BIBBO ASSOCIATES, L.L.P.

Consulting Engineers

November 9, 2020

Town of North Castle Residential Project Review Committee 17 Bedford Road Armonk, NY 10504-1898

ATTN: Mr. Adam Kaufman, Chairman

RE: Pool Plan

Brandon Yasgur 4 Hardscrabble Circle

Dear Members of the Committee:

Please find attached the following materials in support of the Building Permit Application for the above referenced project:

- 1 copy R.P.R.C. Checklist
- 1 copy R.P.R.C. Application (w/ scan of check)
- 1 copy Pool Plan (2 Sheets)
- 1 copy Drainage Calculations
- 1 copy Aerial Map
- 1 copy Gross Land Coverage Worksheet

Our client is proposing to construct a new in-ground pool and associated mechanical equipment located at the above address.

We respectfully request this matter be placed on your next available meeting agenda for your review. Please feel free to contact us with any questions or comments you may have.

Very truly yours,

Nicholas Gaboury, P.É.

NG/aw Enclosures

cc: B. Yasgur

Drainage Calculations

Yasgur Addition 4 Hardscrabble Circle

Prepared By:



Nicholas Gaboury, P.E. N.Y.S. License #: 099349

Rev.

Date: 11/4/2020

Enclosed herewith are stormwater calculations for the proposed site improvements at 4 Hardscrabble Circle, located in the Town of North Castle.

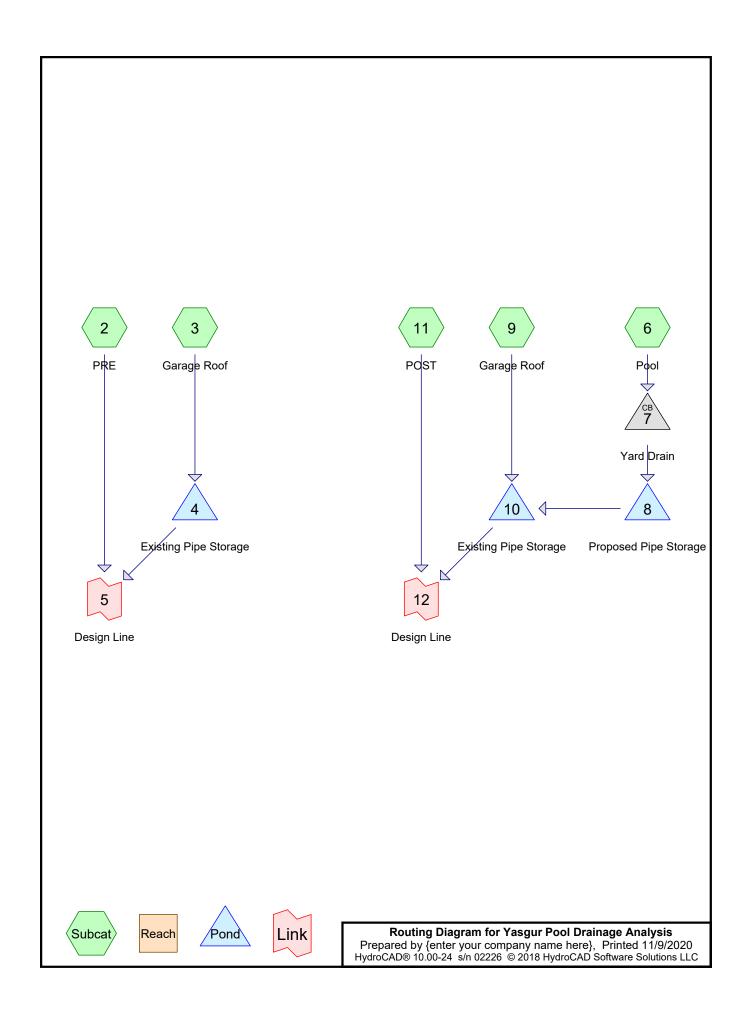
Currently at the site there is an existing single family residence serviced by a public water supply and an onsite sewage disposal system. A ridgeline is located on the western side of the site and the site slopes towards the east. The property is located in the Mianus River Watershed Basin and the total area of disturbance is 0.43 Acres. Soils identified within the area of disturbance consist of Charlton-Chatfield, Chatfield-Charlton, and Paxton Loam. Charlton-Chatfield and Chatfield-Charlton belong to hydrologic group B. Paxton Loam belongs to hydrologic group C. A soil map has been provided herewith.

The applicant is proposing to construct a new in-ground pool with minimal associated patio edging. The project will result in an addition of 809 sqft. of impervious coverage on the site. In order to mitigate the additional impervious surfaces, a stormwater management system has been designed for the site. Runoff will be collected from the new impervious areas with a proposed trench drain. The stormwater enters a yard drain which then conveys it to an underground pipe detention system. The underground pipe system consists of one (1) row of 24" HDPE pipe on a bed of gravel. The row of pipe shall be 52 feet in length and shall be set level. The length of pipe shall connect to the existing pipe detention system which has an outlet structure to control the release of stormwater to the existing rock outlet protection pad located behind the existing garage. All proposed stormwater conveyance piping shall be high density polyethylene pipe (HDPE) unless otherwise noted.

The proposed pipe detention system has been sized to attenuate the post-development peak discharge from the 25-year storm to predevelopment rates, as required by the North Castle Town Code. In order to analyze the impact of the proposed construction, a stormwater model of the pool area was developed for both predevelopment and post-development conditions. A design line was selected on the neighboring property to the east where runoff from the Yasgur site collects at the existing low point. Please see the Watershed Map attached with this report.

HydroCAD v. 10.0, a computer-modeling program based upon TR-20, was used to generate peak flows from the subcatchments. In the program, the user inputs various characteristics for each subcatchment including a curve number and time of concentration. These two parameters relate runoff to the specific land characteristics of the subcatchment. Based upon the inputted data, peak flows are generated for the 25-year storm events for the pre-development and post-development subcatchments. HydroCAD output reports are located at the end of this report. The HydroCAD reports demonstrate that the stormwater management design will result in reduced peak flows from the project site under the post-construction conditions.

HydroCAD Output Reports



Yasgur Pool Drainage Analysis

Type III 24-hr 25 year storm Rainfall=6.00" Printed 11/9/2020

Prepared by {enter your company name here}

HydroCAD® 10.00-24 s/n 02226 © 2018 HydroCAD Software Solutions LLC

Time span=0.00-360.00 hrs, dt=0.01 hrs, 36001 points
Runoff by SCS TR-20 method, UH=SCS, Weighted-CN
Reach routing by Dyn-Stor-Ind method - Pond routing by Dyn-Stor-Ind method

Subcatchment2: PRE Runoff Area=110,111 sf 15.46% Impervious Runoff Depth=2.90"

Flow Length=378' Tc=18.5 min CN=71 Runoff=5.94 cfs 26,599 cf

Subcatchment 3: Garage Roof Runoff Area=658 sf 100.00% Impervious Runoff Depth=5.76"

Tc=6.0 min CN=98 Runoff=0.09 cfs 316 cf

Pond 4: Existing Pipe Storage Peak Elev=643.95' Storage=73 cf Inflow=0.09 cfs 316 cf

Outflow=0.03 cfs 316 cf

Link 5: Design Line Inflow=5.97 cfs 26,915 cf

Primary=5.97 cfs 26,915 cf

Subcatchment 6: Pool Runoff Area=2,718 sf 29.76% Impervious Runoff Depth=3.88"

Tc=0.0 min CN=81 Runoff=0.35 cfs 880 cf

Pond 7: Yard Drain Peak Elev=648.89' Inflow=0.35 cfs 880 cf

6.0" Round Culvert n=0.013 L=16.8' S=0.3274 '/' Outflow=0.35 cfs 880 cf

Pond 8: Proposed Pipe Storage Peak Elev=644.92' Storage=161 cf Inflow=0.35 cfs 880 cf

6.0" Round Culvert n=0.013 L=10.6' S=0.0000 '/' Outflow=0.20 cfs 880 cf

Subcatchment9: Garage Roof Runoff Area=658 sf 100.00% Impervious Runoff Depth=5.76"

