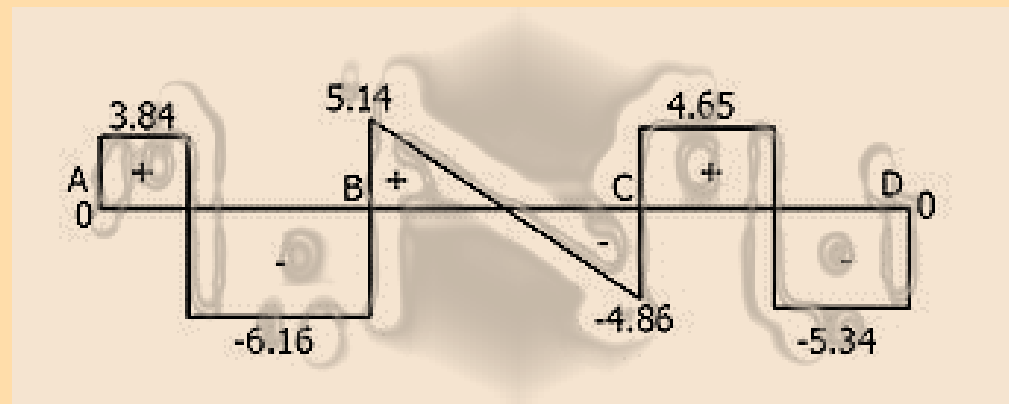
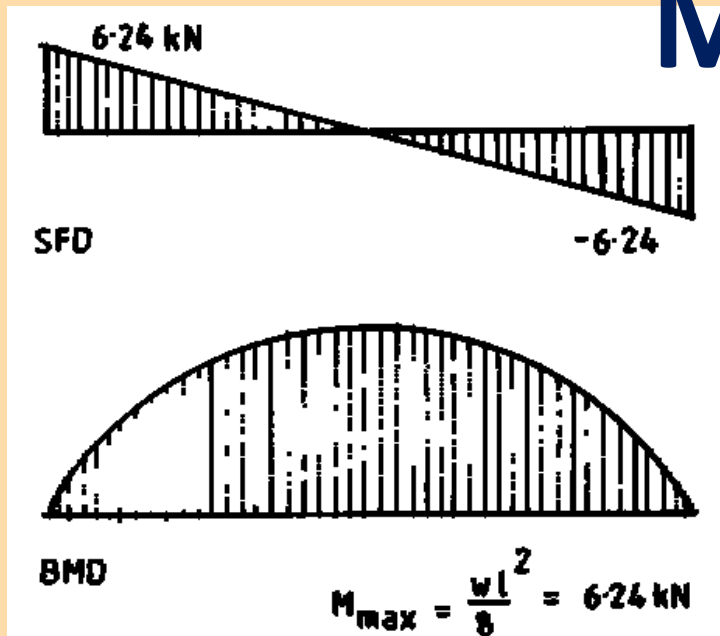
