ALLEY·POYNER
MACCHIETTO
ARCHITECTURE

ADDENDUM #2

PHOENIX ACADEMY OMAHA
APMA Project # 15008
7 August 2015

(1110 N 66TH STREET)

This addendum is issued by the Owner to all known bidders prior to receipt of Proposal. Bidders shall acknowledge receipt of this Addendum by so indicating on the Proposal Form. All information and instruction given herein shall become a part of the Contract Documents.

This addendum contains the following information:

Total **(3) 8 ½" x 11"** Sheets
(11) 30" x 42" Sheets

If this addendum appears incomplete, notify the architect immediately.

GENERAL INFORMATION

A. SEE ETI ADDENDUM attached to this document.
B. WINDOW TYPE A & TYPE B CLASSROOM WINDOWS to have integral blinds. REMOVE ALL RECESSED ROLLER SHADES FROM TYPE A & TYPE B CLASSROOM WINDOWS. REMOVE recessed gypsum board soffit for roller shades at these locations. Acoustical ceiling to extend to exterior wall.
C. CHANGE all polished concrete floors to sealed concrete.
D. ROOF INSULATION shall have a minimum insulation value of R-30.

DRAWINGS

CIVIL DRAWINGS

1. SHEET C1.0 – DEMOLITION PLAN
2. SHEET C3.0 – SITE UTILITY PLAN
3. SHEET C4.0 – SITE PAVING PLAN

ARCHITECTURAL DRAWINGS

1. SHEET AD1.0 – DEMOLITION PLANS – LOWER LEVEL & FIRST FLOOR
2. SHEET A9.1– FINISH PLANS – LOWER LEVEL & FIRST FLOOR

STRUCTURAL DRAWINGS

1. SHEET S1.0 – STRUCTURAL NOTES
2. SHEET S1.1 – FOUNDATION AND FLOOR FRAMING PLANS
3. SHEET S1.2 – ROOF FRAMING PLAN
4. SHEET S3.1 – STRUCTURAL SECTIONS
5. SHEET S3.2 – STRUCTURAL SECTIONS
6. SHEET S3.3 – STRUCTURAL SECTIONS

PROJECT MANUAL

1. Section 033000-Cast-in-place concrete; 2.4, B, add Barrier-bac as approved manufacturer.
2. Section 075423-EPDM; 2.3, A, 1, Add e Johns Manville as approved manufacturer.
3. Section 051200 Structural Steel: Revise paragraph 2.2.D.1 to read "**Studs shall have a minimum tensile strength of 65,000 psi and minimum elongation of 20%.**"

Engineering Technologies Inc

ADDENDUM

DATE ISSUED	August 6, 2015	ADDENDUM #	2
ENGINEER	Engineering Technologies, Inc. 1111 North 13th Street, Suite 216 Omaha, NE 68102	PROJECT	Phoenix Academy Addition and Renovation
		ETI PROJECT #	2015-022

The Architect issues this Addendum to all known bidders before receipt of proposals. Bidder shall acknowledge the receipt of this addendum on their proposal sheet and all information contained herein shall become a part of the contract documents.

ADDENDUM:

PRIOR APPROVAL – MECHANICAL

1. The following manufacturers have received prior approval for bidding purposes subject to shop drawing review:
   - A. Grilles/Register/Diffusers — Price Industries
   - B. Roof Hoods — Loren Cook
   - C. FEMA Louvers — Ruskin
   - D. Drainage Products — Watts
   - E. Plumbing Accessories — Watts
   - F. Wall Hydrants — Watts

DRAWINGS – MECHANICAL

1. Sheet M2.0 – Lower Level HVAC Plan
   - A. Sheet Note #3 – Flues shown shall indicate a pair of flues (6 total) termination with concentric wall cap. The three existing Armstrong furnaces are all 75MBH fuel input, 68.5MBH output. Confirm exact sizes.

PRIOR APPROVAL – ELECTRICAL

1. The following lighting manufacturers have received prior approval for bidding purposes subject to shop drawing review:
   - A. Types 1, 1E, 2, 2E, 4, 7, 9, 9E, 12, 12E, 16 — Daybrite
   - B. Type 3 — Peerless Electric
   - C. Types 5, 6 — Chloride
   - D. Types 10, 10E, 15, 15E — Lightolier
   - E. Type 11 — LBL Lighting
   - F. Type 14 — Mobern Lighting

DRAWINGS – ELECTRICAL

1. Sheet E3.0, Electrical Riser Diagram, Schedules, Details, and Symbols
   - A. Reference Panel MDP schedule - Provide a new breaker for Panel F, as designated in note 2.

END OF ADDENDUM

LINCOLN: 825 M Street, Suite 200, Lincoln, NE 68508 P 402.476.1273 F 402.476.1274
OMAHA: 1111 N.13th Street, Suite 216, Omaha, NE 68102 P 402.330.2772 F 402.330.2630

PHOENIX ACADEMY OMAHA

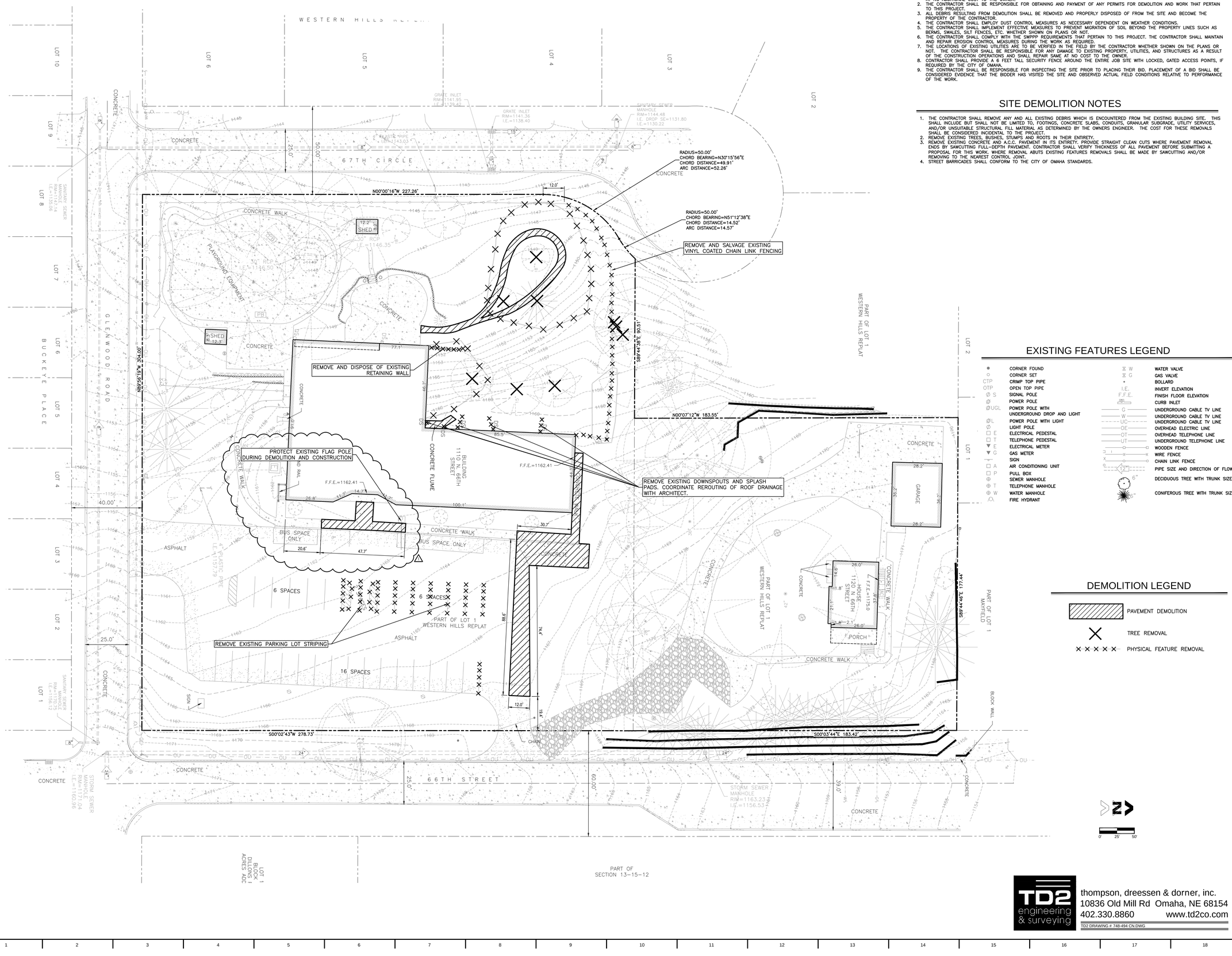
1110 N 66TH STREET

GENERAL DEMOLITION NOTES

1. THE CONTRACTOR SHALL LIMIT REMOVAL AND DEMOLITION WORK TO THAT SPECIFICALLY CALLED FOR IN THE DOCUMENTS. ANY STRUCTURES TO REMAIN WHICH ARE DAMAGED DURING DEMOLITION OPERATIONS SHALL BE REPAIRED BACK TO THEIR ORIGINAL DESIGN AT NO ADDITIONAL COST TO THE OWNER.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR OBTAINING AND PAYMENT OF ANY PERMITS FOR DEMOLITION AND WORK THAT PERTAIN TO THIS PROJECT.
3. ALL DEBRIS RESULTING FROM DEMOLITION SHALL BE REMOVED AND PROPERLY DISPOSED OF FROM THE SITE AND BECOME THE PROPERTY OF THE CONTRACTOR.
4. THE CONTRACTOR SHALL EMPLOY DUST CONTROL MEASURES AS NECESSARY DEPENDENT ON WEATHER CONDITIONS.
5. THE CONTRACTOR SHALL IMPLEMENT EFFECTIVE MEASURES TO PREVENT MIGRATION OF SOIL BEYOND THE PROPERTY LINES SUCH AS BERMS, SWALES, SILT FENCES, ETC. WHETHER SHOWN ON PLANS OR NOT.
6. THE CONTRACTOR SHALL COMPLY WITH THE SWPPP REQUIREMENTS THAT PERTAIN TO THIS PROJECT. THE CONTRACTOR SHALL MAINTAIN AND REPAIR EROSION CONTROL MEASURES DURING THE WORK AS REQUIRED.
7. THE LOCATIONS OF EXISTING UTILITIES ARE TO BE VERIFIED IN THE FIELD BY THE CONTRACTOR WHETHER SHOWN ON THE PLANS OR NOT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DAMAGE TO EXISTING PROPERTY, UTILITIES, AND STRUCTURES AS A RESULT OF THE CONSTRUCTION OPERATIONS AND SHALL REPAIR SAME AT NO COST TO THE OWNER.
8. CONTRACTOR SHALL PROVIDE A 6 FEET TALL SECURITY FENCE AROUND THE ENTIRE JOB SITE WITH LOCKED, GATED ACCESS POINTS, IF REQUIRED BY THE CITY OF OMAHA.
9. THE CONTRACTOR SHALL BE RESPONSIBLE FOR INSPECTING THE SITE PRIOR TO PLACING THEIR BID. PLACEMENT OF A BID SHALL BE CONSIDERED EVIDENCE THAT THE BIDDER HAS VISITED THE SITE AND OBSERVED ACTUAL FIELD CONDITIONS RELATIVE TO PERFORMANCE OF THE WORK.

SITE DEMOLITION NOTES

1. THE CONTRACTOR SHALL REMOVE ANY AND ALL EXISTING DEBRIS WHICH IS ENCOUNTERED FROM THE EXISTING BUILDING SITE. THIS SHALL INCLUDE BUT SHALL NOT BE LIMITED TO, FOOTINGS, CONCRETE SLABS, CONDUITS, GRANULAR SUBGRADE, UTILITY SERVICES, AND/OR UNSUITABLE STRUCTURAL FILL MATERIAL AS DETERMINED BY THE OWNERS ENGINEER. THE COST FOR THESE REMOVALS SHALL BE CONSIDERED INCIDENTAL TO THE PROJECT.
2. REMOVE EXISTING TREES, BUSHES, STUMPS AND ROOTS IN THEIR ENTIRETY.
3. REMOVE EXISTING CONCRETE AND A.C.C. PAVEMENT IN ITS ENTIRETY. PROVIDE STRAIGHT CLEAN CUTS WHERE PAVEMENT REMOVAL ENDS BY SAWCUTTING FULL-DEPTH PAVEMENT. CONTRACTOR SHALL VERIFY THICKNESS OF ALL PAVEMENT BEFORE SUBMITTING A PROPOSAL FOR THIS WORK. WHERE REMOVAL ABUTS EXISTING FEATURES REMOVALS SHALL BE MADE BY SAWCUTTING AND/OR REMOVING TO THE NEAREST CONTROL JOINT.
4. STREET BARRICADES SHALL CONFORM TO THE CITY OF OMAHA STANDARDS.



EXISTING FEATURES LEGEND

●	CORNER FOUND	W	WATER VALVE
○	CORNER SET	G	GAS VALVE
CTP	CRIMP TOP PIPE	•	BOLLARD
OTP	OPEN TOP PIPE	I.E.	INVERT ELEVATION
Ø S	SIGNAL POLE	F.F.E.	FINISH FLOOR ELEVATION
Ø	POWER POLE		CURB INLET
ØUGL	POWER POLE WITH UNDERGROUND DROP AND LIGHT	G	UNDERGROUND CABLE TV LINE
ØL	POWER POLE WITH LIGHT	W	UNDERGROUND CABLE TV LINE
Ø	LIGHT POLE	UC	UNDERGROUND CABLE TV LINE
E	ELECTRICAL PEDESTAL	OE	OVERHEAD ELECTRIC LINE
T	TELEPHONE PEDESTAL	OT	OVERHEAD TELEPHONE LINE
E	ELECTRICAL METER	UT	UNDERGROUND TELEPHONE LINE
G	GAS METER		WOODEN FENCE
	SIGN		WIRE FENCE
A	AIR CONDITIONING UNIT		CHAIN LINK FENCE
P	PULL BOX	12	PIPE SIZE AND DIRECTION OF FLOW
	SEWER MANHOLE	6"	DECIDUOUS TREE WITH TRUNK SIZE
T	TELEPHONE MANHOLE		CONIFEROUS TREE WITH TRUNK SIZE
W	WATER MANHOLE		
	FIRE HYDRANT		

DEMOLITION LEGEND

- PAVEMENT DEMOLITION
- TREE REMOVAL
- PHYSICAL FEATURE REMOVAL

0' 25' 50'

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thompson, dreessen & dorner, inc.
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402.330.8860 www.td2co.com

TD2 DRAWING # 748-494 CN.DWG

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ELECTRICAL ENGINEER
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OMAHA, NE, 68102
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PROFESSIONAL CIVIL ENGINEER — DOUGLAS D. DILLON — 9738 — STATE OF NEBRASKA