Tc=6.0 min CN=98 Runoff=0.09 cfs 316 cf

Pond 10: Existing Pipe Storage Peak Elev=644.88' Storage=153 cf Inflow=0.29 cfs 1,196 cf

Outflow=0.28 cfs 1,196 cf

Subcatchment 11: POST Runoff Area=107,393 sf 15.85% Impervious Runoff Depth=2.90"

Flow Length=378' Tc=18.5 min CN=71 Runoff=5.80 cfs 25,942 cf

Link 12: Design Line Inflow=5.94 cfs 27,138 cf

Primary=5.94 cfs 27,138 cf

Total Runoff Area = 221,538 sf Runoff Volume = 54,052 cf Average Runoff Depth = 2.93" 83.67% Pervious = 185,371 sf 16.33% Impervious = 36,167 sf

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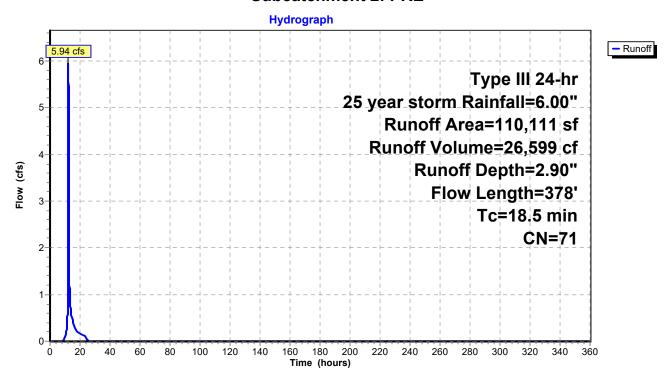
Summary for Subcatchment 2: PRE

Runoff = 5.94 cfs @ 12.26 hrs, Volume= 26,599 cf, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs Type III 24-hr 25 year storm Rainfall=6.00"

| A | rea (sf) | CN E | escription | | |
|-------|----------|---------|------------|-------------|--|
| | 17,021 | 98 F | aved park | ing & roofs | |
| | 36,607 | 74 > | 75% Gras | s cover, Go | ood, HSG C |
| | 16,066 | 61 > | 75% Gras | s cover, Go | ood, HSG B |
| | 18,967 | 70 V | Voods, Go | od, HSG C | |
| | 21,450 | 55 V | Voods, Go | od, HSG B | |
| 1 | 10,111 | 71 V | Veighted A | verage | |
| | 93,090 | 8 | 4.54% Per | vious Area | |
| | 17,021 | 1 | 5.46% Imp | ervious Ar | ea |
| | | | | | |
| Tc | Length | Slope | Velocity | Capacity | Description |
| (min) | (feet) | (ft/ft) | (ft/sec) | (cfs) | |
| 17.5 | 100 | 0.0300 | 0.10 | | Sheet Flow, |
| | | | | | Woods: Light underbrush n= 0.400 P2= 3.50" |
| 1.0 | 278 | 0.0755 | 4.42 | | Shallow Concentrated Flow, |
| | | | | | Unpaved Kv= 16.1 fps |
| 18.5 | 378 | Total | | | |

Subcatchment 2: PRE



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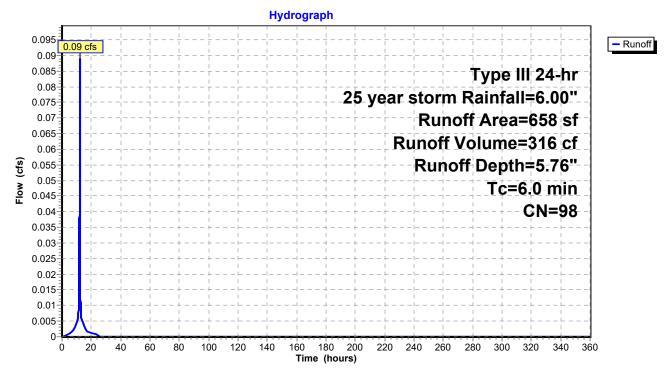
Summary for Subcatchment 3: Garage Roof

Runoff = 0.09 cfs @ 12.08 hrs, Volume= 316 cf, Depth= 5.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs Type III 24-hr 25 year storm Rainfall=6.00"

| _ | Α | rea (sf) | CN I | Description | | |
|---|-------|----------|---------|--------------|--------------|---------------|
| | | 658 | 98 | Jnconnecte 4 | ed roofs, HS | SG C |
| | | 658 | | 100.00% Im | pervious A | rea |
| | | 658 | | 100.00% Uı | nconnected | i |
| | т. | 1 41- | Ol | \ | 0 | Description |
| | | 9 | Slope | , | Capacity | Description |
| | (min) | (feet) | (ft/ft) | (ft/sec) | (cfs) | |
| | 6.0 | • | | | • | Direct Entry. |

Subcatchment 3: Garage Roof



Yasgur Pool Drainage Analysis

Type III 24-hr 25 year storm Rainfall=6.00" Printed 11/9/2020

Prepared by {enter your company name here}

HydroCAD® 10.00-24 s/n 02226 © 2018 HydroCAD Software Solutions LLC

Summary for Pond 4: Existing Pipe Storage

Inflow Area = 658 sf,100.00% Impervious, Inflow Depth = 5.76" for 25 year storm event

Inflow = 0.09 cfs @ 12.08 hrs, Volume= 316 cf

Outflow = 0.03 cfs @ 12.42 hrs, Volume= 316 cf, Atten= 72%, Lag= 20.0 min

Primary = 0.03 cfs @ 12.42 hrs, Volume= 316 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs

Peak Elev= 643.95' @ 12.42 hrs Surf.Area= 100 sf Storage= 73 cf

Flood Elev= 647.00' Surf.Area= 0 sf Storage= 157 cf

Plug-Flow detention time= 21.7 min calculated for 316 cf (100% of inflow)

Center-of-Mass det. time= 21.7 min (766.8 - 745.1)

| Volume | Invert | Avail.Stora | age Storage Description |
|--------|----------|-------------|---|
| #1 | 643.00' | 15 | 7 cf 24.0" Round Pipe Storage L= 50.0' |
| Device | Routing | Invert | Outlet Devices |
| #1 | Primary | | 6.0" Round Culvert L= 24.0' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 643.00' / 642.00' S= 0.0417 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| #2 | Device 1 | | 4.0' long x 0.5' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 Coef. (English) 2.80 2.92 3.08 3.30 3.32 |
| #3 | Device 1 | 643.00' | 1.0" Vert. Orifice/Grate C= 0.600 |

Primary OutFlow Max=0.03 cfs @ 12.42 hrs HW=643.95' TW=0.00' (Dynamic Tailwater)

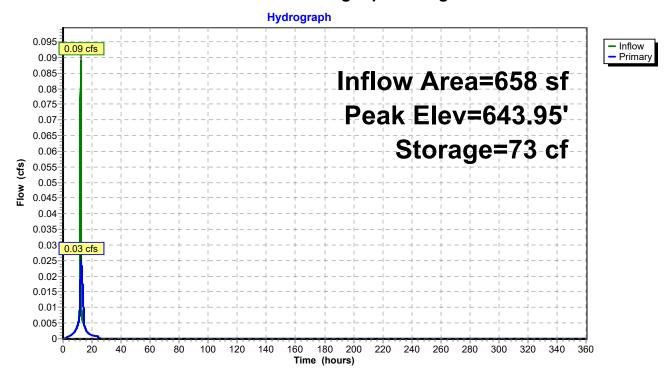
-1=Culvert (Passes 0.03 cfs of 0.79 cfs potential flow)

2=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

-3=Orifice/Grate (Orifice Controls 0.03 cfs @ 4.58 fps)

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Pond 4: Existing Pipe Storage



Yasgur Pool Drainage Analysis

Type III 24-hr 25 year storm Rainfall=6.00" Printed 11/9/2020

Prepared by {enter your company name here}

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Summary for Link 5: Design Line