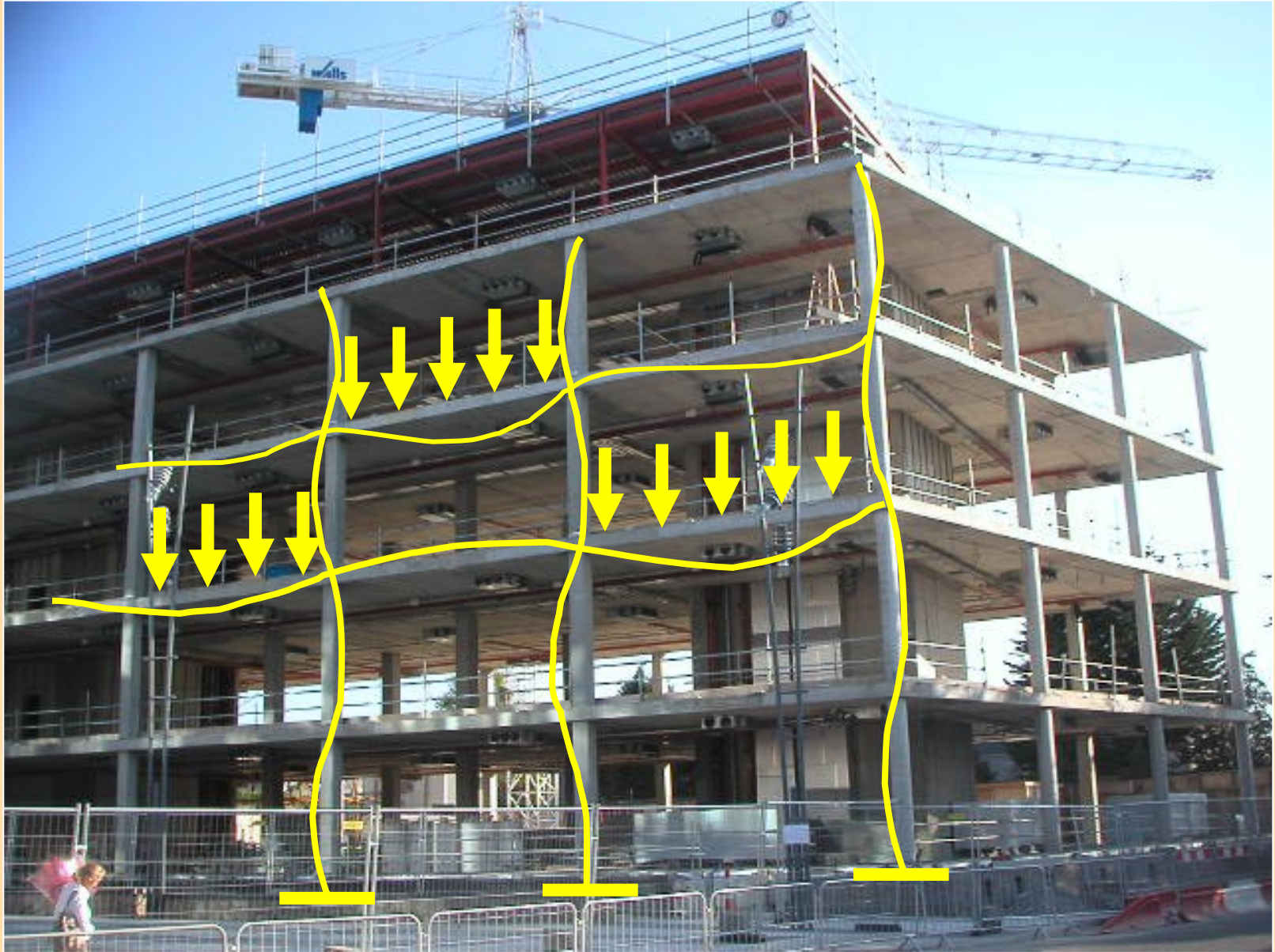