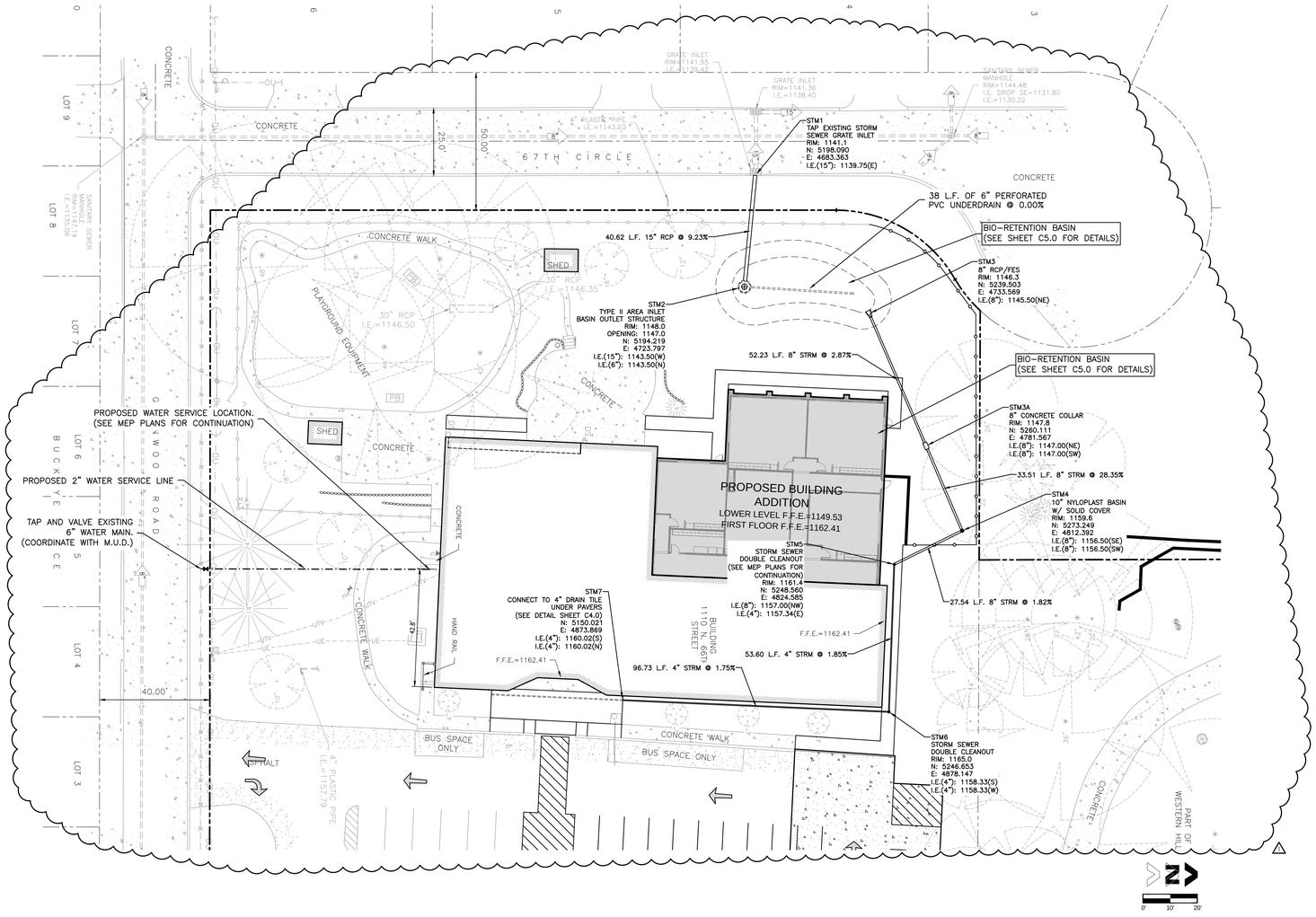
REVISION	DATE
2 ADDENDUM 2	8/7/15

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

DEMOLITION PLAN

C1.0



GENERAL NOTES

1. CONSTRUCTION OBSERVATION SHALL BE PAID FOR BY THE OWNER.
2. ALL PROJECT PROCEDURES, MATERIALS, BONDS AND RESERVES SHALL CONFORM TO THE CITY OF OMAHA STANDARD SPECIFICATIONS EXCEPT AS AMENDED BY THE CONTRACT DOCUMENTS FOR THIS PROJECT.
3. ALL MATERIAL SHALL CONFORM TO THE CITY OF OMAHA STANDARD SPECIFICATIONS, 2014 EDITION, EXCEPT AS AMENDED BY THE CONTRACT DOCUMENTS AND SPECIFICATIONS FOR THIS PROJECT.
4. ELEVATIONS SHOWN ARE REFERENCED TO BENCHMARK AS NOTED ON SHEET C0.0.
5. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR ANY DAMAGE TO EXISTING PROPERTY, UTILITIES AND STRUCTURES, AND WILL REPAIR SAME AT HIS OWN EXPENSE.
6. UTILITIES ARE SHOWN AS A CONVENIENCE FOR THE CONTRACTOR. THE LOCATIONS OF ALL AERIAL AND UNDERGROUND UTILITY FACILITIES MAY NOT BE INDICATED IN THESE PLANS. UNDERGROUND UTILITIES, WHETHER INDICATED OR NOT, WILL BE LOCATED AND FLAGGED BY THE UTILITY COMPANIES AT THE CONTRACTOR'S REQUEST. THE CONTRACTOR SHALL NOTIFY OF UTILITY COMPANIES BEFORE WORK IS STARTED TO VERIFY UTILITY LOCATIONS (ONE CALL 811). NO EXCAVATION WILL BE PERMITTED IN THE AREA OF THE UNDERGROUND UTILITIES UNTIL ALL FACILITIES HAVE BEEN LOCATED AND IDENTIFIED TO THE SATISFACTION OF ALL PARTIES AND THEN ONLY WITH EXTREME CARE TO AVOID ANY POSSIBILITY OF DAMAGES TO THE FACILITIES.
6. THE CONTRACTOR SHALL OBSERVE GOOD SOIL CONSERVATION PRACTICES IN THE CONSTRUCTION OF THIS PROJECT. AS DIRECTED BY THE ENGINEER, ALL EXPOSED AREAS DISTURBED BY CONSTRUCTION OPERATION SHALL BE SEEDED AS SOON AS POSSIBLE AFTER CONSTRUCTION IS COMPLETE.
7. THE ENGINEER/INSPECTOR MUST NOTIFY THE FOLLOWING CITY OF OMAHA PUBLIC WORKS DEPARTMENT PERSONNEL 48 HOURS PRIOR TO ALL PRECONSTRUCTION MEETINGS AND 48 HOURS PRIOR TO THE START OF ANY CONSTRUCTION: MATT GROSSE (PHONE: 402-444-5282; (EMAIL: mgrosse@cityofomaha.org) AND TRAFFIC MAINTENANCE (PHONE: 402-444-5160).
8. THE ENGINEER/INSPECTOR MUST SUBMIT WEEKLY PROGRESS REPORTS TO THE FOLLOWING CITY OF OMAHA PUBLIC WORKS DEPARTMENT EMPLOYEE NO LATER THAN THE FOLLOWING WEEK ENDING DATE: MATT GROSSE (EMAIL: mgrosse@cityofomaha.org).
9. THE ENGINEER/INSPECTOR MUST NOTIFY THE FOLLOWING CITY OF OMAHA PUBLIC WORKS DEPARTMENT PERSONNEL 48 HOURS PRIOR TO LANE CLOSURES AND 24 HOURS PRIOR TO LANE RESTRICTIONS: MATT GROSSE (PHONE: 402-444-5282; (EMAIL: mgrosse@cityofomaha.org) AND TRAFFIC MAINTENANCE (PHONE: 402-444-5160).
10. ALL DEBRIS RESULTING FROM CONSTRUCTION OPERATIONS SHALL BE HAULED OFF THE SITE AND DISPOSED OF PROPERLY. (NO PAY ITEM)
11. THE CONTRACTOR SHALL DISPOSE OF ALL UNSUITABLE MATERIALS ENCOUNTERED IN THE REMOVAL AND GRADING OPERATIONS OFF THE PROJECT SITE, INCLUDING CONCRETE, ASPHALT, OIL MAT, BRICK, ROCK, ETC. NO UNSUITABLE MATERIAL AS DETERMINED BY THE ENGINEER, SHALL BE USED FOR BACKFILLING OR EMBANKMENT CONSTRUCTION.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTENANCE OF EROSION CONTROL MEASURES ON THE SITE, AS REQUIRED BY THE CITY OF OMAHA.
13. MANHOLES, APPURTENANCES, AND SPECIAL STRUCTURES INSTALLATION; INSTALL THE MANHOLES, APPURTENANCES, AND SPECIAL STRUCTURES AT THE LOCATIONS AND ELEVATIONS INDICATED IN THE CONTRACT DOCUMENTS OR AS DIRECTED BY THE ENGINEER. INSTALL EXTERNAL FRAME SEALS ON ALL NEW CONSTRUCTION UNLESS OTHERWISE INDICATED IN THE CONTRACT DOCUMENTS.
14. THE CONTRACTOR SHALL BE RESPONSIBLE TO COMPLY WITH ALL OSHA REGULATIONS IN THE PROSECUTION OF WORK UNDER THIS CONTRACT.
15. THE FOLLOWING STANDARD PLATES ON FILE AT THE CITY OF OMAHA PUBLIC WORKS DEPARTMENT SHALL GOVERN:

ITEMS	STANDARD PLATE NO.
GEOTEXTILE FABRIC SILT FENCE	100-01
SEWER BEDDING DETAIL	700-01
STORM MANHOLE DETAIL	700-40
MANHOLE RING, SOLID COVER & STEPS	700-90

STORM SEWER NOTES

1. ALL INLET STATIONING SHOWN IS TO THE CENTER OF THE MANHOLE.
2. THE LOCATIONS OF EXISTING UTILITIES ARE TO BE VERIFIED IN THE FIELD BY THE CONTRACTOR.
3. THE CONTRACTOR WILL BE HELD RESPONSIBLE FOR ANY DAMAGE TO EXISTING PROPERTY, UTILITIES AND STRUCTURES AND REPAIR SAME AT HIS OWN EXPENSE.
4. ALL DEBRIS RESULTING FROM CONSTRUCTION OPERATIONS SHALL BE HAULED OFF SITE AND DISPOSED OF PROPERLY. (NO PAY ITEM)
5. REINFORCED CONCRETE SEWER PIPE 30 INCHES AND SMALLER IN DIAMETER SHALL CONFORM TO THE REQUIREMENTS OF ASTM C-76, CLASS III, LATEST REVISION. REINFORCED CONCRETE SEWER PIPE 36 INCHES AND LARGER IN DIAMETER SHALL CONFORM TO THE D-LOAD SPECIFIED TO PRODUCE A 0.01 INCH CRACK IN ACCORDANCE WITH ASTM C-655, LATEST ADDITION. PIPE JOINT SEALING COMPOUND SHALL CONFORM TO FED. SPEC. SS-C-153, OR APPROVED EQUAL.
6. THE CONTRACTOR SHALL NOTIFY THE CITY OF OMAHA PUBLIC WORKS DEPARTMENT 48 HOURS PRIOR TO THE START OF CONSTRUCTION.
7. THE ENGINEER/INSPECTOR MUST INSURE ALL CONCRETE STORM SEWER PIPE USED FOR CONSTRUCTION HAS BEEN CERTIFIED BY THE AMERICAN CONCRETE PIPE ASSOCIATION (ACPA). ALL PIPE SHALL DISPLAY THE Q-CAST SYMBOL TO VERIFY THE MANUFACTURER HAS MET THE ACPA'S CERTIFICATION PROGRAM. VISUAL INSPECTIONS FOR DEFECTS SHALL CONTINUE TO TAKE PLACE ON SITE.

UTILITY NOTES

1. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS IN THE FIELD PRIOR TO THE START OF CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ANY DAMAGE TO EXISTING PROPERTY, UTILITIES, AND STRUCTURES, AND WILL REPAIR AT HIS OWN EXPENSE. THE CONTRACTOR SHALL HAVE ALL EXISTING UTILITIES FIELD LOCATED IN THE FIELD PRIOR TO CONSTRUCTION THROUGH THE "ONE CALL" SYSTEM AT 811.
2. THE CONTRACTOR SHALL EMPLOY DUST CONTROL MEASURES AS NECESSARY DEPENDENT ON WEATHER CONDITIONS.
3. SEE SPECIFICATIONS FOR COMPACTION AND FILL REQUIREMENTS.
4. THE CONTRACTOR SHALL TAKE PRECAUTIONS TO PREVENT SILT LADEN RUNOFF FROM ENTERING STREET AND RUNNING ONTO ADJACENT PROPERTY. EROSION CONTROL SHALL BE MAINTAINED THROUGHOUT CONSTRUCTION AND SUBJECT TO APPROVAL BY THE OWNER. THE CONTRACTOR SHALL COMPLY WITH SWPPP REQUIREMENTS.
5. SITEWORK UTILITY CONTRACTOR TO EXTEND ALL PIPING TO WITHIN 5' OF BUILDING AND CAP FOR CONNECTION BY BUILDING UTILITY CONTRACTOR. THE CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY THE PROPOSED UTILITY LINE LOCATIONS AT THE BUILDING AGAINST THE LOCATIONS SHOWN ON THE MECHANICAL AND ELECTRICAL PLANS AND TO COORDINATE THE CONTINUATION OF THE LINES INTO THE BUILDING.
6. COMPLY WITH ALL CODES AND REGULATIONS. OBTAIN PERMITS AS NECESSARY.
7. REFER TO SPECIFICATIONS FOR MATERIAL, PROCEDURAL AND CONSTRUCTION REQUIREMENTS.
8. COORDINATE UTILITY CONSTRUCTION WORK WITH THE WORK OF ALL OTHER TRADES ON THE SITE.
9. SEE BUILDING AND SITE PLUMBING PLANS FOR CONTINUATION OF ALL UTILITY LINES INTO BUILDINGS. COORDINATE WITH OTHER CONTRACTORS AS NECESSARY FOR CONSTRUCTION AND TESTING.
10. THE CONTRACTOR SHALL ADJUST ALL UTILITY STRUCTURES TO GRADE AS REQUIRED. THESE INCLUDE, BUT NOT LIMITED TO MANHOLES, FIRE HYDRANTS, STORM SEWER INLETS, JUNCTION BOXES, VALVE BOXES, AND CURB STOPS. SEWER CLEAN-OUTS AND ANY OTHER ITEMS THAT MAY FALL INTO THIS CLASSIFICATION. THESE ADJUSTMENTS SHALL BE (NO PAY ITEMS).
11. PRIOR TO CONSTRUCTION, THE CONTRACTOR SHALL VERIFY ALL GRADES AND TAP ELEVATIONS WHERE PROPOSED UTILITIES ARE TO TIE INTO EXISTING UTILITIES. THE CONTRACTOR IS RESPONSIBLE TO STUDY THE PROPOSED PLANS AND NOTIFY THE OWNERS ENGINEER ON ANY INCONSISTENCIES, DISCREPANCIES OR CONTRACTIBILITY PROBLEMS IF THEY MAY ARISE.
12. THE "TEN STATE STANDARDS" FOR THE SEWER AND WATER SEPARATION SHALL BE CONFORMED TO.
13. THE CONTRACTOR SHALL PROVIDE ALL DE-WATERING AND UNSTABLE TRENCH BEDDING AS REQUIRED. A SUBSURFACE INVESTIGATION MAY BE OBTAINED FROM THE OWNER FOR THE CONTRACTOR'S INFORMATION.

UTILITY NOTES

CITY OF OMAHA PUBLIC WORKS-SEWERS
MR. BRIAN LODES
1819 FARNAM STREET
OMAHA, NE 68102
(402) 444-5105

METROPOLITAN UTILITIES DISTRICT-GAS
MR. MARK MASEK
3001 SO 61ST AVENUE
OMAHA, NE 68106
(402) 504-7910

METROPOLITAN UTILITIES DISTRICT-WATER
MR. DON NOONAN
3001 SOUTH 61ST AVENUE
OMAHA, NE 68106
(402) 504-7988

OMAHA PUBLIC POWER DISTRICT-POWER
MR. JOHN IMIG
444 SOUTH 16TH STREET MALL
OMAHA, NE 68102
(402) 552-5386

CENTURY LINK
MR. JACK DODENDORF
7404 NORTH 78TH STREET, BLDG A
OMAHA, NE 68122
(402) 572-5814

COX COMMUNICATIONS
MR. STEVE MIESKA, SUPERVISOR
(EAST OF 84TH ST)
3031 N 120TH STREET
OMAHA, NE 68164
(402) 934-0259 (OFFICE)
(402) 510-1755 (CELL)

MR. KEN WINTER, SUPERVISOR
(WEST OF 84TH ST)
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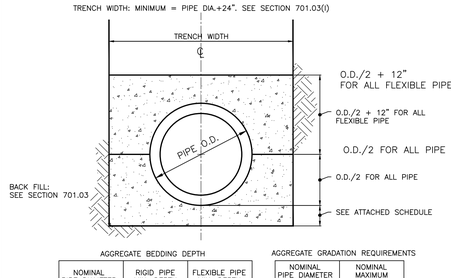
(402) 510-1185 (CELL)

LEGEND

- PROPOSED STORM SEWER
- EXISTING STORM SEWER
- EXISTING STORM SEWER
- EXISTING SANITARY SEWER
- EXISTING WATER MAIN
- EXISTING GAS MAIN
- EXISTING WATER VALVE
- EXISTING ELECTRICAL LINE



AGGREGATE BEDDING DEPTH

NOMINAL PIPE DIAMETER (INCHES)	RIGID PIPE MIN. DEPTH (INCHES)	FLEXIBLE PIPE MIN. DEPTH (INCHES)
≤ 12	4**	6
≤ 30	6	6***
≤ 60	12	12***
>60	18	N/A

AGGREGATE GRADATION REQUIREMENTS

NOMINAL PIPE DIAMETER (INCHES)	NOMINAL MAXIMUM SIZE
≤ 18	3/4"
≤ 60	1 1/2"
>60	2" or 3"

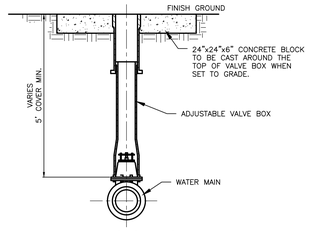
* SEE ASTM C1479 FOR MINIMUM DEPTH FOR PCC PIPE.

** AS REQUIRED BY THE ENGINEER

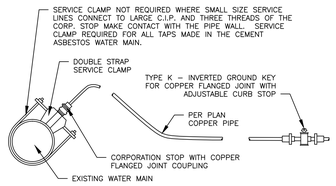
*** OR ONE-FORTH (1/4) OF THE DIAMETER, WHICHEVER IS GREATER.

1. RIGID PIPE INCLUDES BUT IS NOT LIMITED TO PCC, DUCTILE IRON, VITRIFIED CLAY, FIBERGLASS, AND PVC TRUSS. FLEXIBLE PIPE IS DEFINED AS ANY PIPE HAVING A PIPE STIFFNESS OF ONE HUNDRED FIFTY-THREE (153) POUNDS PER INCH PER INCH OR LESS. FLEXIBLE PIPE INCLUDES BUT IS NOT LIMITED TO CORRUGATED STEEL, CORRUGATED ALUMINUM, SOLID WALL PVC, CORRUGATED PVC, AND PROFILE WALL PVC. THE ENGINEER SHALL DETERMINE WHETHER A PIPE MATERIAL NOT LISTED IS RIGID OR FLEXIBLE.
2. AGGREGATE BEDDING MATERIAL SHALL CONSIST OF RECYCLED PCC, CRUSHED LIMESTONE, QUARTZITE, OR DOLOMITE MEETING THE REQUIREMENTS FOR CLASS 1S COARSE AGGREGATE AS DEFINED BY ASTM C33, STANDARD SPECIFICATION FOR CONCRETE AGGREGATES. THE AGGREGATE SHALL CLASSIFY AS A WELL-GRADED GRAVEL, POORLY GRADED GRAVEL, WELL GRADED GRAVEL WITH SAND, OR A POORLY GRADED GRAVEL WITH SAND WHEN CLASSIFIED USING ASTM D2487, STANDARD CLASSIFICATION OF SOILS FOR ENGINEERING PURPOSES (UNIFIED SOIL CLASSIFICATION SYSTEM). THE MAXIMUM AMOUNT OF MATERIAL PASSING A #200 (75 UM) STANDARD SIEVE SHALL BE EIGHT (8) PERCENT AND THE NOMINAL MAXIMUM SIZE SHALL CONFORM TO THE REQUIREMENTS OUTLINED IN THE TABLE ABOVE.