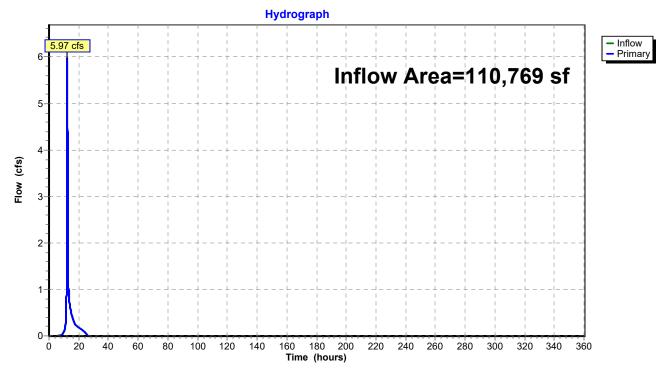
Inflow Area = 110,769 sf, 15.96% Impervious, Inflow Depth = 2.92" for 25 year storm event

Inflow = 5.97 cfs @ 12.27 hrs, Volume= 26,915 cf

Primary = 5.97 cfs @ 12.27 hrs, Volume= 26,915 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs

Link 5: Design Line



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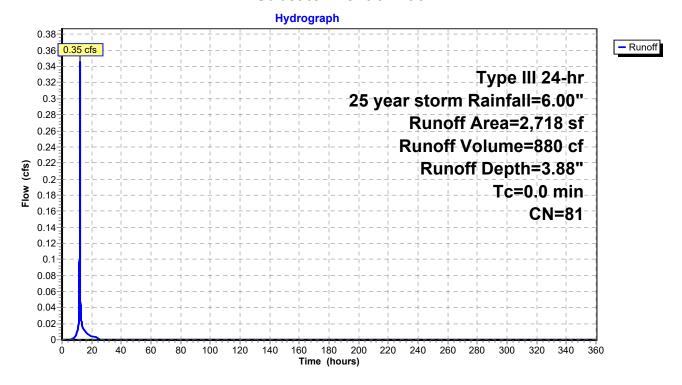
Summary for Subcatchment 6: Pool

Runoff = 0.35 cfs @ 12.00 hrs, Volume= 880 cf, Depth= 3.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs Type III 24-hr 25 year storm Rainfall=6.00"

| | Area (sf) | CN | Description | | | |
|---|-----------|----|-------------------------------|--|--|--|
| * | 809 | 98 | Pool & Equip Pad | | | |
| | 1,909 | 74 | >75% Grass cover, Good, HSG C | | | |
| | 2,718 | 81 | Weighted Average | | | |
| | 1,909 | | 70.24% Pervious Area | | | |
| | 809 | | 29.76% Impervious Area | | | |

Subcatchment 6: Pool



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Summary for Pond 7: Yard Drain

Inflow Area = 2,718 sf, 29.76% Impervious, Inflow Depth = 3.88" for 25 year storm event

Inflow = 0.35 cfs @ 12.00 hrs, Volume= 880 cf

Outflow = 0.35 cfs @ 12.00 hrs, Volume= 880 cf, Atten= 0%, Lag= 0.0 min

Primary = 0.35 cfs @ 12.00 hrs, Volume= 880 cf

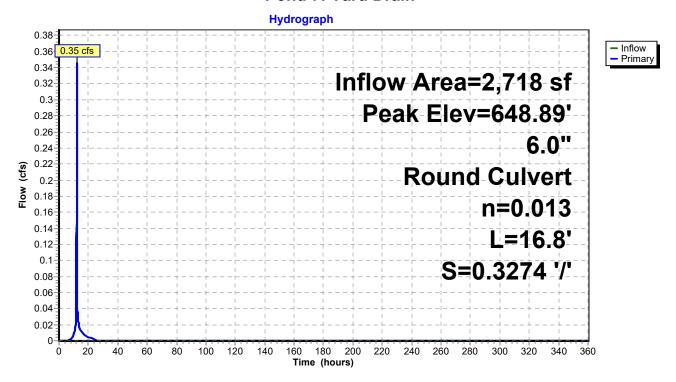
Routing by Dyn-Stor-Ind method, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs

Peak Elev= 648.89' @ 12.00 hrs

| Device | Routing | Invert | Outlet Devices |
|--------|---------|---------|--|
| #1 | Primary | 648.50' | 6.0" Round Culvert |
| | | | L= 16.8' CPP, square edge headwall, Ke= 0.500 |
| | | | Inlet / Outlet Invert= 648.50' / 643.00' S= 0.3274 '/' Cc= 0.900 |
| | | | n= 0.013 Corrugated PE_smooth interior_Flow Area= 0.20 sf |

Primary OutFlow Max=0.34 cfs @ 12.00 hrs HW=648.89' TW=644.43' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.34 cfs @ 2.12 fps)

Pond 7: Yard Drain



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Summary for Pond 8: Proposed Pipe Storage

Inflow Area = 2,718 sf, 29.76% Impervious, Inflow Depth = 3.88" for 25 year storm event

Inflow 0.35 cfs @ 12.00 hrs. Volume= 880 cf

0.20 cfs @ Outflow 12.07 hrs, Volume= 880 cf, Atten= 42%, Lag= 4.3 min

0.20 cfs @ 12.07 hrs, Volume= Primary 880 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs

Peak Elev= 644.92' @ 12.08 hrs Surf.Area= 42 sf Storage= 161 cf

Flood Elev= 647.00' Surf.Area= 0 sf Storage= 163 cf

Plug-Flow detention time= 52.6 min calculated for 880 cf (100% of inflow)

Center-of-Mass det. time= 53.1 min (859.1 - 806.0)

Volume Avail.Storage Storage Description Invert #1 24.0" Round Pipe Storage 643.00' 163 cf L = 52.0'

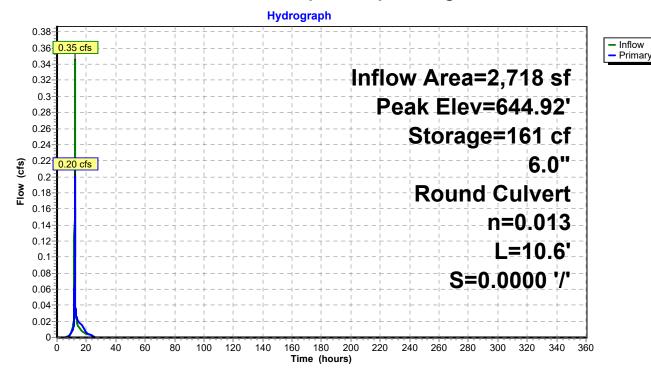
Device Routing Invert **Outlet Devices**

#1 **Primary** 643.00' 6.0" Round Culvert

> L= 10.6' CPP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 643.00' / 643.00' S= 0.0000 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf

Primary OutFlow Max=0.17 cfs @ 12.07 hrs HW=644.91' TW=644.88' (Dynamic Tailwater) -1=Culvert (Inlet Controls 0.17 cfs @ 0.88 fps)

Pond 8: Proposed Pipe Storage



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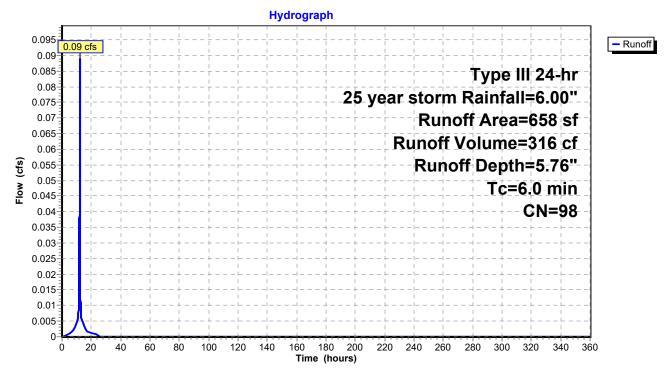
Summary for Subcatchment 9: Garage Roof

Runoff = 0.09 cfs @ 12.08 hrs, Volume= 316 cf, Depth= 5.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs Type III 24-hr 25 year storm Rainfall=6.00"

| A | rea (sf) | CN E | Description | | |
|-------|----------|---------|--------------------------|------------|---------------|
| | 658 | 98 L | Unconnected roofs, HSG C | | |
| | 658 | 1 | 00.00% Im | pervious A | rea |
| | 658 | 1 | 00.00% Ur | nconnected | l |
| _ | | 01 | | | D |
| Tc | Length | Slope | Velocity | Capacity | Description |
| (min) | (feet) | (ft/ft) | (ft/sec) | (cfs) | |
| 6.0 | | | | | Direct Entry, |