# CHAPTER 2

## Shear Force And Bending Moment





# INTRODUCTION

	Effects	Action
Loading	Shear Force	Design Shear reinforcement
Loading	Bending Moment	Design flexure reinforcement

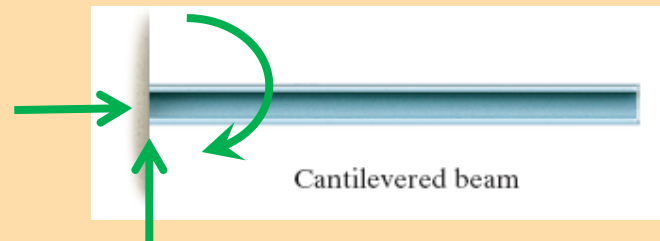
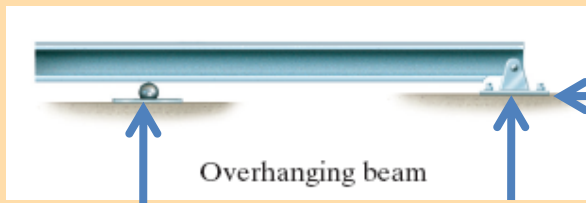


# SHEAR FORCE & BENDING MOMENT

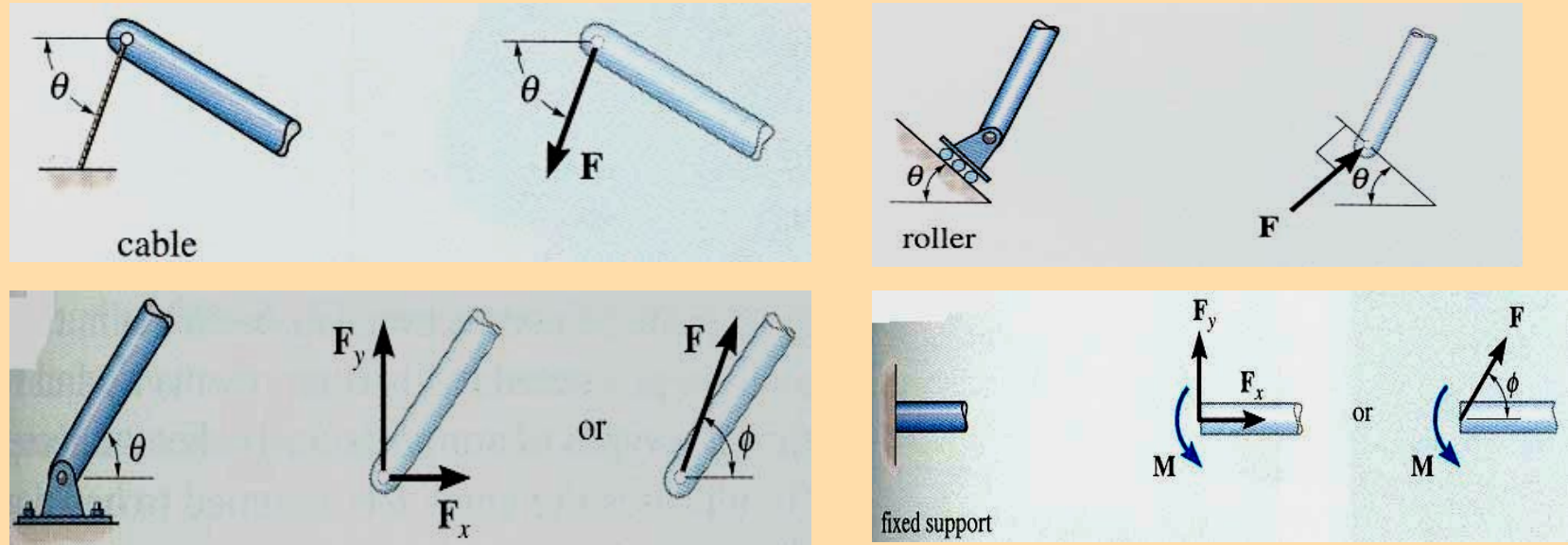
- Introduction
  - Types of beams
  - Effects of loading on beams
  - The force that cause shearing is known as shear force
  - The force that results in bending is known as bending moment
  - Draw the shear force and bending moment diagrams

# SHEAR FORCE & BENDING MOMENT

- Members with support loadings applied perpendicular to their longitudinal axis are called **beams**.
- Beams classified according to the way they are supported.



# TYPES OF SUPPORT



As a general rule, if a **support prevents translation** of a body in a given direction, then **a force is developed** on the body in the opposite direction. Similarly, if **rotation is prevented**, a **couple moment** is exerted on the body.

# SHEAR FORCE & BENDING MOMENT

- Types of beam

- a) **Determinate Beam**

The force and moment of reactions at supports can be determined by using the 3 equilibrium equations of statics i.e.