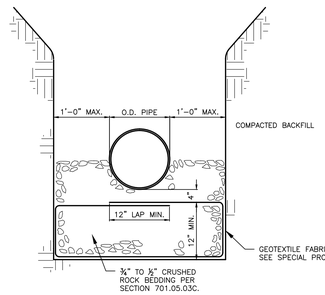
STORM & SANITARY SEWER PIPE BEDDING
NOT TO SCALE



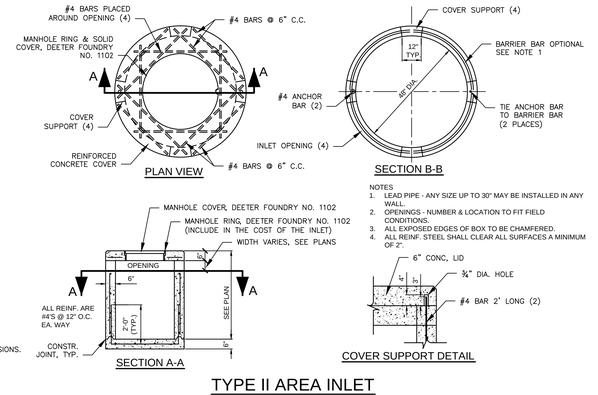
VALVE BOX SETTING
NOT TO SCALE



CORPORATION STOP DETAIL
NOT TO SCALE



UNSTABLE TRENCH BEDDING DETAIL
NOT TO SCALE



TYPE II AREA INLET
NOT TO SCALE

PHOENIX ACADEMY OMAHA
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1	ADDENDUM 2	8/7/15

REVISION	DATE

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

SITE UTILITY PLAN

C3.0

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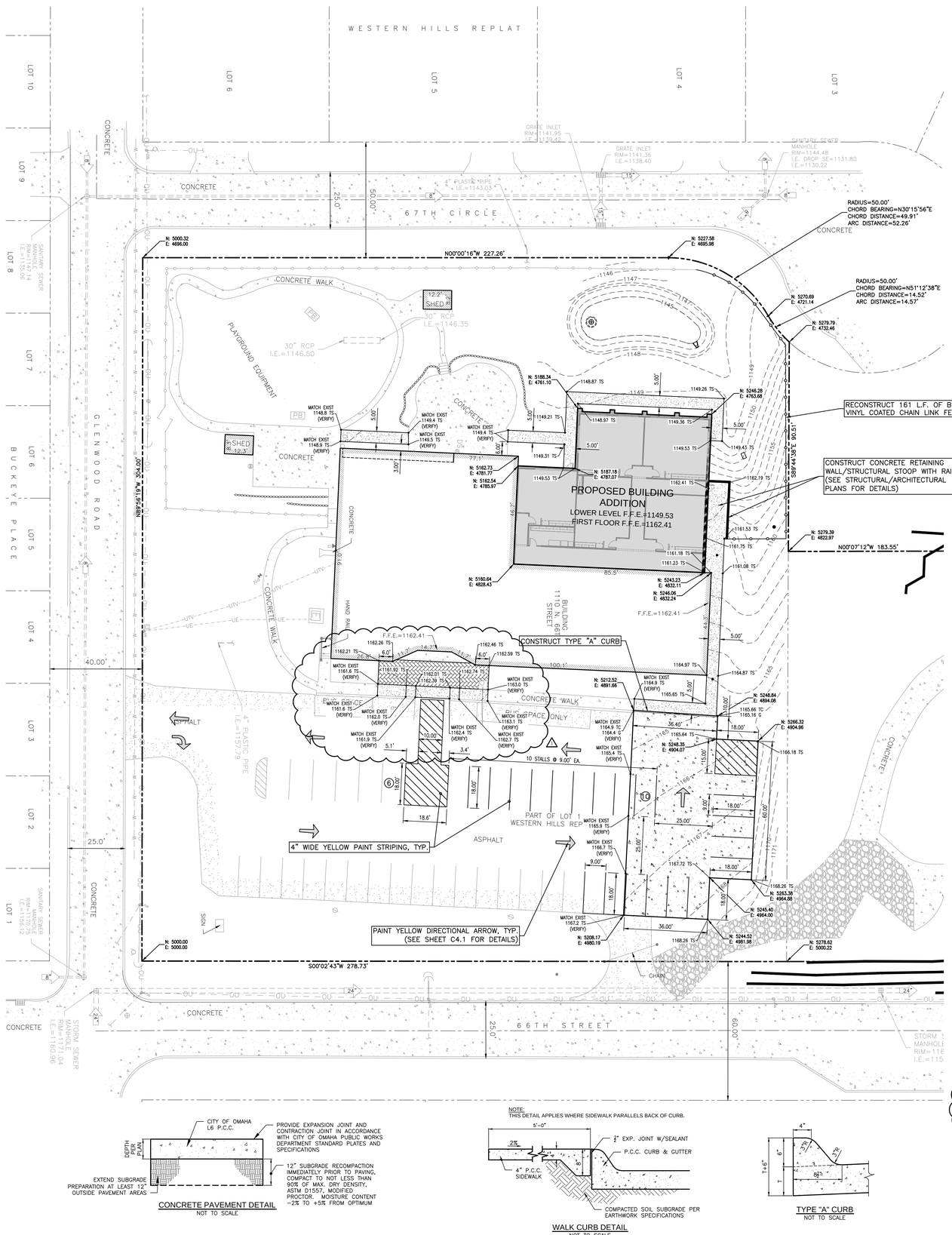
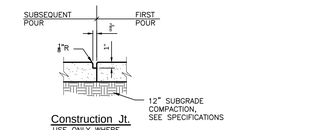
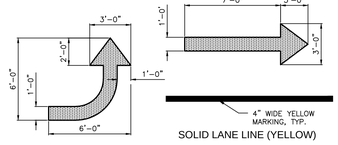
402.330.8860 www.td2co.com
TD2 DRAWING # 748-494 CN.DWG



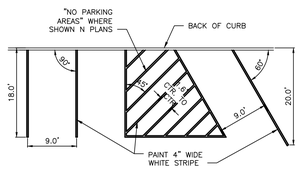


SIDEWALK JOINT DETAILS
NOT TO SCALE

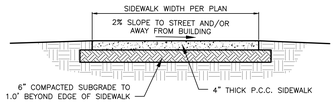


SOLID LANE LINE (YELLOW)

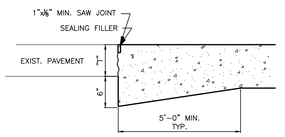
TYPICAL PAVEMENT MARKING DETAILS
NOT TO SCALE



STANDARD PARKING STALL STRIPING DETAIL
NOT TO SCALE



TYPICAL SIDEWALK SECTION
NOT TO SCALE



THICKENED EDGE DETAIL
NOT TO SCALE

PAVING NOTES

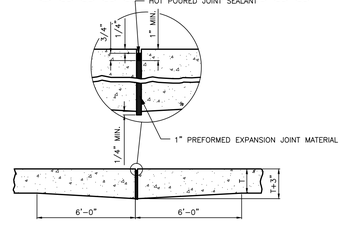
1. THE CONTRACTOR SHALL HAVE ALL UTILITIES LOCATED PRIOR TO CONSTRUCTION THROUGH THE NEBRASKA 811 "ONE CALL" NOTIFICATION SYSTEM.
2. THE CONTRACTOR SHALL VERIFY ALL UTILITY LOCATIONS IN THE FIELD PRIOR TO THE START OF CONSTRUCTION OPERATIONS.
3. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ANY DAMAGE TO EXISTING PROPERTY, UTILITIES, AND STRUCTURES, AND WILL REPAIR OR REPLACE DAMAGED IMPROVEMENTS AT HIS OWN EXPENSE.
4. DIMENSIONS SHOWN ARE TO EDGE OF SLAB, BACK OF CURB, OR FACE OF BUILDING AS APPLICABLE. CONTRACTOR SHALL VERIFY THE LAYOUT AND DIMENSIONS OF IMPROVEMENTS WITH THE OWNER AND ARCHITECT BEFORE BEGINNING CONSTRUCTION.
5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROVIDING ALL BARRICADES, FLAGMEN, AND TRAFFIC CONTROL DURING CONSTRUCTION OPERATIONS. NO SEPARATE PAYMENT WILL BE MADE FOR TRAFFIC CONTROL.
6. SEE LANDSCAPING PLANS FOR PLANTING AND LANDSCAPING REQUIREMENTS.
7. COORDINATE WORK WITH ALL OTHER TRADES.
8. THE CONTRACTOR SHALL COMPLY WITH ALL OSHA REGULATIONS IN THE EXECUTION OF THE WORK UNDER THIS CONTRACT.
9. THE CONTRACTOR SHALL ADJUST ALL UTILITIES TO FINISHED PAVEMENT GRADE OR AS REQUIRED IN LANDSCAPED AREAS. (NO PAY ITEM)
10. VERIFY ELEVATIONS AT ALL OUTSIDE DOOR LOCATIONS WITH ARCHITECT TO ENSURE CODE COMPLIANCE AND ACCESSIBILITY. SEE ARCHITECTURAL PLANS FOR STOOPS.
11. WATER REDUCING ADMIXTURES SHALL NOT BE USED UNLESS APPROVED BY THE ENGINEER.
12. CONCRETE PAVEMENT SHALL BE CURED USING A WHITE PIGMENTED LIQUID MEMBRANE FORMING CURING COMPOUND THAT HAS BEEN APPROVED BY THE CITY OF OMAHA. MINIMUM RATE OF APPLICATION SHALL BE 200 SQ. FEET PER GALLON IF A MECHANICAL POWER SPRAYER IS USED, AND 100 SQ. FEET PER GALLON IF A HAND SPRAYER IS USED.
13. SEE SHEET C6.0 FOR SPECIFICATIONS.

LEGEND

PROPOSED 7" P.C.C. PAVEMENT OR CURB & GUTTER

PROPOSED 4" P.C.C. SIDEWALK

PROPOSED PERVIOUS PAVER AREA



EXPANSION JOINT DETAIL
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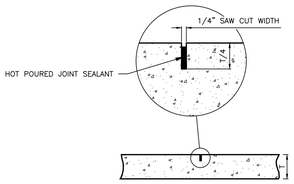
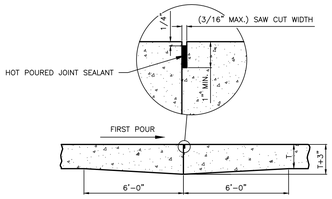
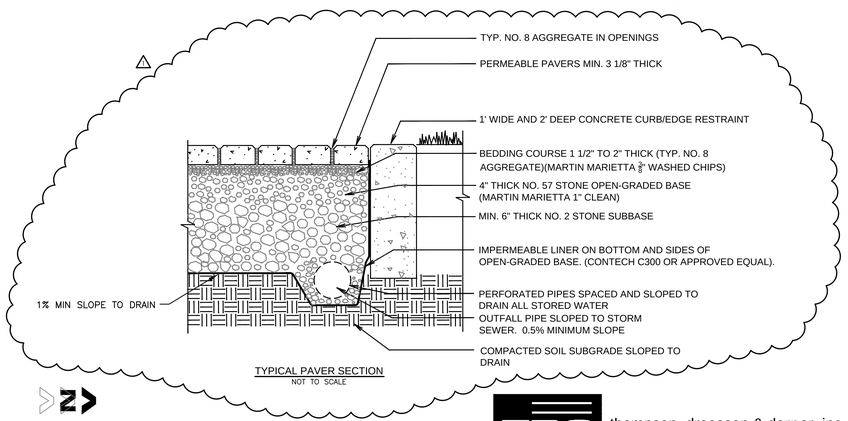


CONTRACTION JOINT DETAILS
NOT TO SCALE



TRANSVERSE CONSTRUCTION JOINT DETAIL
NOT TO SCALE



TYPICAL PAVER SECTION
NOT TO SCALE

CONCRETE PAVEMENT DETAIL
NOT TO SCALE

WALK CURB DETAIL
NOT TO SCALE

TYPE "A" CURB
NOT TO SCALE

0' 10' 20'

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⚠	REVISION	DATE
2	ADDENDUM 2	8/7/15

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

SITE PAVING PLAN

C4.0

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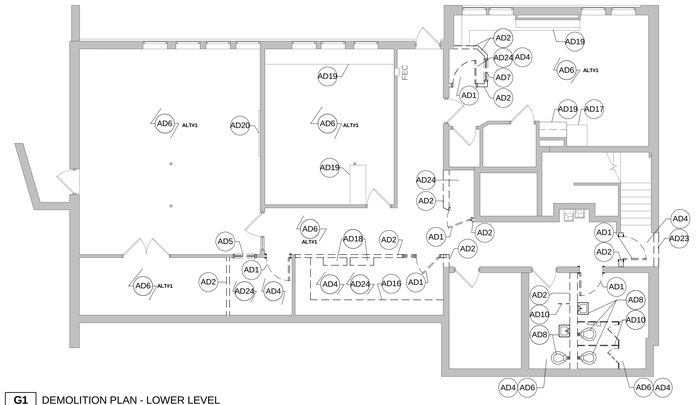
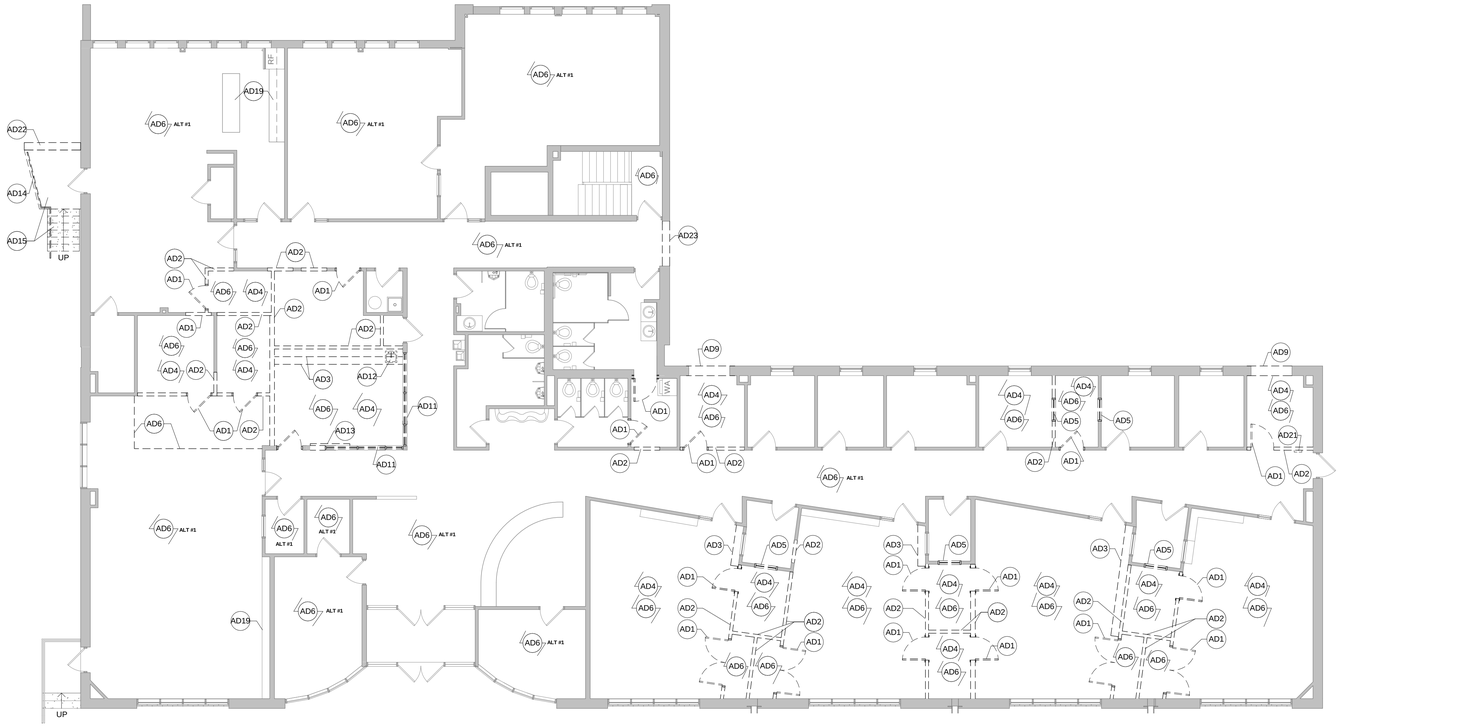
TD2 DRAWING # 748-494 CN.DWG



G1 DEMOLITION PLAN - LOWER LEVEL
AD1.0 SCALE: 1/8" = 1'-0"



P1 DEMOLITION PLAN - FIRST LEVEL
AD1.0 SCALE: 1/8" = 1'-0"

