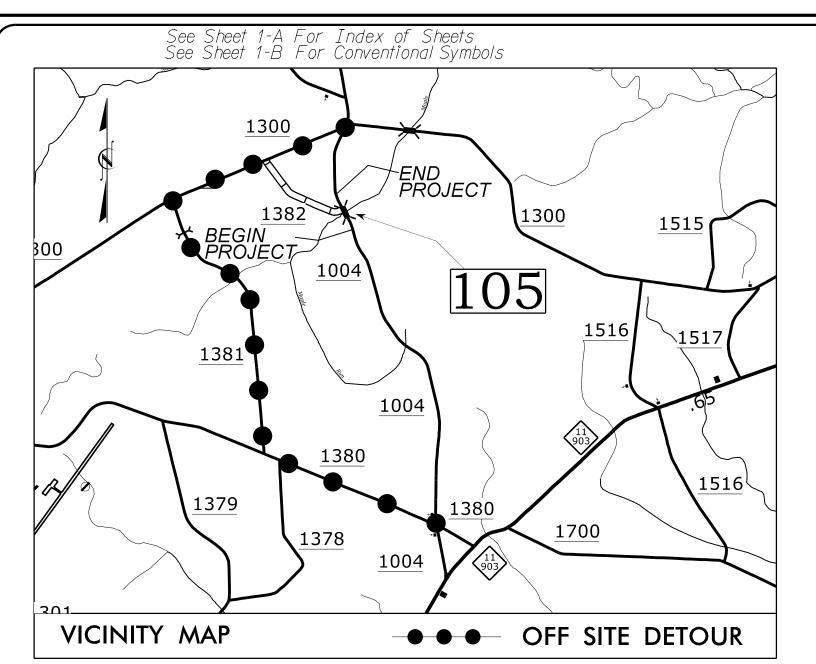
17BP.3.R.29

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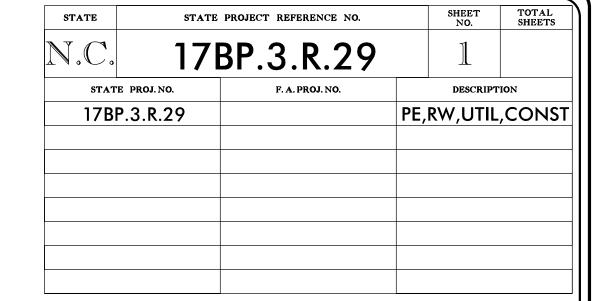


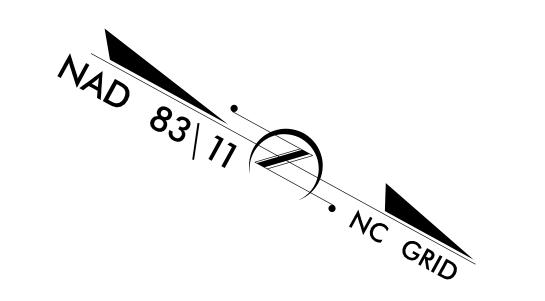
STATE OF NORTH CAROLINA
DIVISION OF HIGHWAYS

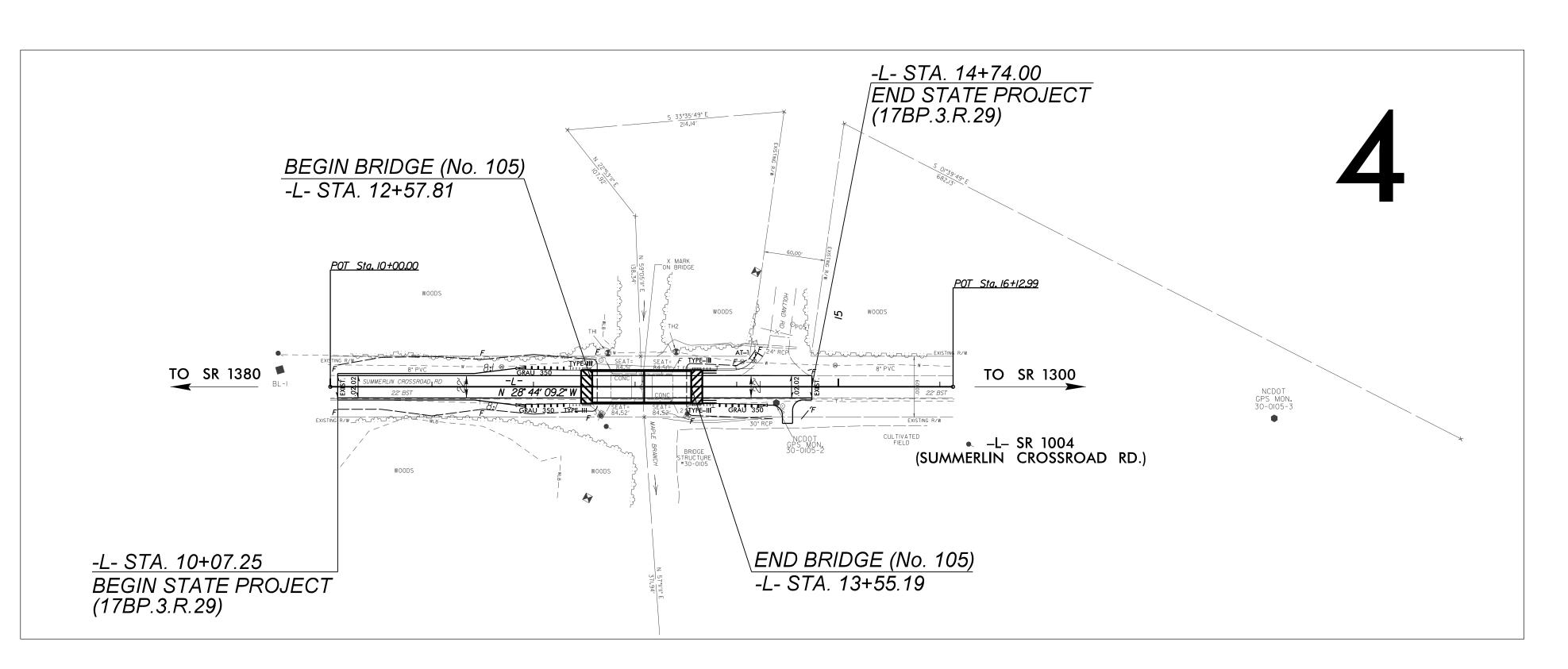
DUPLIN COUNTY

LOCATION: BRIDGE NO. 105 OVER MAPLE RUN
ON (SR 1004) SUMMERLIN CROSSROAD ROAD

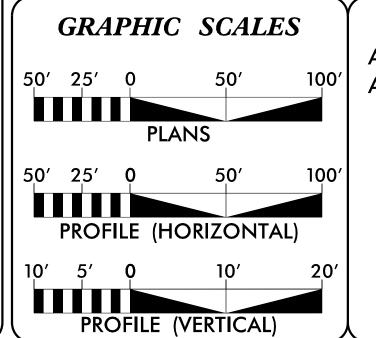
TYPE OF WORK: GRADING, DRAINAGE, PAVING, & STRUCTURE







PREPARED FOR
DEPARTMENT OF TRANSPORTATION
DIVISION OF HIGHWAYS
RALEIGH, NC
PLANS COORDINATED BY:
Trevor Carroll – Div. 3 Bridge Maintenance Engineer



DESIGN DATA

ADT 2013 = 2,500 ADT 2033 = 3,050 K = N/A

D = NA T = 6%

V = STATUTORY 55 MPH FUNC = RURAL CLASS = LOCAL

SUBREGIONAL TIER

PROJECT LENGTH

LENGTH ROADWAY PROJECT 17BP.3.R.29 = 0.070 Miles

TOTAL STRUCTURE PROJECT 17BP.3.R.29 = 0.018 Miles

TOTAL LENGTH STATE PROJECT 17BP.3.R.29 = 0.088 Miles

Prepared in the Office of:

LOCHNER

H. W. LOCHNER, INC.
2840 PLAZA PLACE, SUITE 202
RALEIGH, NC 27612

NC License Number F-0159

BRIAN K. EASON, PE

PROJECT ENGINEER

MAY 30, 2014

DOUG WHEATLEY, PE

PROJECT DESIGNER

LETTING DATE:
OCTOBER 15, 2015

TREVOR CARROLL

NCDOT CONTACT

HYDRAULICS
ENGINEER

SEAL
20147

Frank 7 Fleming

P.E.

ROADWAY DESIGN
ENGINEER

ENGINEER

DocuSigned by:

SEAL
20147

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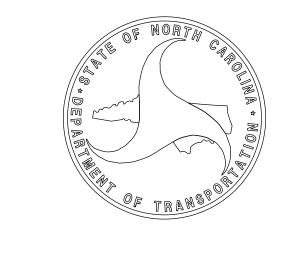
P.E.

SEAL
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— 38503BAD7047465...

SIGNATURE:



SHEET NUMBER SHEET

1 TITLE SHEET

1-A INDEX OF SHEETS, GENERAL NOTES, LIST OF

STANDARD DRAWINGS, AND CENTERLINE COORDINATE LIST

1-B CONVENTIONAL SYMBOLS

2 PAVEMENT SCHEDULE, TYPICAL SECTIONS, MILLING DETAIL, AND WEDGING DETAIL

2A GEOTEXTILE OVERLAP DETAIL, AND ROCK PLATING DETAIL

SUMMARY OF DRAINAGE QUANTITIES
SUMMARY OF GUARDRAIL, EARTHWORK
SUMMARY, SHOULDER BERM GUTTER
SUMMARY, AND RIGHT OF WAY SUMMARY

4 PLAN / PROFILE SHEET

TMP-1 THRU TMP-2 TRANSPORTATION MANAGEMENT PLAN

STRUCTURE PLANS

SP-1 SIGN DESIGN PLANS

EC-1 THRU EC-5 EROSION CONTROL PLANS

X-A THRU X-3 CROSS-SECTIONS

EFFECTIVE: 01-17-12 REVISED: 07/30/12

2012 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Highway Design Branch -

N. C. Department of Transportation - Raleigh, N. C., Dated January, 2012 are applicable to this project and by reference hereby are considered a part of these plans:

STD.NO. TITLE

DIVISION 2 - EARTHWORK

S-1 THRU S-18

200.02 Method of Clearing - Method II

225.02 Guide for Grading Subgrade - Secondary and Local

225.04 Method of Obtaining Superelevation - Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation

DIVISION 4 - MAJOR STRUCTURES

422.10 Reinforced Bridge Approach Fills DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction - High Side of Superelevated Curve - Method I

DIVISION 6 - ASPHALT BASES AND PAVEMENTS

654.01 Pavement Repairs
DIVISION 8 - INCIDENTALS

840.00 Concrete Base Pad for Drainage Structures

840.25 Anchorage for Frames - Brick or Concrete or Precast

840.29 Frames and Narrow Slot Flat Grates

840.35 Traffic Bearing Grated Drop Inlet – for Cast Iron Double Frame and Grates

840.46 Traffic Bearing Precast Drainage Structure

846.01 Concrete Curb, Gutter and Curb & Gutter

846.04 Drop Inlet Installation in Shoulder Berm Gutter

862.01 Guardrail Placement

862.02 Guardrail Installation

862.03 Structure Anchor Units (Details in Lieu of Standard Drawing

as March 2013 Letting)

876.02 Guide for Rip Rap at Pipe Outlets



NC License Number F–0159

PROJECT REFERENCE NO.

17BP.3.R.29

ROADWAY DESIGN
ENGINEER

CARO

CESSION

Docusigned by:
SEAL

38508BAD7047465.

SHEET NO.

/-A

GENERAL NOTES:

2012 SPECIFICATIONS
EFFECTIVE: 01-17-12
REVISED: 10/31/14

GRADE LINE:

GRADING AND SURFACING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. GRADE LINES MAY BE ADJUSTED AT THEIR BEGINNING AND ENDING AND AT STRUCTURES AS DIRECTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD II.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

SUBSURFACE PLANS:

NO SUBSURFACE PLANS ARE AVAILABLE FOR THIS PROJECT. THE CONTRACTOR SHOULD MAKE HIS OWN INVESTIGATIONS AS TO THE SUBSURFACE CONDITIONS.

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE DUPLIN COUNTY UTILITIES, TRI-COUNTY ELECTRIC, AND AT&T. ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

CENTERLINE COORDINATE LIST

POINT NO.	SURVEY LINE	STATION	northing (Y)	EASTING (X)
1	-L-	10+00.00	466,929.334	2,315,898.632
2	-L-	10 + 50.00	466,973.177	2,315,874.593
3		11 + 00.00	467,017.019	2,315,850.555
4		11 + 50.00	467,060.861	2,315,826.516
5		12 + 00.00	467,104.703	2,315,802.478
6		12 + 50.00	467,148.546	2,315,778.439
7	-L-	13 + 00.00	467,192.388	2,315,754.400
8		13 + 50.00	467,236.230	2,315,730.362
9		14+00.00	467,280.072	2,315,706.323
10	-L-	14 + 50.00	467,323.915	2,315,682.284
11	-L-	15 + 00.00	467,367.757	2,315,658.246
12	-L-	15 + 50.00	467,411.599	2,315,634.207
13	-L-	16+00.00	467,455.442	2,315,610.169
14	-L-	16+12.99	467,466.828	2,315,603.926

8/3/2015 R:\Roadway\Proj\300105_rdy_psh_01A.dgn

*S.U.E. =	Subsurface	Utility	Engineering	

BOUNDARIES AND PROPERTY:

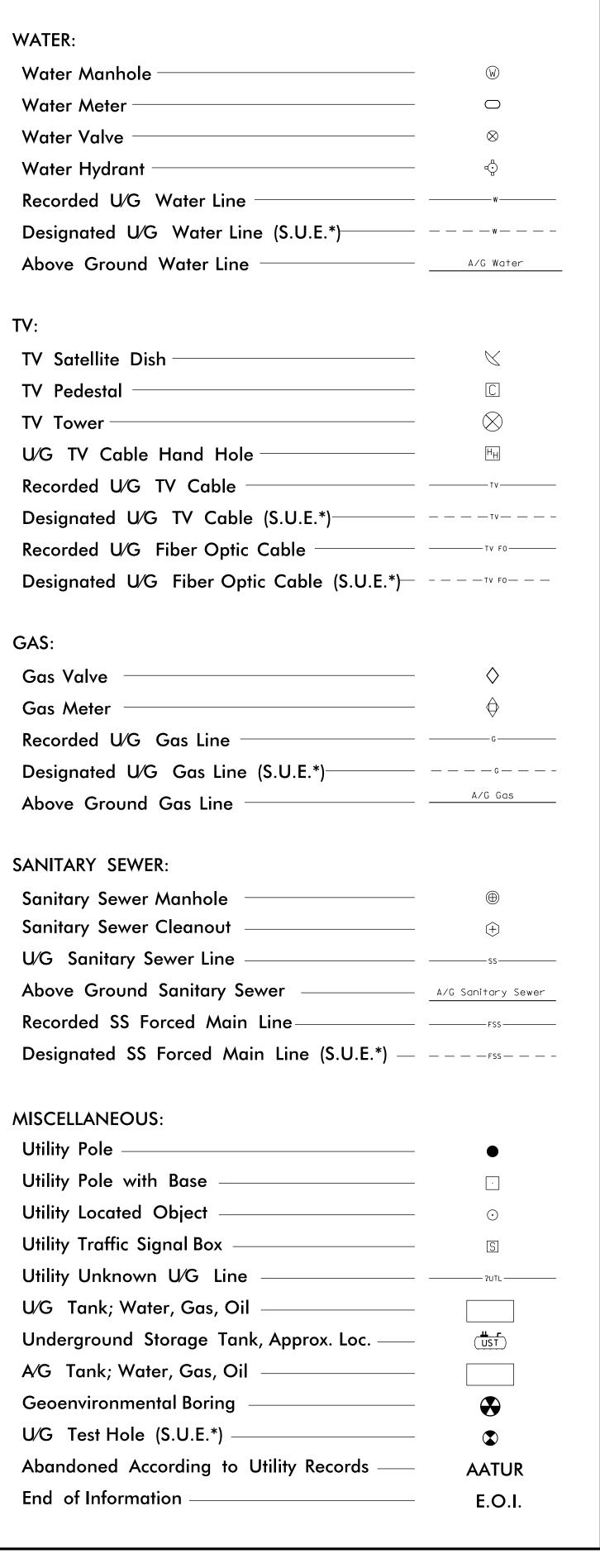
TATE	\mathbb{OF}	NORT	CAROLINA
DIVI	SION	V OF	GHWAYS

	PROJECT REFERENCE NO.	SHEET NO.
	<i>17BP.3.R.29</i>	IB
LC	CHN	ER
2840 PL	CHNER, INC. AZA PLACE, SUITE 202 , NC 27612	NC License

CONVENTIONAL PLAN SHEET SYMBOLS

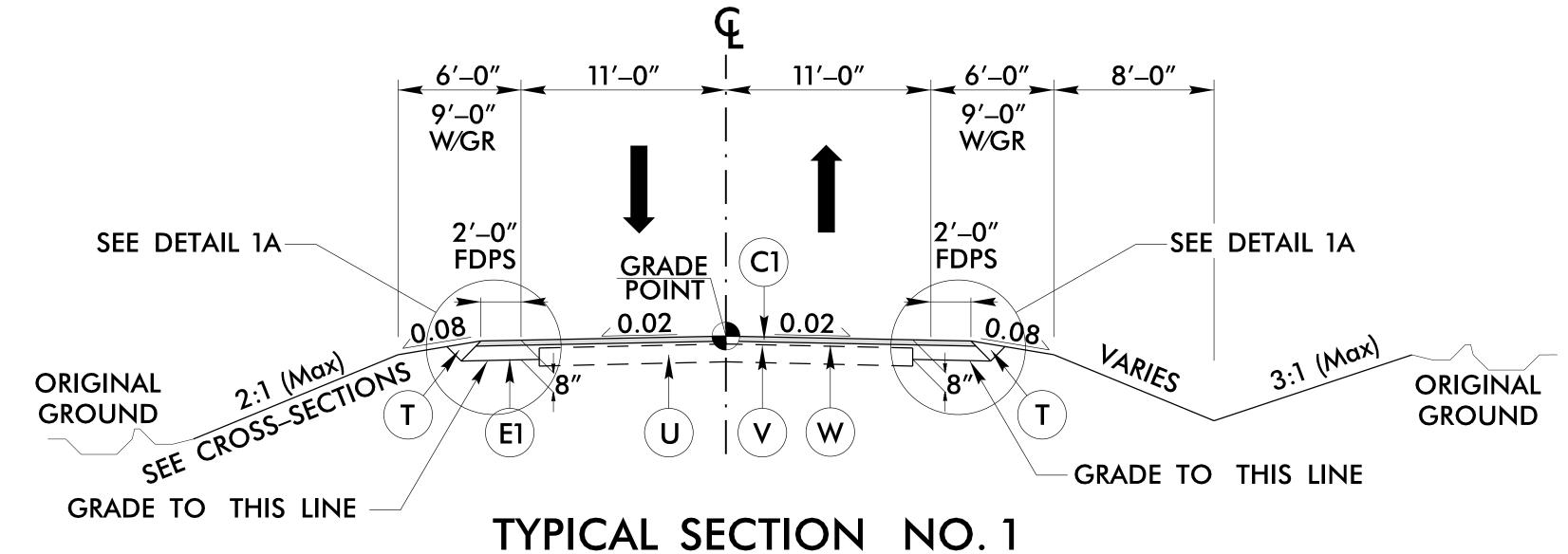
State Line			
County Line		RAILROADS:	
Township Line		Standard Gauge ————	CSX TRANSPORTATION
City Line		RR Signal Milepost ————————————————————————————————————	⊙ MILEPOST 35
Reservation Line	·	Switch ————	SWIT CH
Property Line		RR Abandoned	
Existing Iron Pin	<u>.</u>	RR Dismantled	
Property Corner	×	RIGHT OF WAY:	
Property Monument	 ECM	Baseline Control Point	•
Parcel/Sequence Number		Existing Right of Way Marker	
Existing Fence Line		Existing Right of Way Line	
Proposed Woven Wire Fence	— — — — — — — — — — — — — — — — — — —	Proposed Right of Way Line	$\frac{\overline{R}}{W}$
Proposed Chain Link Fence	_	Proposed Right of Way Line with	\overline{R}
Proposed Barbed Wire Fence		Iron Pin and Cap Marker	W
Existing Wetland Boundary		Proposed Right of Way Line with Concrete or Granite R/W Marker	$\frac{\mathbb{R}}{\mathbb{R}}$
Proposed Wetland Boundary	WLB	Proposed Control of Access Line with	
Existing Endangered Animal Boundary	EAB	Concrete C/A Marker	
Existing Endangered Plant Boundary	ЕРВ ———	Existing Control of Access	——————————————————————————————————————
Known Soil Contamination: Area or Site	% - %	Proposed Control of Access ————	
Potential Soil Contamination: Area or Site —		Existing Easement Line ————————————————————————————————————	—— E ——
BUILDINGS AND OTHER CULT	TURE:	Proposed Temporary Construction Easement –	———Е———
Gas Pump Vent or U/G Tank Cap		Proposed Temporary Drainage Easement ——	—— TDE ——
Sign —	<u>©</u>	Proposed Permanent Drainage Easement ——	PDE
Well —		Proposed Permanent Drainage / Utility Easemen	t
Small Mine		Proposed Permanent Utility Easement ———	PUE
Foundation —		Proposed Temporary Utility Easement ———	——— TUE ———
Area Outline		Proposed Aerial Utility Easement ————	———AUE———
Cemetery		Proposed Permanent Easement with	\triangle
Building —		Iron Pin and Cap Marker	(
School —		ROADS AND RELATED FEATURE	7.S. :
Church		Existing Edge of Pavement	
Dam		Existing Curb	
		Proposed Slope Stakes Cut	
HYDROLOGY:		Proposed Slope Stakes Fill ————	
Stream or Body of Water —		Proposed Curb Ramp	
Hydro, Pool or Reservoir		Existing Metal Guardrail	
Jurisdictional Stream		Proposed Guardrail ————	
Buffer Zone 1		Existing Cable Guiderail	
Buffer Zone 2		Proposed Cable Guiderail	
Flow Arrow — Disappearing Stream — Disappear		Equality Symbol	•
Spring ————————————————————————————————————		Pavement Removal	
Wetland		VEGETATION:	
Proposed Lateral, Tail, Head Ditch —		Single Tree	
False Sump	<−−− FLOW	Single Shrub	
i dise sullip		Hedge ————	
		Woods Line	-ښ-ښ-ښ-ښ-ښ-

Vineyard ————————————————————————————————————	Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert	CONC
Bridge Wing Wall, Head Wall and End Wall –) CONC WW (
MINOR:	
Head and End Wall	CONC HW
Pipe Culvert	
Footbridge	
Drainage Box: Catch Basin, DI or JB	СВ
Paved Ditch Gutter	
Storm Sewer Manhole	(\$)
Storm Sewer —	S
UTILITIES:	
POWER:	
Existing Power Pole —————	•
Proposed Power Pole —	4
Existing Joint Use Pole	
Proposed Joint Use Pole ————	-6-
Power Manhole ————————————————————————————————————	P
Power Line Tower ————	
Power Transformer ———————————————————————————————————	otin
U/G Power Cable Hand Hole	
H_Frame Pole ————————————————————————————————————	•—•
Recorded U/G Power Line	P
Designated U/G Power Line (S.U.E.*)	P
TELEPHONE:	
Existing Telephone Pole	-•-
Proposed Telephone Pole ————	-0-
Telephone Manhole	\bigcirc
Telephone Booth	[2]
Telephone Pedestal ————————————————————————————————————	T
Telephone Cell Tower	,
U/G Telephone Cable Hand Hole ———	H _H
Recorded U/G Telephone Cable ————	тт
Designated U/G Telephone Cable (S.U.E.*)—	
Recorded U/G Telephone Conduit	
Designated U/G Telephone Conduit (S.U.E.*)	
Recorded U/G Fiber Optics Cable —	
Designated U/G Fiber Optics Cable (S.U.E.*)	
· ,	



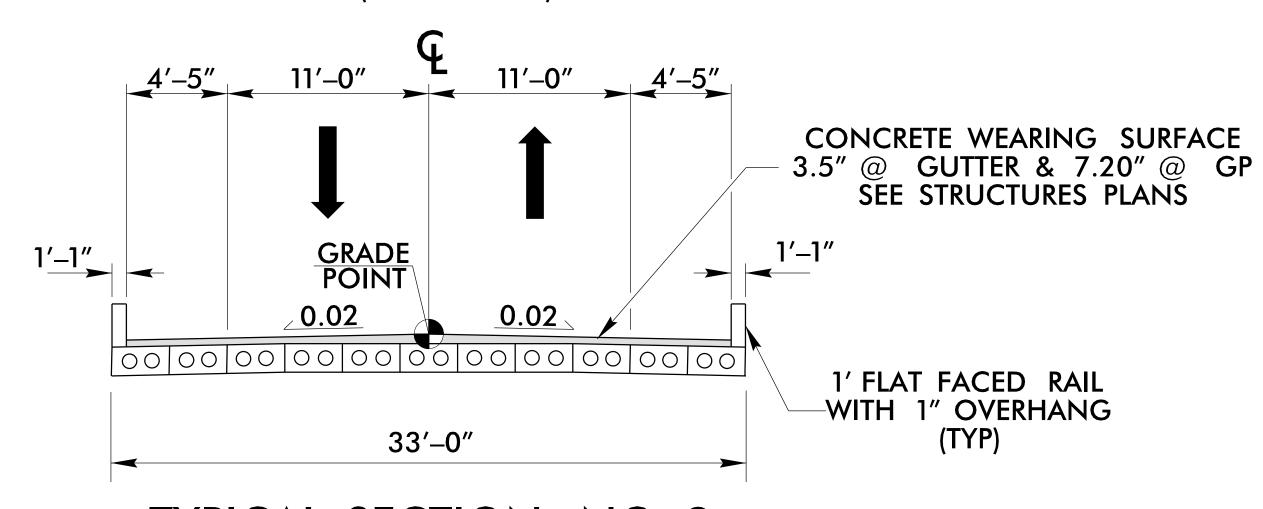
	PAVEMEN	Т 5	SCHEDULE
C1	PROP. APPROX. 3" ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 165 LBS. PER SQ. YD. IN EACH OF TWO LAYERS	Т	EARTH MATERIAL.
C2	PROP. VAR. DEPTH ASPHALT CONCRETE SURFACE COURSE, TYPE SF9.5A, AT AN AVERAGE RATE OF 110 LBS PER SQ YD PER 1" DEPTH TO BE PLACED IN LAYERS NOT TO EXCEED 1 1/2" IN DEPTH	U	EXISTING PAVEMENT.
E1	PROP. APPROX. 5" ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 570 LBS. PER SQ. YD.	V	MILLING (VARIABLE DEPTH)
E2	PROP. VAR. DEPTH ASPHALT CONCRETE BASE COURSE, TYPE B25.0B, AT AN AVERAGE RATE OF 114 LBS. PER SQ. YD. PER 1" DEPTH. TO BE PLACED IN LAYERS NOT LESS THAN 3" IN DEPTH OR GREATER THAN 5½" IN DEPTH.	W	VARIABLE DEPTH ASPHALT PAVEMENT (SEE WEDGING DETAIL)
R1	SHOULDER BERM GUTTER		

NOTE: PAVEMENT EDGE SLOPES ARE 1:1 UNLESS SHOWN OTHERWISE.