$$\Sigma F_x = 0, \Sigma F_y = 0 \text{ and } \Sigma M = 0$$

- b) **Indeterminate Beam**

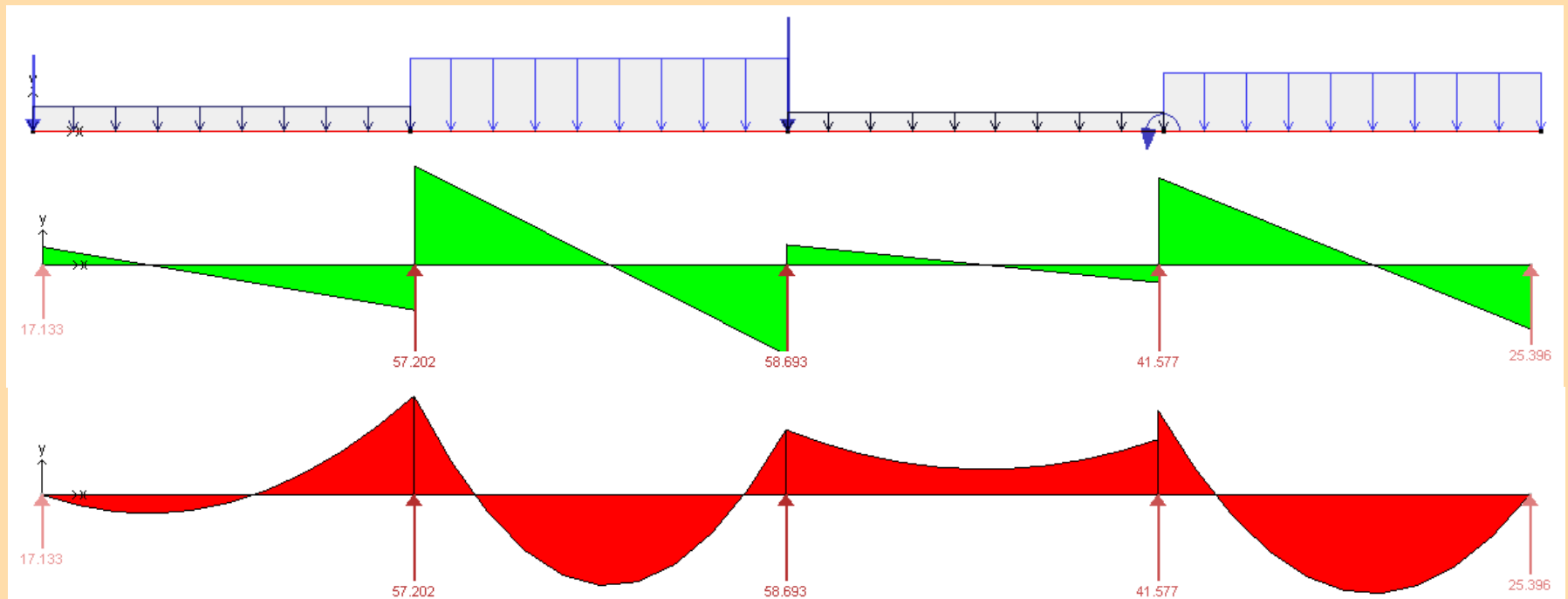
The force and moment of reactions at supports are more than the number of equilibrium equations of statics.

(The extra reactions are called redundant and represent the amount of degrees of indeterminacy).



# SHEAR FORCE & BENDING MOMENT

- In order to properly design a beam, it is important to know the **variation** of the shear and moment along its axis in order to find the points where these values are a maximum.

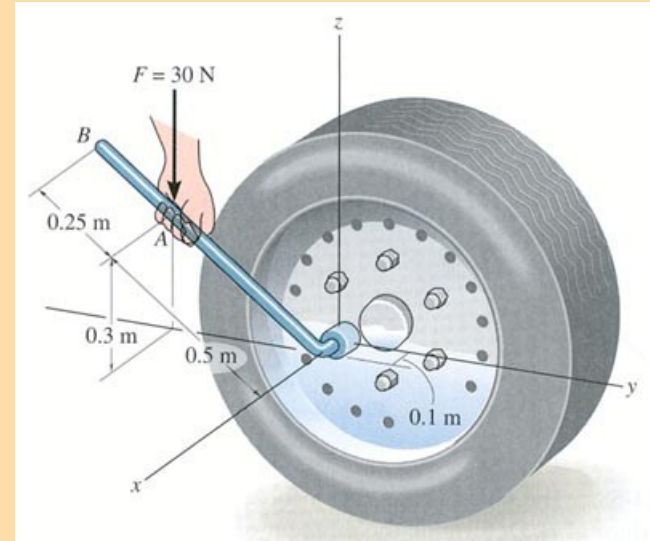
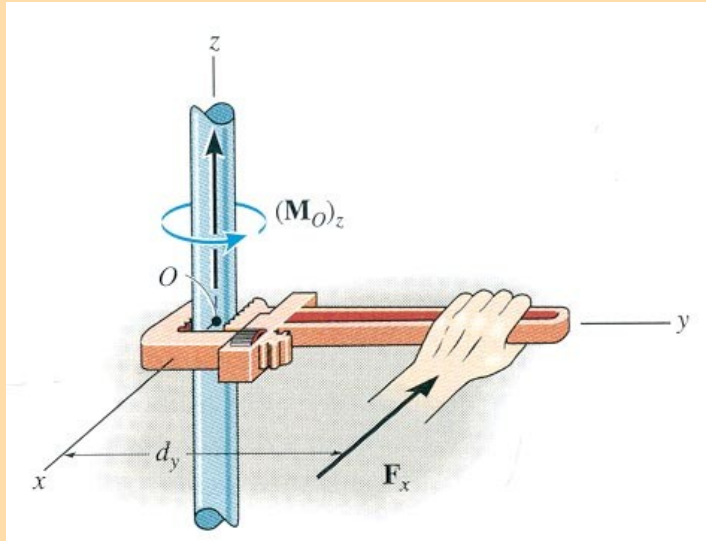