GENERAL NOTES - DEMO PLANS

1. DEMOLITION PLAN NOTES APPLY TO ALL DEMOLITION PLAN SHEETS.
2. COORDINATE ALL DEMOLITION/PHASING EFFORTS WITH THE ARCHITECT-ENGINEER AND OWNER'S REPRESENTATIVES. EVERY EFFORT SHALL BE MADE TO MINIMIZE DISRUPTION OF BUILDING OPERATION AND TO PROVIDE OCCUPANT SAFETY. EXCESSIVE NOISE OR VIBRATION SHALL BE PRE-APPROVED AND COORDINATED WITH THE OWNER'S REPRESENTATIVE.
3. COORDINATE WITH THE OWNER ANY PRE-APPROVED DISRUPTION AND VERIFICATION OF SERVICE WITHIN THE EXISTING BUILDING SO AS TO MINIMIZE THE DISRUPTION OF SERVICE.
4. CONSTRUCT TEMPORARY CONSTRUCTION PARTITIONS WITHIN THE EXISTING BUILDING WHICH OFFER A ONE-HOUR ENCLOSURE TO ISOLATE ANY DEMOLITION/CONSTRUCTION WORK FROM THE GENERAL PUBLIC AND AS DEEMED NECESSARY BY THE OWNER. COORDINATE LOCATIONS WITH THE OWNER AND MAINTAIN MEANS OF EGRESS THROUGHOUT THE WORK.
5. MAINTAIN A SECURE, WEATHER-TIGHT ENCLOSURE AT THE EXTERIOR OF THE EXISTING BUILDING THROUGHOUT THE DEMOLITION/CONSTRUCTION PORTION OF THE WORK IN THAT AREA. BUILDING SECURITY SHALL BE COORDINATED WITH THE OWNER.
6. VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS AND NOTIFY THE ARCHITECT-ENGINEER OF ANY DISCREPANCIES.
7. ITEMS TO BE DEMOLISHED ARE SHOWN GRAPHICALLY WITH DASHED LINES.
8. REFER TO KEYED NOTES FOR SPECIFIC ITEMS TO BE REMOVED. ALL ITEMS ARE NOT KEYED. REMOVE IN THEIR ENTIRETY ALL EXISTING WALLS, DOORS, MILLWORK, PLUMBING FIXTURES ETC. AS REQUIRED TO EXECUTE THE DEMOLITION/CONSTRUCTION WORK DESCRIBED BY THE ARCHITECT-ENGINEER.
9. REMOVE ALL DEMOLITION MATERIALS FROM THE SITE UNLESS NOTED OTHERWISE. THE OWNER SHALL RESERVE THE RIGHT TO SALVAGE ANY MATERIALS.
10. PROVIDE PROTECTION FOR ALL EXISTING BUILDING MATERIALS AND EQUIPMENT FROM DAMAGE DUE TO ANY DEMOLITION OR CONSTRUCTION-RELATED INCIDENT PERFORMED UNDER THIS CONTRACT.
11. REPAIR OR REPLACE ANY WALLS, FLOORS, OR EQUIPMENT DAMAGED AS A RESULT OF DEMOLITION OR CONSTRUCTION TO MATCH EXISTING FINISH AND/OR CONDITION.
12. THE CONTRACTOR SHALL COORDINATE WITH THE ARCHITECT-ENGINEER ANY MATERIALS TO BE REUSED AND WILL BE RESPONSIBLE FOR VERIFYING AND MAINTAINING THE FUNCTIONAL AND AESTHETIC INTEGRITY OF THE MATERIALS.
13. VERIFY AND MAINTAIN THE LOCATION OF EXISTING POWER, COMMUNICATION AND DATA CABLES SO AS NOT TO INADVERTENTLY INTERRUPT THE CONTINUITY OF THEIR SERVICE.
14. PATCH ALL FLOOR AND CEILING PENETRATIONS RESULTING FROM REMOVAL OR REROUTING OF NEW OR EXISTING PIPING, DUCTWORK, CONDUIT, ETC. AS REQUIRED TO MAINTAIN FIRE SEPARATIONS. FINISH AS REQUIRED FOR NEW OR EXISTING ADJACENT SURFACES.
15. REMOVE ALL ABANDONED PIPING, ELECTRICAL CONDUIT, MISCELLANEOUS HANGERS, MISCELLANEOUS DUCTWORK, AND ALL OTHER ABANDONED ITEMS UNLESS NOTED OTHERWISE.
16. THE CONTRACTOR SHALL SHORE AS REQUIRED ANY AREAS AFFECTED OR COMPROMISED BY DEMOLITION ACTIVITIES.
17. DEMOLITION CONTRACTOR SHALL COORDINATE WITH MECHANICAL AND ELECTRICAL CONTRACTORS FOR LOCATION OF ASSOCIATED SAWCUTS AND CONCRETE FLOOR DEMOLITION.

KEYED NOTES

AD1	REMOVE EXISTING DOOR AND FRAME SYSTEM COMPLETE
AD2	REMOVE EXISTING WALL COMPLETE TO EXTENT SHOWN. PATCH & REPAIR ADJACENT WALL/FLOOR SURFACE/STRUCTURE TO MATCH EXISTING. PREP FOR NEW FINISH
AD3	REMOVE CASEWORK TO EXTENT SHOWN. SALVAGE TO OWNER FOR REUSE. PATCH & REPAIR ADJACENT WALL/FLOOR SURFACE AS REQUIRED TO MATCH EXISTING OR TO RECEIVE NEW FINISH
AD4	REMOVE EXISTING CEILING FINISHES & FIXTURES IN THIS AREA COMPLETE. SALVAGE EXISTING CEILING TILES & FIXTURES TO OWNER FOR REUSE
AD5	REMOVE EXISTING WINDOW SYSTEM COMPLETE. PATCH OPENING TO MATCH EXISTING ADJACENT SURFACE. SALVAGE WINDOW TO OWNER FOR REUSE IN NEW LOCATION
AD6	REMOVE EXISTING FLOOR COVERING & VINYL BASE COMPLETE IN THIS AREA. PATCH AND REPAIR FLOOR & WALLS AS NEEDED TO RECEIVE NEW FLOOR FINISH
AD7	REMOVE EXISTING WINDOW SYSTEM COMPLETE. SALVAGE TO OWNER FOR REUSE
AD8	REMOVE EXISTING PLUMBING FIXTURES. PATCH & REPAIR ADJACENT WALL/FLOOR SURFACE AS REQUIRED TO MATCH EXISTING OR TO RECEIVE NEW FINISH
AD9	SAW CUT EXISTING MASONRY WALL FOR NEW OPENING TO +8'-0" AFF. PROVIDE TEMPORARY SUPPORT OF EXISTING MASONRY WALL ABOVE
AD10	REMOVE EXISTING TOILET PARTITION COMPLETE
AD11	REMOVE EXISTING WINDOW AND FRAME COMPLETE
AD12	REMOVE EXISTING SINK - SALVAGE TO OWNER FOR REUSE
AD13	REMOVE EXISTING STAFF MAILBOXES COMPLETE - SALVAGE TO OWNER FOR REUSE
AD14	REMOVE EXISTING GUARDRAIL COMPLETE
AD15	REMOVE EXISTING CONCRETE STAIR TREADS/LANDING COMPLETE
AD16	REMOVE EXISTING LIBRARY CASEWORK COMPLETE - SALVAGE TO OWNER FOR REUSE
AD17	REMOVE EXISTING REFRIGERATOR - SALVAGE TO OWNER FOR REUSE
AD18	REMOVE EXISTING LIBRARY BOOKSHELVES COMPLETE - SALVAGE TO OWNER FOR REUSE
AD19	EXISTING CASEWORK TO REMAIN
AD20	REMOVE EXISTING MARKER BOARD & PROJECTOR ABOVE - SALVAGE TO OWNER FOR REUSE
AD21	REMOVE EXISTING SEMI-RECESSED FIRE EXTINGUISHER CABINET - SALVAGE TO OWNER FOR REUSE
AD22	REMOVE EXISTING WING WALL TO TOP OF NEW CONC SLAB - SALVAGE EXISTING BRICK FOR REUSE - PATCH & REPAIR ADJACENT WALL SURFACE/STRUCTURE TO MATCH EXISTING
AD23	EXISTING DRAWINGS INDICATE LINTELS FOR FUTURE OPENING - VERIFY LINTEL EXISTS AND REMOVE WALL TO BOTTOM OF EXISTING LINTEL - COORD W/ ARCHITECT IF NEW LINTELS ARE REQD
AD24	IF ALTERNATE 1 IS NOT ACCEPTED, PATCH & REPAIR FLOORING TO MATCH EXISTING

NOTE:
SEE FINISH PLANS FOR ADD ALTERNATE INDICATING EXTENTS & LOCATIONS TO PROVIDE NEW FLOOR FINISHES

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LICENSED PROFESSIONAL ENGINEER — NICHOLAS P. ALLEY — A-2050 — STATE OF NEBRASKA

JULY 24, 2015

2	ADDM 2	Date 2

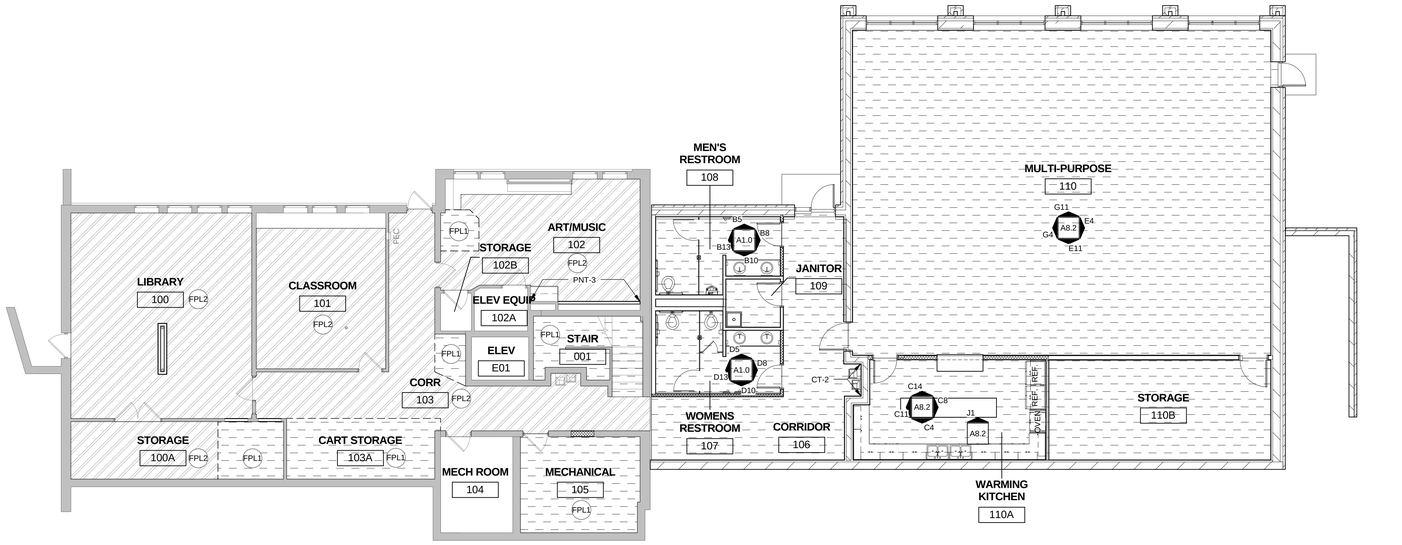
△REVISION	DATE

PROJECT NUMBER: 15008

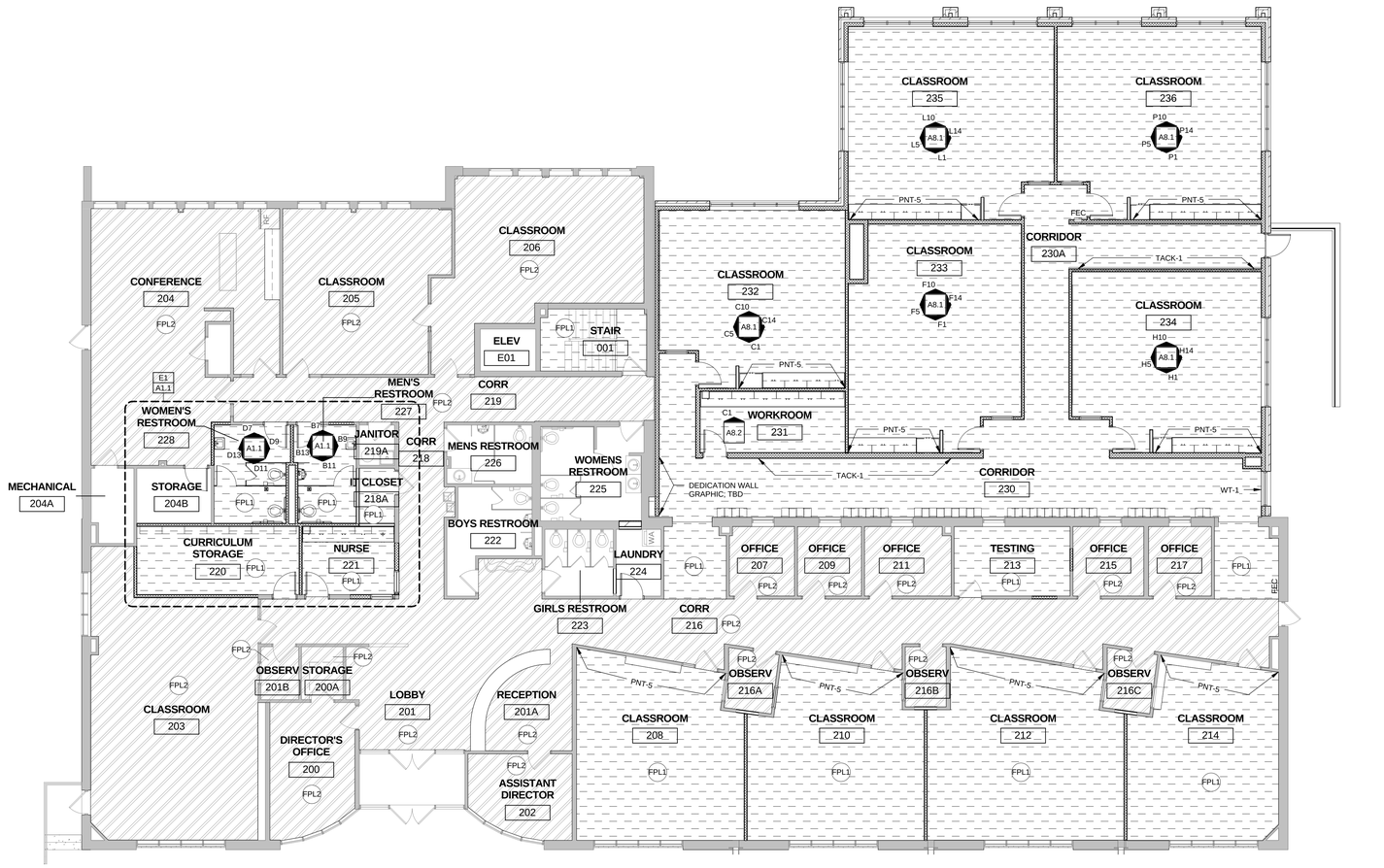
DATE: JULY 24, 2015

DEMOLITION PLANS - LOWER LEVEL & FIRST FLOOR

AD1.0 2



F1 FINISH FLOOR PLAN - LOWER LEVEL
A9.1 SCALE: 1/8" = 1'-0"



P1 FINISH FLOOR PLAN - FIRST FLOOR
A9.1 SCALE: 1/8" = 1'-0"

GENERAL NOTES - FINISH PLANS

1. GENERAL NOTES APPLY TO ALL FINISH PLAN SHEETS.
2. UNLESS NOTED OTHERWISE:
   EXTERIOR DIMENSIONS: ARE TAKEN FROM FACE OF EXTERIOR FINISH MATERIAL TO FACE OF EXTERIOR FINISH MATERIAL
   INTERIOR DIMENSIONS: ARE TAKEN FROM FACE OF FINISH MATERIAL TO FACE OF FINISH MATERIAL
3. REFER TO INTERIOR ELEVATIONS, FINISH SPECIFICATIONS, AND ROOM FINISH SCHEDULE FOR ADDITIONAL WALL FINISH INFORMATION
4. FLOOR FINISH TRANSITIONS TO OCCUR AT MIDPOINT OF DOOR, UNO
5. MISC. METALS, GRILLES, ETC TO BE PAINTED TO MATCH ADJACENT WALL SURFACE
6. PROVIDE WINDOW TREATMENTS (WT) AT WINDOWS AS INDICATED ON FINISH PLANS. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION
7. REFER TO DOOR SCHEDULE FOR DOOR FINISHES
8. PROVIDE 7'-0" H x 1" STAINLESS STEEL CORNER GUARDS AT ALL EXPOSED GYPSUM WALL LOCATIONS

NEW FLOOR COVERING; SEE FINISH SCHED.

ADD ALTERNATE NEW FLOOR COVERING; SEE FINISH SCHED.

KEYED NOTES	
FPL1	PREPARE EXISTING FLOOR FOR NEW FLOOR COVERING; SEE ROOM FINISH SCHEDULE FOR MORE INFORMATION
FPL2	ADD ATLERNATE: PREPARE EXISTING FLOOR FOR NEW FLOOR COVERING; SEE ROOM FINISH SCHEDULE FOR MORE INFORMATION

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JULY 24, 2015

2	ADDM 2	Date 2

△REVISION	DATE

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

FINISH PLANS - LOWER LEVEL & FIRST FLOOR

A□.1 2

STRUCTURAL NOTES, GENERAL

1. CODE: 2006 International Building Code
City of Omaha Amendment

2. STRUCTURAL DESIGN LOADS:

LIVE LOADS:
Floors: 100 PSF
Note: Live load reduction has been applied per Section 1607.9.1 of the 2006 International Building Code.

ROOF LIVE LOADS:
Flat and Sloped Roofs: 25 PSF
Storm Shelter Roof (First Floor): 100 PSF

SNOW:
Ground Snow Load: 25 PSF (Plus Snow Drifting)
Snow Exposure Factor: C_e = 1.0
Thermal Factor: C_t = 1.0
Occupancy Category: III (I_s = 1.1)
Note: Buildings have been designed for snow loads per ASCE 7-05 as required by Section 1608.1 of the 2006 International Building Code.

WIND:
Basic Wind Speed: 90 MPH (Exposure C)
Occupancy Category: III (I_w = 1.15)
Note: Buildings have been designed for wind loads per ASCE 7-05 as required by Section 1609.1.1 of the 2006 International Building Code.

STORM SHELTER - Tornado:
Basic Storm Wind Speed: 250 MPH (Exposure C)
Importance Factor: I = 1.0
Internal Pressure Coefficient: GC_{pi} = 0.55
Note: Storm Shelters have been designed for storm wind loads per ASCE 7-05 as required by Section 3.3.1 of FEMA P-361 - 2008.

SEISMIC:
Spectral Response Accelerations: S_s = 0.125; S_1 = 0.041
Site Class: D
Occupancy Category: III (I_e = 1.25)
Seismic Design Category: A
Note: Buildings have been designed for seismic loads per ASCE 7-05 as required by Section 1613.1 of the 2006 International Building Code.

3. FOUNDATION DESIGN CRITERIA:

A. Foundation design based on the Geotechnical Engineering Report completed by Thompson, Dreessen and Dorner, Inc., Omaha, Nebraska dated June 12, 2015 and addendum dated July 14, 2015. Verify all site preparation within 15 feet of the building and site walls has been performed and inspected in accordance with the 2006 IBC.
Net Allowable Soil Bearing Pressure: 1500 psf.