USE TYPICAL SECTION NO. 1

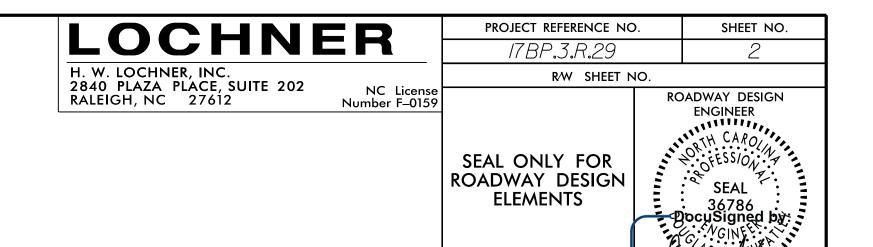
-L- STA. 10+07.25 TO STA. 12+57.81 (BEGIN BRIDGE) -L- STA. 13+55.19 (END BRIDGE) TO STA. 14+74.00



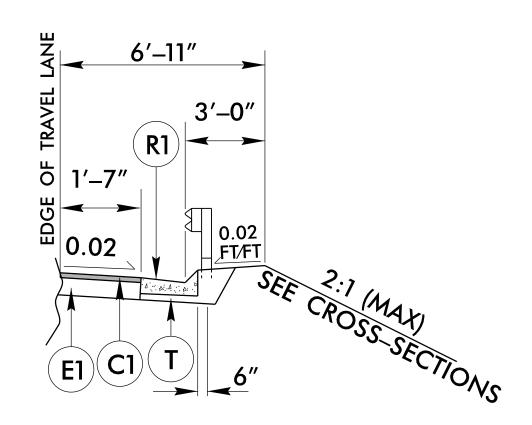
TYPICAL SECTION NO. 2

CORED SLAB BRIDGE WITH CONCRETE OVERLAY

USE TYPICAL SECTION NO. 2 -L-12+57.81 (BEGIN BRIDGE) TO STA. 13+55.19 (END BRIDGE)



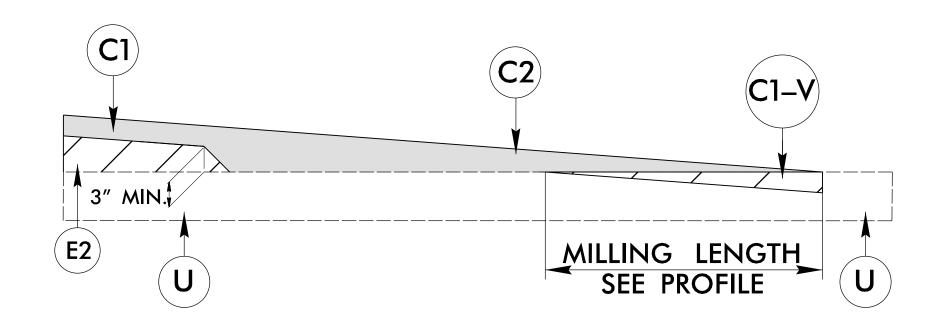
DETAIL OF SHOULDER BERM GUTTER



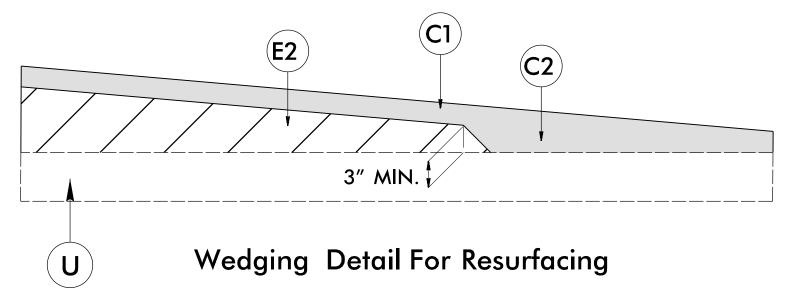
PARTIAL TYPICAL SECTION NO. 1A

USE PARTIAL TYPICAL 1A IN CONJUNCTION WITH TYPICAL 1 AT:

-L- STA. 13+66.19 RT. TO 13+80.19 RT. -L- STA. 13+66.19 LT. TO 13+80.19 LT.



MILLING DETAIL



WEDGING DETAIL

H. W. LOCHNER, INC.
2840 PLAZA PLACE, SUITE 202
RALEIGH, NC 27612

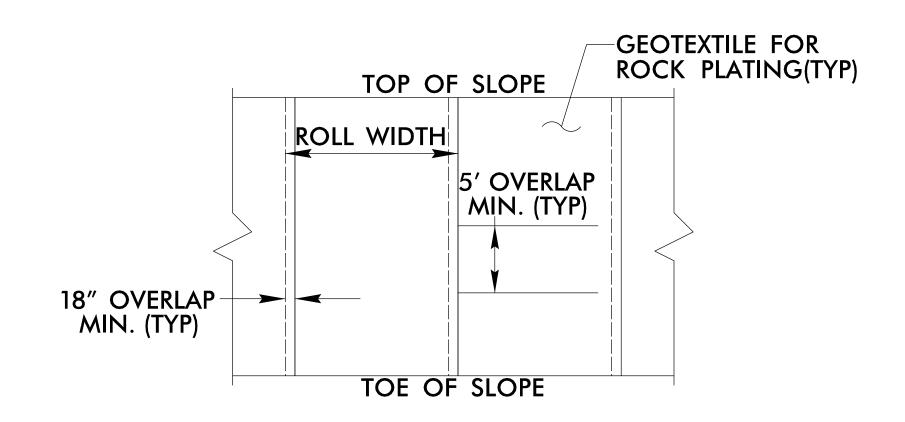
PROJECT REFERENCE NO.

17BP.3.R.29

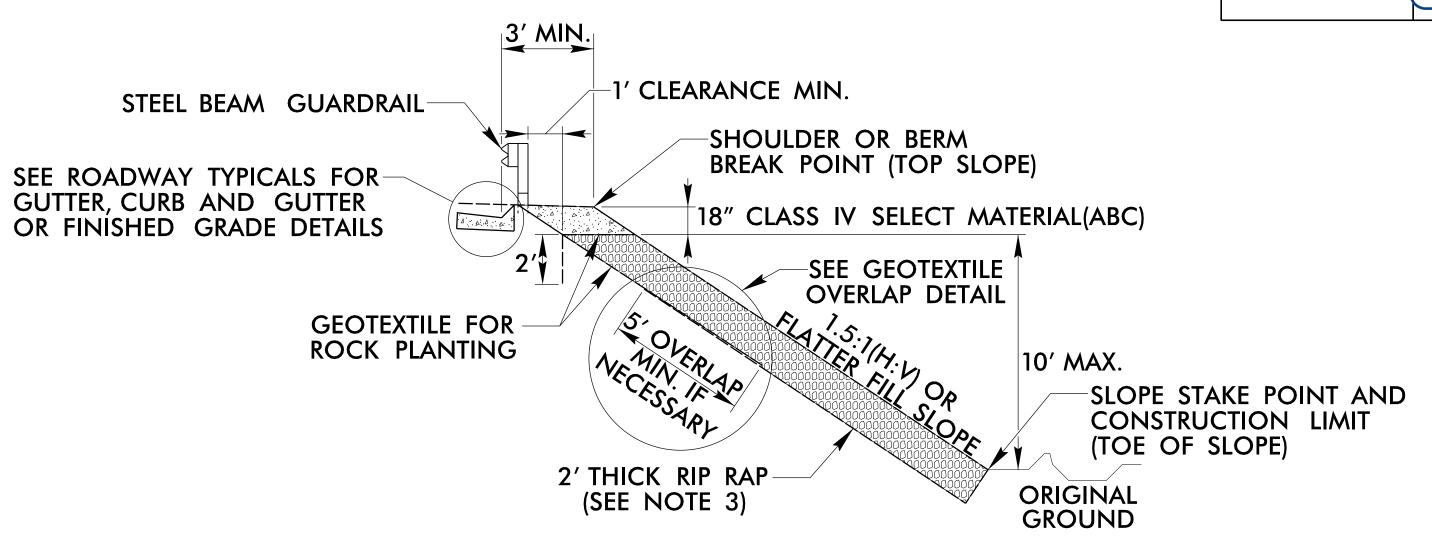
RW SHEET NO.

NC License Number F-0159

SHEET NO.



GEOTEXTILE OVERLAP DETAIL (PLAN VIEW)



ROCK PLATING DETAIL— TYPICAL SECTION

- -L- STA. 10 + 50 TO BRIDGE EMBANKMENT LT.
- -L- STA. 10 + 75 TO BRIDGE EMBANKMENT RT.
- -L- BRIDGE EMBANKMENT TO STA. 13 + 75 LT.
- -L- BRIDGE EMBANKMENT TO STA. 14+30 RT.

NOTES:

- 1. SEE ROADWAY PLANS AND SUMMARY SHEETS FOR ROCK PLATING LOCATIONS.
- 2. FOR STANDARD ROCK PLATING. SEE SECTION 275 OF THE STANDARD SPECIFICATIONS.
- 3. USE CLASS 1, 2 OR B RIP RAP UNLESS REQUIRED OTHERWISE IN THE ROADWAY SUMMARY SHEETS.

\Proj\300105_rdy_psh_02_typ.dgr INER

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

LOCHNER

H. W. LOCHNER, INC.
2840 PLAZA PLACE, SUITE 202
RALEIGH, NC 27612

PROJECT REFERENCE NO.

178P.3.R.29

SHEET NO.

ECOLOGICAL

NC License Number F–0159 NC FIRM LICENSE No: F-1148 1151 SE Cary Parkway Suite 101 Cary, NC 27518 (919) 557-0929

RIGHT OF WAY AREA DATA

PARCEL NO.	PROPERTY OWNERS NAMES	TOTAL ACREAGE	AREA TAKEN	AREA REMAINING RT.	AREA REMAINING LT.	CONST. EASE.	PERM. DRAIN. EASE.	TEMP. DRAIN. EASE.
1	MICHAEL DEAN ROBINSON					0.033 AC		
2	BENJAMIN SCOTT TURNER					0.061 AC		
3	WILLIAM S DRAUGHON & KAYWOOD G DRAUGHON					0.056 AC	0.038 AC	

SUMMARY OF EARTHWORK

STATION	STATION	UNCLASSIFIED EXCAVATION CY	EMBANK +%	BORROW CY	WASTE CY
10+07.25	12 + 50.00	20	150	130	0
14+00.00	14 + 74.00	10	68	58	0
PROJECT TOTAL		30	218	188	0
EST. 5% TO REPLACE	TOP SOIL ON BOR	OW PIT		10	
GRAND TOTAL		30		198	
SAY		40		210	

NOTE: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Shoulder Borrow, Fine Grading, Clearing and Grubbing, Breaking of Existing Pavement, and Removal of Existing Pavement will be paid for at the contract lump sum price for "Grading".

SHOULDER BERM GUTTER SUMMARY

SURVEY LINE	STATION	STATION	LENGTH
L (LT)	13 + 66.19	13 + 80.19	14
L (RT)	13 + 66.19	13 + 80.19	14
TOTAL			28
		SAY	30 LF

NOTE: Invert Elevations are for Bid Purposes only and shall not be used for project construction stakeout. See "Standard Specifications For Roads and Structures, Section 300–5".

SUB-REGIONAL & REGIONAL LIST OF PIPES, ENDWALLS, ETC. (FOR PIPES 48" & UNDER)

STATION	CATION (LT,RT, OR CL)	STRUCTURE NO.	P ELEVATION	VERT ELEVATION	VERT ELEVATION	OPE CRITICAL	15" 3	SP, CAAI		i, or PVC)		." 15"			PIPE D OTHRV		42"	48"	CLASS (UNLESS OT		PE	PE	STD. 838 STD. 838 OR STD. 838 (UNLES NOTE OTHERW	3.01, 8.11 3.80 SS D	STRUCTURES * TOTAL L.F. FOR PAY * TOTAL L.F. FOR PAY * TOTAL L.F. FOR PAY * TOTAL L.F. FOR PAY	R STD. 840.02	:	FRAME, GRATES AND HOOD STANDARD 840.03	I OR STD. 840.15	SRATE STD. 840.16 STD. 840.17 OR 840.26	STD. 840.18 OR 840.27	STD. 840.19 OR 840.28 WITH GRATE STD. 840.22	WITH TWO GRATES STD. 840.22	AME WITH GRATE STD. 840.24 ME WITH TWO GRATES STD. 840.24	1 OR 840.32	N.S. FLAT) W/2 GRATES STD. 840.29	PIPE FIROW		BOWS NO. & SIZE RS CL. "B" C.Y. STD 840.72	E PLUG, C.Y. STD.	C.B. N.D.I. D.I. G.D.I. G.D.I. J.B. M.H.	DROP INLET
THICKNESS OR GAUGE	<u> </u>	FROM		<u>Z</u>	<u> </u>			NOT USE	NOT USE		DO NOT USE PV	.064	.064	620.	620.	109	001	601.			" SIDE DRAIN	18" SIDE DRAIN PIP 24" SIDE DRAIN PIF	_	C.S.P.	5.0' THRU 10.0'	10.0' AND ABOVE C.B. STD. 840.01	E	TYPE OF GRATE	D.I. STD. 840.14	G.D.I. TYPE "A"	G.D.I. TYPE "B"	G.D.I. TYPE "D" G.D.I. FRAME V	G.D.I. FRAME V	G.D.I. (N.S.) FR. G.D.I. (N.S.) FRA	D. 8	G.D.I. FRAME ((TOANIAMO "51		CORR. STEEL EL	CONC. & BRIC	T.B.J.E	
13 + 75 _L_	LT	0401	85.72	2 82.7	2																•			1												1 1						
		0401 0402	2	82.7	2 82.	58													28																							
13 + 75 _L_	RT	0402	85.72	2 82.2	3																			1												1 1						
		0402 OU	Г	82.2	80.	50	16	Х																													2	2			ANCH	IOR TO PREVENT FLOATING
14 + 50 -L-	RT	0403		78.4	78.	.2	2	28																																		
TOTALS							16 2	28											28					2												2	2	2				

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL.

TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.

FLARE LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL.

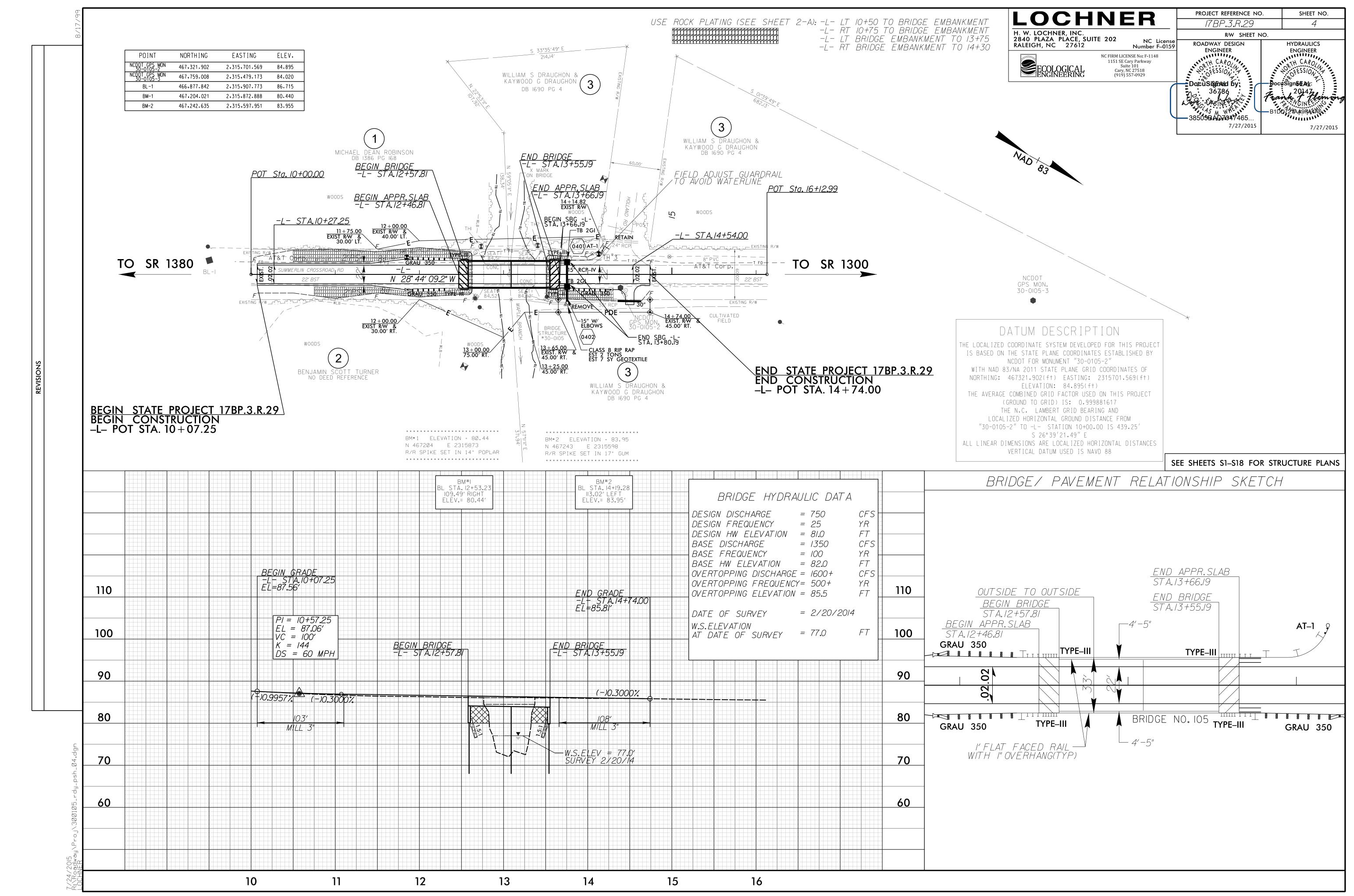
W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL.

W = TOTAL WIDTH OF FLARE FROM BEGINNING
G = GATING IMPACT ATTENUATOR TYPE 350

NG = NON-GATING IMPACT ATTENUATOR TYPE 350

GUARDRAIL SUMMARY

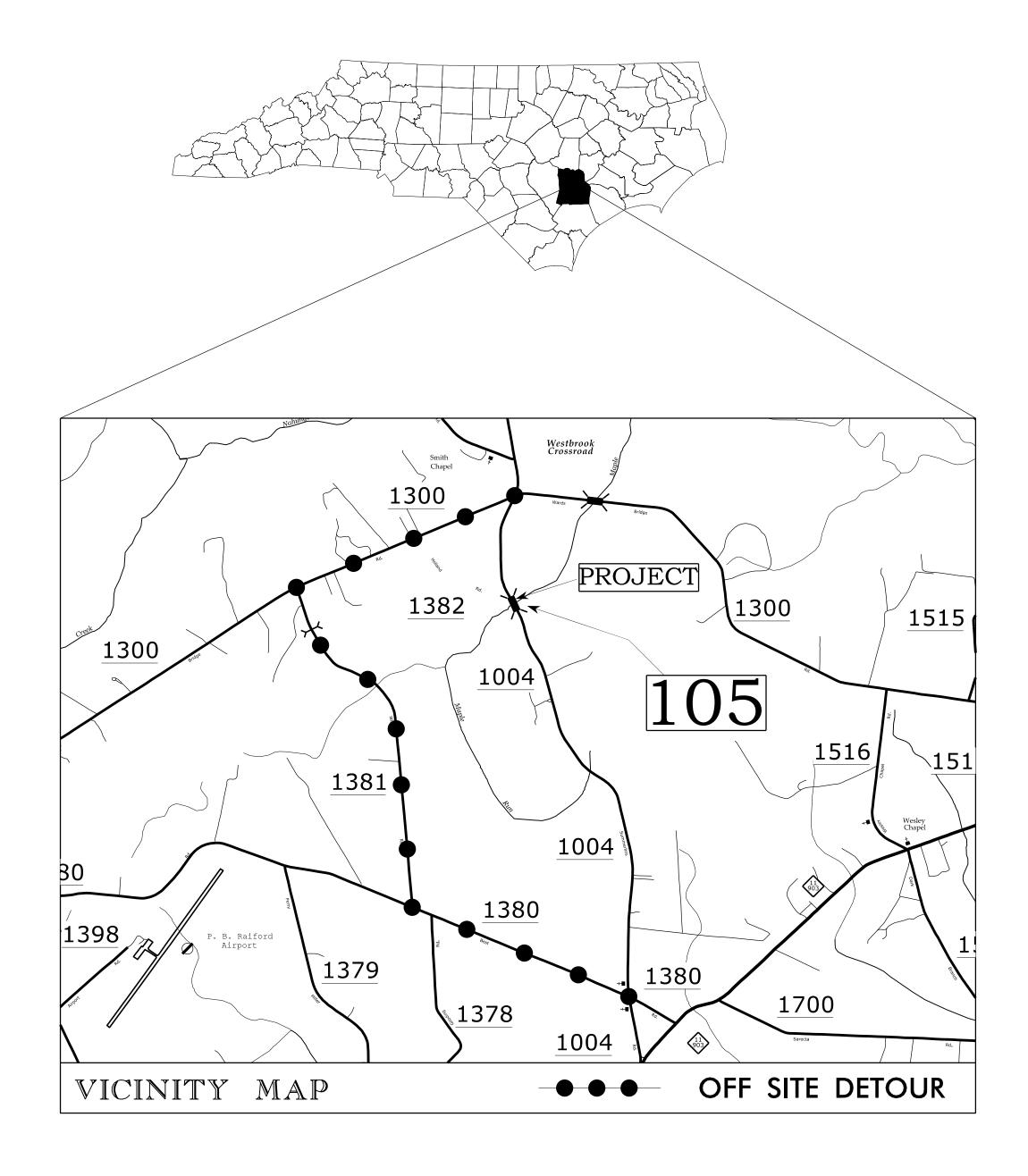
SURVEY	DEC. STA	5115 074	LOCATION		LENGTH		WARRANT POINT		"N" DIST.	TOTAL	FLARE L	FLARE LENGTH V		V	ANCHORS							IMPACT ATTENUATOR	OR SINGLE	REMOVE	REMOVE AND STOCKPILE	
LINE	BEG. STA.	END STA.	LOCATION	STRAIGHT	SHOP CURVED	DOUBLE FACED	APPROACH END	TRAILING END	FROM E.O.L.	E.O.L. WIDTH APPRI	APPROACH END	TRAILING END	APPROACH END	TRAILING END	XI MOD	XI GRA	RAU M-	_350 TYPE III	CAT-1	VI MOD BIC	AT-1	TYPE 350 EA G NG	FACED GUARDRAIL	EXISTING GUARDRAIL	EXISTING GUARDRAIL	REMARKS
-L-	11 + 82.81	12 + 57.81	LT	75.00					4.42			50		1		1	1	1								
-L-	11 + 82.81	12 + 57.81	RT	75.00					4.42		50		1			1	1	1								
-L-	13 + 55.19	14+20.64	LT	28.67	25.00				4.42		50		1					1			1					FIELD ADJUST GUARDRAIL TO AVOID CONFLICT WITH 8" WATE
-L-	13 + 55.19	14+30.19	RT	75.00					4.42			50		1		1	1	1								
			SUB-TOTAL	253.67	25.00											3	3	4			1					
	LESS ANCHORS:																									
		GRAU 350	3@50.00	-150																						
		TYPE-III	4@18.75	–75																						
		AT-1	1@6.25	-6.25																						
			TOTAL	22.42	25.00																					
			SAY	25	25											3	3	4			1					



STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

TRANSPORTATION MANAGEMENT PLAN

DUPLIN COUNTY



WORK ZONE SAFETY & MOBILITY "from the MOUNTAINS to the COAST"

N.C.D.O.T. WORK ZONE TRAFFIC CONTROL 1561 MAIL SERVICE CENTER (MSC) RALEIGH, NC 27699-1561 750 N. GREENFIELD PARKWAY, GARNER, NC 27529 (DELIVERY) PHONE: (919) 773-2800 FAX: (919) 771-2745

KATHERINE HITE, PE DIVISION TRAFFIC ENGINEER



INDEX OF SHEETS

SHEET NO. <u>TITLE</u>

TITLE SHEET, VICINITY MAP AND INDEX OF SHEETS AND LIST OF APPLICABLE ROADWAY STANDARD TMP-1

PHASING, GENERAL NOTES AND LOCAL NOTES TMP-1A

TMP-2 DETOUR SIGNING SPECIAL SIGN DESIGN SP - 1

ROADWAY STANDARD DRAWINGS

STD. NO.	<u>TITLE</u>
1101.03	TEMPORARY ROAD CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1145.01	BARRICADES
1205.01	PAVEMENT MARKINGS - LINE TYPES & OFFSETS
1205.02	PAVEMENT MARKINGS - 2 LANE & MULTILANE ROADWAYS
1205.12	PAVEMENT MARKINGS - BRIDGES
1250.01	RAISED PAVEMENT MARKERS INSTALLATION SPACING
1251.01	RAISED PAVEMENT MARKERS - (TEMPORARY & PERMANENT)
1261.01	GUARDRAIL AND BARRIER DELINEATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATION TYPE
1262.01	GUARDRAIL AND DELINEATION

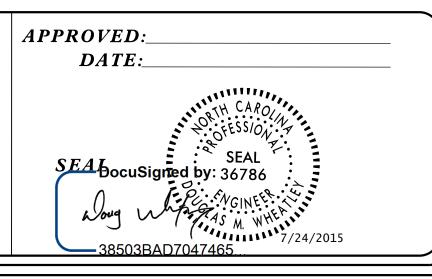
LOCHNER

H. W. LOCHNER, INC. 2840 PLAZA PLACE, SUITE 202 RALEIGH, NC 27612 LICENSE # F-0159

B Eason, PE QC ENGINEER

D. Wheatley, PE PROJECT ENGINEER

D Martin DESIGN ENGINEER / TECHNICIAN



PROJ. REFERENCE NO. SHEET NO. 17BP.3.R.29 TMP-1A

PHASING

PHASE I

PRIOR TO ANY CONSTRUCTION OPERATIONS, INSTALL AND COVER DETOUR SIGNS AS SHOWN ON TMP-2 AND IN ACCORDANCE WITH ROADWAY STANDARD DRAWING 1101.03 SHEET 1 OF 9. SIGNS SHALL BE COVERED IF DETOUR IS NOT OPENED WITHIN 3 DAYS OF SIGN INSTALLATION.