# PRINCIPLE OF MOMENTS

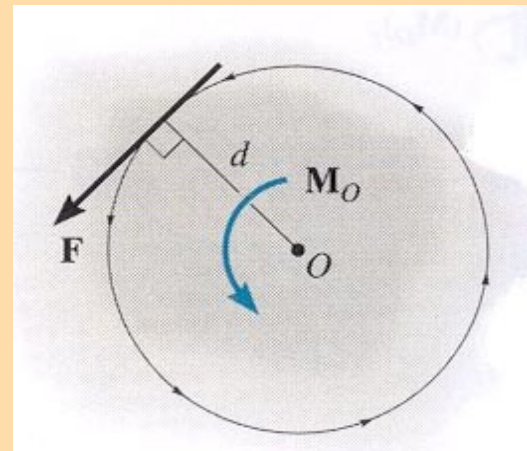
- The moment of a force indicates the tendency of a body to turn about an axis passing through a specific point O.
- The principle of moments, which is sometimes referred to as ***Varignon's Theorem*** (Varignon, 1654 – 1722) states that ***the moment of a force about a point is equal to the sum of the moments of the force's components about the point.***

# PRINCIPLE OF MOMENTS



In the 2-D case, the magnitude of the moment is:

$$M_o = \text{Force} \times \text{distance}$$

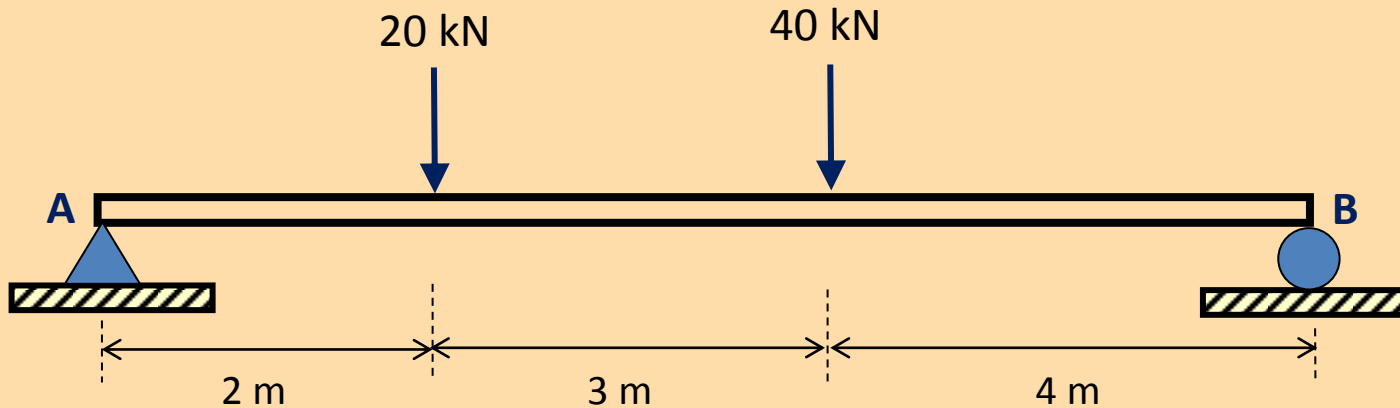


# BEAM'S REACTION

- If a support *prevents translation* of a body in a particular direction, then the support exerts a *force* on the body in that direction.
- Determined using  $\Sigma F_x = 0$ ,  $\Sigma F_y = 0$  and  $\Sigma M = 0$