4. EXCAVATION AND BUILDING WALL BACKFILL:

A. Construction Procedure: Building walls that retain earth are shown on the plans. The walls shown shall be constructed prior to construction of the building floor structure above. The building floor structure above the walls and the slab on grade floor at the base of the walls shall be constructed prior to backfilling behind the wall or the contractor shall provide temporary bracing for the top and base of the concrete walls prior to backfilling behind walls. If temporary bracing is used, backfill shall be performed in accordance with the bracing shop drawings.

5. GENERAL NOTES:

A. All work shall comply with requirements of the 2006 International Building Code, with recommendations of manufacturers, and with recognized workmanship and material standards.

B. Comply with all applicable codes, ordinances, and regulations including those promulgated and enforced by OSHA. The structural design represented by the drawings and specifications is based on interaction of the various components, materials, and systems shown or required by all of the drawings and specifications. The contractor shall determine the need for and provide all required bracing or other means to insure stability and safety until all work required by the contract documents is complete. When and where necessary to comply with these requirements, the contractor shall provide appropriate additional temporary or permanent connections and/or members or, in the alternative, shall make appropriate modifications of specified connections and/or members. Where additions to or modifications of specified requirements are proposed, they shall be submitted to the Architect for review and approval. Such review and approval will be only for compliance with the structural and architectural design intent for the work. The adequacy for construction phase stability and safety is the responsibility of the contractor.

C. Adapt requirements of details, sections, plans, and notes at all locations of which conditions are similar.

D. The structural drawings are to be read in view of all other drawings and all specifications. Coordinate all work shown with all other work.

E. Shop drawings for any part of the work shall show the interface with and provisions for related other work including such adaptations of requirements given as may be necessary.

F. Contractor shall cross check dimensions and elevations between architectural, mechanical, and structural plans and notify Architect of any variance before contractor begins work.

6. MECHANICAL, ELECTRICAL, AND PIPING SYSTEMS WORK:

A. Do not support concentrated loads from steel roof deck.

B. Secondary framing, bridging, or other means shall be provided to distribute loads to structural members. Such framing, bridging, or other means shall be shown on the shop drawings for the work of the mechanical, electrical and piping systems.

C. For limitations on excavations, see Cast-in-Place Concrete Work notes regarding Footing Work.

D. For limitations on openings and sleeves in footings, see Cast-in-Place Concrete Work note 4. For limitations on openings and embedded items in concrete slabs-on-metal deck, see Cast-in-Place Concrete Work note 7.

E. Maximum mechanical equipment weights are shown on the plans. These weights include all components of the units, curb, screen, etc. The contractor shall notify the engineer of actual weight if it exceeds the maximum weight published on the plans and revisions to the structural framing may be required. Attach all mechanical and electrical roof top equipment to the building structure as required by the equipment manufacturer to maintain equipment stability.

7. SPECIAL INSPECTION:

Special Inspection in accordance with Section 1704 of the 2006 International Building Code will be performed for the following types of work:

A. Soils: Special Inspection in accordance with Section 1704.7 of the IBC is required for site preparation, fill placement, and evaluation on in-place density of fill material.

B. Concrete & Reinforcing Steel : Special Inspection in accordance with Table 1704.4 of the IBC is required for all reinforced concrete, excluding slabs-on-grade that do not have reinforcing bars and site work concrete fully supported on earth. Cast-In anchor bolts and embed steel shall be inspected.

C. Structural Steel : Special Inspection in accordance with Section 1704 and Table 1704.3 of the IBC is required for all structural steel. In-plant special inspection is required for storm shelter structural steel and precast concrete.

D. Masonry & Reinforcing Steel : Special Inspection Level 1 in accordance with Table 1704.5.1 of the IBC is required for reinforced masonry. Prism tests are not required.

E. Post-Installed Anchors: Provide special inspection in accordance with the product's ICC/ES Report. All stress increases have been utilized.

F. Cold-Formed Metal Framing: Provide special inspection in accordance with Specification Section 05 40 00-"Cold-Formed Metal Framing" and AWS D1.3.

The Contractor shall provide the Special Inspector sufficient notification to allow the required inspections to be made without delaying the construction schedule. The Contractor shall confirm that ALL inspections have been completed and approved by the Special Inspector prior to proceeding with Work.

8. ABBREVIATIONS

ARCH.	Architect	HORIZ.	Horizontal
B.O.	Bottom Of	H.S.	Headed Stud
B.O.L.	Bottom Of Lintel	HSS.	Hollow Structural Section
B.O.S.	Bottom Of Steel	JST. BRG.	Joist Bearing
BOTT.	Bottom	o.c.	On Center
BRG.	Bearing	P.A.F.	Powder Actuated Fastener
BTWN.	Between	REINF.	Reinforcing
CLR.	Clear	REQ'D.	Required
COL.	Column	SIM.	Similar
CONC.	Concrete	STD.	Standard
CONT.	Continuous	STL.	Steel
COORD.	Coordinate	THK.	Thick
C.J.	Const. or Control Joint	T.O.	Top Of
d.b.a.	Deformed Bar Anchor	T.O.F.	Top Of Footing
DBL.	Double	T.O.S.	Top Of Steel
DBS.	Dowel Bar Substitute (Mechanical Splice)	T.O.W.	Top Of Wall
DET.	Detail	TYP.	Typical
EA.	Each	U.N.O.	Unless Noted Otherwise
EXIST.	Existing	VERT.	Vertical
EXP.	Expansion	w/	With
FDN.	Foundation		
FTG.	Footing		

STRUCTURAL STEEL WORK

1. MATERIALS:

Structural Members (W shapes):	ASTM A992, Grade 50, U.N.O.
Angles, Channels, Plates, and Bars:	ASTM A36, U.N.O.
Steel Tubes:	ASTM A500 Grade B
Steel Pipe:	ASTM A53, Type E or S, Grade B
Headed Studs:	ASTM A108, Fu = 65 ksi
Anchor Bolts:	ASTM F1554, Gr. 36, headed type; U.N.O.
Non-High Strength Bolts:	ASTM A307
High Strength Bolts:	ASTM A325 bolts, U.N.O.
Adhesive Anchors:	Hilti "HIT-RE 500-SD", Simpson "SET-XP", when anchoring to concrete. Hilti "HY 70", when anchoring to solid masonry.
Welding Electrodes:	E70
Deformed Bar Anchors:	ASTM A496, with a minimum tensile strength of 80 ksi.

2. STRUCTURAL STEEL:

A. All steel work shall comply with the 2006 International Building Code and AISC - American Institute of Steel Construction - "Code of Standard Practice for Steel Buildings and Bridges".

B. Comply with all applicable codes, ordinances, and regulations including those promulgated and enforced by OSHA. See STRUCTURAL GENERAL NOTE 5.B.

C. All structural steel shapes, plates, bolts, etc. exposed to weather or in direct contact with treated lumber shall be galvanized.

D. Install all post installed anchors in accordance with the manufacturer's written instructions and the product's ICC-ES report. Submit product ICC-ES report for all post installed anchors with shop drawings.

3. STEEL JOISTS:

A. All steel joist work shall comply with SJI - Steel Joist Institute - "Standard Specifications for Open Web Steel Joists".
Exception: Joist ends shall be designed to span to center of bearing plates as shown in the contract documents.

B. Steel Joists are designated on the drawings using Steel Joist Institute designations. Provide special joist designs as specifically noted on the plans or details. See load diagram 17/S3.2 for special extended joist ends and joist loading. Do not reduce the main span capacity of the steel joist due to the cantilever loading.

C. Steel joists shall be designed for a minimum net uplift of 15 psf unless noted otherwise and without a 1/3 stress increase.

D. Joist Bridging:

1. All bridging shall be continuous. Bridging may be terminated where necessary (e.g. for mechanical work) provided diagonal cross bridging is provided at each adjacent joist space. Bridging shall not be interrupted at two adjacent joist spaces.

2. Where diagonal cross bridging is used, provide horizontal bridging at first joist space adjacent to walls.

E. Do not place concentrated loads in excess of 100 lbs. between panel points of steel joists unless web members are installed in accordance with 9/S3.2. Joist reinforcing details assumes rod web members. If angle web members are provided the joist manufacturer shall provide an alternate detail of equivalent strength on the shop drawings.

F. Joist locations shall be shown on the shop drawings. Any deviations in locations for those shown on the drawings shall be highlighted.

G. All steel joists shall be designed for a minimum of 25% shear reversal. Snowdrifts have been accounted for in the standard joist designations shown on the plans.

4. METAL DECK:

A. Composite Metal Floor Deck:

(1). Vulcraft "3VLI", "3VLJ" or Approved Equal with the following minimum section properties:

Depth:	3"
Thickness:	18 ga.
Finish:	Galvanized
Yield Stress:	50 ksi

(2). All metal floor deck work shall comply with SDI - Steel Deck Institute "Specifications and Commentary for Composite Steel Floor Deck".

(3). Metal floor deck shown on the drawings shall be used in 2 or more span condition, unless noted or detailed otherwise. Metal deck will require shoring where clear span exceeds 11'-9".

(4). Metal floor deck shall be attached as follows, unless noted otherwise on the drawings:

a. Fasten deck to supports with 5/8" diameter puddle welds at each flute and at 18" o.c. where deck is parallel to supports, unless noted otherwise.

b. Fasten side laps of individual sheets together between supports with #10 Teks screws (w/ Vulcraft VLJ deck) or button punches at a spacing of 18"o.c. maximum between supports.

(5). Openings thru Slab-on-Metal Deck. Some but not all openings are shown on the plans. See Cast-in-place Concrete Work note 7.C.

B. Metal Roof Deck:

(1) Vulcraft "1.5B", or Approved Equal with the following minimum section properties:

Depth:	1 1/2"
Thickness:	22 ga.
Finish:	Painted
Yield Stress:	33 ksi

(2). Metal roof deck shown on the drawings shall be used in 3 or more span condition, unless noted or detailed otherwise.

(3). Metal roof deck shall be attached as follows, unless noted otherwise on the drawings:

a. Fasten deck to perpendicular supports at each flute. For deck to structural steel, use 5/8" dia. puddle welds.

b. Fasten deck to parallel supports at 12" o.c. For deck to structural steel, use 5/8" dia. puddle welds.

c. Fasten side laps of individual sheets together with #10-16 Tek screws at 12"o.c., unless noted otherwise on the plans.

(4). All edges and openings in deck shall be supported. Unless noted or approved otherwise, provide support in accordance with the following:

Openings less than 8" in either direction and spaced farther that 2'-6" o.c.:
Add a 2'-0" (min.) wide x 3'-0" long piece of deck nested on top of deck screw flutes together with #10 screws at 6" centers and the opening may be cut through the center of the deck. If openings are spaced closer than 2'-6" o.c. frame as noted below for larger openings.

Openings greater than 8" in either direction or openings for ductwork:
Support edges of opening with steel framing supported by steel joists or beams as shown on 14/S3.2. steel deck shall be welded to framing with welds at 12" centers. See Detail 14/S3.2 for steel framing required around opening.

(5). Up to 20 pounds may be supported from 1 1/2" metal deck provided the attachment to the deck distributes the load to at least (3) deck flutes and there is only (1) attachment to any deck flute in any particular span.

(6). Provide reinforcing in the roof deck per 15/S3.2 at locations of equipment and concentrated loads on top of the roof deck.

CAST-IN-PLACE CONCRETE WORK

1. MATERIALS:

Concrete:

Class	Locations	28-Day Strength	Max. Slump	Max. Aggregate	Air Entrainment
TYPE I	Footings	3000 PSI	5"	1"	5%-7%
TYPE II	Exterior Exposed Concrete, Structural Stoops*, Columns, Walls, Stem Walls, Ext.	4000 PSI	5"	3/4"	5%-7%
TYPE III	Interior Slab on Grade	3000 PSI	5"	1"	Not Req'd
TYPE IV	Concrete Slab on Metal Deck	4000 PSI	4"	3/4"	Not Req'd

* Exterior concrete slabs shall conform to NDOR Mix Design 47BD-4350. Type F fly ash shall be used at these locations and Type C fly ash is not permitted at these locations. Modify maximum aggregate size as required above.

Other Materials:

Reinforcing Bars:	ASTM A615 Grade 60, deformed
Weldable Reinforcing Bars:	ASTM A706 Grade 60, deformed
Deformed Bar Anchors:	ASTM A496, with a minimum tensile strength of 80 ksi.
Welded Wire Fabric:	ASTM A185, flat sheet type.
Anchor Bolts:	ASTM F1554, Gr. 36, unless noted otherwise.
Adhesive Anchors:	Hilti "HIT-RE 500-SD", Simpson "SET-XP", when anchoring to concrete.
Expansion Anchors:	Hilti "Kwik Bolt TZ" or Simpson "Strong-Bolt"

2. CONTINUITY:

All reinforcing shall be continuous unless noted otherwise. Continuity at corners and intersections shall be achieved using corner bars and contact lap splices, see detail 1/S3.1. Continuity at other locations may be achieved using contact lap splices shown on approved shop drawings. Location of lap splices shall be shown on the shop drawings. Unless noted otherwise, the following lap splices shall be used:

Location:	#3	#4	#5	#6	#7	#8	#9	#10	#11
3000 PSI and 4000 PSI Concrete:									
Top Bars (*):	21"	28"	35"	46"	63"	82"	104"	132"	162"
Other Bars:	16"	22"	27"	35"	48"	63"	80"	102"	125"

(*) Top bars are horizontal reinforcing where more than 12" of concrete is cast in the member below the reinforcing.

Mechanical connections may be used in lieu of lap splices provided approval is obtained from the Architect/Engineer. Connections shall develop in tension 125 percent of the specified yield strength of the bar. All mechanical connections shall be shown on the shop drawings and be installed in accordance with the manufacturer's written instructions and the product's ICC-ES report. Submit the product's ICC-ES report for mechanical splice products with shop drawings.

3. GENERAL:

A. All concrete work shall comply with ACI 301.

B. Coordinate work with all other work.

C. All reinforcing shall be continuous, see notes above. All reinforcing, anchor bolts, and other embedded items shall be secured in place prior to placing concrete. Do not interrupt reinforcing, anchor bolts, or embedded items with plumbing or sleeves.

D. Construction joints shall be keyed joints, unless noted otherwise, with reinforcing continuous through the joint. Concrete on one side of construction joints shall not be placed less than 48 hours after placement of concrete on the opposite side of the construction joint.

E. Clear cover from reinforcing to surfaces of concrete shall be as shown. Clear distance between parallel bars in a layer shall be as shown on the plans with minimum of 1 1/2" or the diameter of the reinforcing, whichever is greater. Clear distance between parallel bars in two or more layers shall be as shown on the plans with a minimum of 1 1/2" or the diameter of the reinforcing, whichever is greater.

F. Install all post installed adhesive anchors in accordance with the manufacturer's written instructions and the product's ICC-ES report. Submit product ICC-ES report for all post installed adhesive anchors with shop drawings.

4. FOOTING WORK:

A. See plans for 'Footing Schedule'. Coordinate footing work with all other work.

B. Pipes, underground ductwork, and other work which require trenching adjacent to pad footings and parallel to continuous footings shall not be located below lines extending downward from the bottom edges of the footing at a 45-degree angle from the horizontal. Pipes and other work perpendicular to continuous footings may be located beneath the footing. Footing elevations may be lowered if approved on the footing shop drawings.

C. Provide steps in footings per 2/S3.1. Show all footing step locations and reinforcing on the footing reinforcing shop drawings.

D. Protect soils under footings from frost during cold weather construction and protect soils under footings from moisture variation per specifications.

E. Sleeves shall consist of galvanized standard pipe. Do not displace the reinforcing steel. Provide 2" minimum concrete clear cover between reinforcing steel and sleeve and place edge of sleeves 6" minimum from the top and bottom of footings. Where pipes are sleeved through footings, provide space all around pipe to allow for settlement of footing. Vertical penetrations are not permitted through footings.

5. INTERIOR SLAB-ON-GRADE WORK:

A. Coordinate slab-on-grade work with all other work. Provide slopes, thickened slabs, depressed slabs, equipment pads, blockouts, etc. as needed.

B. Sawcut control joints in slab to a depth equal to 1/3 the slab thickness.

C. Slab-on-grade Requirements:

(1). Thickness: 4" minimum.
(2). Reinforcing: 6x6 - W1.4xW1.4 welded wire fabric (w.w.f.)
(3). Control Joints: Sawcut at 10'-0"o.c. maximum each way along column lines and at equal spaces between column lines unless noted otherwise.

D. All slabs-on-grade shall have a vapor barrier beneath the slab. Place a 4" compacted aggregate leveling course over compacted subgrade.

E. All interior slabs-on-grade shall have the subgrade scarified to 12" depth and re-compacted to the requirements of structural fill immediately prior to placing granular leveling course and slab-on-grade.

F. Separate slab-on-grade from all columns, walls, or other vertical surfaces w/ 3/8" expansion joint material. Differential movement could occur between foundations and slab-on-grade.

6. STRUCTURAL STOOPS:

A. Slabs at structural stoops shall be 4" minimum/8" maximum concrete slab on 4" biodegradable void forms, unless noted otherwise. See 3/S3.1 for stoop construction and reinforcing.

B. Stoop slabs shall bear on and/or be supported on all sides by concrete walls, masonry walls, or footings as shown on the drawings. Coordinate stoop size with Architectural Drawings.

7. CONCRETE SLABS on METAL DECK:

A. Place concrete slab over metal deck with the thickness described on the plans. The contractor shall maintain the slab thickness when placing the floor slab. The floor slab thickness will not change due to beam deflection or beam camber.

B. See plans and sections for slab reinforcing requirements.

C. Openings in Slabs:

(1) Some but not necessarily all openings are shown on the Structural Drawings. Contractor shall coordinate location and size of all openings with other work [e.g.: Mech., Elec., etc.].