PHASE II

INSTALL BARRICADES AND UNCOVER DETOUR SIGNS. CLOSE -L- (SR 1004 / SUMMERLIN CROSSROAD RD.) TO TRAFFIC AS SHOWN ON TMP-2. CONSTRUCT BRIDGE, APPROACHES, AND ROADWAY UP TO AND INCLUDING THE FINAL LAYER OF SURFACE COURSE.

PHASE III

UPON COMPLETION OF BRIDGE, APPROACHES AND ROADWAY, PLACE FINAL PAVEMENT MARKINGS AND MARKERS IN ACCORDANCE WITH ROADWAY STANDARD DRAWINGS. REMOVE ALL ROAD CLOSURE SIGNS AND BARRICADES AND OPEN -L- (SR 1004 / SUMMERLIN CROSSROAD RD.) TO THROUGH TRAFFIC.

GENERAL NOTES

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESRIED OVERLAPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

LANE AND SHOULDER CLOSURE REQUIREMENTS

- A) REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED, OR AS DIRECTED BY THE ENGINEER.
- B) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.

TRAFFIC PATTERN ALTERATIONS

C) NOTIFY THE ENGINEER TWENTY ONE (21) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

<u>signing</u>

- D) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- E) PROVIDE SIGNING AND DEVICES REQUIRED TO CLOSE THE ROAD ACCORDING TO THE ROADWAY STANDARD DRAWINGS AND TRAFFIC CONTROL PLANS.
- PROVIDE SIGNING REQUIRED FOR THE OFF-SITE DETOUR ROUTE AS SHOWN ON SHEET TMP-2.
- F) COVER OR REMOVE ALL SIGNS AND DEVICES REQUIRED TO CLOSE THE ROAD WHEN ROAD CLOSURE IS NOT IN OPERATION.

 COVER OR REMOVE ALL SIGNS REQUIRED FOR THE OFF-SITE DETOUR WHEN THE DETOUR IS NOT IN OPERATION.
- G) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.

TRAFFIC CONTROL DEVICES

H) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2
ATTACHED, OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

S) INSTALL PAVEMENT MARKINGS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME

LINES.

<u>MARKING</u> PAINT <u>MARKER</u> RAISED

SUMMERLIN CROSSROAD ROAD

V) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING

- W) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS BY THE END OF EACH DAY'S OPERATION.
- T) PHASING ZONE WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.

APPROVED: _______ DATE: ______

SEAL

SEAL

SEAL

ON GINE

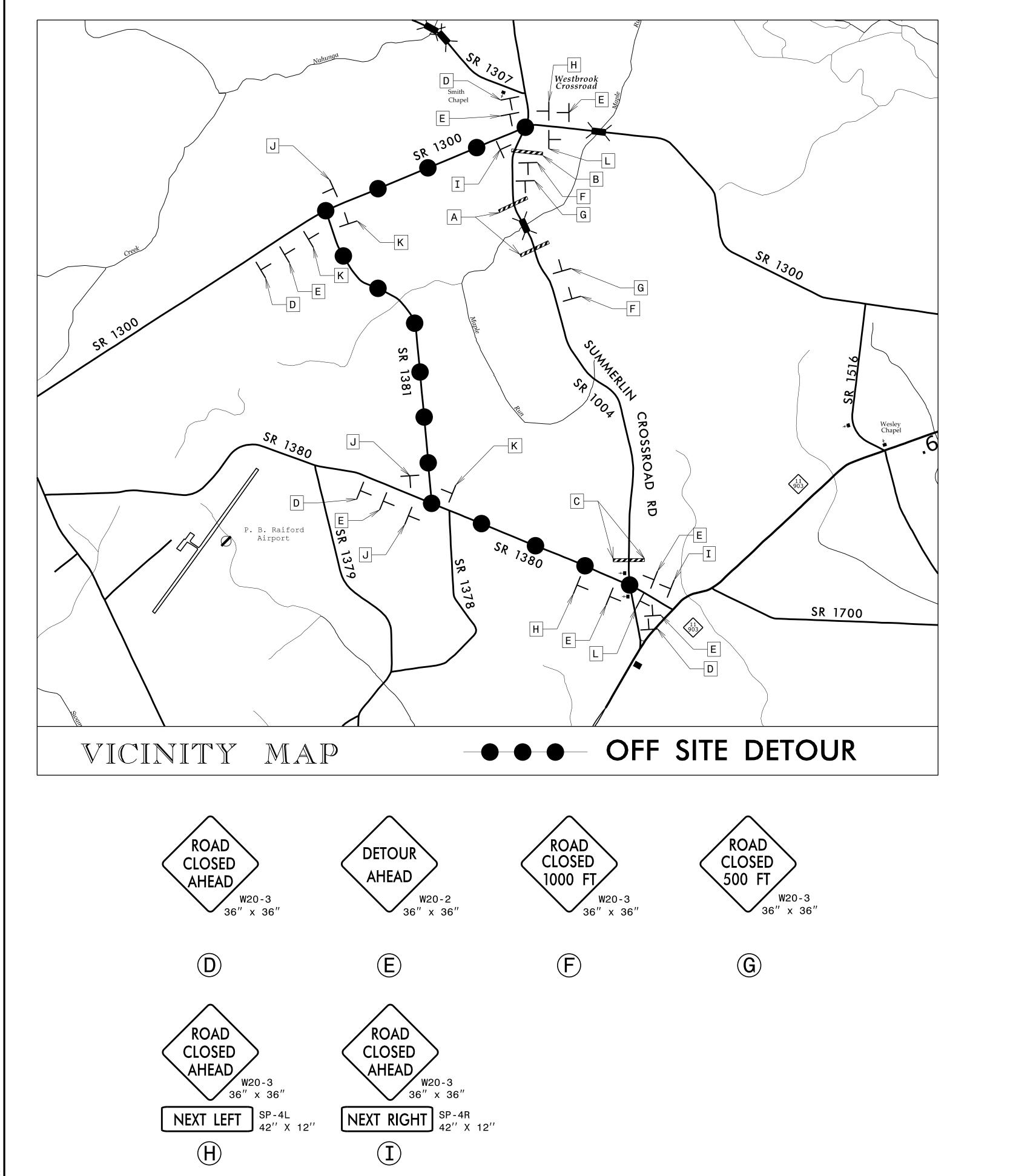
NOTE STATE

SEAL

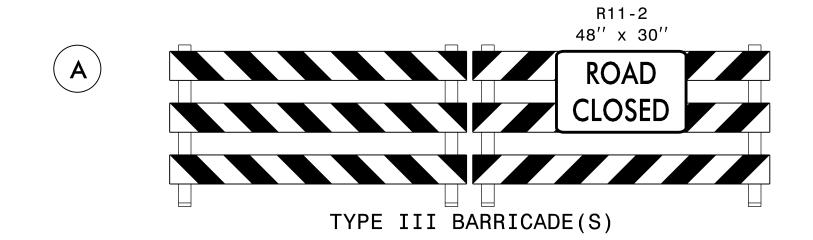
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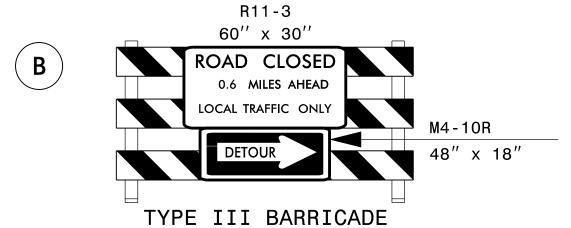


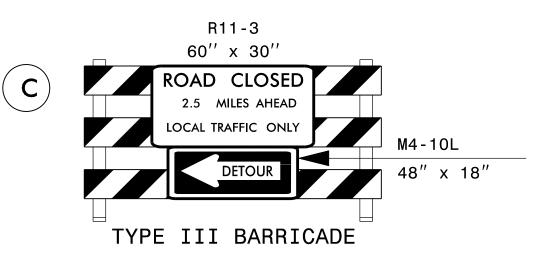
TRANSPORTATION
OPERATION
PLAN

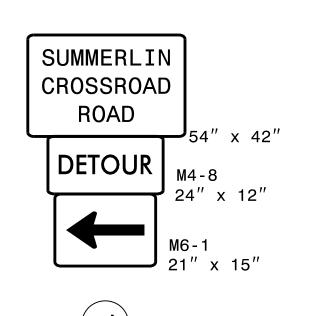


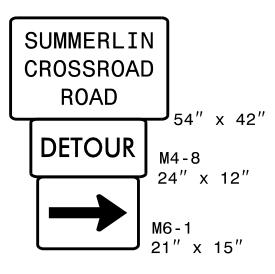
PROJ. REFERENCE NO. SHEET NO. 17BP.3.R.29 TMP-2

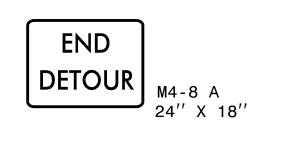








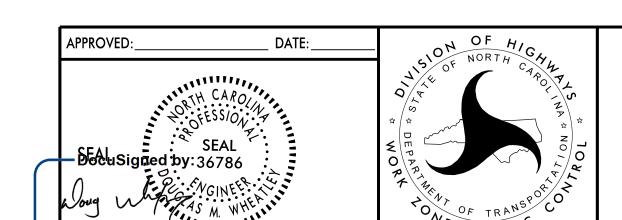




(K)



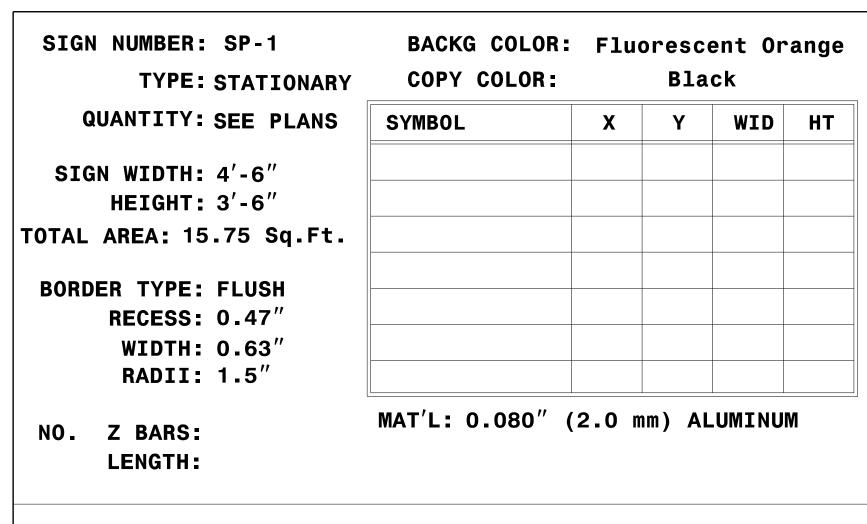
SEE TMP-3 FOR SIGN DESIGN



DETOUR SIGNING

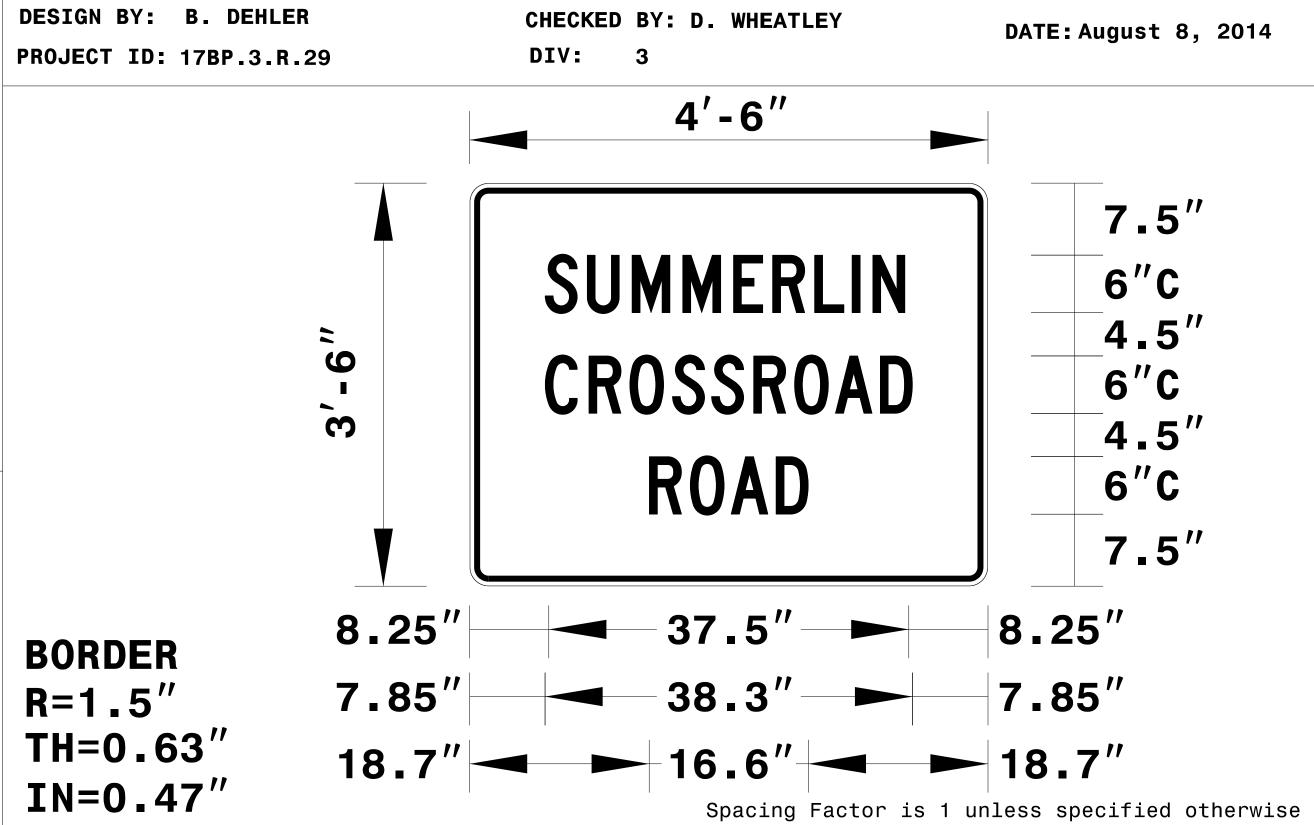
iffic\TrafficControl\TCP\300105_TCP_02.dgn

PROJ. REFERENCE NO. SHEET NO. 17BP.3.R.29 SP-1



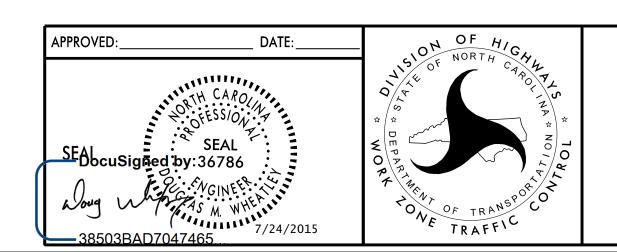
USE NOTES: 1,2

- 1. Legend and border shall be direct applied black non-reflective sheeting.
- 2. Background shall be NC GRADE B fluoresent orange retroreflective sheeting.



LETTER POSITIONS

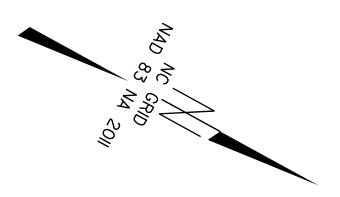
Letter locations are panel edge to lower left corner						Series/Si Text Leng							
S	U	M	M	E	R	L	I	N					C 2000
7.9	12.2	16.9	22.2	27.5	31.6	35.9	39.8	42.0					37.5
С	R	0	S	S	R	0	A	D					C 2000
7.9	12.4	16.7	21.1	25.2	29.6	33.8	38.1	42.8					38.3
R	0	Α	D										C 200
18.7	23.0	27.2	31.9										16.6



SPECIAL SIGN DESIGN

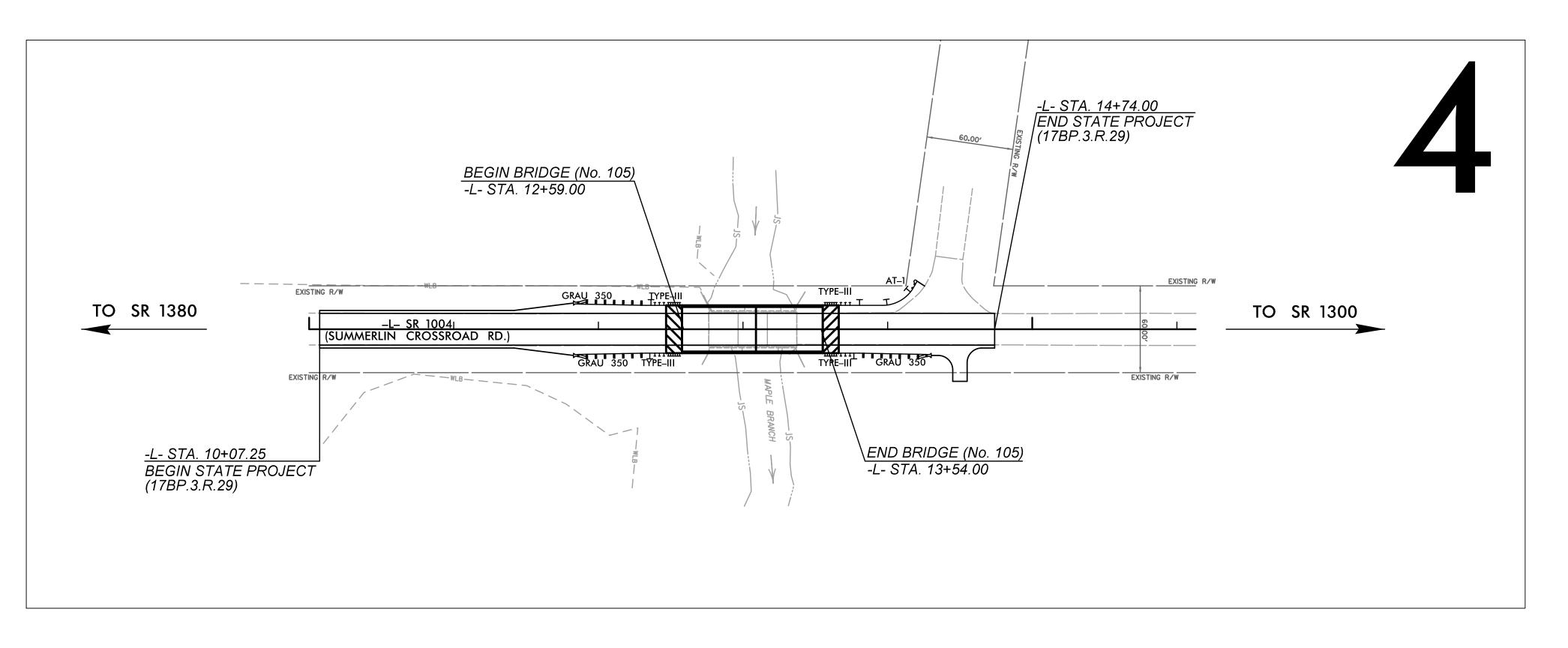
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PLAN FOR PROPOSED HIGHWAY EROSION CONTROL



LOCATION: BRIDGE NO. 105 OVER MAPLE BRANCH ON (SR 1004) SUMMERLIN CROSSROAD ROAD

TYPE OF WORK: GRADING, DRAINAGE, PAVING, & STRUCTURE



STATE	STATE PROJECT REFERENCE NO.	SHEET NO.	TOTAL SHEETS
N.C.	17BP.3.R.29	EC-1	
STATE PROJ.N	O. F. A. PROJ. NO.	DESCRIPT	ION

EROSIO	N AND SEDIMENT CONTROL MEASURES
<u>Séd. #</u>	Description Symbol
1630.03	Temporary Silt Ditch
1630.05	Temporary Diversion TD
1605.01	Temporary Silt Fence
1606.01	Special Sediment Control Fence
1622.01	Temporary Berms and Slope Drains
1630.02	Silt Basin Type B.
1633.01	Temporary Rock Silt Check Type-A
	Temporary Rock Silt Check Type-A with Matting and Polyacrylamide (PAM)
1633.02	Temporary Rock Silt Check Type-B
	Wattle / Coir Fiber Wattle
	Wattle / Coir Fiber Wattle with Polyacrylamide (PAM).
1634.01	Temporary Rock Sediment Dam Type-A
1634.02	Temporary Rock Sediment Dam Type-B
1635.01	Rock Pipe Inlet Sediment Trap Type-A
1635.02	Rock Pipe Inlet Sediment Trap Type-B
1630.04	Stilling Basin
1630.06	Special Stilling Basin
	Rock Inlet Sediment Trap:
1632.01	Туре А
1632.02	Туре В
1632.03	Type C
	Skimmer Basin
	Tiered Skimmer Basin
	Infiltration Basin

GRAPHIC SCALE

PLANS PROFILE (HORIZONTAL) PROFILE (VERTICAL)

THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG-010000 GENERAL CONSTRUCTION PERMIT EFFECTIVE AUGUST 3, 2011 AND ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENT AND NATURAL RESOURCES DIVISION OF WATER RESOURCES.

Prepared in the Office of:



NC FIRM LICENSE No: F-1148 1151 SE Cary Parkway Cary, NC 27518

2012 STANDARD SPECIFICATIONS

Designed by:

BRANDON BARHAM, PE LEVEL III CERTIFICATION NO.