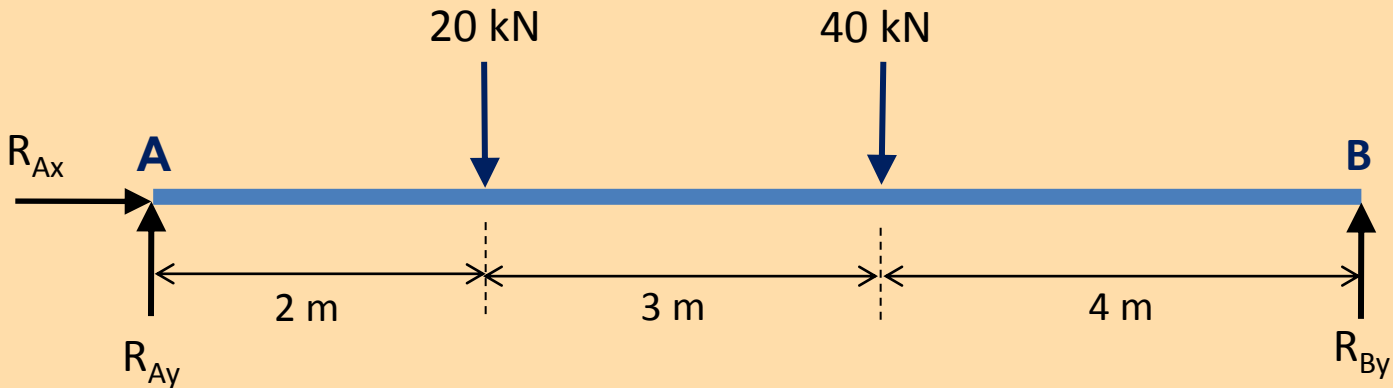
# EXAMPLE 1

The beam shown below is supported by a pin at A and roller at B. Calculate the reactions at both supports due to the loading.



# EXAMPLE 1 – Solution

Draw the free body diagram:



By taking the moment at B,

$$\Sigma M_B = 0$$

$$R_{Ay} \times 9 - 20 \times 7 - 40 \times 4 = 0$$

$$9R_{Ay} = 140 + 160$$

$$R_{Ay} = 33.3 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} - 20 - 40 = 0$$

$$R_{By} = 20 + 40 - 33.3$$

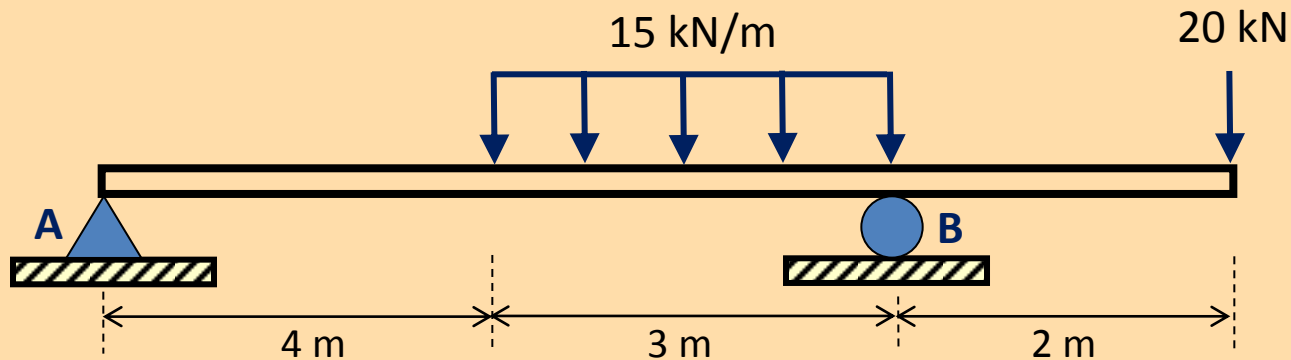
$$R_{By} = 26.7 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