(2) Openings in concrete slabs on composite metal deck shall be reinforced:

a. For openings less than 6" in maximum direction and provided there are no other openings within 12" (edge to edge), slab reinforcing bars shall be relocated to each side of opening and one additional #4 x 6'-0" placed in bottom of flute each side of opening.

b. Several individual openings located near each other may be considered as one opening in determining the reinforcing requirements. For example, (3)-2" diameter openings at 3" center - center along a single line may be considered as a 2" x 8" opening.

c. Openings with a side greater than 6": Provide supplemental reinforcing per 16/S3.2.

(3) Any openings which can not be made in accordance with the requirements above shall be brought to the attention of the Architect/Engineer for determination of reinforcing requirements.

(4) All openings and reinforcing provisions for each opening must be shown on the approved slab reinforcing shop drawings.

D. Embedded mechanical and electrical work: Embedded boxes shall be reinforced as openings as described above. Embedded conduit and piping in the slab shall not be larger than 1" in diameter, shall not be spaced closer than 6" o.c., shall not interfere with the reinforcing placement shown on these drawings, shall be placed 1" clear from the top of concrete, bottom of concrete, and reinforcing bars and W.W.F.

E. Do not support piping in excess of 500 lb from slab-on-metal deck. Any location where larger loads occur supplemental framing shall be provided to transfer the loading to the steel framing members. All plumbing attachments to slabs and structure shall be designed by the plumbing contractor's engineer.

COLD-FORMED METAL FRAMING NOTES

1. GENERAL:

A. All cold-formed metal framing work shall comply with the 2009 International Building Code and AISI - American Iron and Steel Institute - "Cold-Formed Steel Design Manual".

B. Structural cold-formed metal framing is all joists, load-bearing walls, exterior walls, soffits, parapets, cold-formed channel framing (Unistrut), and cold-formed metal roof system.

C. Structural cold-formed metal framing shall be constructed to achieve the geometry shown on the Architectural or Structural drawings.

D. See Specification Section 05 40 00 for submittal requirements and additional information not noted below.

E. Studs shall be 6"-18ga. and spaced at 16"o.c. unless noted otherwise on the plans.

F. Field cutting of steel framing members shall be by saw or shear. Torch cutting will not be permitted.

G. Temporary bracing shall be provided and remain in place until work is permanently stabilized.

H. Connect multiple studs and track together with #10-16 TEK screws at 12" unless noted otherwise on the drawings.

I. P.A.F.'s shall be 0.157" DIA. Hilti X-U or Simpson PDPA pins. All tracks shall be attached to steel or concrete with (2) pins at 16" o.c., unless noted otherwise on the drawings. Pins attached to concrete shall be embedded 1 1/4" into the concrete unless noted otherwise. Do not damage concrete reinforcing with pins. Install all pins per the product's ICC-ES report and the manufacturer's written instructions.

2. METAL STUD WALLS:

A. Where sheathing does not occur on both sides of wall, provided lateral bracing per 1/S1.0 unless otherwise shown on the drawings.

B. At track butt joints, abutting pieces of track shall be securely anchored to a common stud. Where splicing of wall track is necessary between stud spacings, a piece of stud shall be placed in the adjoining track sections and fastened to the track flanges at both sides of the wall with (3)-#12-14 TEK screws on each side of the joint unless noted otherwise on the drawings.

C. Wall studs shall be installed seated squarely (within 1/16") against the web of the top and bottom track, unless a movement joint is specifically detailed. Studs shall be plumbed, aligned and secured to the continuous runner tracks with a minimum of (1) #10-16 TEK screw at each flange.

D. Wall tracks shall be 18ga. minimum, unless noted otherwise on the drawings.

E. All openings shall have headers and jamb studs. See 3/S3.3 for the requirements, unless noted otherwise.

MASONRY WORK

1. MATERIALS:

Reinforcing Bars:	ASTM A615 Grade 60, deformed.
Concrete Masonry Units:	ASTM C90, Grade N; F'm = 1500 psi (Type M or S mortar)
Grout Strength:	ASTM C476, 2000 psi @ 28 days
Adhesive Anchors:	Hilti "HY 70" or approved equal

2. CONTINUITY:

All reinforcing shall be continuous unless noted otherwise. Continuity at corners and intersections shall be achieved using corner bars and contact lap splices, see detail 6/S3.1. Continuity at other locations may be achieved using contact lap splices shown on approved shop drawings. Location of lap splices shall be shown on the shop drawings. Unless noted otherwise, the following lap splices shall be used:

Location:	#3	#4	#5	#6
Masonry	18"	24"	30"	57"

Mechanical connections may be used in lieu of lap splices provided approval is obtained from the Architect/Engineer. Connections shall develop in tension 125 percent of the specified yield strength of the bar. All mechanical connections shall be shown on the shop drawings and be installed in accordance with the manufacturer's written instructions and the product's ICC-ES report. Submit the product's ICC-ES report for mechanical splice products with shop drawings.

3. Masonry work shall comply with the 2006 International Building Code, ACI 530-05 and ACI 530.1-05. All bars must be held in correct position prior to special inspection and grouting operations. Use mechanical devices to secure bars in the correct position prior to grouting such as rebar positioners.

4. Requirements for masonry wall construction are given on the drawings, Wall Schedule, Lintel Schedule, and in the notes below. All reinforcing shall be continuous in grout-filled cells. See note above for continuity requirements.

5. Provide dowels in footings and concrete walls at each vertical rebar. Dowels shall be same size as vertical reinforcing.

6. Locate vertical reinforcing at corners, jambs, intersections, end of wall, and at spacings noted on the drawings. Provide (1)-#4 minimum unless noted otherwise. Reinforcing at jambs shall be continuous from footing (thickened slab) to the top of the wall. See note 2 for continuity requirements.

7. All openings in CMU walls shall have a lintel at the head. Provide 16" minimum between adjacent individual openings. For openings less than 4'-0" provide Lintel 'L1'. Provide a knock-out bond beam with (2)-#4's extending 24" past jambs at each sill. For openings greater than 4'-0", if a lintel is not indicated on the structural drawings, submit opening size and location to the Architect/Engineer for determination of the lintel to be used.

8. Core drilled holes are not permitted in lintels or masonry cells with reinforcing, holes in these areas must be formed or sleeved. Holes less than 6" in diameter at other locations may be core drilled, formed or sleeved without a lintel provided they are located at least 16" from another opening.

9. Embedded electrical conduit shall be 1 1/2" O.D. max, placed 2 1/2" CLR. reinforcing steel, and centered in the CMU wall where occurring. Embedded plumbing shall be placed in cells without reinforcing steel bars. If masonry webs or face shells are required to be cut for embedding plumbing or electrical conduit horizontal joint reinforcing shall be provided in the wall at 8" o.c. within 4'-0" of the location.

10. CONTROL JOINTS. Locate control joints at 20'-0" o.c. maximum. Panels between control joints shall not exceed an aspect ratio of 1 to 1.5 (height to length). Do not locate a control joint closer than 16" from an opening. Locate control joints within 4'-0" at one side of corners and two sides at intersections. Construct control joints per detail 7/S3.1. All horizontal joint reinforcing shall be terminated at the control joint.

11. VENEER. All veneer shall be supported at the head of openings. Unless noted otherwise, see detail 2/S1.0.

12. Where continuous vertical reinforcing is interrupted by a steel beam or steel joist, locate reinforcing to an adjacent cell and lap reinforcing one splice length plus the distance of the offset.

13. Where continuous horizontal reinforcing is not at the same elevation, lap reinforcing one lap splice length plus the vertical distance between the rebars. Where continuous reinforcing is interrupted by a steel beam, locate reinforcing below the beam and lap with the continuous reinforcing as noted above.

14. Submit reinforcing shop drawings for masonry work. Provide wall elevations showing the location of openings in masonry and the location of reinforcing including lap splices. Vertical reinforcing lengths shall be coordinated with heights of CMU to be placed.

15. Infill openings in existing walls where infill is less than 6'-0" wide at locations shown on Architectural Drawings. Provide #4's at 4'-0" o.c. horizontal centered in knock-out bond beams and embed reinforcing 6" minimum each end of existing opening in adhesive with screen tubes in addition to vertical reinforcing described in note 6 above. For other infill locations submit existing opening size and location to the Architect/Engineer for determination of infill requirements.

EXISTING WORK

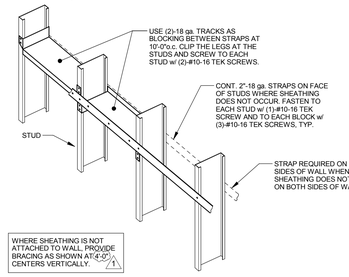
1. Existing conditions shown or noted on the drawings were obtained from existing plans or or were assumed. If conditions other than those shown exist, immediately notify Architect before proceeding with the work at that location. If conditions other than those shown exist, alternate methods of construction may need to be used.

2. Where specifically noted on the drawings that existing construction be verified, notify Architect/Engineer in writing of the findings.

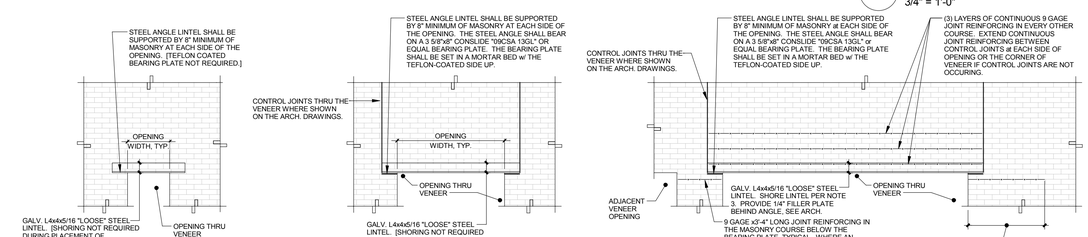
3. Use appropriate construction methods and equipment as necessary to support existing structures and to avoid overstressing the existing structures.

5. All new openings in existing walls shall have a lintel at the head. Lintels to be used are shown on the Architectural Demolition drawings or on the Structural drawings. For openings and lintels not shown on those sheets, use 4/S3.3 for openings less than 6'-0" at existing masonry veneer over existing cold formed metal framing. Do not locate openings which are not shown on the structural drawings beneath existing joists or beams. For any openings which cannot be made in accordance with the requirements noted above, submit opening size and location to the Architect/Engineer for determination of the lintel to be used.

6. New openings in existing roof deck shall be supported as required for openings in new metal roof deck, see 14/S3.2 for requirements.



1 LIGHTGAGE STUDWALL BRACING
3/4" = 1'-0"



NOTES FOR OPENING WIDTH GREATER THAN 5'-0" & LESS THAN OR EQUAL TO 9'-0":
1. JOINT REINFORCING SHALL BE HOT-DIP GALV. TRUSS-TYPE OR LADDER-TYPE FABRICATED FOR 4" NOMINAL VENEER.
2. IF MORE THAN (1) - SECTION OF JOINT REINFORCING IS REQUIRED IN A ROW, OVERLAP 1'-6" MINIMUM.
3. SHORE THE STEEL ANGLE DURING PLACEMENT OF THE MASONRY VENEER UNTIL THE FULL HEIGHT OF VENEER OVER THE OPENING HAS OBTAINED DESIGN STRENGTH. SHORING SHALL BE at 4'-0"o.c. MAXIMUM AND BE DESIGNED TO SUPPORT THE FULL WEIGHT OF THE VENEER ABOVE THE OPENING.
4. NOTIFIY ENGINEER OF ANY OPENINGS GREATER THAN 9'-0" WIDE TO DETERMINE CONSTRUCTION REQUIREMENTS.

2 MASONRY VENEER OPENING DETAILS
3/8" = 1'-0"

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JEREMY T. STEENBOEN E-11656 7/24/15 STATE OF NEBRASKA

1	ADDENDUM 2	08/07/2015

△REVISION	DATE

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

STRUCTURAL NOTES

S1.0

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Omaha, NE 68154
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TD2 PROJECT # 748-478

FOOTING SCHEDULE

MARK	SIZE W	SIZE L	SIZE D	REINFORCING	REMARKS
CF1-0G	1'-0"	CONT.	3'-0"	CONT. #5 LONGITUDINAL TOP, MIDDLE, AND BOTTOM	- STOOP FOOTINGS
CF1-8B	1'-8"	CONT.	1'-4"	(2)-CONT. #4's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF1-8G	1'-8"	CONT.	3'-0"	(2)-CONT. #5's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF2-6B	2'-6"	CONT.	1'-4"	(2)-CONT. #5's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF2-8G	2'-8"	CONT.	3'-0"	(3)-CONT. #5's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF3-6G	3'-6"	CONT.	3'-0"	(4)-CONT. #5's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF4-6G	4'-6"	CONT.	3'-0"	(4)-CONT. #6's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF5-0B	5'-0"	CONT.	1'-4"	(6)-CONT. #5's LONGITUDINAL BOTTOM #4's AT 12"o.c. TRANSVERSE BOTTOM	
CF5-6G	5'-6"	CONT.	3'-0"	(5)-CONT. #6's LONGITUDINAL TOP AND BOTTOM #4 CLOSED STIRRUPS AT 4'-0" o.c.	
CF6-6B	6'-6"	CONT.	1'-4"	(8)-CONT. #5's LONGITUDINAL BOTTOM #5's AT 10"o.c. TRANSVERSE BOTTOM	
PF9-0C	9'-0"	9'-0"	1'-8"	(8)-#6'sx6'-6" EACH WAY BOTTOM	

FOOTING SCHEDULE NOTES:
1. PROVIDE REINFORCING CONTINUITY AT ALL INTERSECTIONS AND CORNERS OF FOOTINGS PER 1/S3.1.
2. WALL FOOTING REINFORCING SHALL BE CONTINUOUS THROUGH COLUMN FOOTINGS.
3. SEE PLANS AND DETAILS FOR ADDITIONAL REINFORCING
4. SEE PLANS FOR INCREASED FOOTING DEPTHS.

LINTEL SCHEDULE

MARK	SIZE AND REINFORCING	DETAIL	REMARKS
L1	8"x8" BOND BEAM w/ (2)-#5's BOTTOM WITH 24" BEARING AT EACH END	18/S3.2	- PROVIDE REINFORCING 3" CLR. BOTTOM OF LINTEL.
L2	12"x8" BOND BEAM w/ (2)-#5's BOTTOM WITH 24" BEARING AT EACH END	12/S3.2 AND 18/S3.2	- PROVIDE REINFORCING 3" CLR. BOTTOM OF LINTEL.
L3	8"x8" BOND BEAMS AT ELEVATIONS 94'-4" 95'-0" w/ (2)-#5's BOTTOM EACH WITH 24" BEARING AT EACH END	12/S3.2 AND 18/S3.2	- PROVIDE REINFORCING 3" CLR. BOTTOM OF LINTEL.
L4	KNOCK-OUT BOND BEAM CONTINUOUS WITH HORIZONTAL WALL REINFORCING	12/S3.2 AND 18/S3.2	- PROVIDE REINFORCING 3" CLR. BOTTOM OF LINTEL.

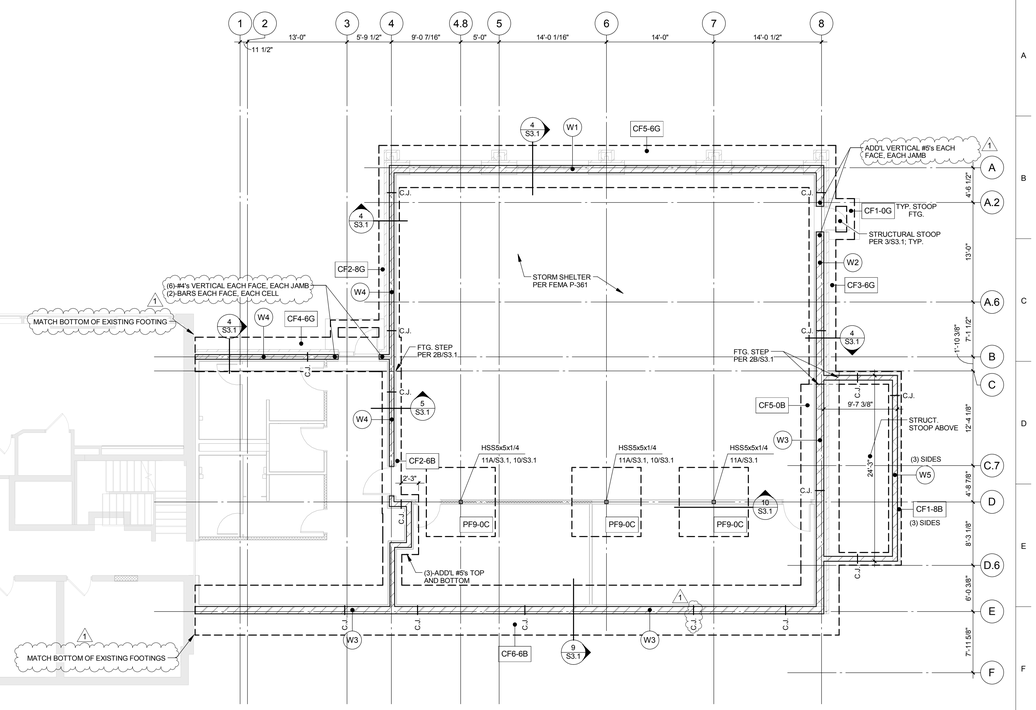
LINTEL SCHEDULE NOTES:
1. BOND BEAM ELEVATIONS ARE NOTED AT THE BOTTOM OF BOND BEAMS.