3368

Reviewed in the Office of:

ROADSIDE ENVIRONMENTAL UNIT

1 South Wilmington St. Raleigh, NC 27611

2012 STANDARD SPECIFICATIONS

Reviewed by:

Roadway Standard Drawings

The following roadway english standards as appear in "Roadway Standard Drawings" – Roadway Design Unit – N. C. Department of Transportation – Raleigh, N. C., dated January 2012 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of

504.01	Railroad Erosion Control De
605.01	Temporary Silt Fence
506.01	Special Sediment Control Fer
607.01	Gravel Construction Entranc
(22.01	Tommonomy Poums and Clone

1630.01 Riser 3asin

1630.02 Silt 3asin Type 3 1630.03 Temporary Silt Ditch 1630.04 Stilling Basin 1630.05 Temporary Diversion 1630.06 Special Stilling Basin

1631.01 Matting Installation

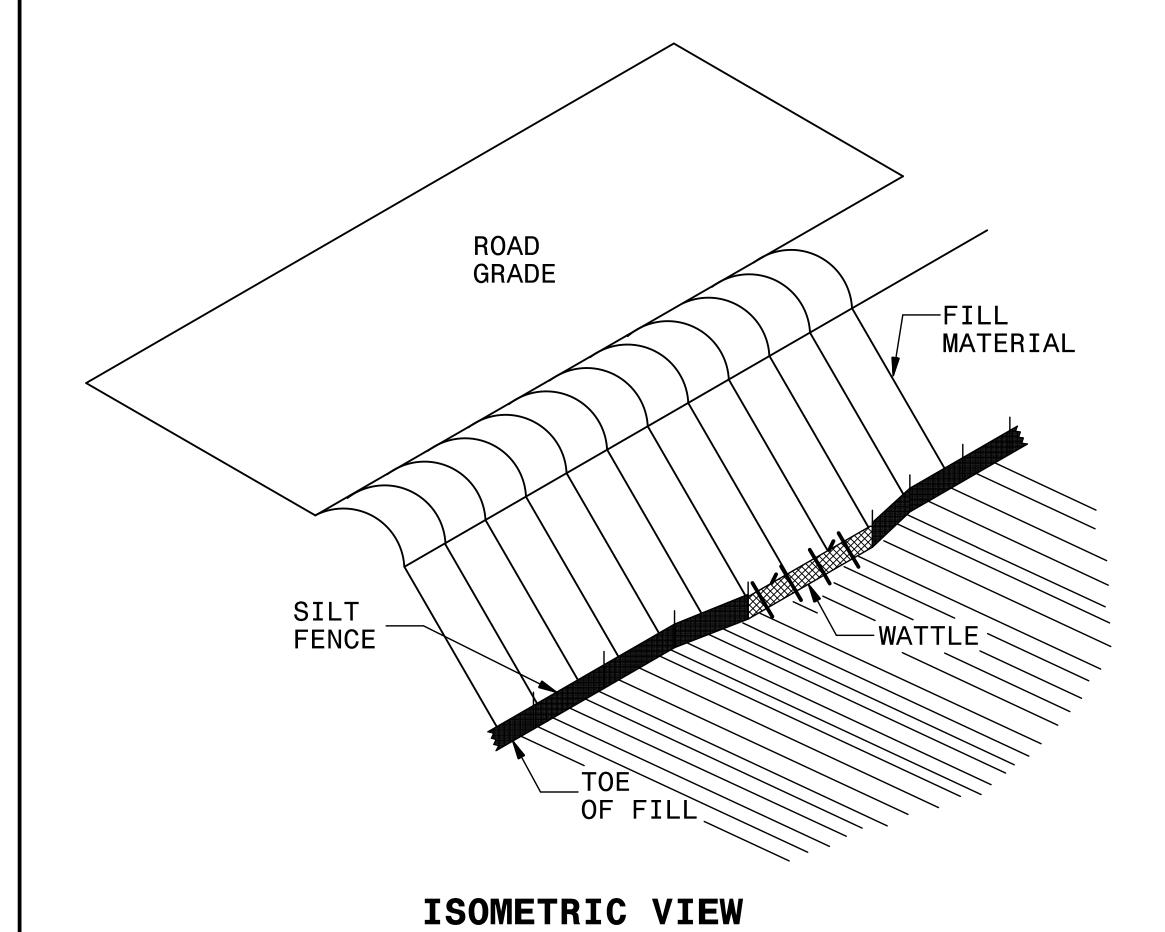
1632.01 Rock Inlet Sediment Trap Type A 1632.02 Rock Inlet Sediment Trap Type 3 1632.03 Rock Inlet Sediment Trap Type C 1633.01 Temporary Rock Silt Check Type A 1633.02 Temporary Rock Silt Check Type 3 1634.01 Temporary Rock Sediment Dam Type A
1634.02 Temporary Rock Sediment Dam Type 3
1635.01 Rock Pipe Inlet Sediment Trap Type A
1635.02 Rock Pipe Inlet Sediment Trap Type 3

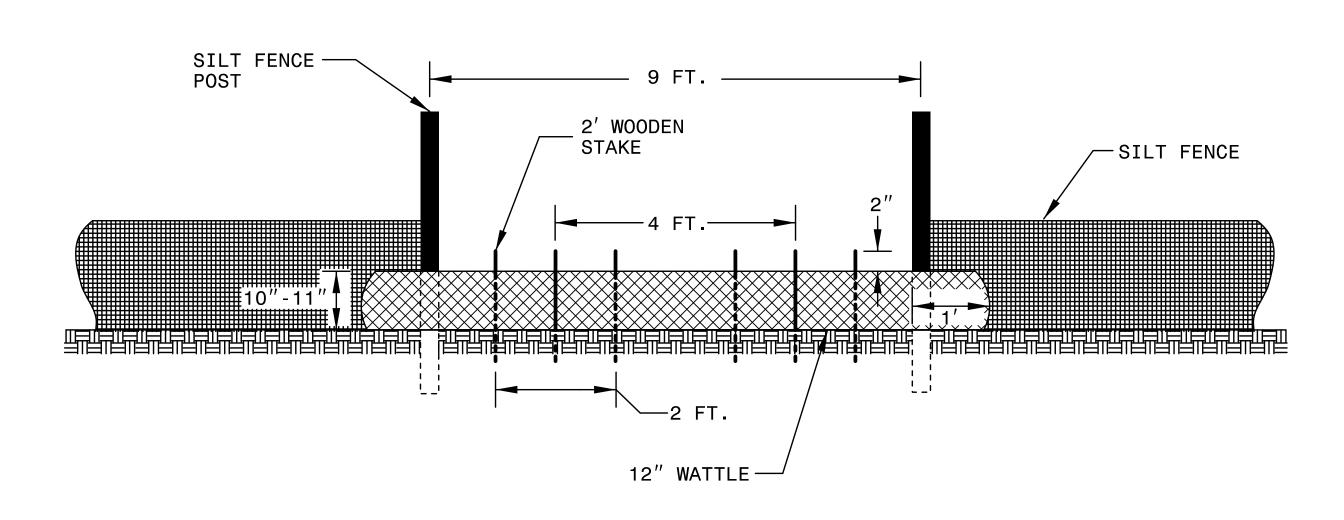
1640.01 Coir Fiber 3affle

1645.01 Temporary Stream Crossing

SILT FENCE COIR FIBER WATTLE BREAK DETAIL

PROJECT REFERENCE NO	SHEET NO.	
<i>17BP.3.R.29</i>		EC-02
R∕W SHEET N	10.	
ROADWAY DESIGN ENGINEER	io.	HYDRAULICS ENGINEER





VIEW FROM SLOPE

NOTES:

USE MINIMUM 12 IN. DIAMETER COIR FIBER (COCONUT FIBER) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 1 TO 2 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLE ON TOE OF SLOPE.

USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

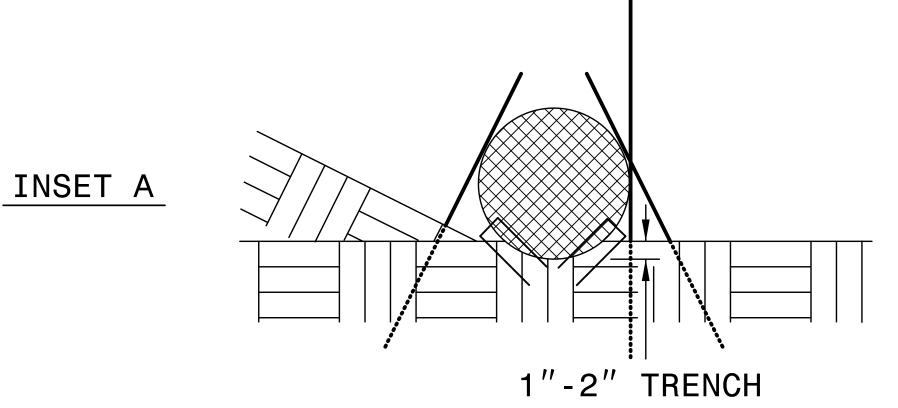
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

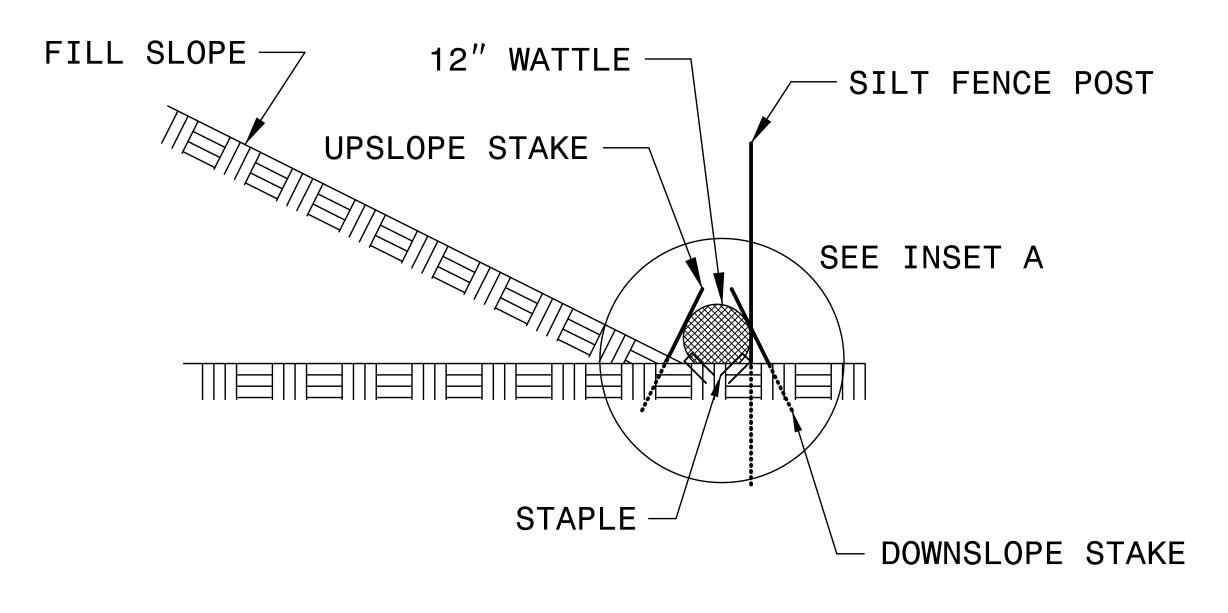
PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

WATTLE INSTALLATION CAN BE ON OUTSIDE OF THE SILT FENCE AS DIRECTED.

INSTALL TEMPORARY SILT FENCE IN ACCORDANCE WITH SECTION 1605 OF THE STANDARD SPECIFICATIONS.

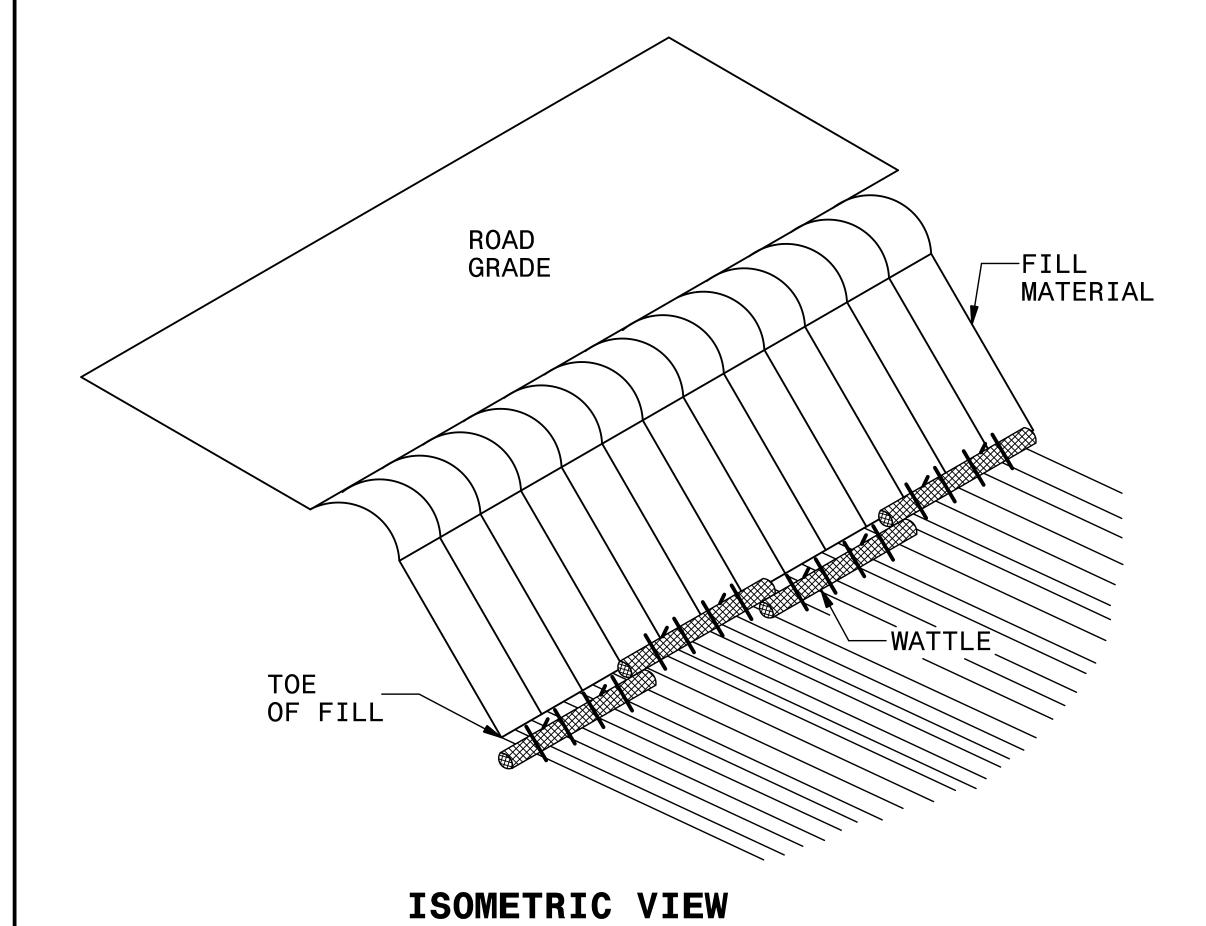


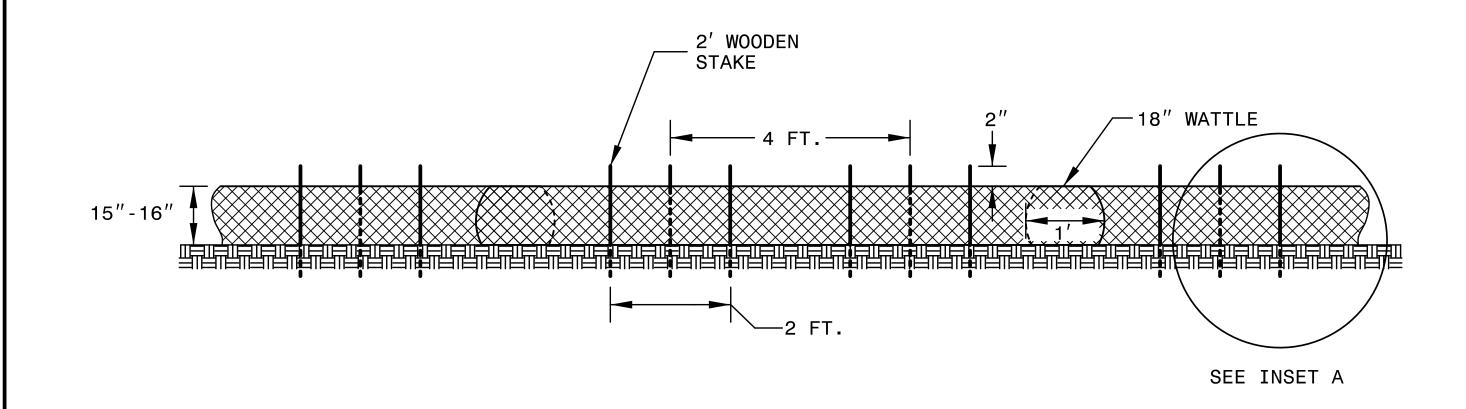


SIDE VIEW

COIR FIBER WATTLE BARRIER DETAIL

PROJECT REFERENCE NO	SHEET NO.				
<i>I7BP.3.R.29</i>		EC-03			
R/W SHEET N	R/W SHEET NO.				
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER			





FRONT VIEW

NOTES:

USE MINIMUM 18 IN. NOMINAL DIAMETER COIR FIBER (COCONUT) WATTLE AND LENGTH OF 10 FT.

EXCAVATE A 2 TO 3 INCH TRENCH FOR WATTLE TO BE PLACED.

DO NOT PLACE WATTLES ON TOE OF SLOPE.

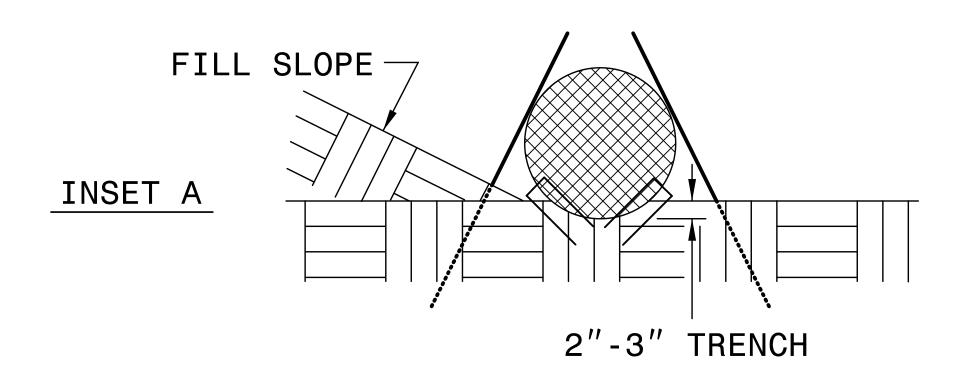
USE 2 FT. WOODEN STAKES WITH A 2 IN. BY 2 IN. NOMINAL CROSS SECTION.

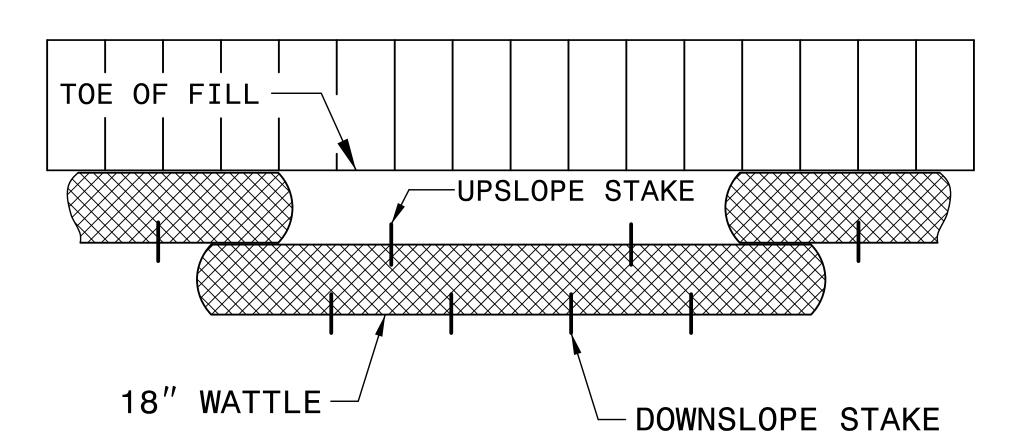
INSTALL A MINIMUM OF 2 UPSLOPE STAKES AND 4 DOWNSLOPE STAKES AT AN ANGLE TO WEDGE WATTLE TO GROUND.

PROVIDE STAPLES MADE OF 0.125 IN. DIAMETER STEEL WIRE FORMED INTO A U SHAPE NOT LESS THAN 12" IN LENGTH.

INSTALL STAPLES APPROXIMATELY EVERY 1 LINEAR FOOT ON BOTH SIDES OF WATTLE AND AT EACH END TO SECURE IT TO THE SOIL.

FOR BREAKS ALONG LARGE SLOPES, USE MAXIMUM SPACING OF 20 FT.





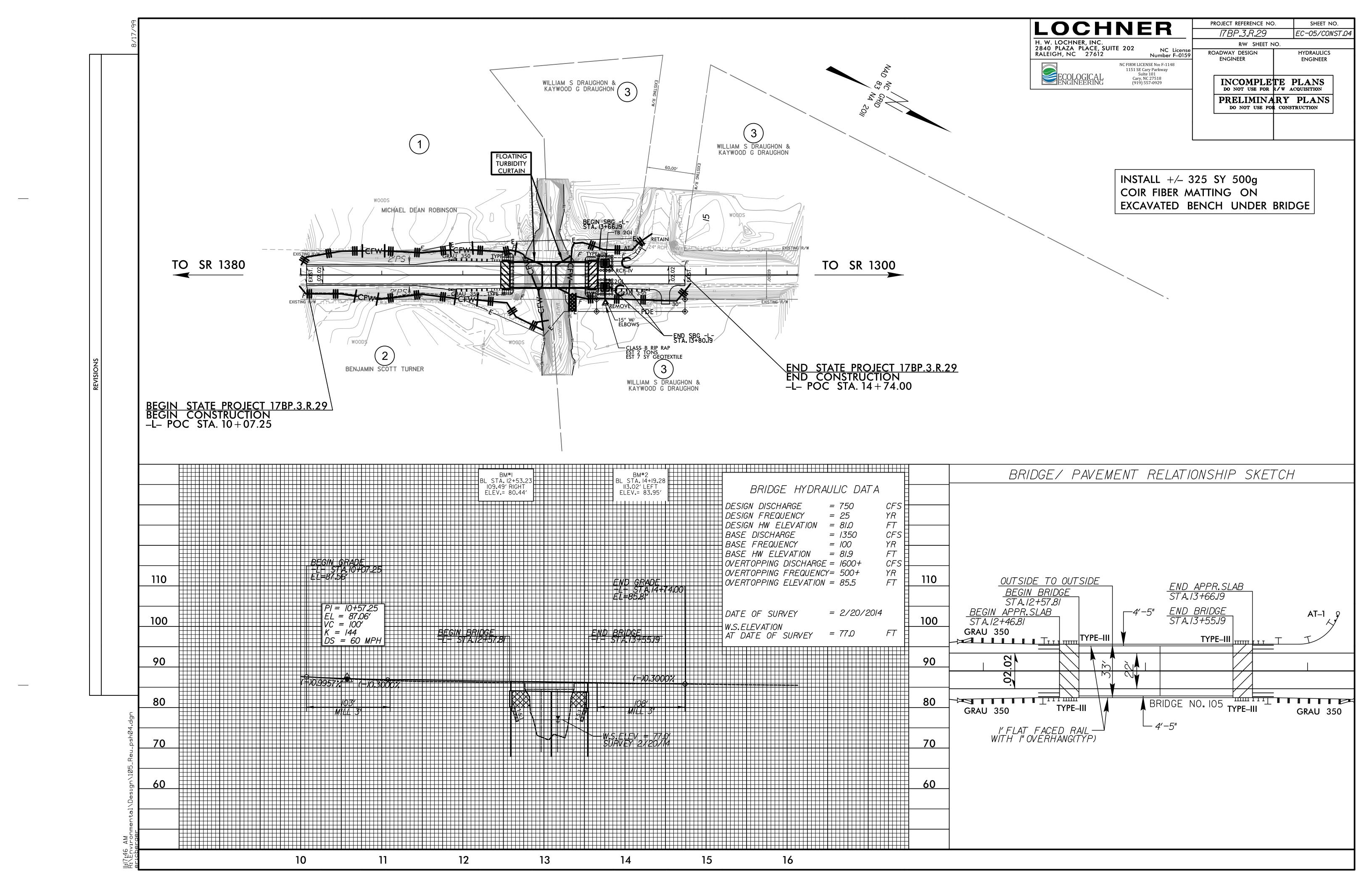
TOP VIEW

DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

PROJECT REFERENCE NO	PROJECT REFERENCE NO.			
<i>17BP.3.R.29</i>	<i>17BP.3.R.29</i>			
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER		

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:1 OR FLATTER	I4 DAYS	7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH.
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	NONE, EXCEPT FOR PERIMETERS AND HQW ZONES.