Subcatchment 9: Garage Roof



Yasgur Pool Drainage Analysis

Type III 24-hr 25 year storm Rainfall=6.00" Printed 11/9/2020

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Summary for Pond 10: Existing Pipe Storage

Inflow Area = 3,376 sf, 43.45% Impervious, Inflow Depth = 4.25" for 25 year storm event

Inflow = 0.29 cfs @ 12.07 hrs, Volume= 1,196 cf

Outflow = 0.28 cfs @ 12.08 hrs, Volume= 1,196 cf, Atten= 2%, Lag= 0.3 min

Primary = 0.28 cfs @ 12.08 hrs, Volume= 1,196 cf

Routing by Dyn-Stor-Ind method, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs

Peak Elev= 644.88' @ 12.08 hrs Surf.Area= 48 sf Storage= 153 cf

Flood Elev= 647.00' Surf.Area= 0 sf Storage= 157 cf

Plug-Flow detention time= (not calculated: outflow precedes inflow)

Center-of-Mass det. time= 37.8 min (866.8 - 829.0)

| Volume | Invert | Avail.Sto | rage Storage Description |
|--------|----------|-----------|--|
| #1 | 643.00' | 15 | 57 cf 24.0" Round Pipe Storage L= 50.0' |
| Device | Routing | Invert | Outlet Devices |
| #1 | Primary | 643.00' | 6.0" Round Culvert |
| | | | L= 24.0' CPP, square edge headwall, Ke= 0.500 |
| | | | Inlet / Outlet Invert= 643.00' / 642.00' S= 0.0417 '/' Cc= 0.900 |
| | | | n= 0.013 Corrugated PE, smooth interior, Flow Area= 0.20 sf |
| #2 | Device 1 | 644.80' | 4.0' long x 0.5' breadth Broad-Crested Rectangular Weir |
| | | | Head (feet) 0.20 0.40 0.60 0.80 1.00 |
| | | | Coef. (English) 2.80 2.92 3.08 3.30 3.32 |
| #3 | Device 1 | 643.00' | 1.0" Vert. Orifice/Grate C= 0.600 |

Primary OutFlow Max=0.28 cfs @ 12.08 hrs HW=644.88' TW=0.00' (Dynamic Tailwater)

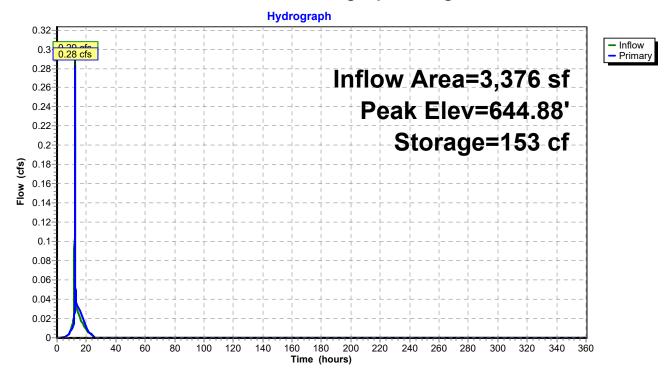
-1=Culvert (Passes 0.28 cfs of 1.21 cfs potential flow)

2=Broad-Crested Rectangular Weir (Weir Controls 0.24 cfs @ 0.78 fps)

-3=Orifice/Grate (Orifice Controls 0.04 cfs @ 6.53 fps)

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Pond 10: Existing Pipe Storage



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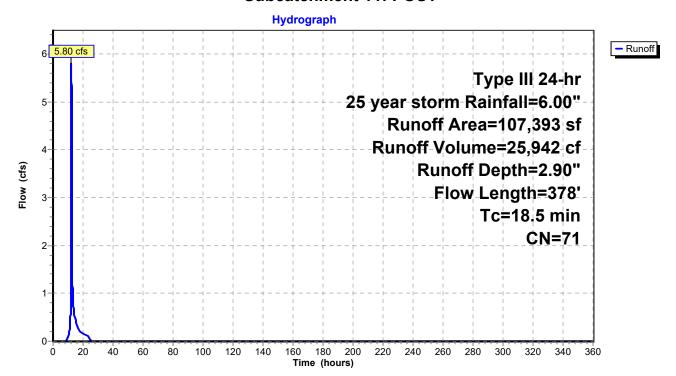
Summary for Subcatchment 11: POST

Runoff = 5.80 cfs @ 12.26 hrs, Volume= 25,942 cf, Depth= 2.90"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs Type III 24-hr 25 year storm Rainfall=6.00"

| _ | Α | rea (sf) | CN [| Description | | |
|---|-------|----------|---------|-------------|-------------|--|
| | | 17,021 | 98 F | Paved park | ing & roofs | |
| | | 33,889 | 74 > | ≻75% Ġras | s cover, Go | ood, HSG C |
| | | 16,066 | 61 > | 75% Gras | s cover, Go | ood, HSG B |
| | | 18,967 | 70 \ | Voods, Go | od, HSG C | |
| | | 21,450 | 55 \ | Voods, Go | od, HSG B | |
| | 1 | 07,393 | 71 \ | Veighted A | verage | |
| | | 90,372 | | | vious Area | |
| | | 17,021 | • | 15.85% lmp | pervious Ar | ea |
| | | | | • | | |
| | Tc | Length | Slope | Velocity | Capacity | Description |
| | (min) | (feet) | (ft/ft) | (ft/sec) | (cfs) | · |
| | 17.5 | 100 | 0.0300 | 0.10 | | Sheet Flow, |
| | | | | | | Woods: Light underbrush n= 0.400 P2= 3.50" |
| | 1.0 | 278 | 0.0755 | 4.42 | | Shallow Concentrated Flow, |
| | | | | | | Unpaved Kv= 16.1 fps |
| _ | 18.5 | 378 | Total | | | · |

Subcatchment 11: POST



Yasgur Pool Drainage Analysis

Type III 24-hr 25 year storm Rainfall=6.00" Printed 11/9/2020

Prepared by {enter your company name here}

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Summary for Link 12: Design Line

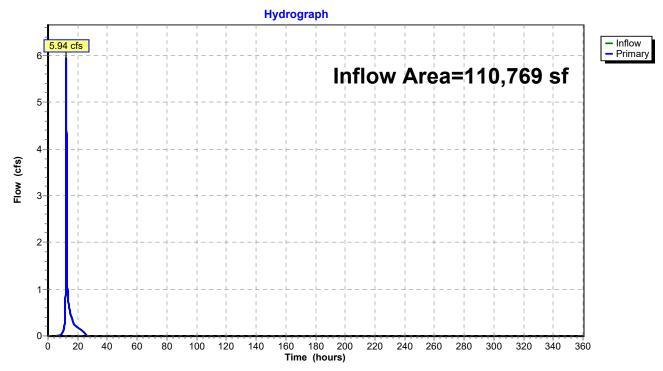
Inflow Area = 110,769 sf, 16.69% Impervious, Inflow Depth = 2.94" for 25 year storm event