WALL SCHEDULE

MARK	TYPE	THICKNESS /SIZE	REINFORCING VERTICAL	REINFORCING HORIZONTAL	REMARKS
W1	MASONRY GROUT SOLID	12" [NOMINAL]	#5's AT 8"o.c. EACH FACE	9ga. HORIZONTAL JOINT REINFORCING AT 16"o.c. AND KNOCKOUT BOND BEAMS w/ (2) CONT. #5's AT 89'-0", 96'-4", 97'-0", AND TOP OF WALL	- FORM BLOCK
W2	MASONRY GROUT SOLID	12" [NOMINAL]	#5's AT 24"o.c. EACH FACE	9ga. HORIZONTAL JOINT REINFORCING AT 16"o.c. AND KNOCKOUT BOND BEAMS w/ (2) CONT. #5's AT TOP OF WALL	- FORM BLOCK
W3	MASONRY GROUT SOLID	12" [NOMINAL]	(2)-#4's AT 8"o.c. INSIDE FACE	9ga. HORIZONTAL JOINT REINFORCING AT 16"o.c. AND KNOCKOUT BOND BEAMS w/ (2) CONT. #5's AT TOP OF WALL	- FORM BLOCK
W4	MASONRY GROUT SOLID	8" [NOMINAL]	#5's AT 8"o.c. EACH FACE	9ga. HORIZONTAL JOINT REINFORCING AT 16"o.c. AND KNOCKOUT BOND BEAMS w/ (2) CONT. #5's AT TOP OF WALL	- FORM BLOCK
W5	MASONRY GROUT SOLID	8" [NOMINAL]	#5's AT 8"o.c. EACH FACE	9ga. HORIZONTAL JOINT REINFORCING AT 16"o.c. AND KNOCKOUT BOND BEAMS w/ (2) CONT. #4's AT TOP OF WALL	- FORM BLOCK

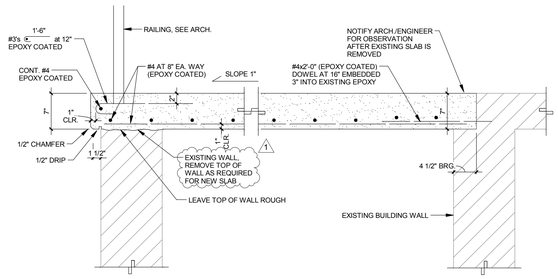
WALL SCHEDULE NOTES:
1. PROVIDE REINFORCING CONTINUITY AT ALL INTERSECTIONS PER 6/S3.1.
2. PROVIDE ADDITIONAL REINFORCING SHOWN ON PLANS AND/OR DETAILS.
3. BOND BEAM ELEVATIONS ARE INDICATED AT BOTTOM OF BOND BEAMS.
4. PLACE BARS AT FACES OF CMU WITH 2 1/4" CLEAR COVER.

FOUNDATION PLAN NOTES:
1. SEE STRUCTRUAL NOTES ON SHEET S1.0 1
2. COORDINATE AND VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND EXIST. CONDITIONS.
3. TOP OF FOOTING ELEVATION = 86'-0" UNLESS NOTED OTHERWISE THUS: T.O.F. = xx'-x"
4. -F---- : FOOTING MARK, SEE SCHEDULE ON SHEET S1.1.
5. W- : WALL MARK, SEE SCHEDULE ON SHEET S1.1.
6. C.J. —— : INDICATES CONTROL JOINT PER 7/S3.1.



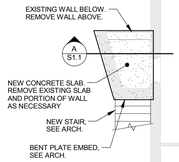
LOWER FOUNDATION PLAN

1/8" = 1'-0"



A NEW STAIR LANDING

1" = 1'-0"



FLOOR FRAMING PLAN NOTES:
1. SEE STRUCTRUAL NOTES ON SHEET S1.0 1
2. COORDINATE AND VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND EXIST. CONDITIONS.
3. TOP OF STEEL ELEVATION = 99'-4" UNLESS NOTED OTHERWISE AS SPECIFIC ELEVATIONS ON INDIVIDUAL MEMBERS.
4. (X) : INDICATES SPAN DIRECTION OF METAL DECK NOTED BELOW:
   (2) : 5" CONCRETE SLAB ON 3"-18ga. VULCRAFT '3VLI' GALVANIZED COMPOSITE METAL FLOOR DECK (5" TOTAL THICKNESS). REINFORCE SLABS-ON-METAL DECK WITH #4's AT 12"o.c. EACH WAY BOTTOM AND #4's AT 12"o.c. EACH WAY TOP. PROVIDE REINFORCEMENT 2 1/2" CLEAR BOTTOM OF DECK FLUTES AND 1" CLEAR TOP OF SLAB AT BARS PARALLEL TO DECK FLUTES. PLACE REINFORCING PERPENDICULAR TO DECK SPAN ON TOP OF DECK FLUTES. ALL REINFORCING BARS SHALL BE SECURED IN CORRECT LOCATIONS PRIOR TO PLACING CONCRETE. SEE PLANS AND DETAILS FOR ADDITIONAL REINFORCING. SHORE METAL DECK WHERE CLEAR SPAN EXCEEDS 11'-9".
5. HEADED STUD (H.S.) REQUIREMENTS
   -STUD SIZE: 3/4"DIA.x6"
   -LOCATE FIRST STUD 12" MAX. FROM END OF BEAM
   -BEAMS:
   **W_x_(xx): THE NUMBER IN PARENTHESIS INDICATES THE NUMBER OF HEADED STUDS REQUIRED; STUDS SHALL BE AT 24"o.c. ALONG THE ENTIRE LENGTH OF THE BEAM w/ HALF OF THE BALANCE OF STUDS PLACED AT 12"o.c. AT EACH END OF BEAM.(AT BEAMS WIDTHS WHICH ARE WIDER THAN 10" UP TO 3 ROWS OF STUDS MAY BE PROVIDED AT 4"o.c. PLACED SYMTRICALLY ABOUT THE CENTER LINE. 1
6. CONNECT BEAMS TO COLUMNS PER 2A/S3.2, UNLESS NOTED OTHERWISE.
7. CONNECT BEAM TO BEAMS PER 1B/S3.2 UNLESS NOTED OTHERWISE. WHERE EQUAL DEPTH BEAMS FRAME ACROSS FROM EACH OTHER, CONNECT PER 1E/S3.2. WHERE BEAMS FRAME INTO EACH OTHER AT A SKEW, CONNECT PER 1D/S3.2. CONNECTION 1C/S3.2 IS ONLY PERMITTED WHERE SPECIFICALLY NOTED. PROVIDE BEAM SPLICES PER 1A/S3.2.
8. (L) : INDICATES LINTEL, SEE LINTEL SCHEDULE ON SHEET S1.1.

FOUNDATION AND FIRST FLOOR FRAMING PLANS

1/8" = 1'-0"

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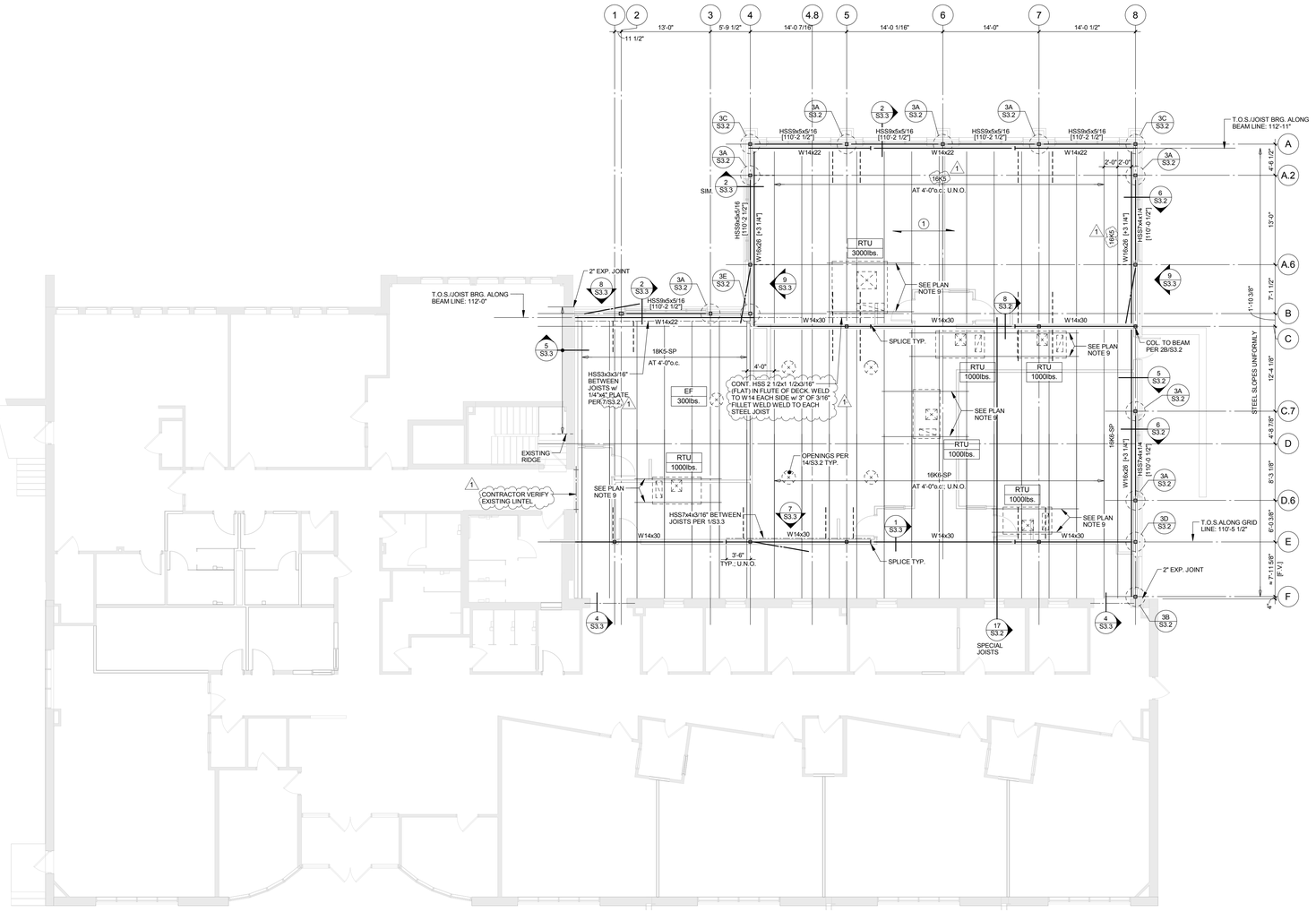
△	REVISION	DATE
1	ADDENDUM 2	08/07/2015

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

FOUNDATION AND FLOOR FRAMING PLANS

S1.1



ROOF FRAMING PLAN NOTES:

1. SEE STRUCTRUAL NOTES ON SHEET S1.0 / 1
2. COORDINATE AND VERIFY ALL DIMENSIONS WITH ARCHITECTURAL DRAWINGS AND EXIST. CONDITIONS.
3. TOP OF STEEL IS NOTED ON THE PLANS.
4. (X) : INDICATES SPAN DIRECTION OF METAL DECK NOTED BELOW:
   (1) : 1 1/2" 22ga. VULCRAFT 1.5B' METAL ROOF DECK.
5. (L) : INDICATES LINTEL. SEE LINTEL SCHEDULE ON S1.1.
6. CONNECT BEAMS TO COLUMNS PER 2A/S3.2 WHERE BEAMS FRAME OVER COLUMNS, UNLESS NOTED OTHERWISE.
7. CONNECT BEAM TO BEAMS PER 1B/S3.2, UNLESS NOTED OTHERWISE. CONNECT BEAMS TO BEAMS PER 1A/S3.2 AT SPICES. CONNECTION 1C/S3.2 IS ONLY PERMITTED WHERE SPECIFICALLY NOTED.
8. : INDICATES L2x2x1/4" BRACING FROM BEAM BOTTOM FLANGE TO JOIST TOP CHORD PANEL POINT. FILED WELD EACH END w/ 1 1/2" OF 3/16" FILLET WELD.
9. (2)-HSS1 1/2x1 1/2x3/16" IN FLUTES OF DECK UNDER EACH RTU. WELD EACH SIDE TO STEEL JOIST.

ROOF FRAMING PLAN

1/8" = 1'-0"

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1	ADDENDUM 2	08/07/2015

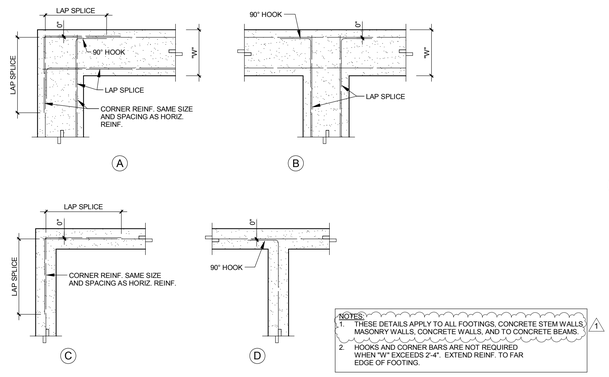
△REVISION	DATE

PROJECT NUMBER: 15008

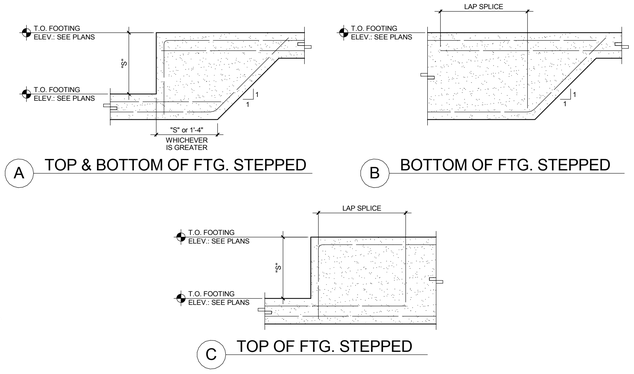
DATE: JULY 24, 2015

ROOF FRAMING PLAN

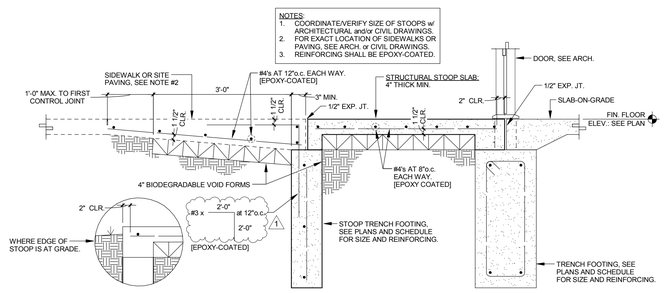
S1.2



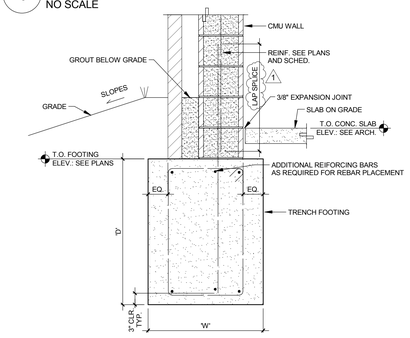
1 CORNER REINFORCING DETAILS
NO SCALE



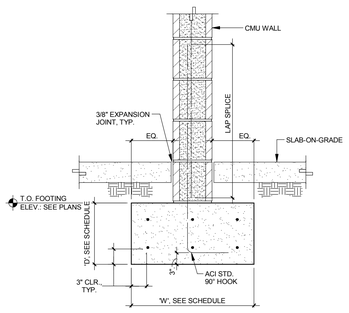
2 FOOTING STEP DETAILS
1/2" = 1'-0"



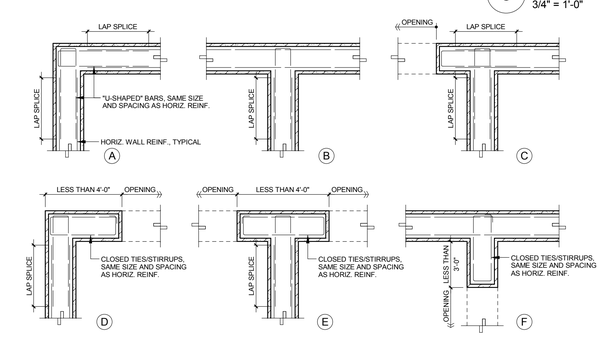
3 STRUCTURAL STOOP
3/4" = 1'-0"



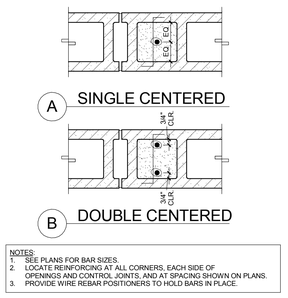
4 TYP. EXTERIOR FTG.
3/4" = 1'-0"



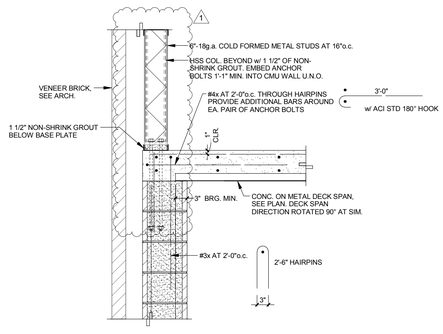
5 FOOTING AT INT. CMU WALL
1" = 1'-0"



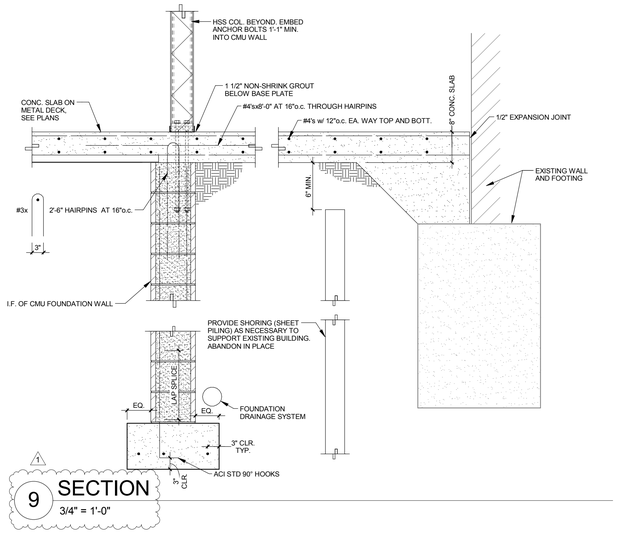
6 MASONRY WALL - REINFORCING DETAILS
NO SCALE



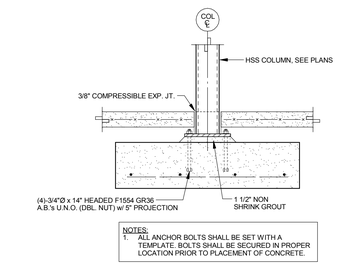
7 PLAN
1 1/2" = 1'-0"