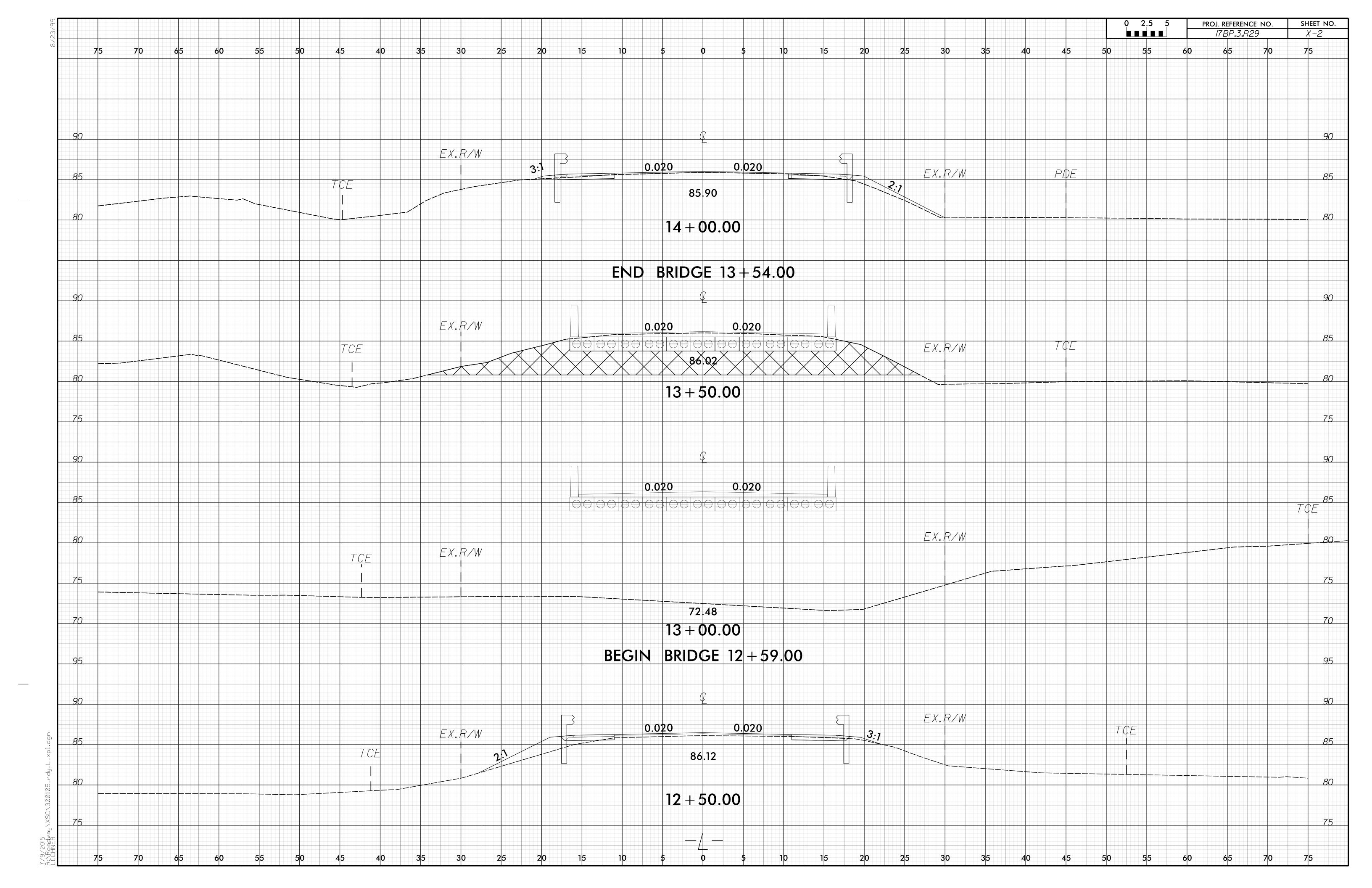


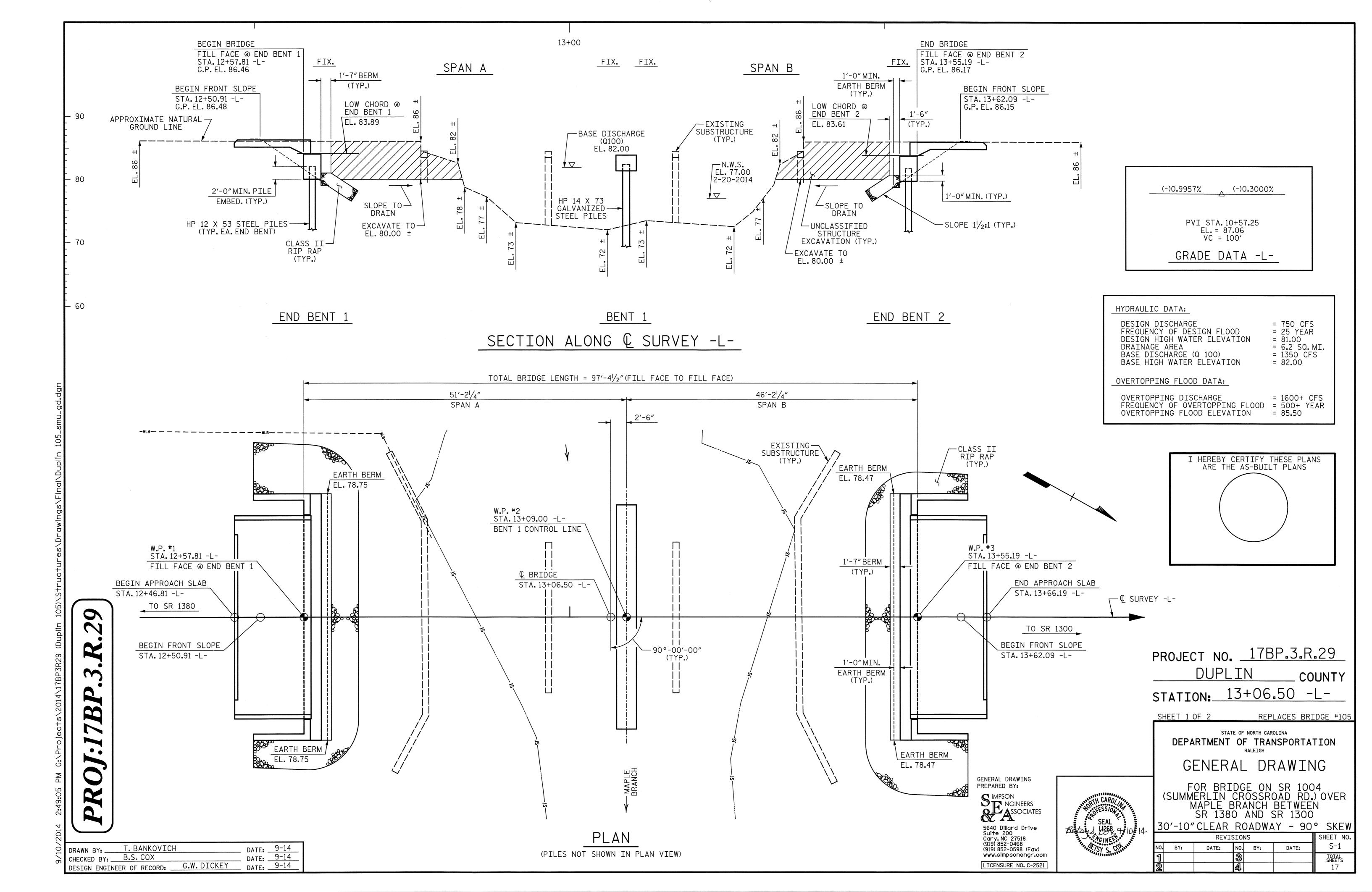
Approximate quantities only. Unclassified excavation, borrow excavation, shoulder borrow, fine grading, clearing and grubbing, breaking of existing pavement and removal of existing pavement will be paid for at the lump sum price for "Grading".

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PROJ. REFERENCE NO.SHEET NO.17BP.3.R.29X-A

NOTE: EMBANKMENT COLUMN INCLUDES BACKFILL FOR UNDERCUT	CROSS-SECTION SUMMARY
Station Uncl. Exc. Embt	
L (cu. yd.) (cu. yd.)	
10+07.25 0 0	
10+50.00 5 16	
11+00.00 4 18 11+50.00 3 24	
12+00.00 3 32	
12+50.00 5 30	
Station Uncl. Exc. Embt	
L (cu. yd.) (cu. yd.)	
14+00.00 0 0	
14+50.00 7 38	
14+74.00 3 16	





FOUNDATION NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS. PILES AT END BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 70 TONS PER PILE.

DRIVE PILES AT END BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 120 TONS PER PILE.

PILES AT BENT 1 ARE DESIGNED FOR A FACTORED RESISTANCE OF 120 TONS PER PILE.

DRIVE PILES AT BENT 1 TO A REQUIRED DRIVING RESISTANCE OF 210 TONS PER PILE. THIS REQUIRED DRIVING RESISTANCE INCLUDES ADDITIONAL RESISTANCE FOR DOWNDRAG OR SCOUR.

PILES AT END BENT 2 ARE DESIGNED FOR A FACTORED RESISTANCE OF 65 TONS PER PILE.

DRIVE PILES AT END BENT 2 TO A REQUIRED DRIVING RESISTANCE OF 110 TONS PER PILE.

INSTALL PILES AT BENT 1 TO A TIP ELEVATION NO HIGHER THAN 45.0 FEET.

THE SCOUR CRITICAL ELEVATION FOR BENT 1 IS ELEVATION 66.0 FEET. SCOUR CRITICAL ELEVATIONS ARE USED TO MONITOR POSSIBLE SCOUR PROBLEMS DURING THE LIFE OF THE STRUCTURE.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 35 TO 45 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT END BENT 1 AND 2. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

IT HAS BEEN ESTIMATED THAT A HAMMER WITH AN EQUIVALENT RATED ENERGY IN THE RANGE OF 40 TO 60 FT-KIPS PER BLOW WILL BE REQUIRED TO DRIVE PILES AT BENT 1. THIS ESTIMATED ENERGY RANGE DOES NOT RELEASE THE CONTRACTOR FROM PROVIDING DRIVING EQUIPMENT IN ACCORDANCE WITH SUBARTICLE 450-3(D)(2) OF THE STANDARD SPECIFICATIONS.

TESTING PILES WITH THE PILE DRIVING ANALYZER (PDA) DURING DRIVING, RESTRIKING OR REDRIVING MAY BE REQUIRED. THE ENGINEER WILL DETERMINE THE NEED FOR PDA TESTING, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS AND FOR PILE DRIVING CRITERIA, SEE PILE DRIVING CRITERIA PROVISION.

NOTES:

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE SHEET SN.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE EXISTING STRUCTURE CONSISTS OF 1 SPAN @ 20'-5", 1 SPAN @ 20'-3", AND 1 SPAN @ 20'-5" WITH A CLEAR ROADWAY WIDTH OF 24 FT. THE SUPERSTRUCTURE CONSISTS OF A REINFORCED CONCRETE DECK ON STEEL I-BEAMS. THE END BENTS AND BENTS ARE REINFORCED CONCRETE CAPS ON TIMBER PILES. THE EXISTING STRUCTURE, WHICH IS LOCATED AT THE SITE OF THE PROPOSED STRUCTURE, SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, THE LOAD LIMIT MAY BE REDUCED AS NECESSARY DURING THE LIFE OF THE PROJECT.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR A DISTANCE OF 30 FT. LEFT AND 70 FT. RIGHT OF CENTERLINE ROADWAY AT END BENT 1 AND FOR A DISTANCE OF 40 FT. LEFT AND 30 FT. RIGHT OF CENTERLINE ROADWAY AT END BENT 2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION. SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THE SUBSTRUCTURE OF THE EXISTING BRIDGE INDICATED ON THE PLANS IS FROM THE BEST INFORMATION AVAILABLE. THIS INFORMATION IS SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR. THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT OF TRANSPORTATION FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN THE EXISTING BRIDGE SUBSTRUCTURE SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH "HEC 18-EVALUATING SCOUR AT BRIDGES."

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

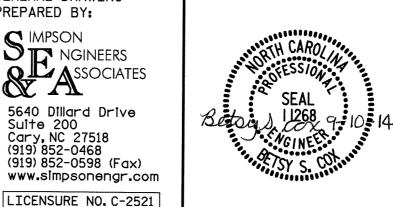
FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS. ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR "REMOVAL OF EXISTING STRUCTURE AT STATION 13+06.50 -L-."

TOTAL BILL OF MATERIAL VERTICAL CONCRETE REMOVAL OF UNCLASSIFIED CONCRETE GROOVING BRIDGE 3'-0" X 1'-9" RIP RAP **GEOTEXTIL** CLASS A REINFORCING HP 12 X 53 ELASTOMERIC EXISTING STRUCTURE WEARING BRIDGE APPROACH GALVANIZED **PRESTRESSED** TESTING CLASS II CONCRETE STEEL PILES FOR STEEL **STRUCTURE** REDRIVES BARRIER EXCAVATION SURFACE **FLOORS** SLABS BEARINGS CONCRETE CORED STEEL PILES (2'-0" THICK) DRAINAGE RAIL SLABS LS EΑ LS SF SF CY LS LB NO. LF NO. LF EΑ LF TON SY LS NO. LF SUPERSTRUCTURE 2,933 3,296 LS 190.50 LS 22 1045.00 END BENT LS 22.5 2,697 490 60 66 BENT 1 11.4 2,226 8 600 END BENT 2 LS 22.5 2,697 455 3 55 61 TOTAL LS LS 2,933 3,296 56.4 LS 7,620 14 945 8 600 10 190.50 115 127 22 | 1045.00

> GENERAL DRAWING PREPARED BY:

C IMPSON NGINEERS ASSOCIATES 5640 Dillard Drive Suite 200 Cary, NC 27518 (919) 852-0468 (919) 852-0598 (Fax)



PROJECT NO. <u>17BP.3.</u>R.29 DUPLIN COUNTY STATION: 13+06.50 -L-

SHEET 2 OF 2

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE ON SR 1004 (SUMMERLÍN CROSSROAD RD.) OVER MAPLE BRANCH BETWEEN SR 1380 AND SR 1300

30'-10" CLEAR ROADWAY - 90° SKEW REVISIONS SHEET NO S-2 BY: DATE: BY: DATE: TOTAL SHEETS

DATE: 9-14 T. BANKOVICH DRAWN BY: ____ CHECKED BY: B.S. COX DATE: 9-14 _ DATE: 9-14 DESIGN ENGINEER OF RECORD: G.W. DICKEY

LOAD AND RESISTANCE FACTOR RATING (LRFR) SUMMARY FOR PRESTRESSED CONCRETE GIRDERS SERVICE III LIMIT STATE STRENGTH I LIMIT STATE SHEAR MOMENT MOMENT DISTRIBUTION FACTORS (DF) LIVE-LOAD FACTORS (Y_{LL}) CONTROLL. RATING DISTI FACT DIST LEFT SPAN 22.000 0.277 1.34 1.24 ER ER 1.75 0.277 1.43 ER 22 0.539 2.2 0.80 1.24 HL-93 (INVENTORY) N/A 0.539 1.61 N/A 0.277 1.85 1.35 ER DESIGN LOAD HL-93 (OPERATING) N/A 1.65 22.000 $\langle 2 \rangle$ 52.852 1.47 ER 0.80 0.277 ER 1.47 1.75 0.277 0.539 RATING 36.000 1.75 ER 22 HS-20 (INVENTORY) 1.90 ER 68.512 1.35 0.277 2.27 N/A ---1.90 ER 0.539 HS-20 (OPERATING) 36.000 22.000 0.277 3.22 0.80 ER 4.29 4.05 13.500 3.22 43.456 1.40 0.277 ER 0.539 SNSH 2.60 22.000 0.80 0.277 ER 0.539 ER 51.980 1.40 0.277 3.46 ER 2.2 2.60 20.000 SNGARBS2 22.000 0.277 2.53 55.751 2.80 ER 0.80 ER 1.40 0.277 3.35 2.53 ER 17.6 0.539 SNAGRIS2 22.000 0.277 1.61 22.000 2.03 ER 0.80 ER 43.796 1.40 0.277 2.14 ER 0.539 1.61 В SNCOTTS3 27.250 0.80 0.277 1.42 22.000 1.75 ER 49.533 0.539 2.2 0.277 1.89 ER SNAGGRS4 34.925 1.42 1.40 1.38 22.000 0.277 0.539 1.81 ER ER 2.2 0.80 1.38 49.115 1.40 0.277 1.84 ER 35.550 SNS5A 22.000 0.277 1.30 0.80 0.277 1.73 0.539 1.69 ER 1.30 51.991 1.40 ER 22 39.950 В SNS6A 22.000 0.80 0.277 1.24 0.539 ER 0.277 1.65 1.70 1.24 52.106 1.40 ER 42.000 LEGAL LOAD SNS7B 22.000 0.277 1.60 ER 0.539 ER 0.80 2.13 ER 1.98 1.60 52.711 1.40 0.277 RATING 33.000 TNAGRIT3 0.539 0.277 1.61 22.000 1.90 ER 0.80 ER ER 33.075 1.61 53.384 1.40 0.277 2.15 TNT4A 0.277 22.000 0.80 1.35 0.539 ER 1.36 56.351 1.40 0.277 1.80 ER 22 1.85 В 41.600 TNT6A 22.000 0.277 1.38 ER 0.277 1.84 ER ER 2.2 1.40 0.539 1.71 1.38 57.992 TNT7A 42.000 0.277 22.000 0.539 ER 2.2 1.44 ER 1.63 0.80 60.375 1.40 0.277 1.91 ER 22 42.000 1.44 TNT7B 0.539 1.57 0.80 0.277 22.000 1.82 ER 58.872 1.40 0.277 43.000 TNAGRIT4 TNAGT5A 22 0.539 1.60 45.000 2.2 0.80 0.277 1.24 22.000 ER 1.66 0.539 1.48 ER 55.976 1.24 1.40 0.277 ER 22 TNAGT5B 45.000

LOAD FACTORS:

DES	IGN	LIMIT STATE	γ_{DC}	$\gamma_{\sf DW}$
LO RAT	ING	STRENGTH I	1.25	1.50
FAC1	rors	SERVICE III	1.00	1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

2.

ے ع

4.

(#) CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

3 LEGAL LOAD RATING **

GIRDER LOCATION

** SEE CHART FOR VEHICLE TYPE

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. 17BP.3.R.29

DUPLIN COUNTY

STATION: 13+06.50 -L-

DEPARTMENT OF TRANSPORTATION
RALEIGH

LRFR SUMMARY FOR PRESTRESSED

CONCRETE GIRDERS (NON-INTERSTATE TRAFFIC)

	SHEET NO.				
BY:	DATE:	NO.	BY:	DATE:	
		3			TOTAL SHEETS
		4]

STD. NO. LRFR1

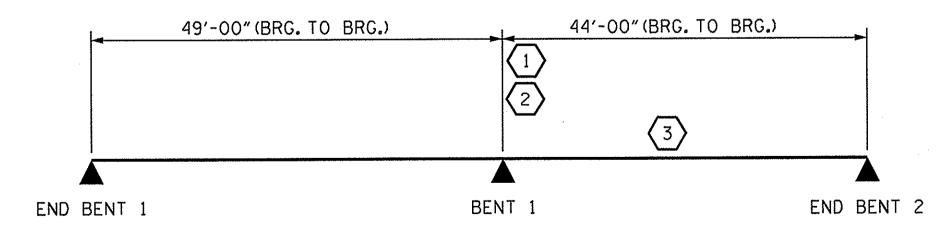
SEAL 21271

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CALLED ON WORKER SEAL 21271

CALLED ON WORKER SEAL 21271



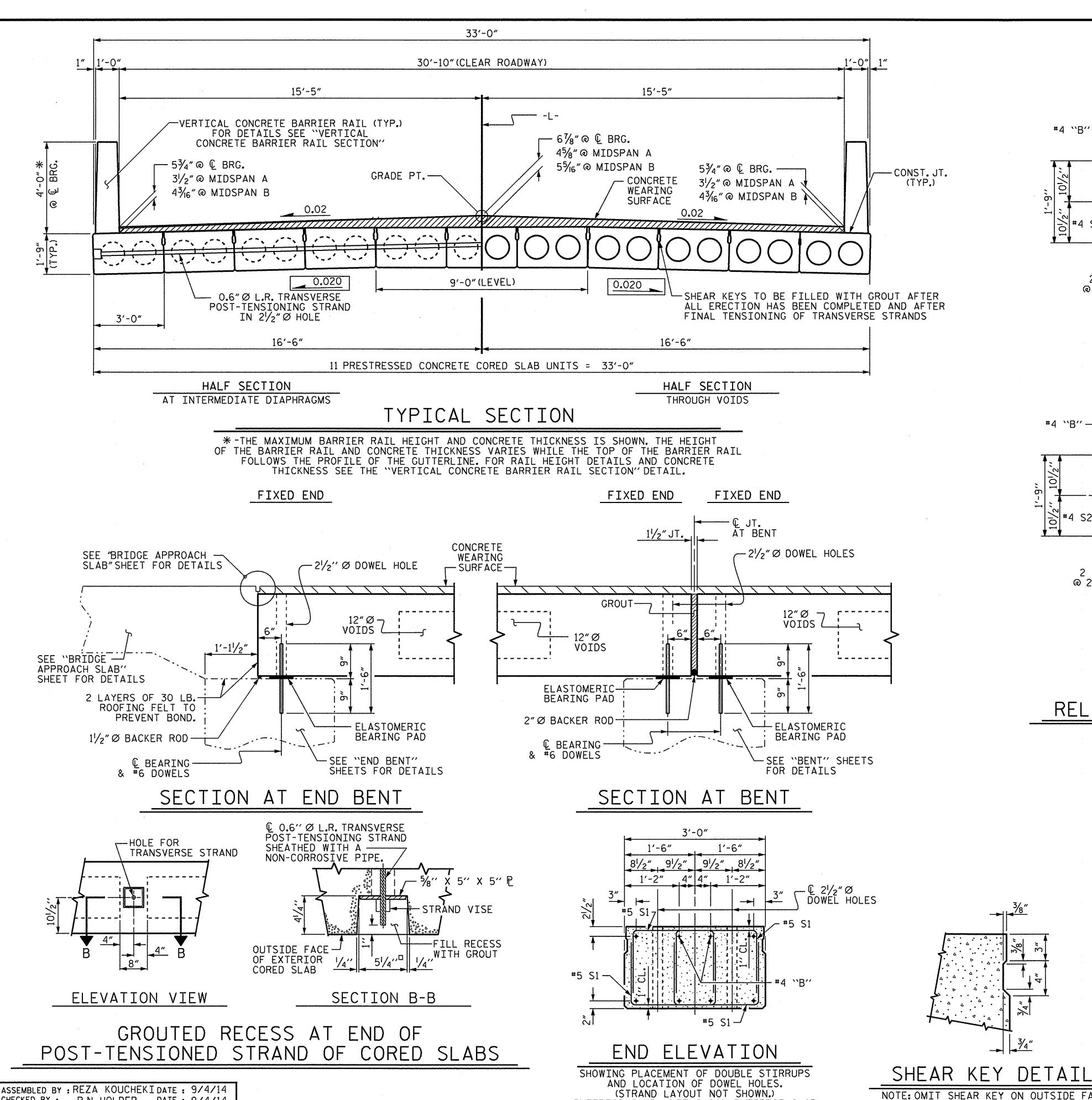
LRFR SUMMARY

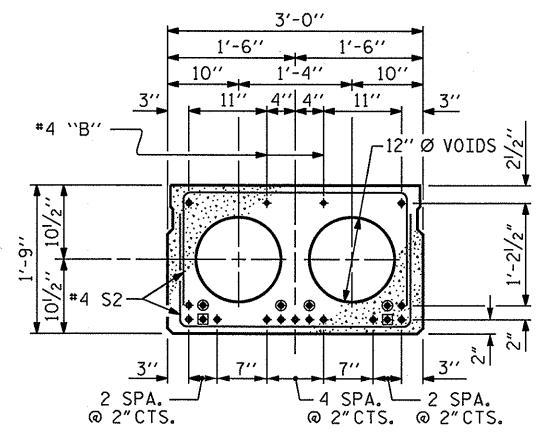
ASSEMBLED BY: REZA KOUCHEKI DATE: 9/4/14
CHECKED BY: P.N. HOLDER DATE: 9/4/14

DRAWN BY: MAA I/O8
CHECKED BY: GM/DI 2/08

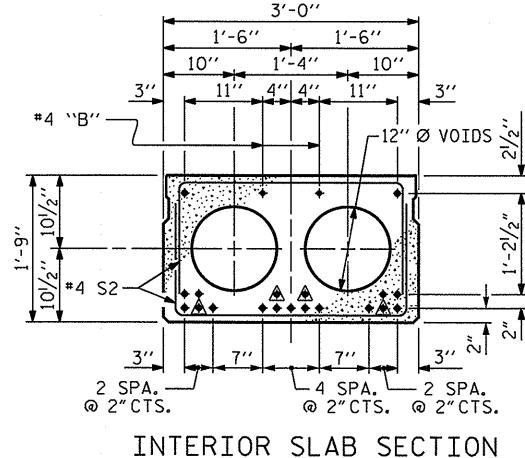
REV. II/I2/O8RR MAA/GM
REV. IO/I/II MAA/GM

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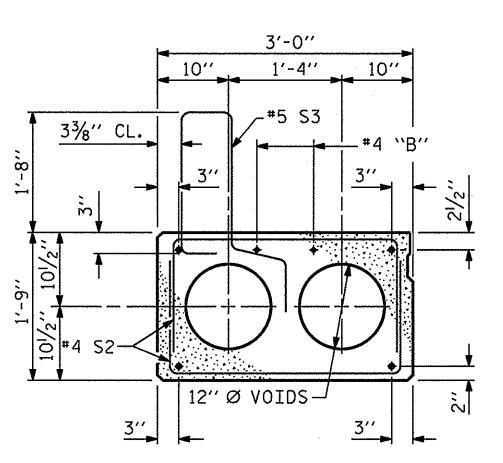
INTERIOR SLAB SECTION (45' UNIT) (15 STRANDS REQUIRED)



0.6" Ø LOW RELAXATION STRAND LAYOUT

(50' UNIT)

(19 STRANDS REQUIRED)



EXT. SLAB SECTION (FOR PRESTRESSED STRAND LAYOUT, SEE

INTERIOR SLAB SECTION.)

- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 6'-O" FROM END OF CORED SLAB UNIT SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- BOND SHALL BE BROKEN ON THESE STRANDS FOR A DISTANCE OF 2'-O"FROM END OF CORED SLAB UNIT. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.
- OPTIONAL FULL LENGTH DEBONDED STRANDS. THESE STRANDS ARE NOT REQUIRED. IF THE FABRICATOR CHOOSES TO INCLUDE THESE STRANDS IN THE CORED SLAB UNIT, THE STRANDS SHALL BE DEBONDED FOR THE FULL LENGTH OF THE UNIT AT NO ADDITIONAL COST. SEE STANDARD SPECIFICATIONS, ARTICLE 1078-7.

DEBONDING LEGEND

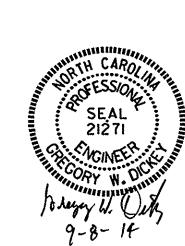
PROJECT NO. 17BP.3.R.29 DUPLIN COUNTY STATION: 13+06.50 -L-

SHEET 1 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD

3'-0" X 1'-9" PRESTRESSED CONCRETE CORED SLAB UNIT 90° SKEW

SHEET NO. REVISIONS DATE: DATE: BY: BY: TOTAL SHEETS

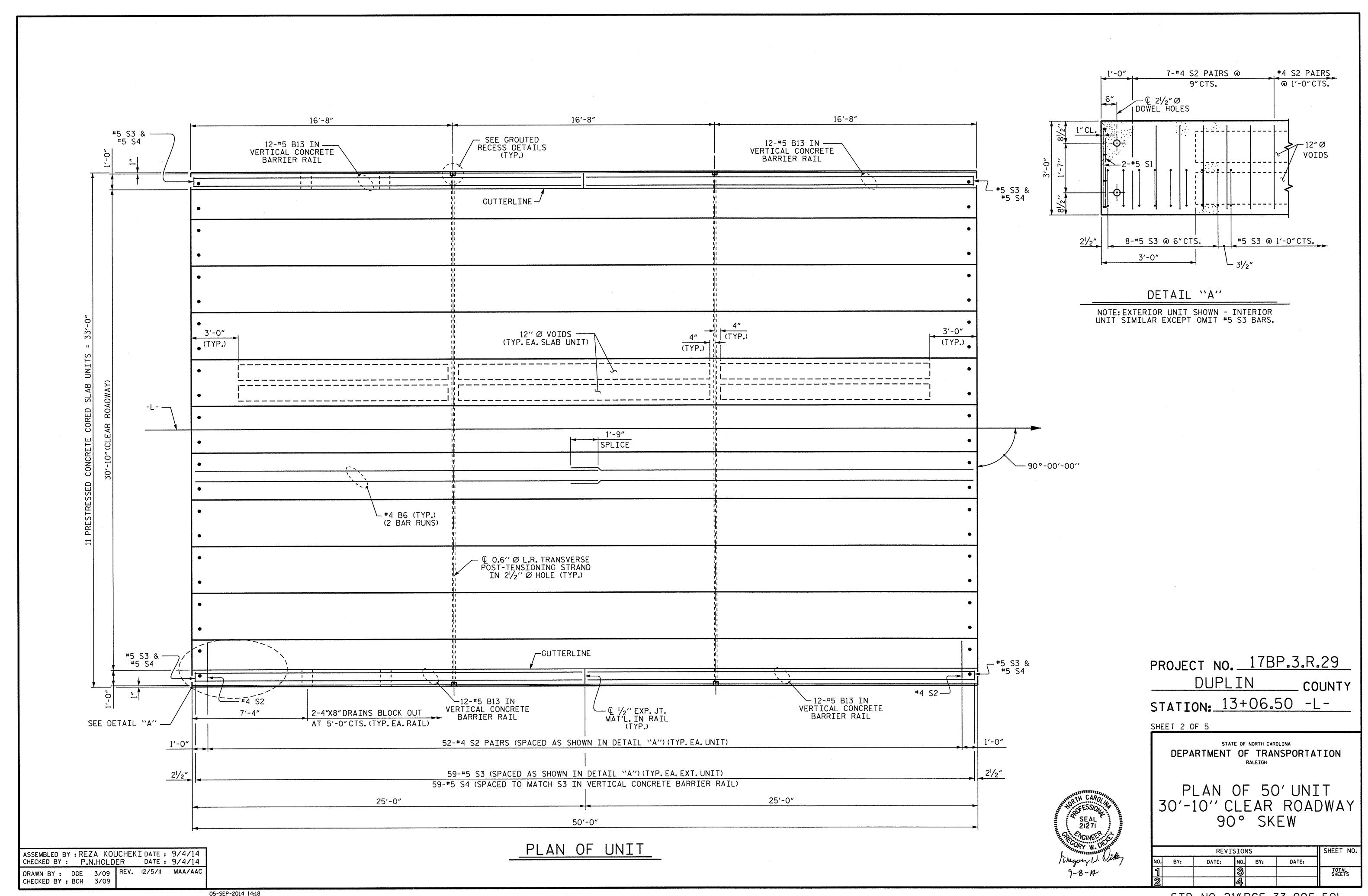


INTERIOR SLAB UNIT SHOWN-EXTERIOR SLAB UNIT SIMILAR EXCEPT SHEAR KEY LOCATION.