Inflow = 5.94 cfs @ 12.26 hrs, Volume= 27,138 cf

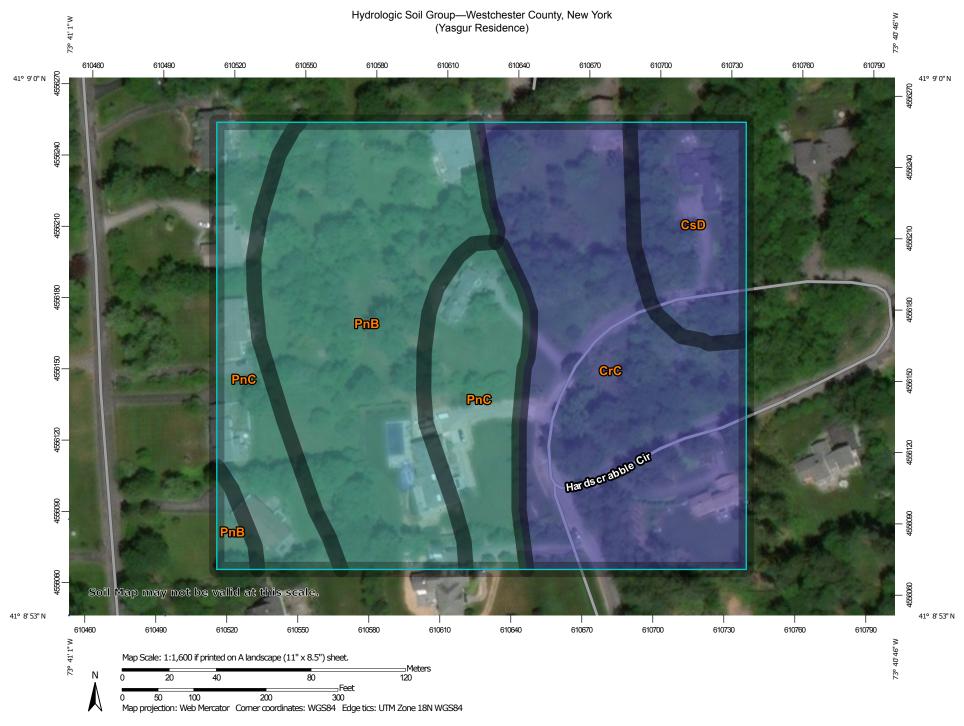
Primary = 5.94 cfs @ 12.26 hrs, Volume= 27,138 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-360.00 hrs, dt= 0.01 hrs

Link 12: Design Line



Soil Map



MAP LEGEND MAP INFORMATION The soil surveys that comprise your AOI were mapped at Area of Interest (AOI) С 1:12.000. Area of Interest (AOI) C/D Soils Warning: Soil Map may not be valid at this scale. D **Soil Rating Polygons** Enlargement of maps beyond the scale of mapping can cause Not rated or not available Α misunderstanding of the detail of mapping and accuracy of soil **Water Features** line placement. The maps do not show the small areas of A/D Streams and Canals contrasting soils that could have been shown at a more detailed Transportation B/D Rails ---Please rely on the bar scale on each map sheet for map measurements. Interstate Highways C/D Source of Map: Natural Resources Conservation Service **US Routes** Web Soil Survey URL: D Major Roads Coordinate System: Web Mercator (EPSG:3857) Not rated or not available -Local Roads Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts Soil Rating Lines Background distance and area. A projection that preserves area, such as the Aerial Photography Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required. This product is generated from the USDA-NRCS certified data as of the version date(s) listed below. Soil Survey Area: Westchester County, New York Survey Area Data: Version 13, Oct 8, 2017 Soil map units are labeled (as space allows) for map scales 1:50,000 or larger. D Not rated or not available Date(s) aerial images were photographed: Dec 31, 2009—Oct 5, 2016 **Soil Rating Points** The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background A/D imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident. B/D

Hydrologic Soil Group

| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
|--------------------------|--|--------|--------------|----------------|
| CrC | Charlton-Chatfield complex, 0 to 15 percent slopes, very rocky | В | 3.6 | 34.5% |
| CsD | Chatfield-Charlton complex, 15 to 35 percent slopes, very rocky | В | 1.0 | 9.8% |
| PnB | Paxton fine sandy loam, 3 to 8 percent slopes | С | 3.5 | 33.5% |
| PnC | Paxton fine sandy loam, 8 to 15 percent slopes | С | 2.3 | 22.3% |
| Totals for Area of Inter | rest | | 10.5 | 100.0% |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

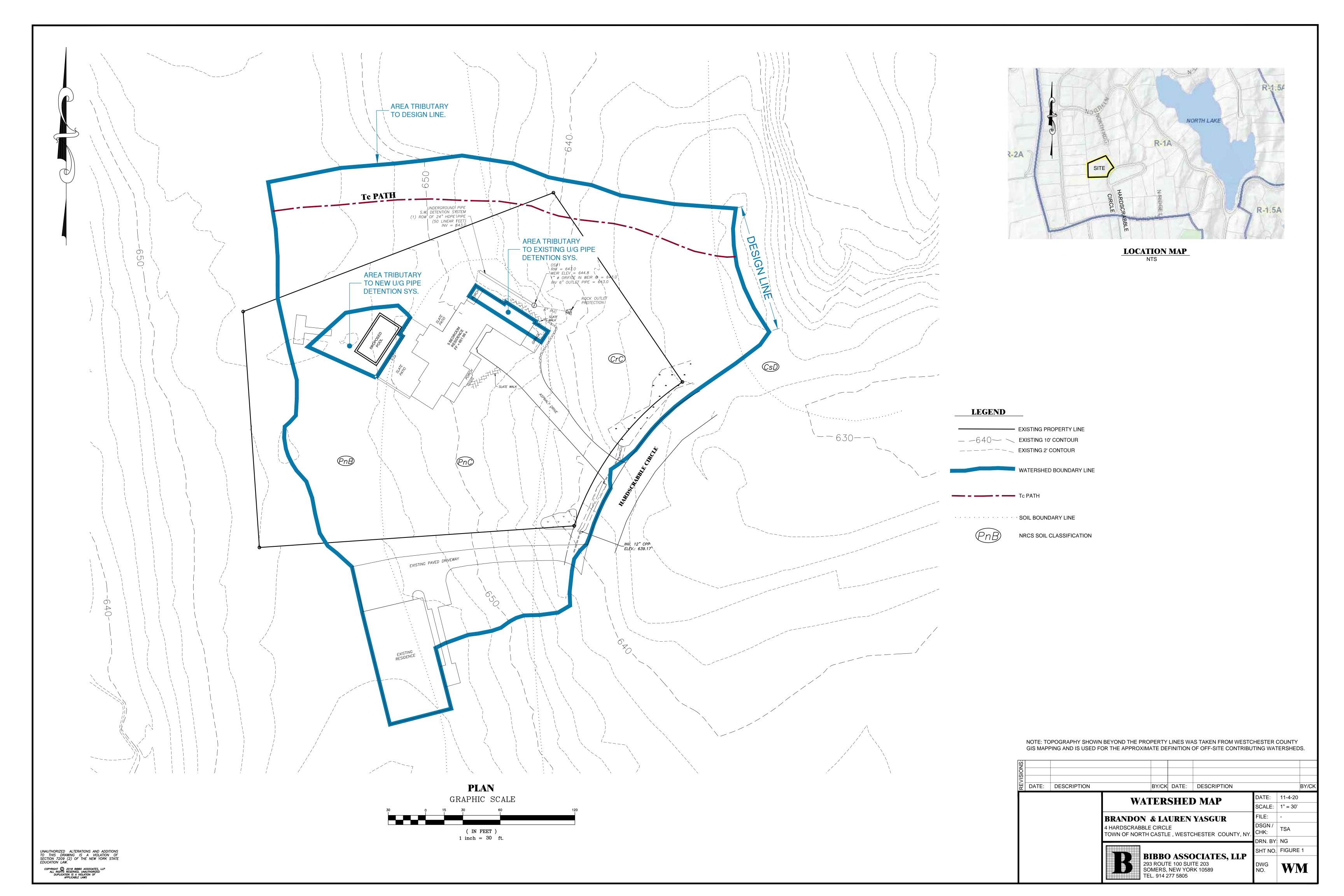
Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

Watershed Map





TOWN OF NORTH CASTLE

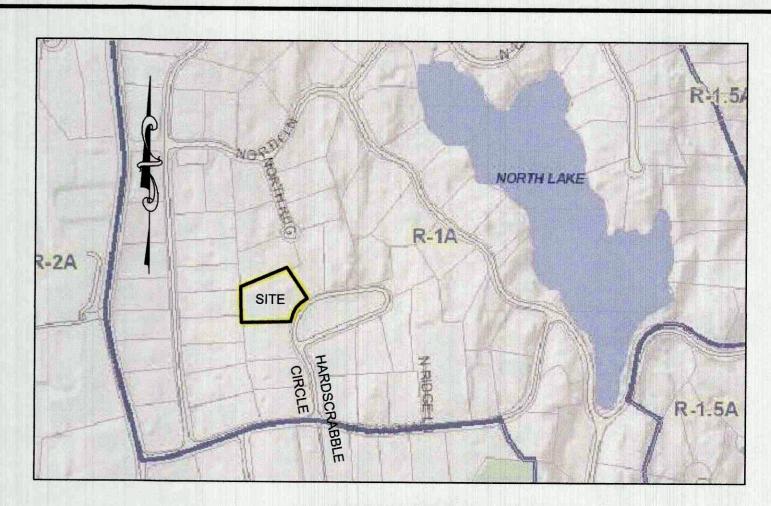
WESTCHESTER COUNTY 17 Bedford Road Armonk, New York 10504-1898