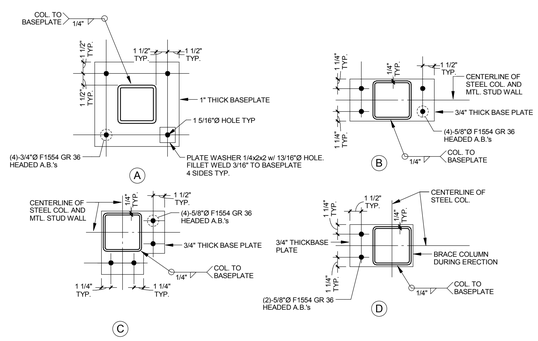
8 SECTION
3/4" = 1'-0"



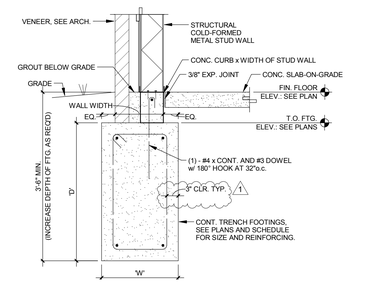
9 SECTION
3/4" = 1'-0"



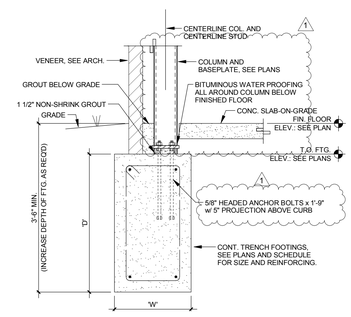
10 HSS COL. AT PAD FOOTING
3/4" = 1'-0"



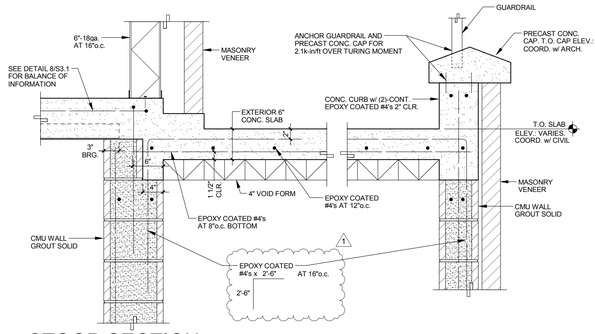
11 BASEPLATE DETAILS
1 1/2" = 1'-0"



12 TYP. EXTERIOR WALL FOOTING
3/4" = 1'-0"



13 COL. AT EXT. WALL FOOTING
3/4" = 1'-0"



14 STOOP SECTION
1" = 1'-0"

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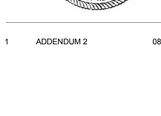


1	ADDENDUM 2	08/07/2015

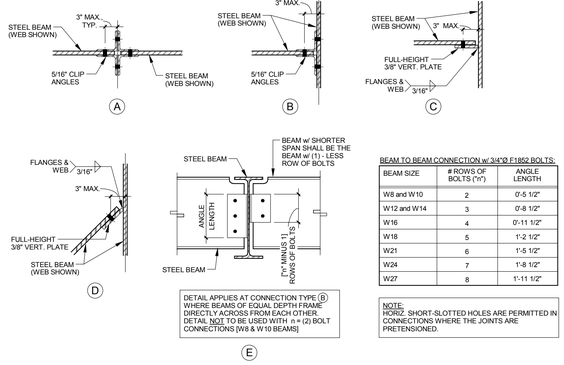
△REVISION	DATE

PROJECT NUMBER: 15008

DATE: JULY 24, 2015

STRUCTURAL SECTIONS

S3.1



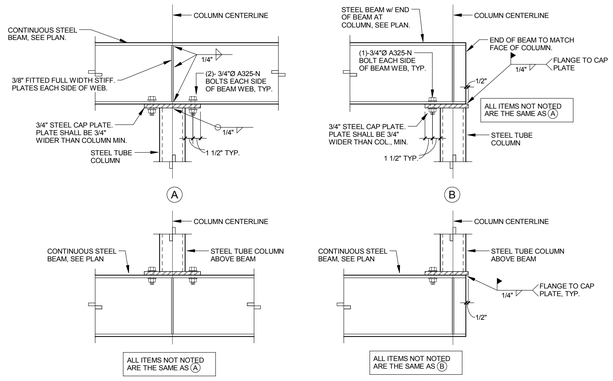
DETAIL APPLIES AT CONNECTION TYPE (B) WHERE BEAMS OF EQUAL DEPTH FRAME DIRECTLY ACROSS FROM EACH OTHER DETAIL NOT TO BE USED WITH n + (2) BOLT CONNECTIONS [W8 & W10 BEAMS]

BEAM TO BEAM CONNECTION w/ 3/4"Ø F1852 BOLTS:

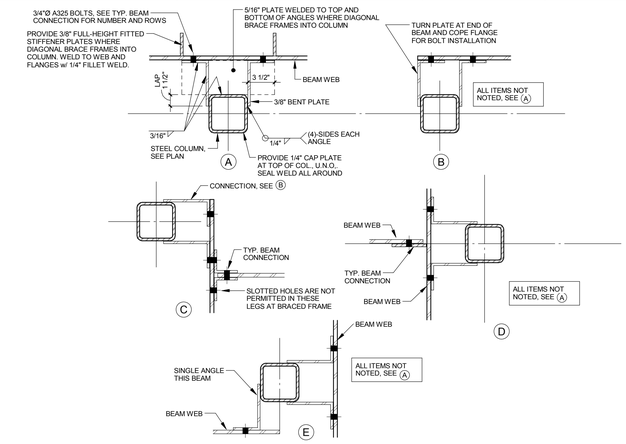
BEAM SIZE	# ROWS OF BOLTS ("n")	ANGLE LENGTH
W8 and W10	2	0'-5 1/2"
W12 and W14	3	0'-8 1/2"
W16	4	0'-11 1/2"
W18	5	1'-2 1/2"
W21	6	1'-5 1/2"
W24	7	1'-8 1/2"
W27	8	1'-11 1/2"

NOTE:
HORIZ. SHORT-SLOTTED HOLES ARE PERMITTED IN CONNECTIONS WHERE THE JOINTS ARE PRETENSIONED.

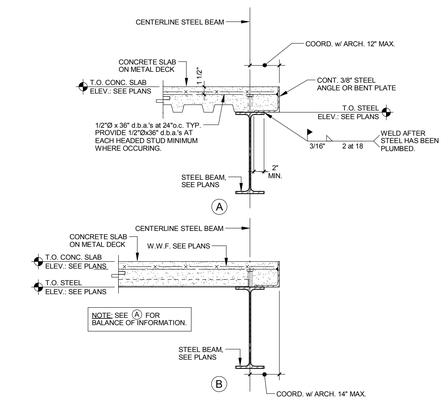
1 TYP. STEEL BEAM TO BEAM CONNECTIONS
1" = 1'-0"



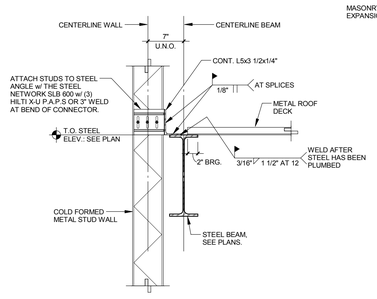
2 BEAM TO STEEL TUBE COLUMN CONNECTIONS
1" = 1'-0"



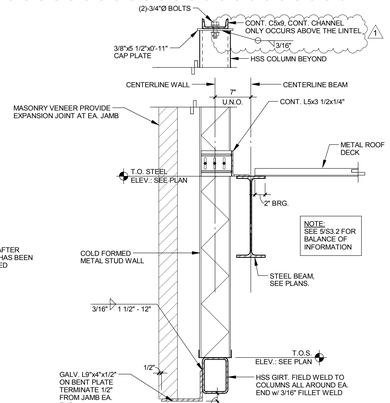
3 BEAM TO HSS COL. CONN.
1 1/2" = 1'-0"



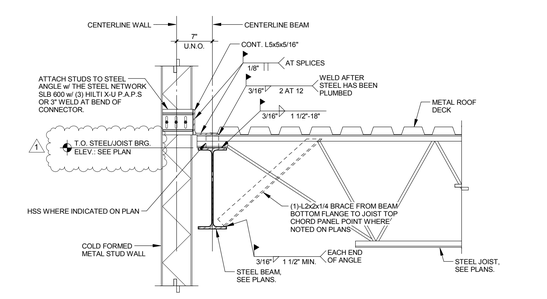
4 SECTION at SLAB EDGE
1" = 1'-0"



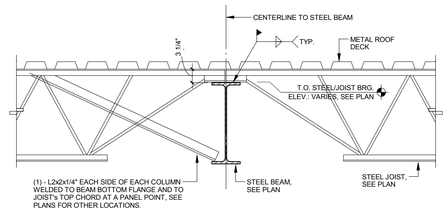
5 DECK BRG. ALONG EXT. BEAM
1" = 1'-0"



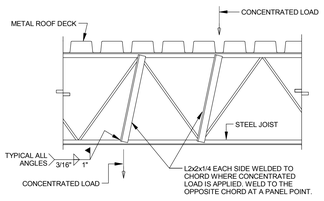
6 SECTION
1" = 1'-0"



7 JOIST BRG. ALONG EXTERIOR BEAM
1" = 1'-0"



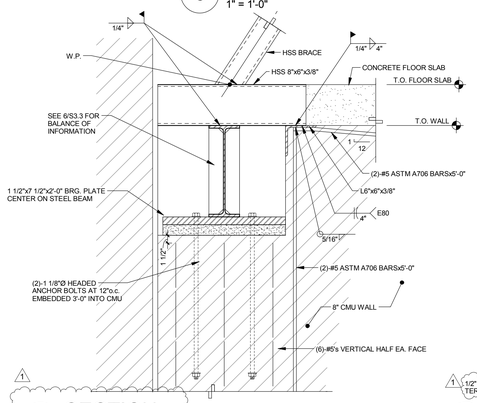
8 JOIST BRG. ALONG INTERIOR BEAM
1" = 1'-0"



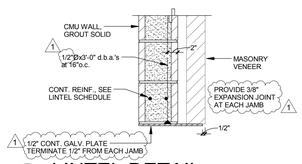
9 JOIST REINFORCING DETAIL
3/4" = 1'-0"

NOT USED

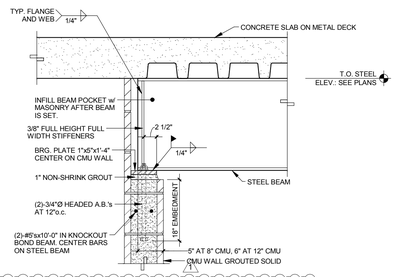
10 NOT USED
1" = 1'-0"



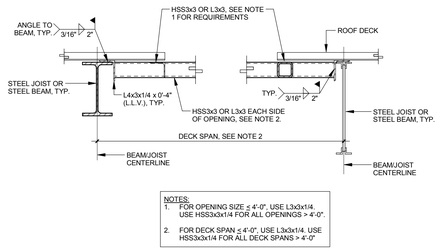
11 SECTION
1" = 1'-0"



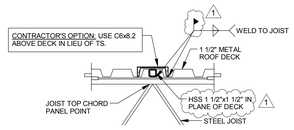
12 LINTEL DETAIL
1" = 1'-0"



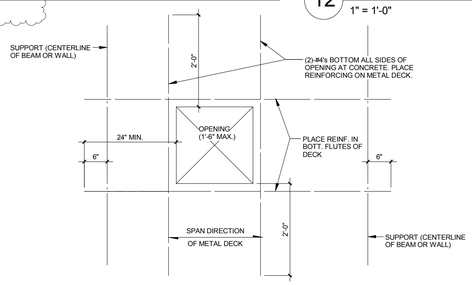
13 BEAM BRG. ON CMU WALL
1" = 1'-0"



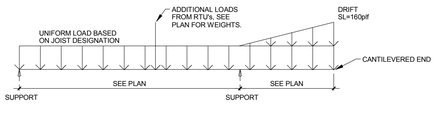
14 OPENINGS IN METAL ROOF DECK
1" = 1'-0"



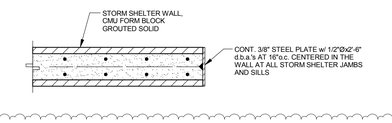
15 ROOF TOP UNIT SUPPORT
1" = 1'-0"



16 SLAB OPENING REINFORCING
3/4" = 1'-0"



17 JOIST LOAD DIAGRAM
NO SCALE



18 STORM SHELTER JAMBS AND SILLS
1" = 1'-0"

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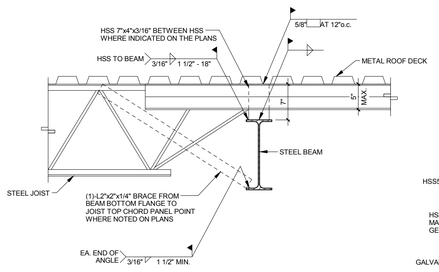
△	REVISION	DATE
1	ADDENDUM 2	08/07/2015

PROJECT NUMBER: 15008

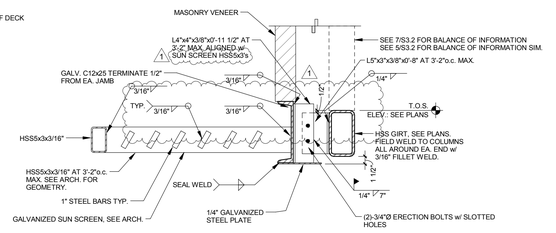
DATE: JULY 24, 2015

STRUCTURAL SECTIONS

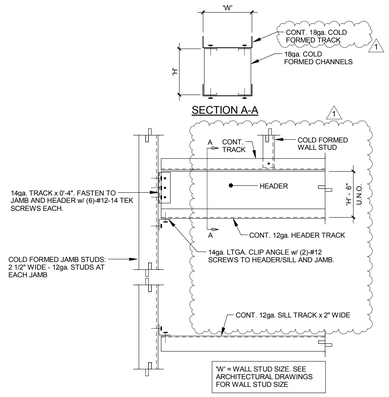
S3.2



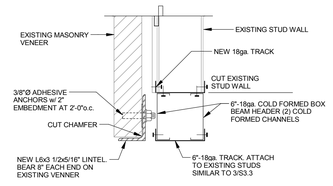
1 SECTION
1" = 1'-0"



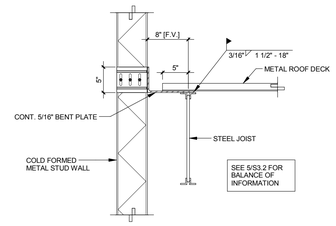
2 SUN SCREEN SUPPORT
1" = 1'-0"



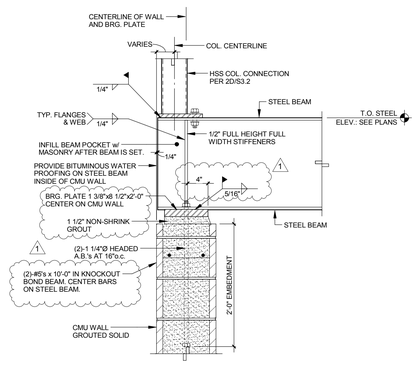
3 STRUCT. LTGA. WALL AT OPENINGS
1 1/2" = 1'-0"



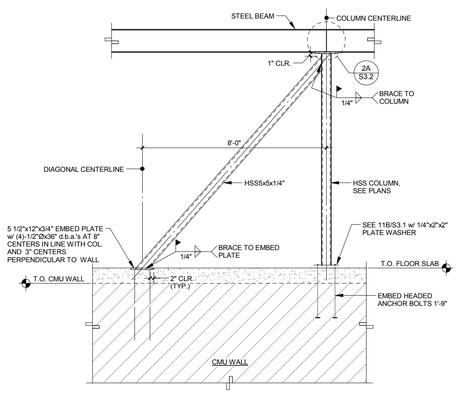
4 SECTION
1 1/2" = 1'-0"



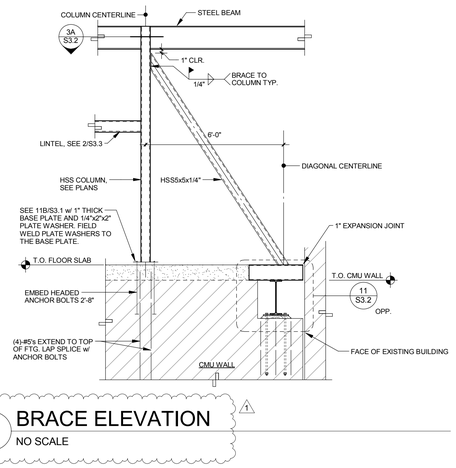
5 SECTION
1" = 1'-0"



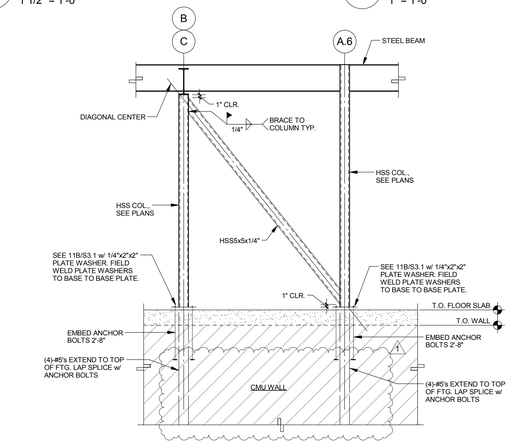
6 BEAM BRG. ON CMU WALL
1" = 1'-0"



7 BRACE ELEVATION
NO SCALE



8 BRACE ELEVATION
NO SCALE



9 BRACE ELEVATION
NO SCALE

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S3.3