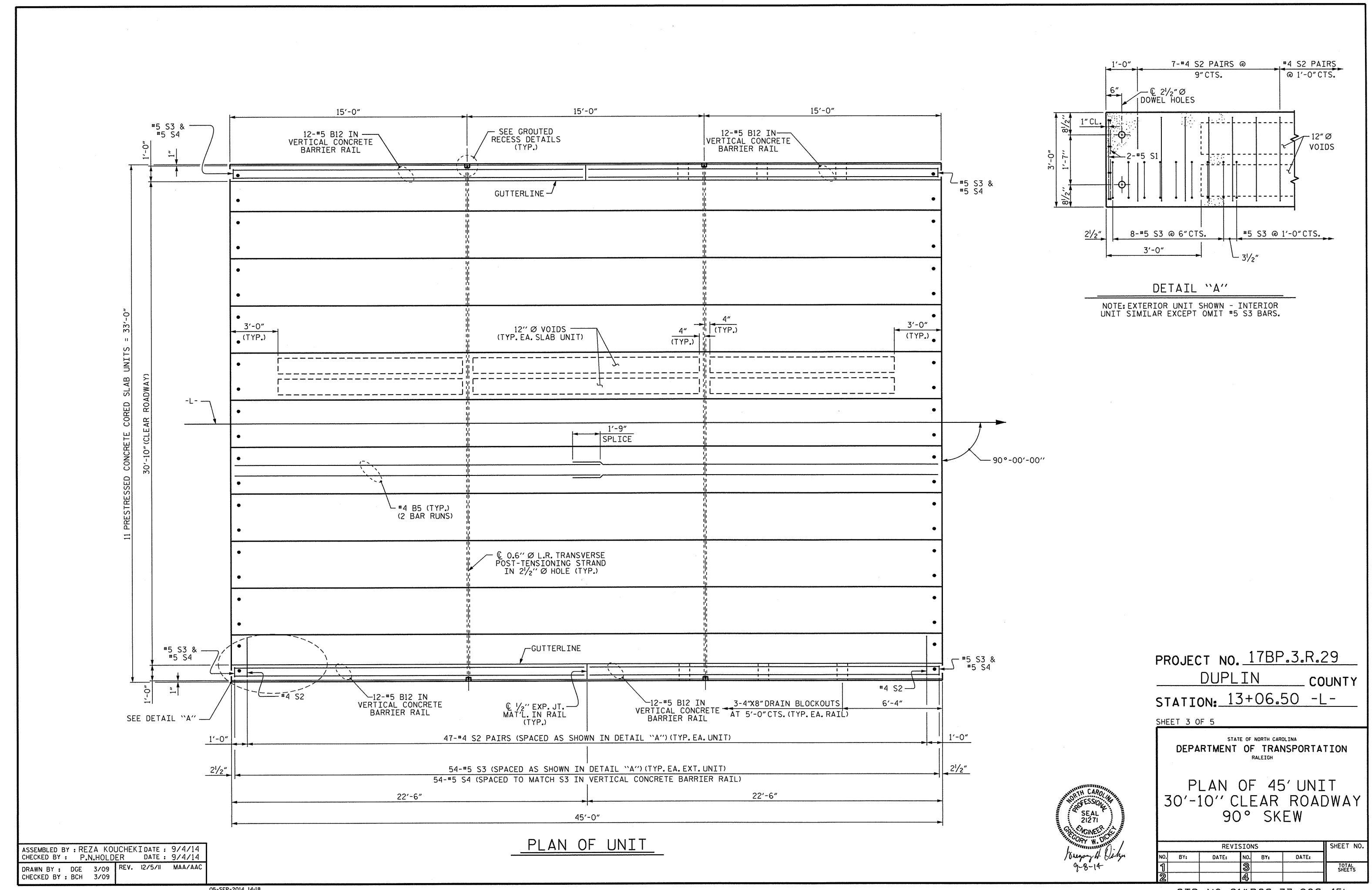
CHECKED BY : P.N. HOLDER DATE : 9/4/14

DRAWN BY : DGE 5/09 REV. 12/11

CHECKED BY : BCH 6/09

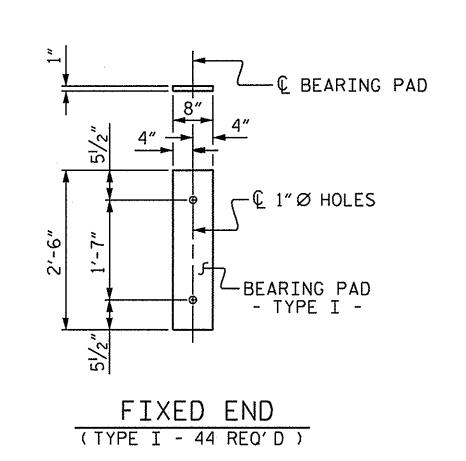


STD. NO. 21" PCS_33_90S_50L



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STD. NO. 21" PCS_33_90S_45L

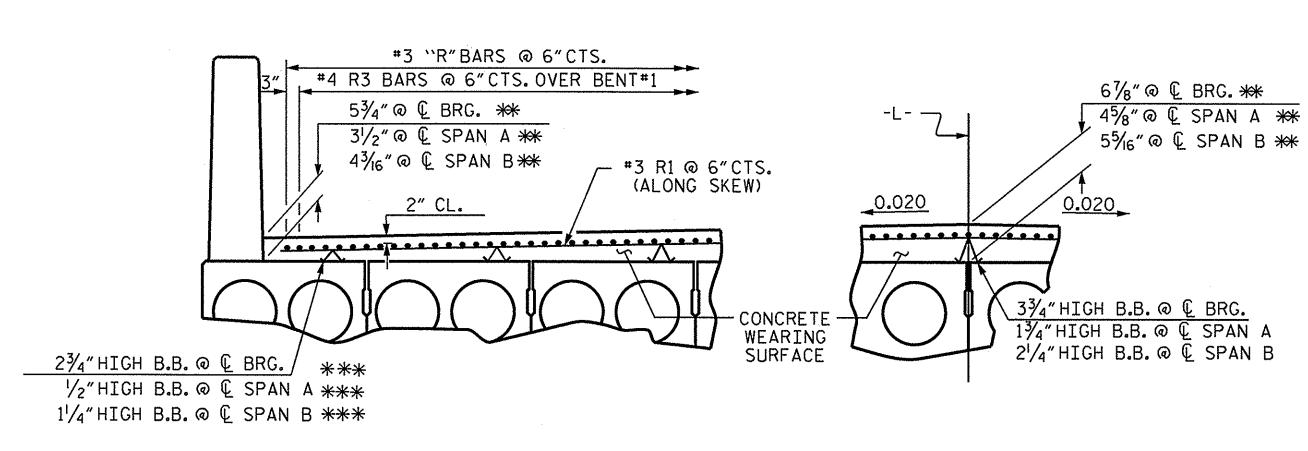


ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 50 DUROMETER HARDNESS.

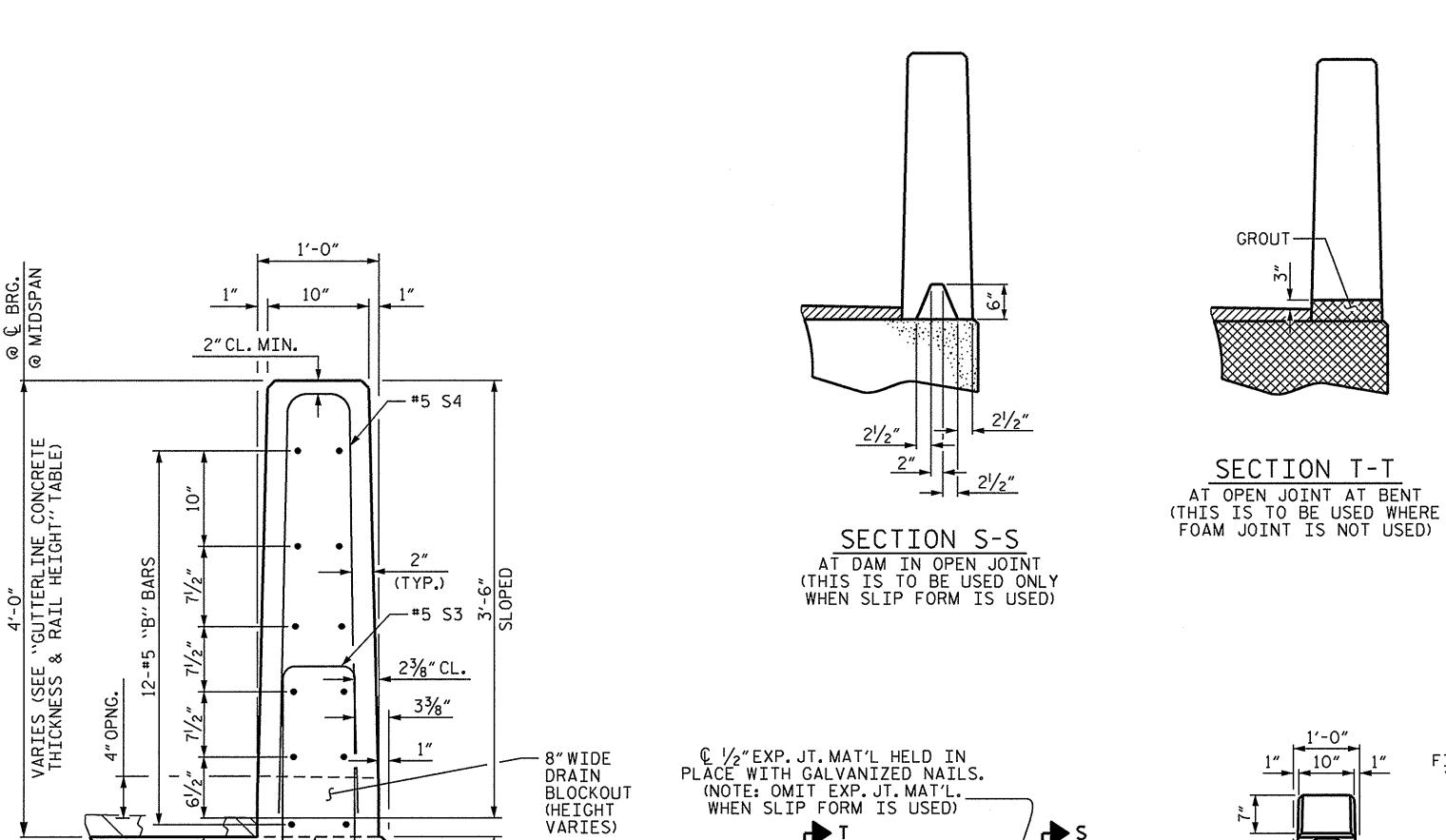
VERTICAL DIM. VARIES

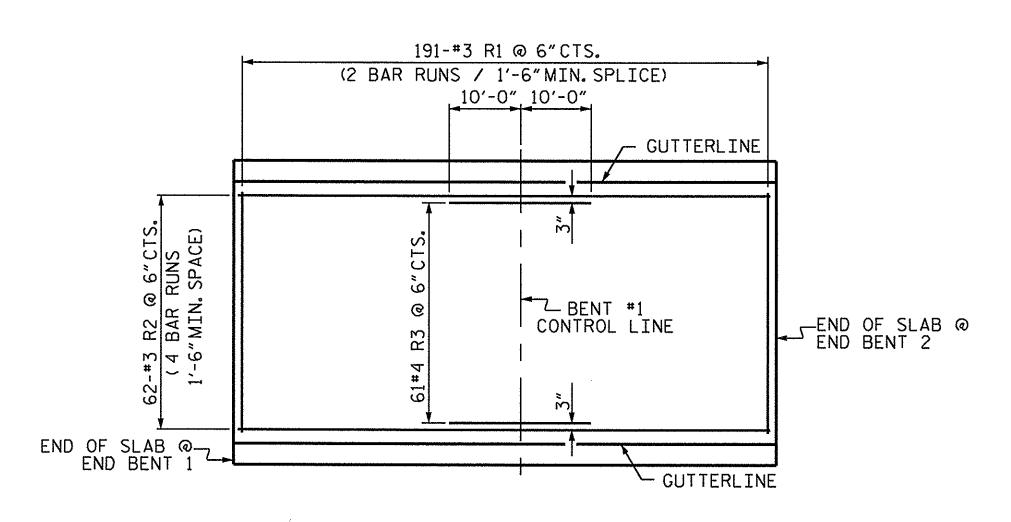
#5 S3 SEE "PLAN OF UNIT" FOR SPACING



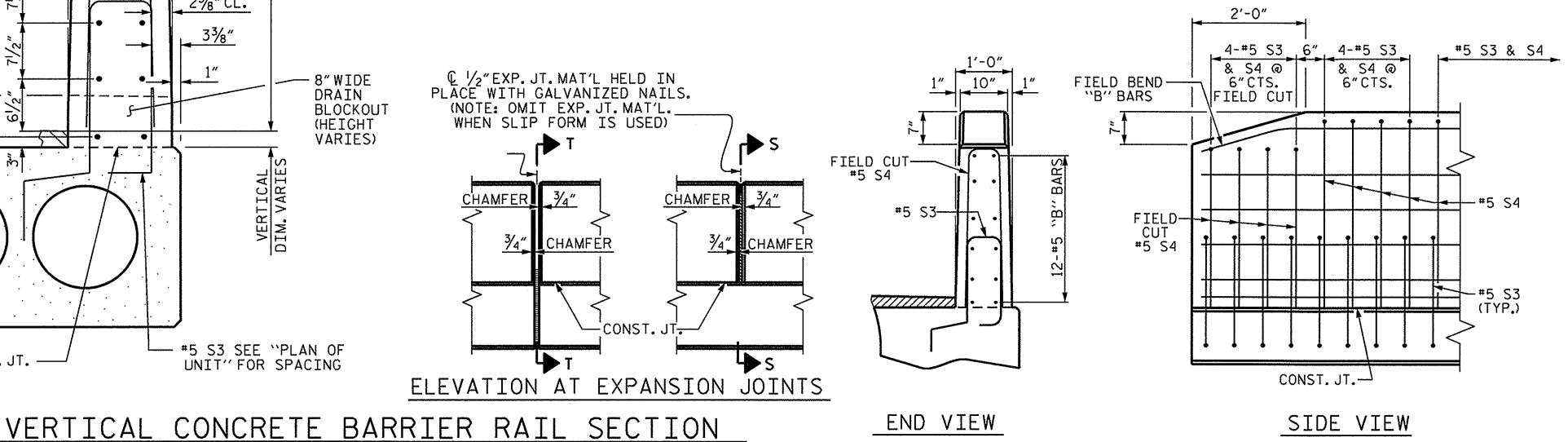
REINFORCING FOR CONCRETE WEARING SURFACE

** BASED ON PREDICTED FINAL CAMBER AND THEORETICAL GRADE LINE ELEVATIONS ** BEAM BOLSTER (B.B.) SHALL BE SPACED AT 2'-0" CENTERS SET 1'-0" FROM GUTTERLINE.





PLAN SHOWING CONCRETE WEARING SURFACE REINFORCING STEEL



ASSEMBLED BY : REZA KOUCHEKIDATE : 9/4/14 CHECKED BY : P.N.HOLDER DATE : 9/4/14 DRAWN BY: DGE 5/09 CHECKED BY: BCH 6/09

CONST. JT. —

END OF RAIL DETAILS

PROJECT NO. 17BP.3.R.29 DUPLIN COUNTY STATION: 13+06.50 -L-

SHEET 4 OF 5

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION STANDARD 3'-0" X 1'-9"
PRESTRESSED CONCRETE
CORED SLAB UNIT
90° SKEW

SHEET NO. **REVISIONS** BY: NO. BY: TOTAL SHEETS

	BILL OF MATERIAL FOR ONE 45' CORED SLAB UNIT						
				EXTERIO	OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B5	4	#4	STR	23'-3"	62	23'-3"	62
S1	8	#5	3	4'-3"	35	4'-3"	35
S2	94	#4	3	5′-4″	335	5'-4"	335
* S3	54	#5	1	6′-2"	347		
REINF(ORCING :	STEEL	LBS	S	432		432
	(Ý COATE IFORCINO		LB:	5.	347		
5000 1	P.S.I.CO	NCRETE	CU. YDS	> •	6.5		6.5
0.6"Ø	L.R. STR	ANDS	No).	15		15

				ATERIA	L FOR O	NE	
				EXTERI	OR UNIT	INTERI(OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
В6	4	#4	STR	25'-9"	69	25'-9"	69
S1	8	#5	3	4'-3"	35	4'-3"	35
S2	104	#4	3	5′-4″	371	5′-4″	371
* S3	59	#5	1	6′-2″	379		
						·	
					4		
REINFORCING STEEL LBS. 475				475			
*EPOXY COATED REINFORCING STEEL LBS. 379							
6500	6500 P.S.I. CONCRETE CU. YDS.			7.1		7.1	
0.6"Ø	0.6"Ø L.R. STRANDS No.).	19		19	

BI	LL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	IER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	45' UNIT					
 ₩ B12	48	48	#5	STR	22'-1"	1106
* S4	108	108	#5	2	7′-2″	807
∗ EP0X	Y COATED REINFORCING STEEL			LBS.		1913
CLASS	AA CONCRETE			CU.YDS.	•	12.2
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN. FT.		90.25

BI	LL OF MATERIAL FOR VERTI	CAL CONC	RETE	BARR	IER R	AIL
BAR	BARS PER PAIR OF EXTERIOR UNITS	TOTAL NO.	SIZE	TYPE	LENGTH	WEIGHT
	50' UNIT					
 ₩B13	48	48	#5	STR	24'-7"	1231
* S4	118	118	#5	2	7′-2″	882
★ EPOX	Y COATED REINFORCING STEEL			LBS.		2113
CLASS	AA CONCRETE			CU.YDS.		13.4
TOTAL	VERTICAL CONCRETE BARRIER RAIL			LN. FT.		100.25

ASSEMBLED BY : REZA KOL	JCHEKI DATE :	9/4/14
CHECKED BY : P.N. HOLI	DER DATE :	9/4/14
DRAWN BY: DGE 5/09	REV. 12/11	MAA/AA(
CHECKED BY : BCH 6/09		

DEAD LOAD DEFLECTION AT	ND CAMBER
	3'-0"× 1'-9"
45' CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	1 ¹¹ / ₁₆ " 🛉
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	/8″ ↓
FINAL CAMBER	1%6″ ♦

DEAD LOAD DEFLECTION AND	ND CAMBER
	3'-0"× 1'-9"
50'CORED SLAB UNIT	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	27⁄16″ ♦
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD	3/16″ ♦
FINAL CAMBER	21/4" 🕴

GUTTERLINE CONC	RETE THICKNESS & RA	IL HEIGHT
30'-10" CLEAR ROADWAY	CONCRETE OVERLAY THICKNESS	RAIL HEIGHT
	@ MID-SPAN	@ MID-SPAN
	NORMAL CROWN SECTION	
45' UNITS	43/16"	3′-105⁄ ₁₆ ″
50' UNITS	31/2"	3'-9%6"

TOTAL	11		495'-0"
	1-1-1-1		
CORED	SLABS	S REQ	UIRED
			TOTAL LENGTH
50' UNIT	TOMO E.T.		
EXTERIOR C.S.	2	50'-0"	100′-0"
INTERIOR C.S.	9	50'-0"	450′-0″

CORED SLABS REQUIRED

INTERIOR C.S. 9 45'-0" 405'-0"

EXTERIOR C.S. 2 45'-0"

45' UNIT

TOTAL

|NUMBER|LENGTH|TOTAL LENGTH

90′-0″

550'-0"

CON	BILL C	F MAI	ERIAL RING S	FOR SURFAC	E
BAR	NO.	SIZE	TYPE	LENGTH	WEIGHT
 ₩R1	382	#3	STR	16'-0"	2298
 ₩R2	248	#3	STR	24'-10"	2316
*R3	61	#4	STR	20′-0″	815
* EPOXY C	OATED RE	INFORCI	NG STEEL	LBS.	5429
CONCRET	E WEARIN	IG SURFA	CE	SQ.FT.	2933

CONCRETE	RELEASE	STRENGTH
UNIT		PSI
45' UNITS		4000
50'UNITS		4900

BAR TYPES

2′-8′′

ALL BAR DIMENSIONS ARE OUT TO OUT

73/4"

1

GRADE 270 S	TRANDS
	0.6" Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH (LBS.PER STRAND)	58,600
APPLIED PRESTRESS (LBS.PER STRAND)	43,950

GROOVING B	RIDGE	FLOORS
APPROACH SLABS	656	SQ.FT.
BRIDGE DECK	2640	SQ.FT.
TOTAL	3296	SQ.FT.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2\frac{1}{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH NON-SHRINK GROUT.

THE BACKER RODS SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN THE VERTICAL CONCRETE BARRIER RAIL SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, 1/2" IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE BARRIER RAIL AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN BARRIER RAIL EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF BARRIER RAIL SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

TRANSVERSE POST TENSIONING OF THE CORED SLAB UNITS SHALL BE DONE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE CORED SLAB UNIT SHALL BE DONE WHEN THE CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN THE REQUIRED STRENGTH SHOWN IN THE "CONCRETE RELEASE STRENGTH" TABLE.

PLACEMENT OF THE CONCRETE WEARING SURFACE SHALL OCCUR AFTER CASTING THE CONCRETE RAIL. THE COST OF THE #3 AND #4 BARS CAST WITH THE CONCRETE WEARING SURFACE SHALL BE INCLUDED IN THE UNIT PRICE BID.

FOR CONCRETE WEARING SURFACE. FOR CONCRETE WEARING SURFACE, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

THE BOTTOM TWO #5 "B" BARS IN THE VERTICAL CONCRETE BARRIER RAIL MAY BE FIELD CUT TO AVOID DRAINS.

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4"X 8". THE HEIGHT OF THE BLOCKOUT IN THE VERTICAL CONCRETE BARRIER RAIL SHALL EXTEND FROM THE TOP OF THE CORED SLAB UNIT TO THE TOP OF THE DRAIN OPENING.

APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE BARRIER RAIL.