-- PLANNING DEPARTMENT Adam R. Kaufman, AICP Director of Planning

Telephone: (914) 273-3542 Fax: (914) 273-3554 www.northcastleny.com

GROSS LAND COVERAGE CALCULATIONS WORKSHEET

| Applicat | tion Name or Identifying Title: Yasgur | Date: 11/9/2020 | |
|---|---|--|------------------|
| Тах Мај | Designation or Proposed Lot No.: 101.02-3-42 | | |
| Gross Le | ot Coverage | | |
| 1. | Total lot Area (Net Lot Area for Lots Created After 12/13/06): | 69697 | |
| 2. | Maximum permitted gross land coverage (per Section 355-26.C(1)(b)): | 9350 | |
| 3. | BONUS maximum gross land cover (per Section 355-26.C(1)(b)): | 2352 | |
| | Distance principal home is beyond minimum front yard setback $x = 10 = 470$ | 470 | |
| 4. | TOTAL Maximum Permitted gross land coverage = Sum of lines 2 and 3 | 12172 | |
| 5. | Amount of lot area covered by principal building: 3686 existing + 0 proposed = | 3686 | |
| 6. | Amount of lot area covered by accessory buildings: o existing + o proposed = | 0 | |
| 7. | Amount of lot area covered by decks: o existing + o proposed = | 0 . | |
| 8. | Amount of lot area covered by porches: $\frac{405}{\text{existing}} + \frac{0}{\text{proposed}} =$ | 405 | |
| 9. | Amount of lot area covered by driveway , parking areas and walkways: 2987 existing + 0 proposed = | 2987 | , |
| 10. | Amount of lot area covered by terraces: 1534 existing + 0 proposed = | 1534 | |
| 11. | Amount of lot area covered by tennis court , pool and mechanical equip : o existing + 809 proposed = | 809 | |
| 12. | Amount of lot area covered by all other structures: o existing + o proposed = | 0 | |
| 13. Prop | osed gross land coverage: Total of Lines $5-12 =$ | 4021 | |
| If Line 1 the projection does not Signature | complete town seem town. | gross land coverage regulation is greater than Line 4 your property of the second seco | ns and oposal |



LOCATION MAP

CONSTRUCTION SEQUENCE:

1. A PRE-CONSTRUCTION MEETING SHALL BE HELD WITH THE TOWN ENGINEER, TOWN BUILDING DEPARTMENT, SITE CONTRACTOR, AND PROJECT ENGINEER PRIOR TO CONSTRUCTION.

2. INSTALL SILT FENCE WHERE INDICATED ON PLANS.

3. STAKE LIMITS OF DISTURBANCE FOR THE PROPOSED IMPROVEMENTS AND PLACE PROTECTIVE FENCING AROUND EXISTING SEWAGE DISPOSAL AREA.

4. GRUB LOT AND STRIP TOPSOIL. STOCKPILE TOPSOIL WHERE INDICATED ON PLANS, AND STABILIZED TOPSOIL AS INDICATED ON TOPSOIL STOCKPILE DETAIL

5. ROUGH GRADING AND BEGIN POOL EXCAVATION. INSTALL NEW POOL FENCE AS DETAILED.

6. INSTALL NEW PIPE DETENTION SYSTEM AND CONNECT TO EXISTING DETENTION SYSTEM AS SHOWN ON THE PLANS. DO NOT CONNECT STORMWATER PIPING TO PIPE DETENTION SYSTEM UNTIL THE TRIBUTARY AREAS HAVE BEEN STABILIZED.

7. FINAL GRADE LAWN AREAS.

8. SEED AND MULCH LAWN AREAS.

9. REMOVE SILT FENCE WHEN FINAL STABILIZATION IS ACHIEVED.

| | | ZONING DA | TA | | | |
|---------------------------|-----------------------|-------------------------|----------------------------|--|--|--|
| TAX MAP DESIGI | VATION | SECTION 1, BLO | SECTION 1, BLOCK 3, LOT 42 | | | |
| ZONING DISTRIC | 7 1 1 1 1 1 | R-1A - RESIDE | NTIAL | | | |
| | | MINIMUM REQUIREMENTS | PROVIDED | | | |
| LOT AREA | (ACRES) | 1.0 | 1.600 (69,696sf) | | | |
| WIDTH | (FT) | 125 | 257.5 | | | |
| DEPTH | (FT) | 150 | 250 | | | |
| FRONT YARD | (FT) | 50 | 93.6 | | | |
| SIDE YARD | (FT) | 25 | 101.2 / 65.6 | | | |
| REAR YARD | (FT) | 40 | 130.0 / 47.1 | | | |
| MAXIMUM BLDG. COVERAGE | (%) | 12% | 5.3% | | | |
| MAX. GROSS LAI | ND COV. | 12,172 S.F. | 4,021 S.F. | | | |

SITE DATA

1.TOTAL AREA OF PARCEL: 1.6 Ac ±

2. OWNER/APPLICANT: BRANDON YASGUR 49 W PATENT ROAD BEDFORD, NY 10507

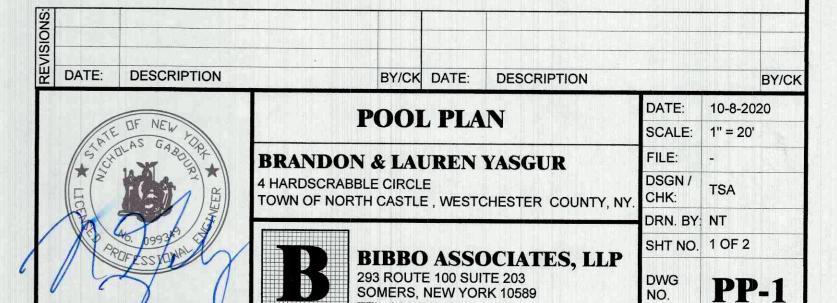
3. ZONING DISTRICT: R-1A RESIDENTIAL

4. TAX I.D. #: SHEET 101.02, BLOCK 3, LOT 42

5. SURVEYOR: EVAN J. FOGLE, PLS LAND SURVEING COMPANY

6. SURVEY LAST UPDATE: MARCH 29, 2018

MOUNT KISCO, NY.



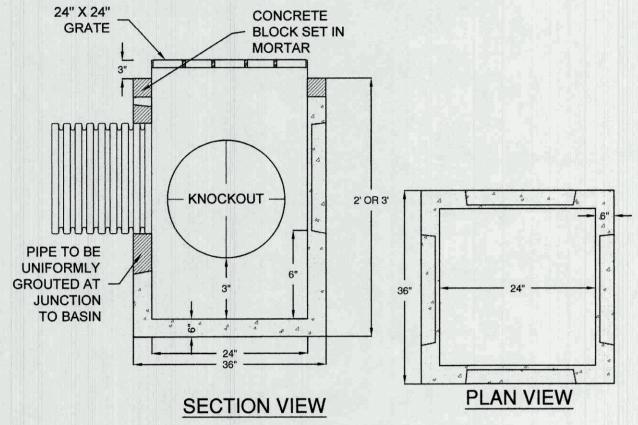
TEL. 914 277 5805

NICHOLAS GABOURY P.E.

DWG NO.