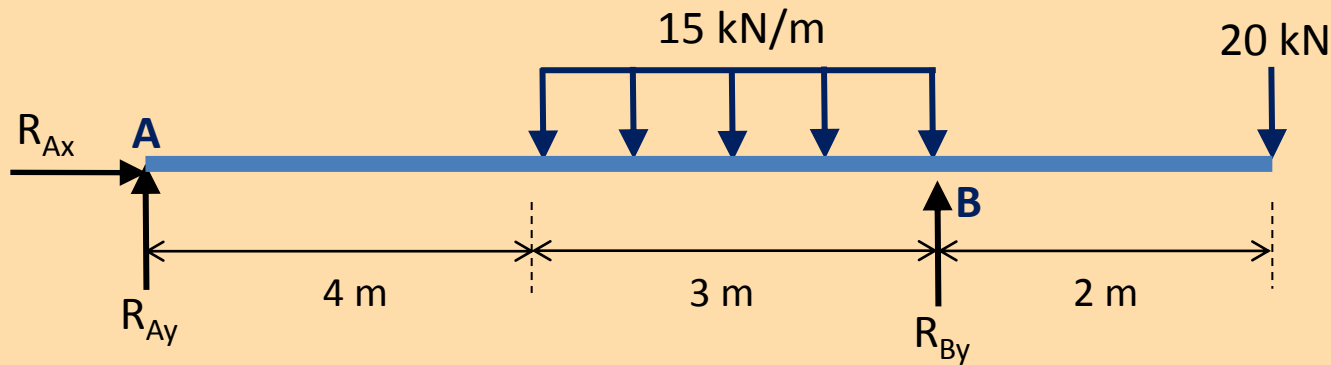
# EXAMPLE 2

Determine the reactions at support A and B for the overhanging beam subjected to the loading as shown.



# EXAMPLE 2 – Solution

Draw the free body diagram:



By taking the moment at A:

$$\Sigma M_A = 0$$

$$-R_{By} \times 7 + 20 \times 9 - (15 \times 3) \times 5.5 = 0$$

$$7R_{By} = 247.5 + 180$$

$$R_{By} = 61.07 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} - 20 - 45 = 0$$

$$R_{Ay} = 20 + 45 - 61.07$$

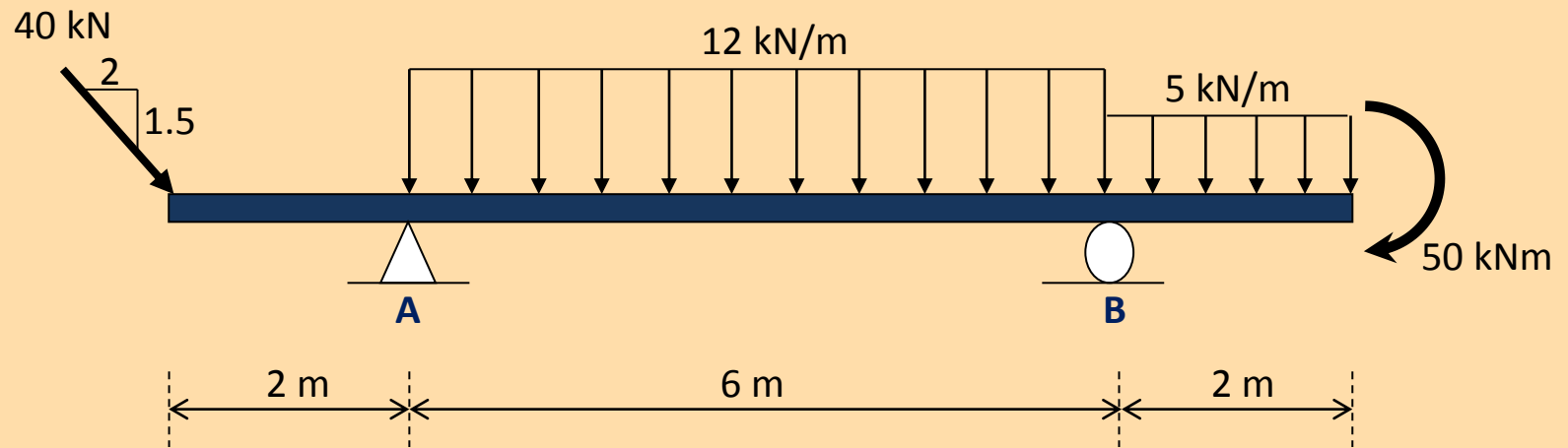
$$R_{Ay} = 3.93 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