PROJECT NO. 17BP.3.R.29

DUPLIN COUNTY

STATION: 13+06.50 -L-

SHEET 5 OF 5

DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

3'-0'' X 1'-9''

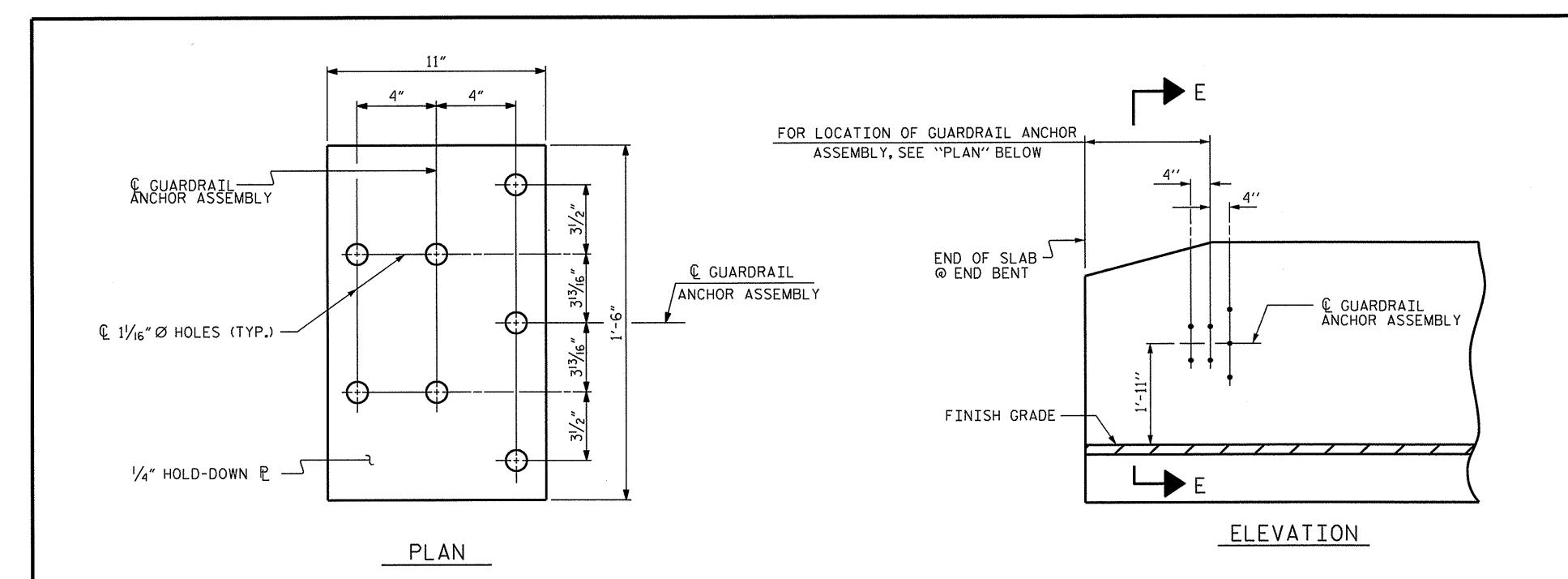
PRESTRESSED CONCRETE

CORED SLAB UNIT

90° SKEW

A. I. I. I.	OR!	eess Sea	ROZ/A	A STATE		
	Q.	SE A 2127	L 	A LANGE	MININ	
CRAIN		VCIM	ER			
h.	13711	PY V	in h	111.		

REVISIONS					SHEET NO
	DATE:	NO.	BY:	DATE:	
		3			TOTAL SHEETS
		4			



NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{1}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)

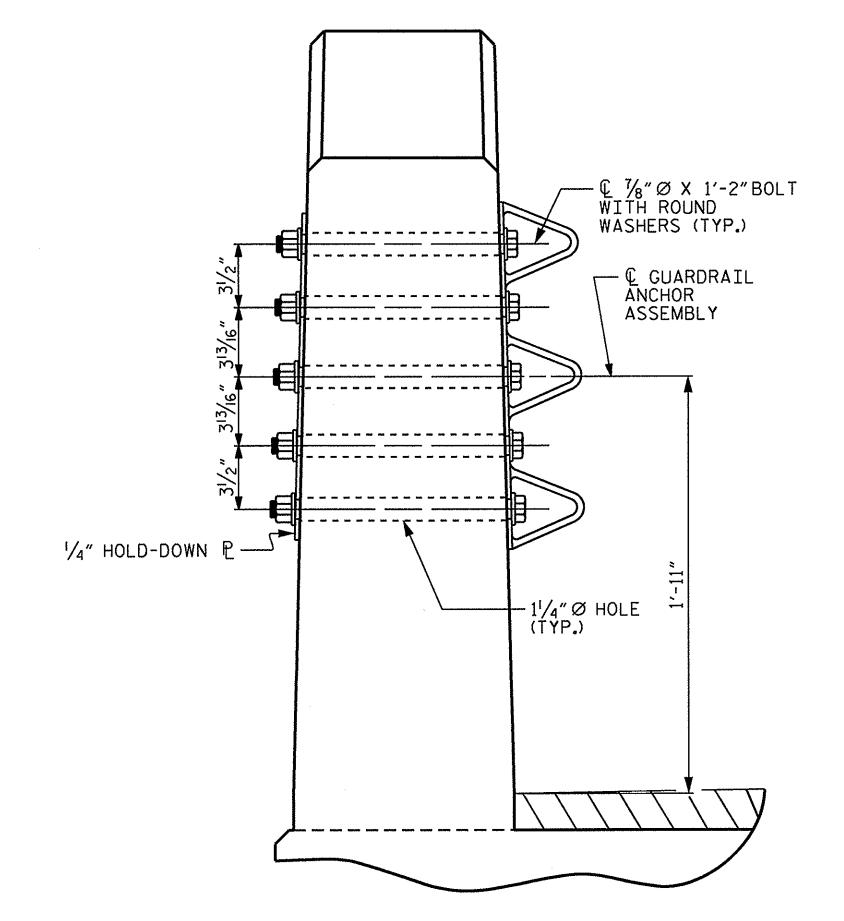
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF BARRIER RAIL. FOR POINTS OF ATTACHMENT. SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

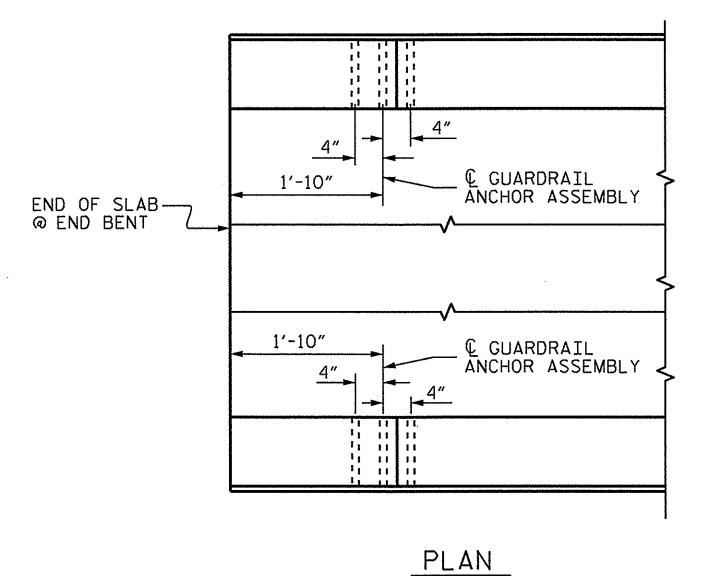
THE COST OF THE GUARDRAIL ANCHOR ASSEMBLY SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR VERTICAL CONCRETE BARRIER RAIL.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE VERTICAL CONCRETE BARRIER RAIL TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ $^{\prime\prime}$ \varnothing HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.

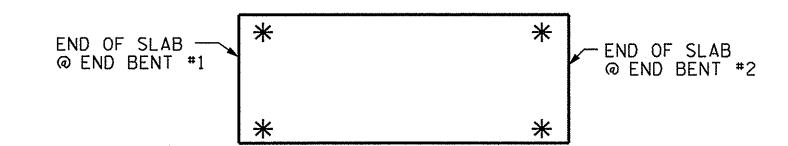


SECTION E-E GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF ANCHORS FOR GUARDRAIL

END BENT #1 SHOWN, END BENT #2 SIMILAR.



SKETCH SHOWING POINTS OF ATTACHMENT

* DENOTES GUARDRAIL ANCHOR ASSEMBLY

PROJECT NO. 17BP.R.3.29 DUPLIN COUNTY STATION: 13+06.50 -L-

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

STANDARD GUARDRAIL ANCHORAGE

FOR VERTICAL CONCRETE BARRIER RAIL

> SHEET NO. REVISIONS BY: DATE: DATE: TOTAL SHEETS



(SHT 1)

STD. NO. GRA3

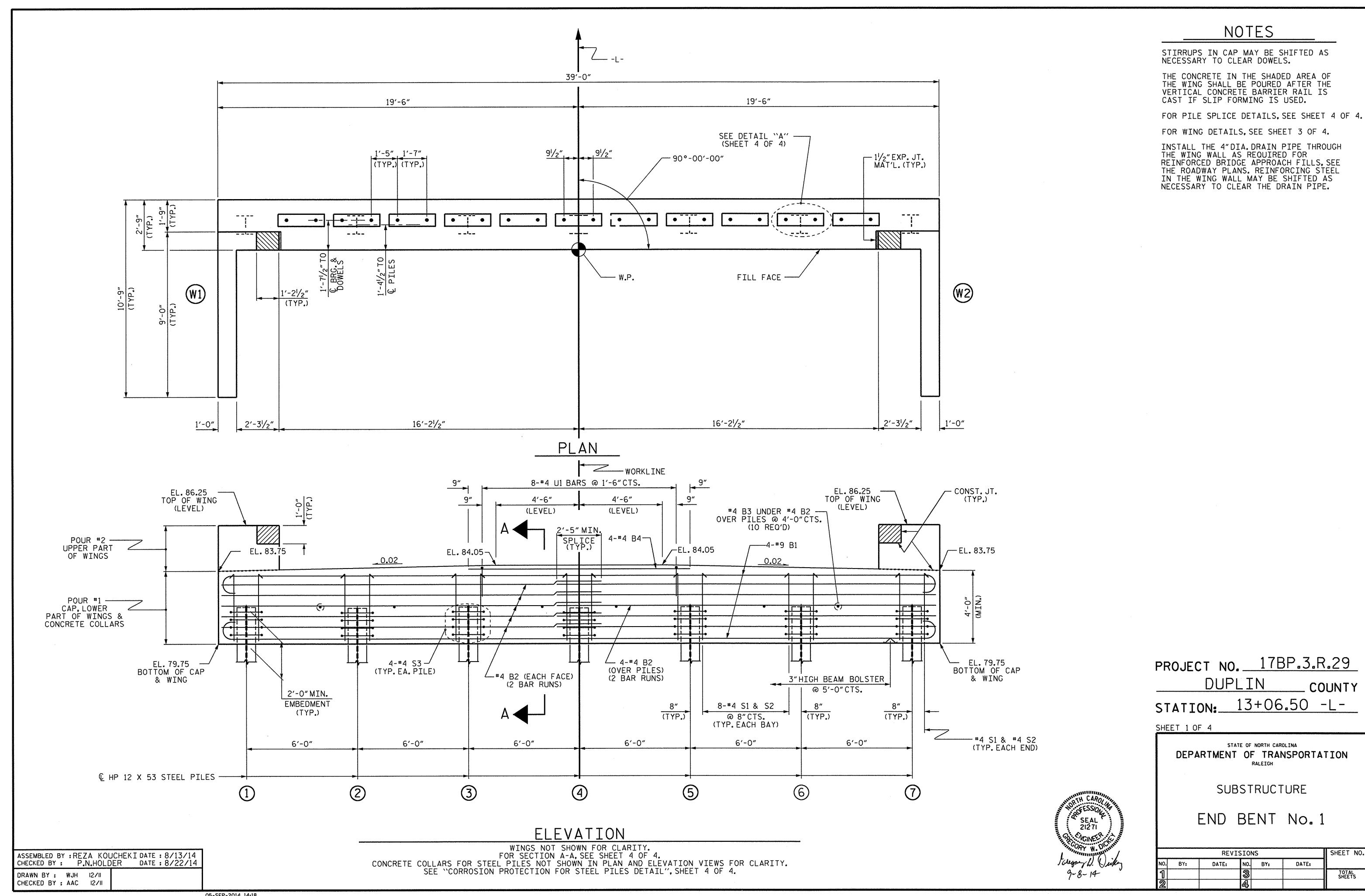
05-SEP-2014 14:18 S:\DPG1\Division3\17BP3R29\FinalPlans\17BP3R29_SD_AB.dgn

ASSEMBLED BY : REZA KOUCHEKI DATE : 9/4/14 CHECKED BY : P.N.HOLDER DATE : 9/4/14

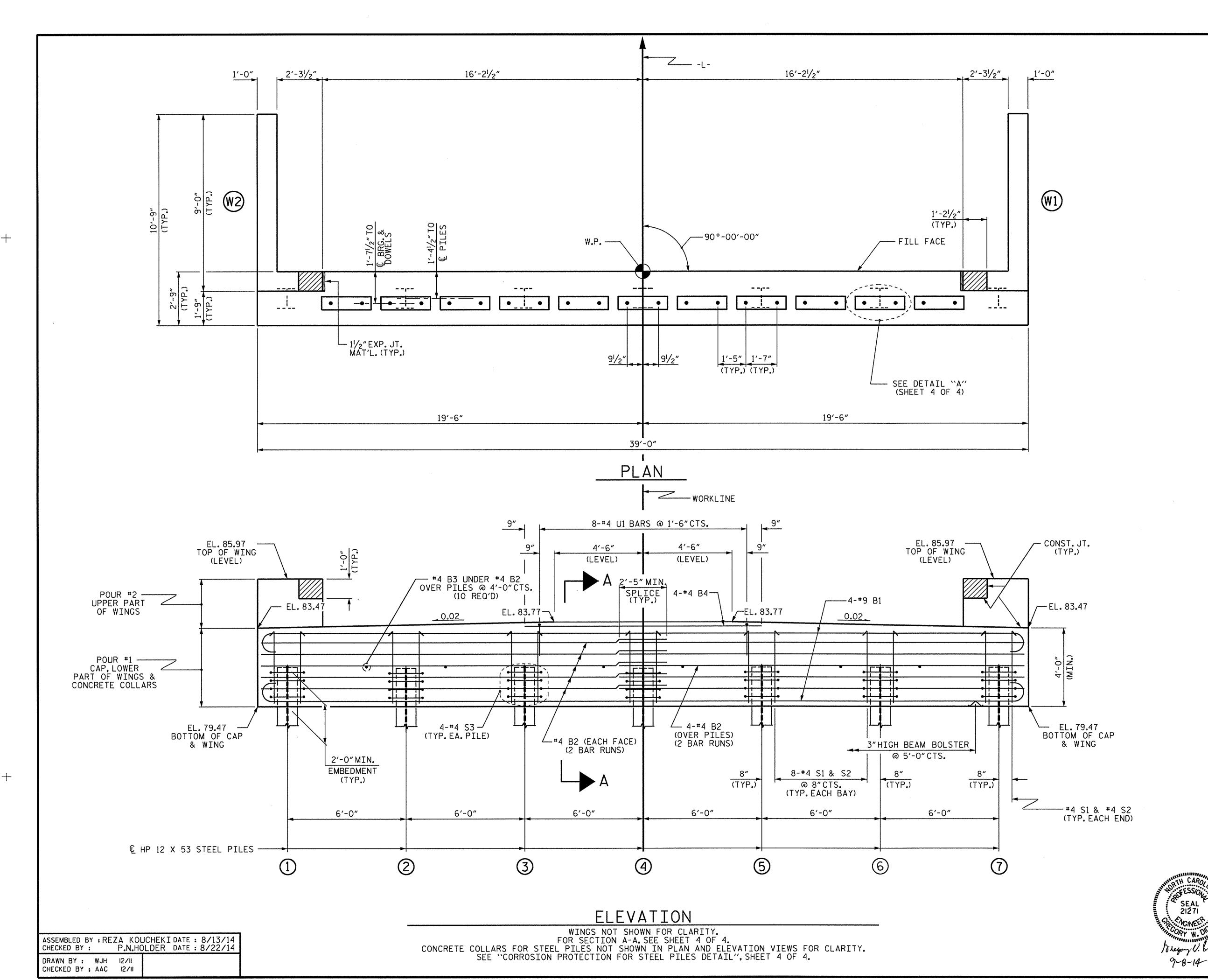
DRAWN BY: MAA 5/10 CHECKED BY: GM 5/10

REV. 10/1/11 REV. 12/5/11 REV. 6/13

MAA/GM MAA/GM MAA/GM



STD. NO. EB_33_90S4



NOTES

STIRRUPS IN CAP MAY BE SHIFTED AS NECESSARY TO CLEAR DOWELS.

THE CONCRETE IN THE SHADED AREA OF THE WING SHALL BE POURED AFTER THE VERTICAL CONCRETE BARRIER RAIL IS CAST IF SLIP FORMING IS USED.

FOR PILE SPLICE DETAILS, SEE SHEET 4 OF 4.

FOR WING DETAILS, SEE SHEET 3 OF 4.

INSTALL THE 4"DIA.DRAIN PIPE THROUGH THE WING WALL AS REQUIRED FOR REINFORCED BRIDGE APPROACH FILLS, SEE THE ROADWAY PLANS. REINFORCING STEEL IN THE WING WALL MAY BE SHIFTED AS NECESSARY TO CLEAR THE DRAIN PIPE.

PROJECT NO. 17BP.3.R.29

DUPLIN COUNTY

STATION: 13+06.50 -L-

SHEET 2 OF 4

DEPARTMENT OF TRANSPORTATION
RALEIGH

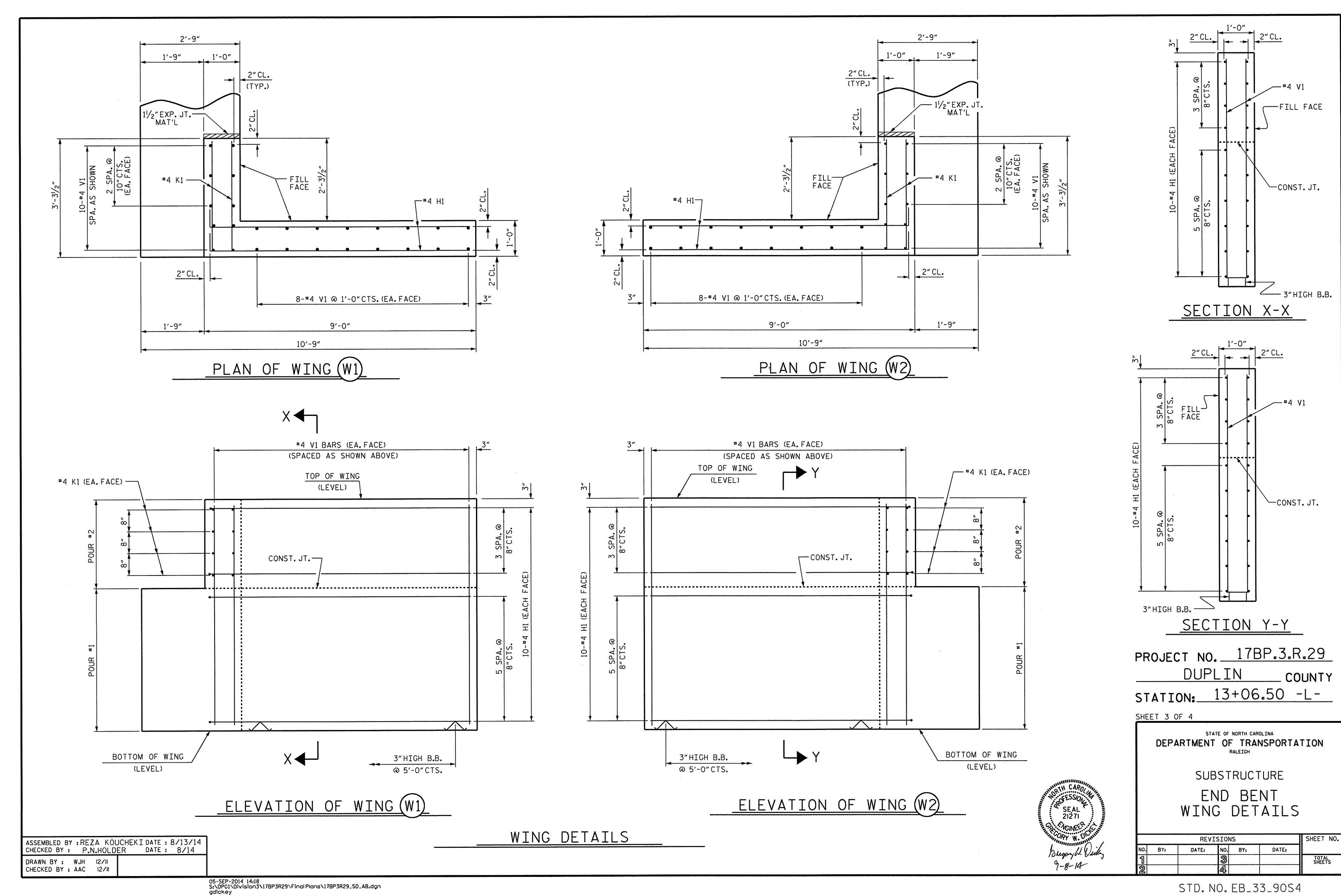
SUBSTRUCTURE

END BENT No. 2

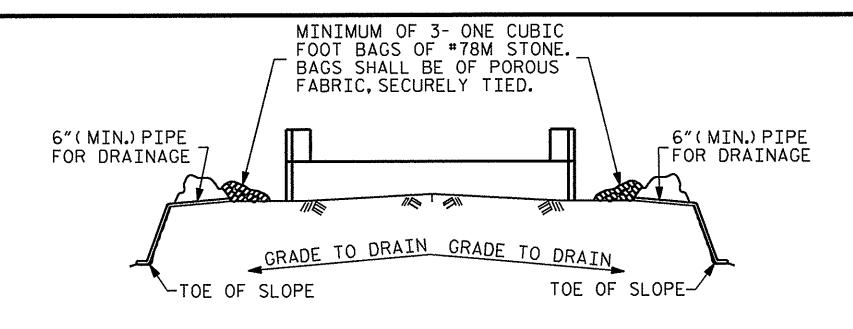
REVISIONS SHEET NO.

BY: DATE: NO. BY: DATE:

3 TOTAL SHEETS



STD. NO. EB_33_90S4

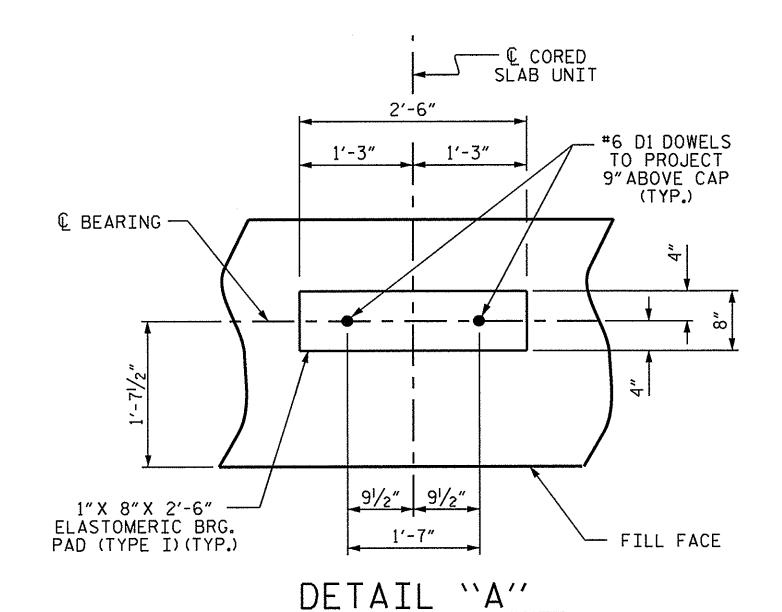


BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

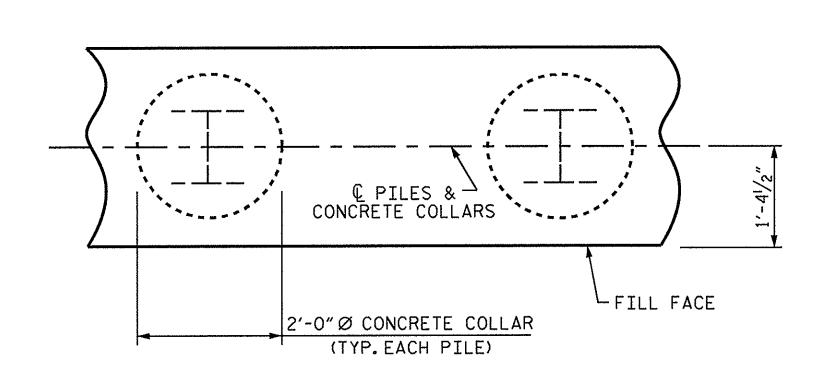
BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT



(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)



PLAN

CORROSION PROTECTION FOR STEEL PILES DETAIL

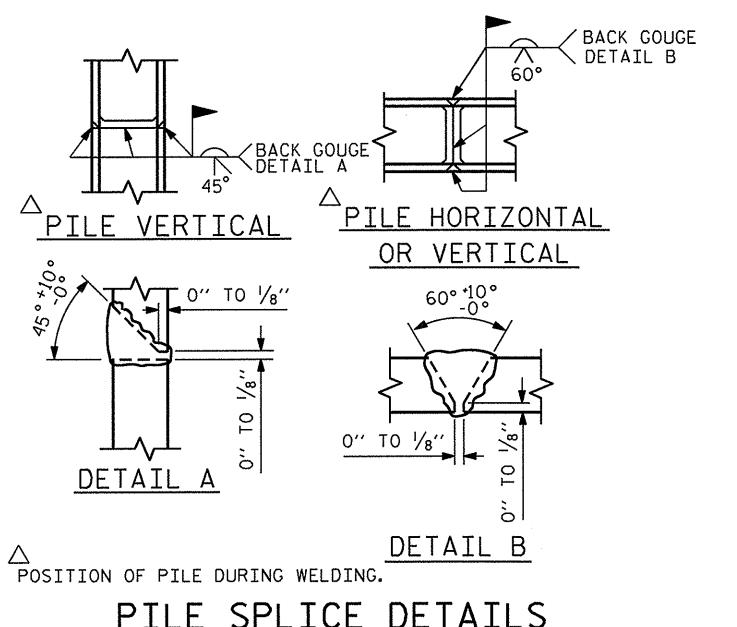
(END BENT No. 1 SHOWN, END BENT No. 2 SIMILAR BY ROTATION)