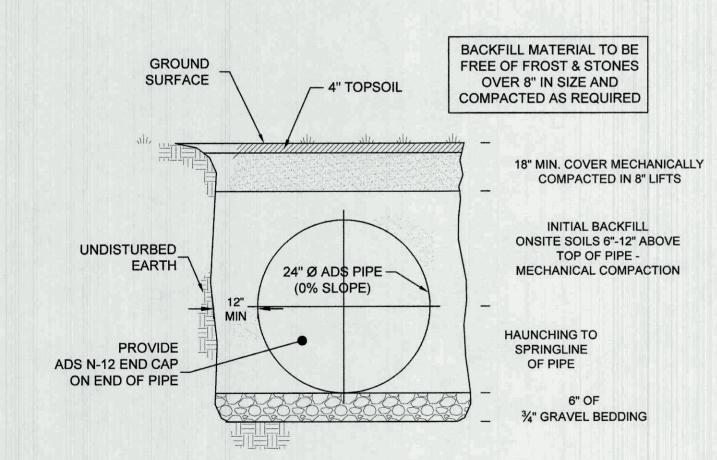
PP-1

8' X 4' POOL EQUIPMENT PAD —

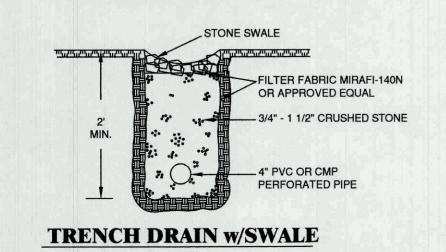


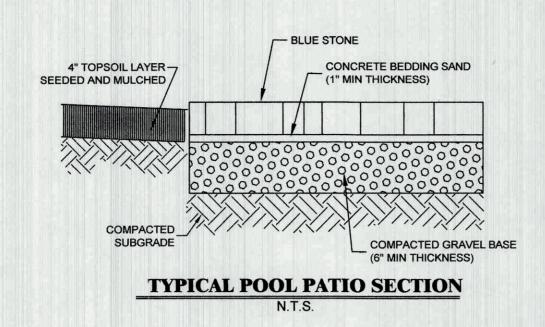
YARD DRAIN DETAIL

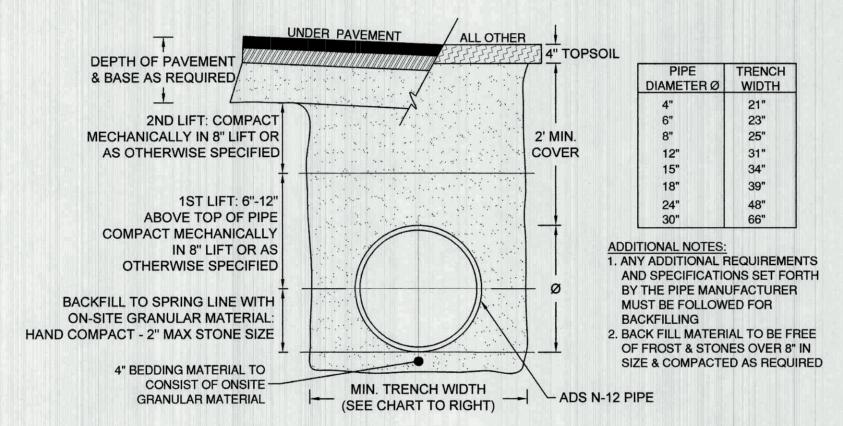
(RESIDENTIAL DRAIN AS MANUFACTURED BY CONNECTICUT PRECAST CO. H-20 LOADING REQUIRED)



UNDERGROUND PIPE DETENTION SYSTEM DETAIL

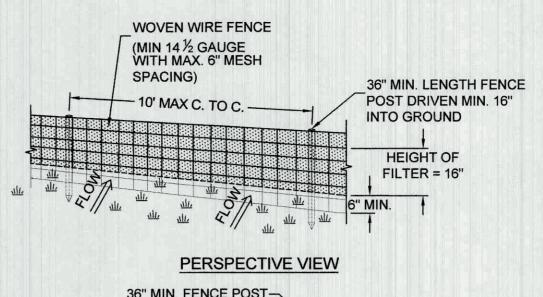


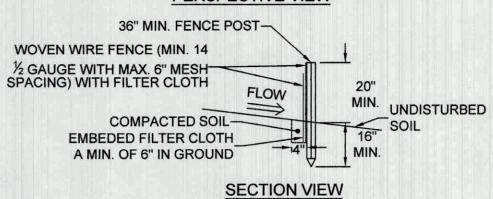




DRAINAGE PIPE INSTALLATION

N.T.S.





CONSTRUCTION SPECIFICATIONS:

1. WOVEN WIRE FENCE TO BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES OR STAPLES. POSTS SHALL BE STEEL WITHER "T" OR "U" TYPE OR HARDWOOD

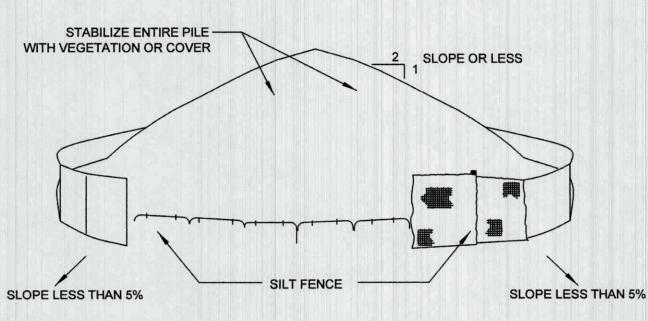
2. FILTER CLOTH TO BE FASTENED SECURELY TO WOVEN WIRE FENCE WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION. FENCE SHALL BE WOVEN WIRE

6" MAXIMUM MESH OPENING. 3. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVER-LAPPED BY SIX INCHES AND FOLDED. FILTER CLOTH SHALL BE EITHER FILTER X, MIRAFI 100X, STABILINKA T140N, OR APPROVED EQUIVALENT.

4. PREFABRICATED UNITS SHALL BE GEOFAB, ENVIROFENCE, OR APPROVED EQUIVALENT. 5. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN

"BULGES" DEVELOP IN THE SILT FENCE.

SILT FENCE DETAIL N.T.S.



INSTALLATION NOTES:

1. AREA CHOSEN FOR STOCKPILING OPERATIONS SHALL BE DRY AND STABLE

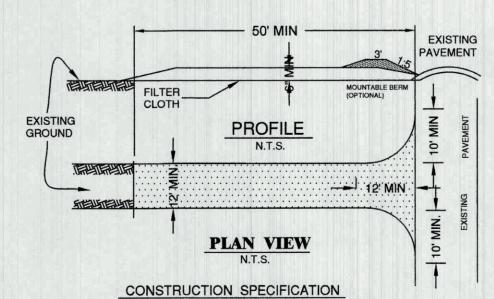
2. MAXIMUM SLOPE OF STOCKPILING SHALL BE 1:2

3. UPON COMPLETION OF SOIL STOCKPILING, EACH PILE SHALL BE SURROUNDED WITH SILT FENCING, THEN STABILIZED WITH VEGETATION OR

COVERED. 4. SEE SILTATION FENCE DETAIL.

SOIL STOCKPILE DETAIL

N. T. S.



1) STONE SIZE- USE 2" STONE, OR RECYCLED CONCRETE EQUIVALENT. 2) LENGTH LESS THAN 50' (EXCEPT ON A SINGLE RESIDENCE LOT WHERE A 30 FOOT MINIMUM

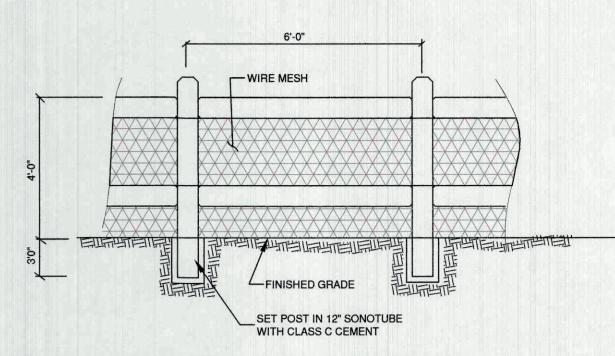
LENGTH WOULD APPLY.) 3) THICKNESS- NOT LESS THAN SIX (6) INCHES.

4) WIDTH - TWELVE (12) FOOT MINIMUM, BUT NOT LESS THAN THE FULL WIDTH AT POINTS WHERE INGRESS OR EGRESS OCCURS. TWENTY-FOUR (24) FOOT IF SINGLE ENTRANCE TO SITE. 5) FILTER CLOTH- WILL BE PLACED OVER THE ENTIRE AREA PRIOR TO PLACING OF STONE. 6) SURFACE WATER- ALL SURFACE WATER FLOWING OR DIVERTED TOWARD CONSTRUCTION ENTRANCES SHALL BE PIPED ACROSS THE ENTRANCE. IF PIPING IS IMPRACTICAL, A MOUNTABLE BERM WITH 5:1 SLOPES WILL BE PERMITTED.

7) MAINTENANCE- THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION WHICH WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHTS-OF-WAY. ALL SEDIMENT SPILLED, DROPPED, WASHED OR TRACKED ONTO PUBLIC RIGHTS-OF-WAY MUST BE REMOVED IMMEDIATELY. 8) WHEN WASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STABILIZED WITH STONE & WHICH DRAINS INTO AN APPROVED SEDIMENT TRAPPING DEVICE. 9) PERIODIC INSPECTION AND NEEDED MAINTENANCE SHALL BE PROVIDED AFTER EACH RAIN

STABILIZED CONSTRUCTION ENTRANCE DETAIL

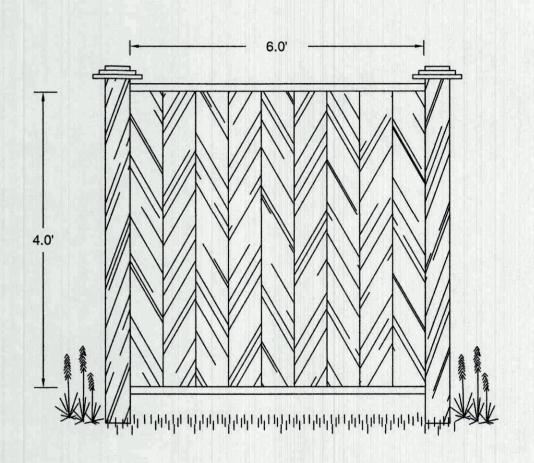
N. T. S.



SPLIT RAIL FENCE DETAIL

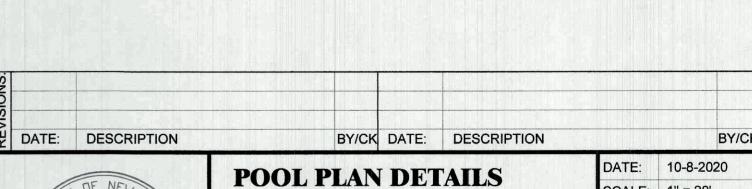
OR APPROVED EQUAL

PLEASE NOTE: THE PROPOSED POOL FENCE GATES ON EACH SIDE OF THE EXISTING RESIDENCE SHALL OPEN OUTWARD (AWAY FROM POOL), BE SELF-CLOSING, SELF-LATCHING, AND EQUIPPED WITH MAGNALATCH AT 54" OFF OF GRADE.



BOARD FENCE DETAIL

N.T.S. OR APPROVED EQUAL





NICHOLAS GABOURY P.E.

BRANDON & LAUREN YASGUR 4 HARDSCRABBLE CIRCLE TOWN OF NORTH CASTLE , WESTCHESTER COUNTY, N'

> BIBBO ASSOCIATES, LLP 293 ROUTE 100 SUITE 203 SOMERS, NEW YORK 10589

DWG NO. PP-1 TEL. 914 277 5805

SCALE: 1" = 20'

SHT NO. 2 OF 2

DSGN / TSA



Section I- PROJECT

TOWN OF NORTH CASTLE

WESTCHESTER COUNTY 17 Bedford Road Armonk, New York 10504-1898

RESIDENTIAL PROJECT REVIEW COMMITTEE Adam R. Kaufman AICP, Chair Telephone: (914) 273-3000 x 43 Fax: (914) 273-3554 www.nortcastleny.com

RESIDENTIAL PROJECT REVIEW COMMITTEE (RPRC) APPLICATION

ADDRESS: 4 HARDSCRABBLE CIRCLE, NORTH CASTLE, NY, 10504

| Section III- DESCRIPTION OF WORK: |
|--|
| CONSTRUCTION OF NEW INGROUND SWIMMING POOL AND ASSOCIATED PATIO AREA |
| |
| |
| |
| |
| |
| Section III- CONTACT INFORMATION: |
| APPLICANT: BRANDON YASGUR |
| ADDRESS: 4 HARDSCRABBLE CIRCLE, NORTH CASTLE, NY, 10504 |
| PHONE: MOBILE:EMAIL: |
| PROPERTY OWNER: BRANDON YASGUR |
| ADDRESS: 4 HARDSCRABBLE CIRCLE, NORTH CASTLE, NY, 10504 |
| PHONE:MOBILE:EMAIL: |
| PROFESSIONAL::NICHOLAS GABOURY, P.E |
| ADDRESS: 293 ROUTE 100 SUITE 203, SOMERS, NY 10589 |
| PHONE: 914-277-5805 MOBILE: |
| EMAIL: NGABOURY@BIBBOASSOCIATES.COM_ |
| Section IV- PROPERTY INFORMATION: |
| Zone: R-1A Tax ID (lot designation) 101.02-3-42 |
| |



Town of North Castle Residential Project Review Committee

17 Bedford Road Armonk, New York 10504 (914) 273-3542 (914) 273-3554 (fax)

RPRC COMPLETENESS REVIEW FORM

This form represents the standard requirements for a completeness review for all Residential Project Review Committee submissions. Failure to provide all of the information requested will result in a determination that the application is incomplete.

| Project Name on Plan: | | |
|--|---|--|
| ☐Initial Submittal ☐Revised Preliminary | | |
| Street Location: | | |
| Zoning District: Property Acreage: Tax Map Parcel ID: | | |
| Date: | | |
| DEPARTMENTAL USE ONLY | | |
| Date Filed: Staff Name: | | |
| Preliminary Plan Completeness Review Checklist Items marked with a "⊠" are complete, items left blank "□" are incomplete and must be completed, "NA" means not applicable. | | |
| □1. | Plan prepared by a registered architect or professional engineer | |
| □ 2. | Aerial photo (Google Earth) showing the applicant's entire property and adjacent properties and streets | |
| □3. | Map showing the applicant's entire property and adjacent properties and streets | |
| □ 4. | A locator map at a convenient scale | |
| □5. | The proposed location, use and design of all buildings and structures | |
| □6. | Existing topography and proposed grade elevations | |
| □7. | Location of drives | |
| □8. | Location of all existing and proposed site improvements, including drains, culverts, retaining walls and fences | |

RPRC COMPLETENESS REVIEW FORM

Page 2

| ☐9. Description of method of water supply and sewage disposal and location of such facilities | |
|--|--|
| ☐10. The name and address of the applicant, property owner(s) if other than the applicant and of the planner, engineer, architect, surveyor and/or other professionals engaged to work | |
| ☐11. Submission of a Zoning Conformance Table depicting the plan's compliance with the minimum requirements of the Zoning District | |
| ☐12. If a tree removal permit is being sought, submission of a plan depicting the location and graphical removal status of all Town-regulated trees within the proposed area of disturbance. In addition, the tree plan shall be accompanied by a tree inventory includes a unique ID number, the species, size, health condition and removal status of each tree. | |
| ☐13. If a wetlands permit is being sought, identification of the wetland and the 100-foot wetland buffer. | |
| More information about the items required herein can be obtained from the North Castle Planning Department. A copy of the Town Code can be obtained from Town Clerk or on the North Castle homepage: http://www.northcastleny.com/townhall.html | |
| On this date, all items necessary for a technical review of the proposed site plan have been submitted and constitute a COMPLETE APPLICATION. | |



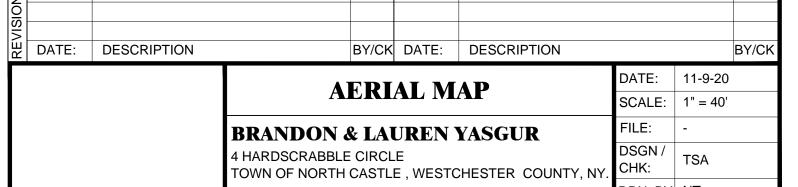


(IN FEET) 1 inch = 40 ft.



LOCATION MAP

NTS



SHT NO. FIGURE 1 BIBBO ASSOCIATES, LLP
293 ROUTE 100 SUITE 203
SOMERS, NEW YORK 10589
TEL. 914 277 5805

AM