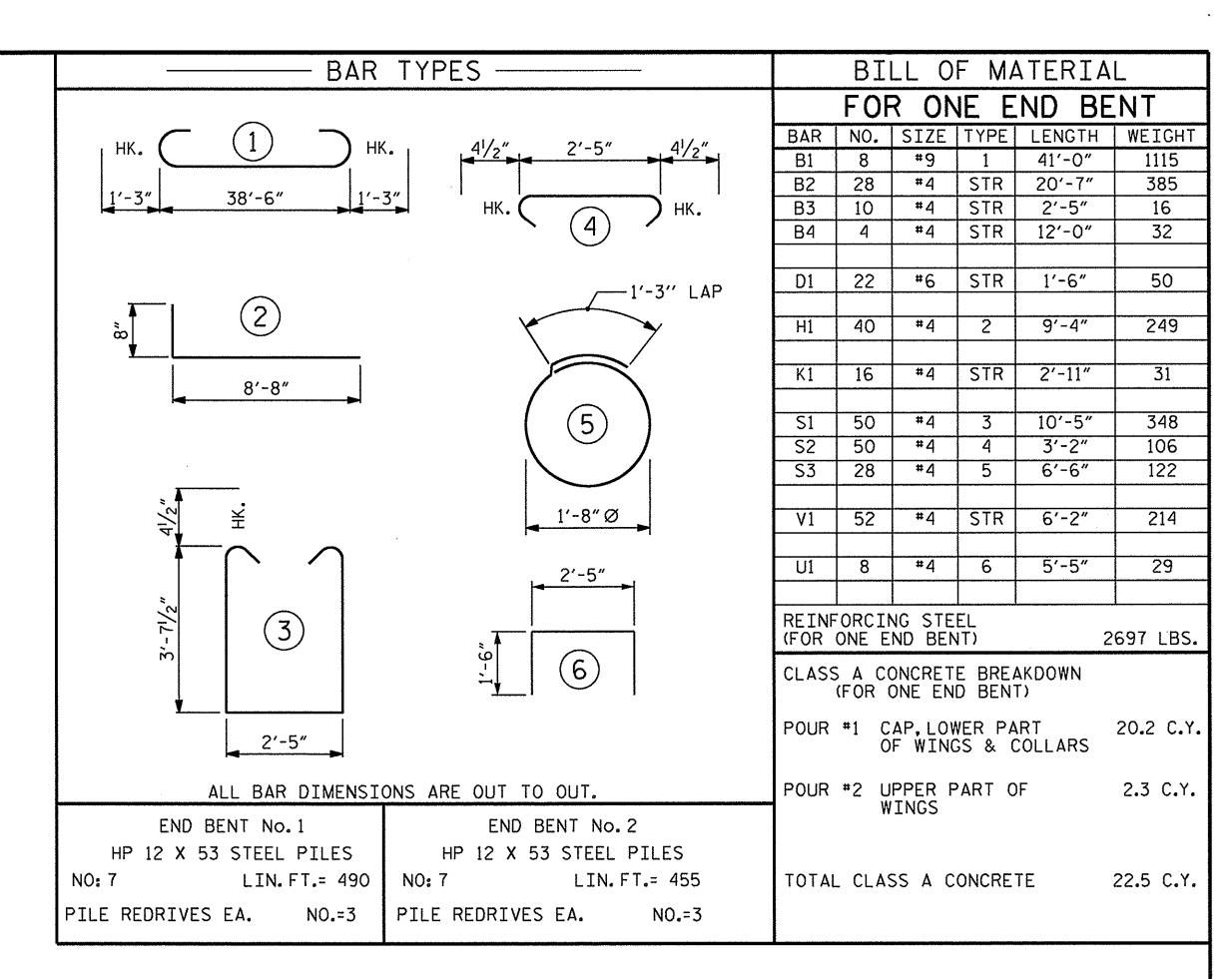
ASSEMBLED BY : REZA KOUCHEKI DATE : 8/13/14 CHECKED BY: P.N.HOLDER DATE: 8/14 DRAWN BY: WJH 12/11 CHECKED BY : AAC 12/11

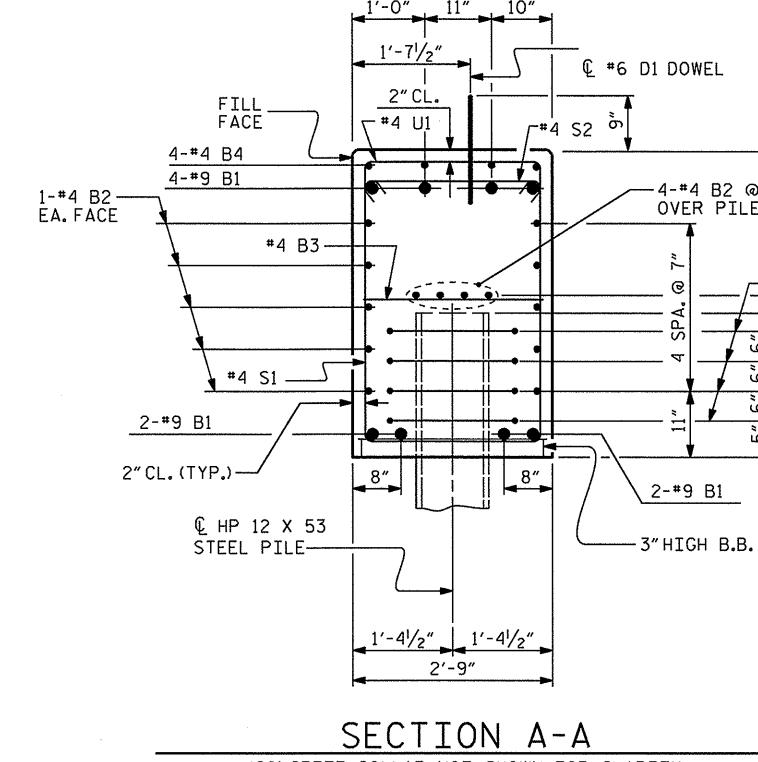
/ BACK GOUGE DETAIL B PILE HORIZONTAL OR VERTICAL VT 0" TO 1/8" 0" TO 1/8" DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

-BOTTOM OF CAP







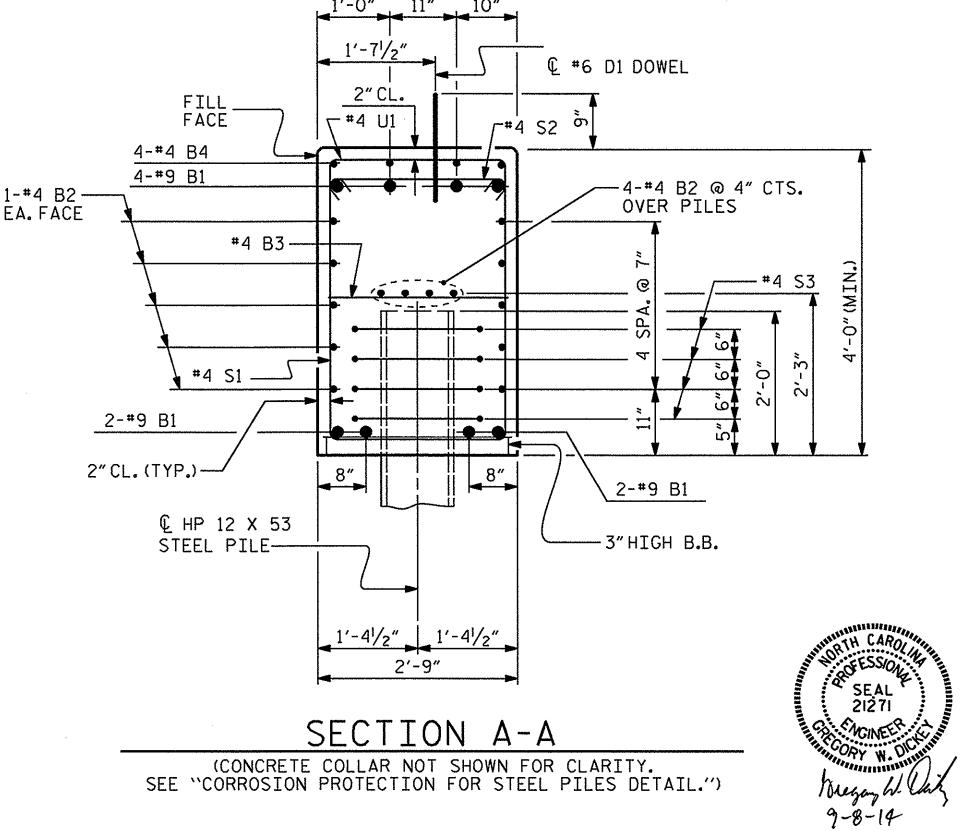
PROJECT NO. 17BP.3.R.29 DUPLIN COUNTY 13+06.50 -L-STATION:__ SHEET 4 OF 4

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SUBSTRUCTURE

END BENT No.1 & 2 DETAILS

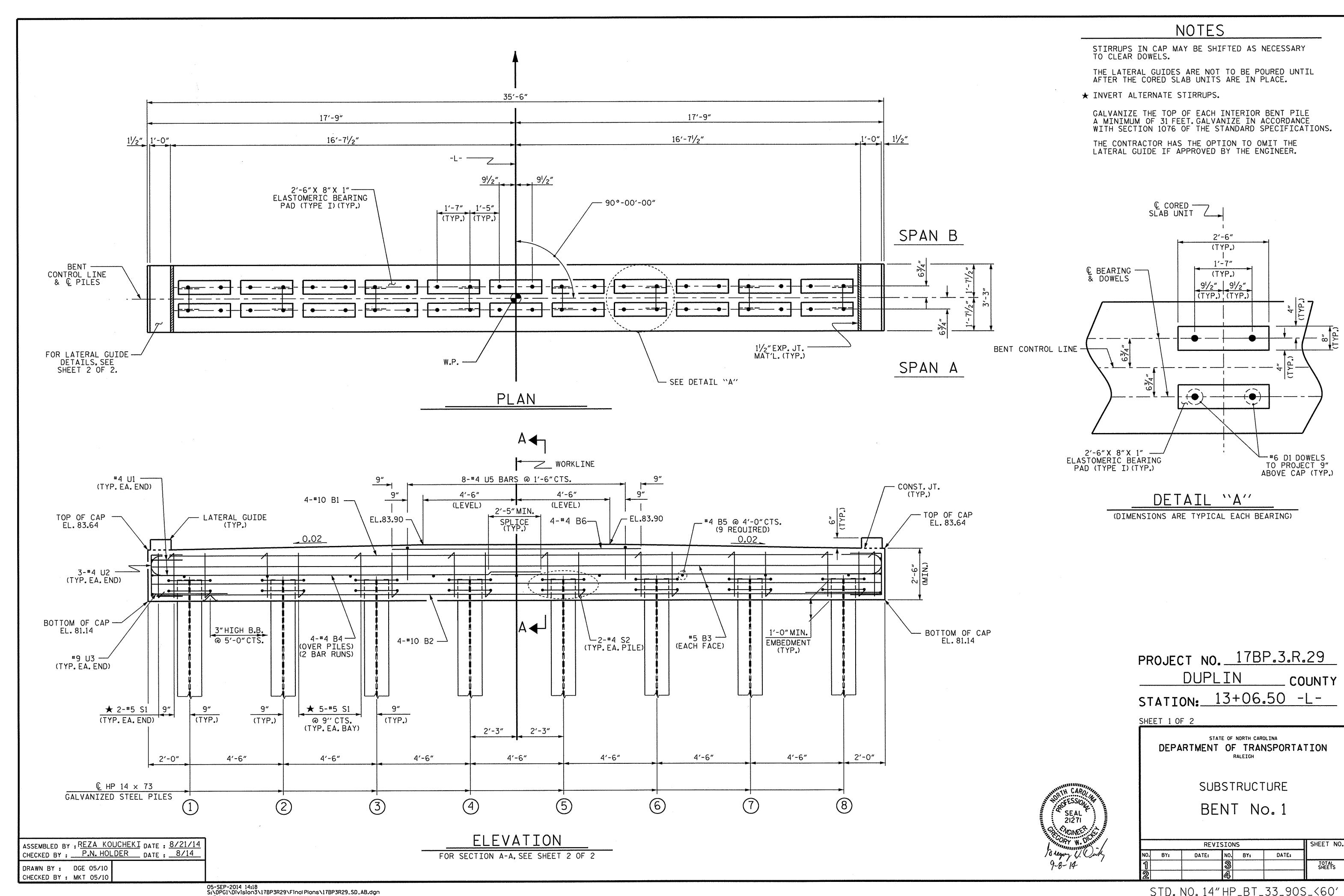
REVISIONS SHEET NO. DATE: DATE: BY: BY: TOTAL SHEETS

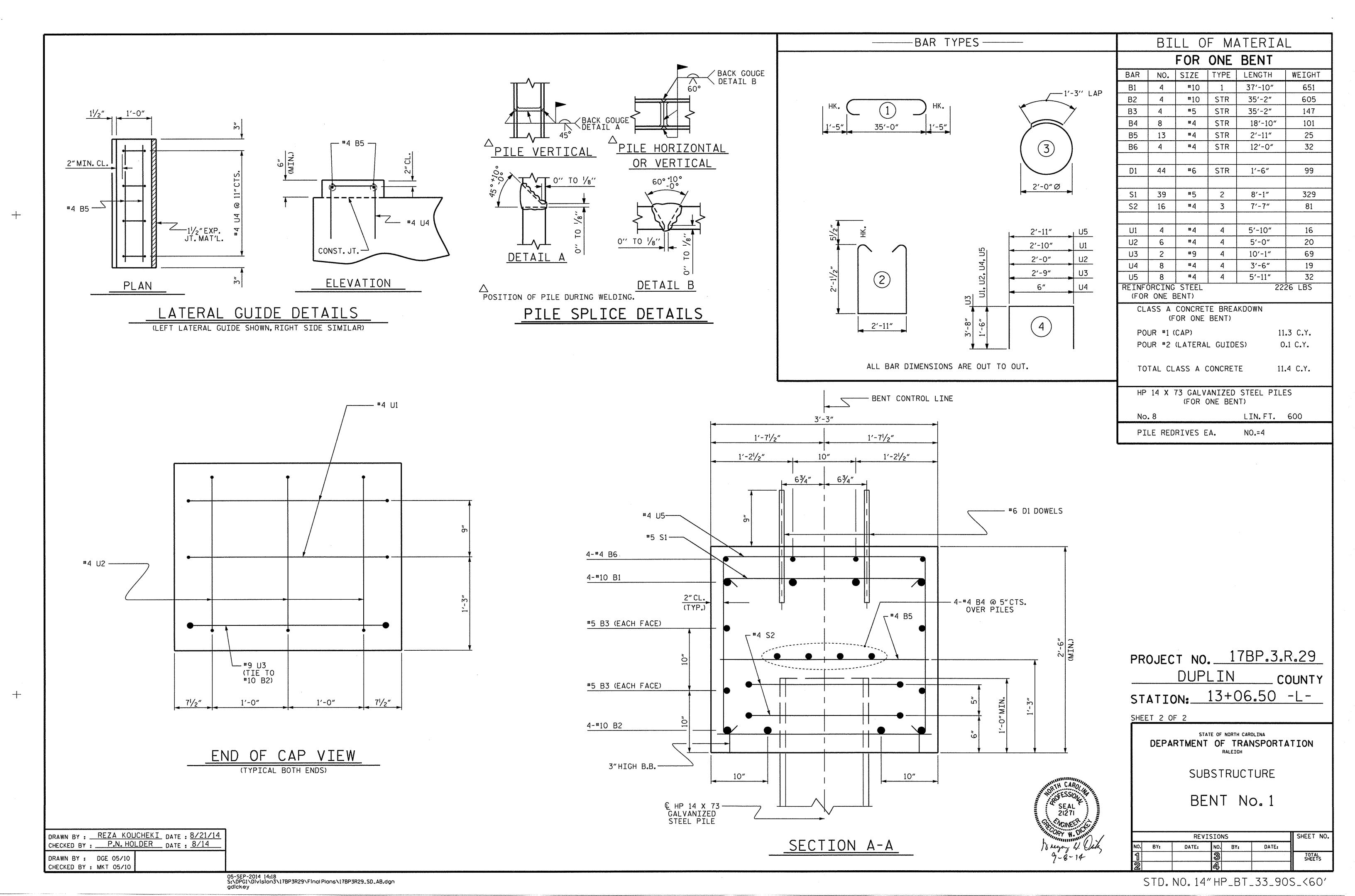


© HP 12 X 53 TEEL PILE

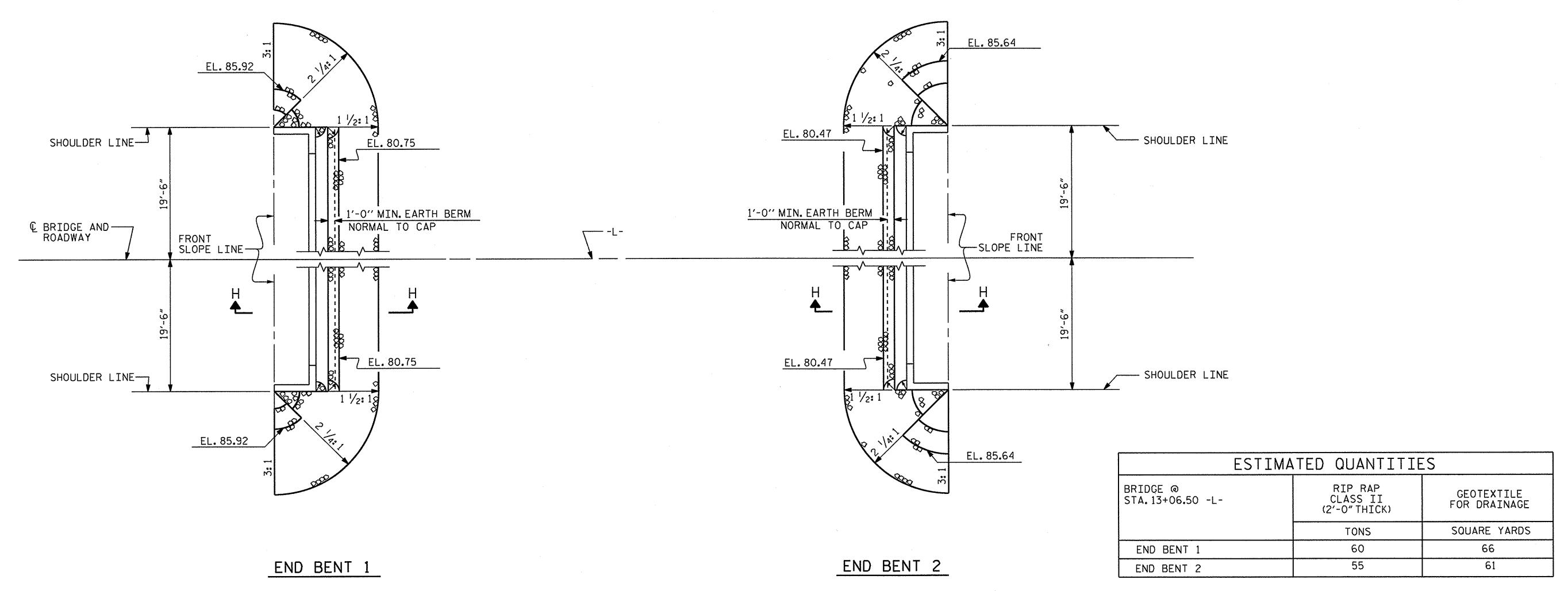
2'-0"

ELEVATION

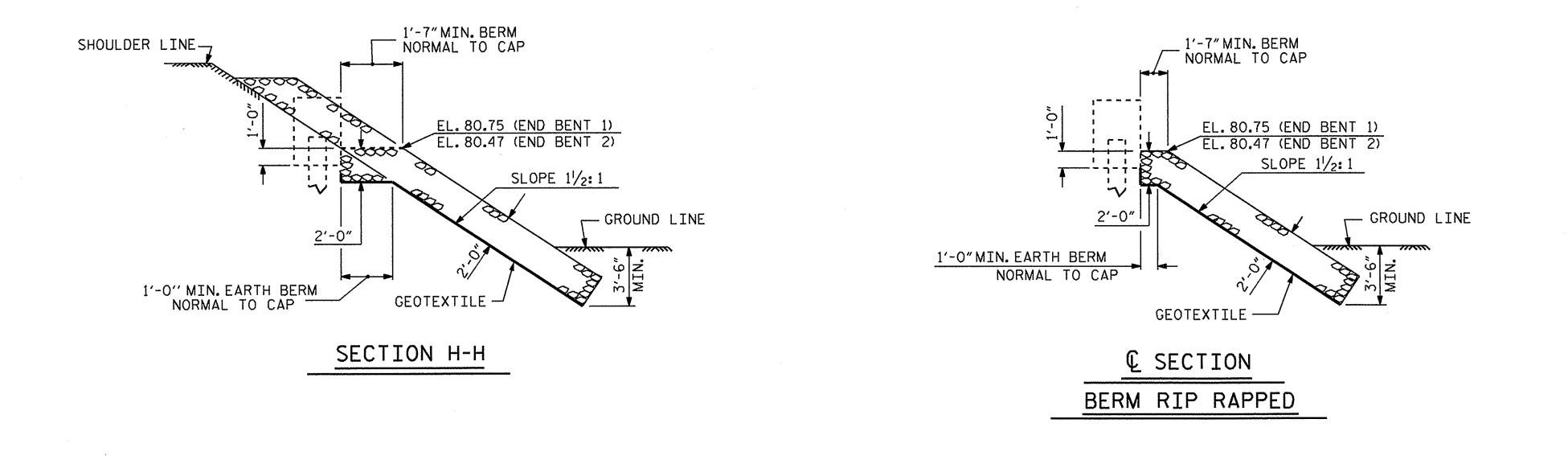








PLAN OF RIP RAP



PROJECT NO. 17BP.3.R.29

DUPLIN COUNTY

STATION: 13+06.50 -L-

STATION: 13100.30 L

DEPARTMENT OF TRANSPORTATION
RALEIGH
STANDARD

-RIP RAP DETAILS-

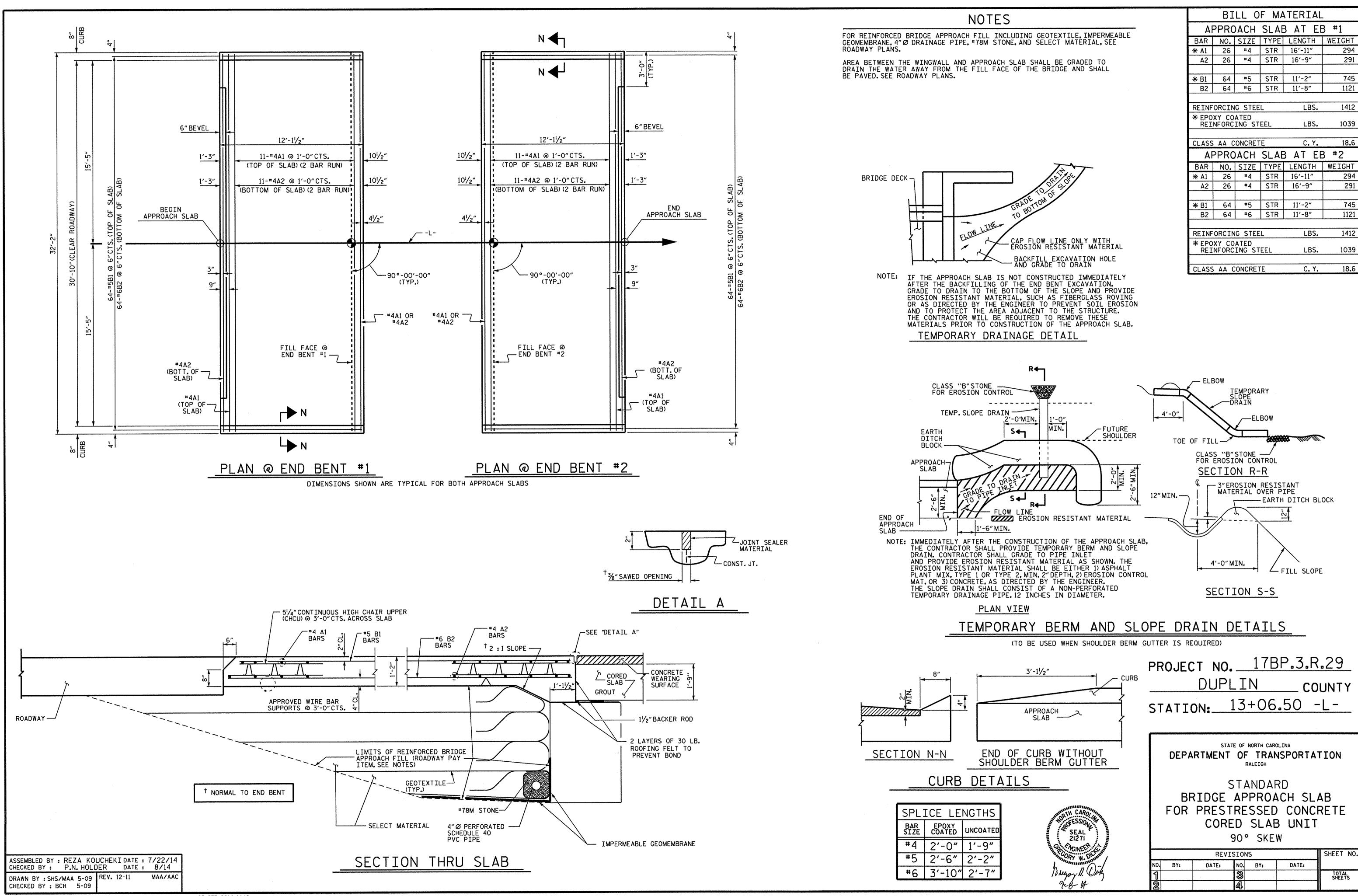
STATE OF NORTH CAROLINA

REVISIONS						SHEET NO.
NO.	BY:	DATE:	NO.	BY:	DATEs	
1			3			TOTAL SHEETS
2			4			

ASSEMBLED BY : REZA KOUCHEKI DATE : 8/21/14 CHECKED BY : P.N.HOLDER DATE : 8/14

DRAWN BY : REK I/84 REV. 5/1/06R TLA/GM REV. 10/1/II MAA/GM REV. 12/21/II MAA/GM

SEAL 21271 21271 CORY W. Dicher



STANDARD NOTES

DESIGN DATA:

---- A.A.S.H.T.O. (CURRENT) SPECIFICATIONS ---- SEE PLANS LIVE LOAD ---- SEE A.A.S.H.T.O. IMPACT ALLOWANCE STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 - 20,000 LBS. PER SO. IN. - AASHTO M270 GRADE 50W - 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 - 27.000 LBS. PER SQ. IN.

REINFORCING STEEL IN TENSION

GRADE 60 - - 24,000 LBS. PER SO. IN. ---- 1,200 LBS. PER SQ. IN. CONCRETE IN COMPRESSION CONCRETE IN SHEAR ---- SEE A.A.S.H.T.O. STRUCTURAL TIMBER - TREATED OR UNTREATED - EXTREME FIBER STRESS - - - - - 1,800 LBS. PER SQ. IN.

COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER ----

375 LBS. PER SQ. IN.

EQUIVALENT FLUID PRESSURE OF EARTH

30 LBS. PER CU. FT.

(MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS: CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS: AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4"RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND

CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE ¾"∅ STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES.ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.

ENGLISH

JANUARY, 1990