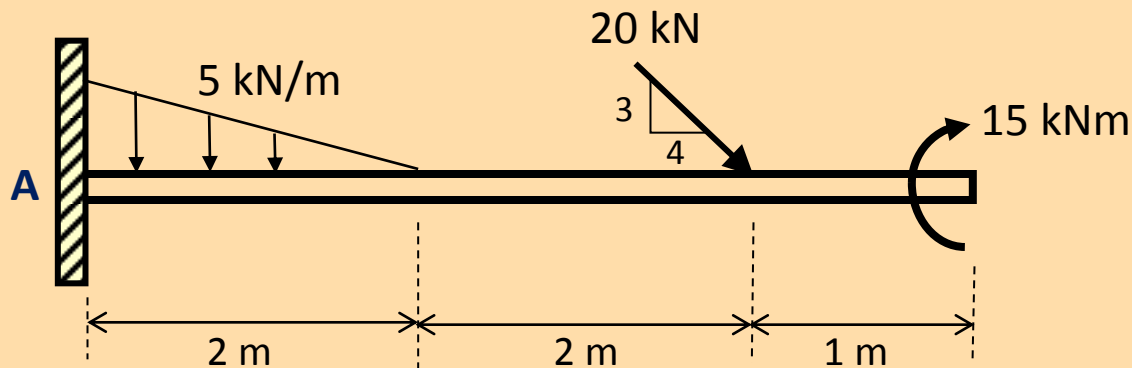


# CLASS EXERCISE – 5 mins?



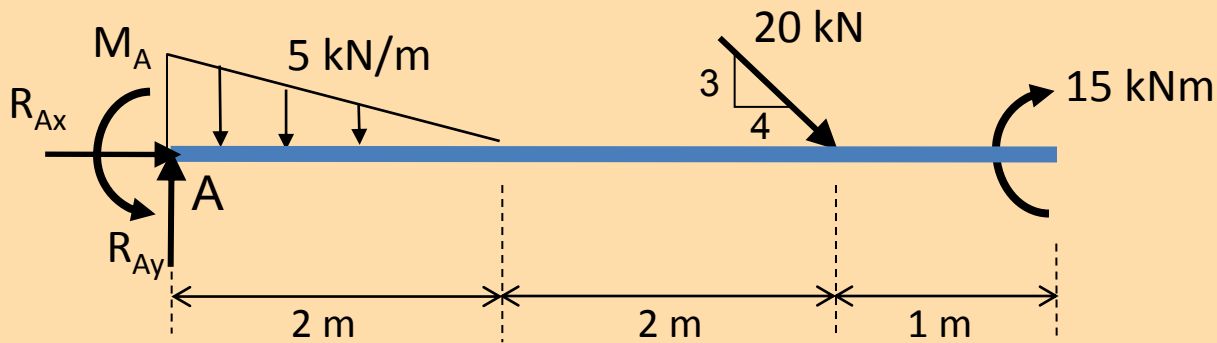
## EXAMPLE 3

A cantilever beam is loaded as shown. Determine all reactions at support A.



# EXAMPLE 3 – Solution

Draw the free body diagram:



$$\Sigma F_x = 0$$

$$- R_{Ax} + 20 (4/5) = 0$$

$$- R_{Ax} = 16 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} - 0.5 (5)(2) - 20(3/5) = 0$$

$$R_{Ay} - 5 - 12 = 0$$

$$R_{Ay} = 17 \text{ kN}$$

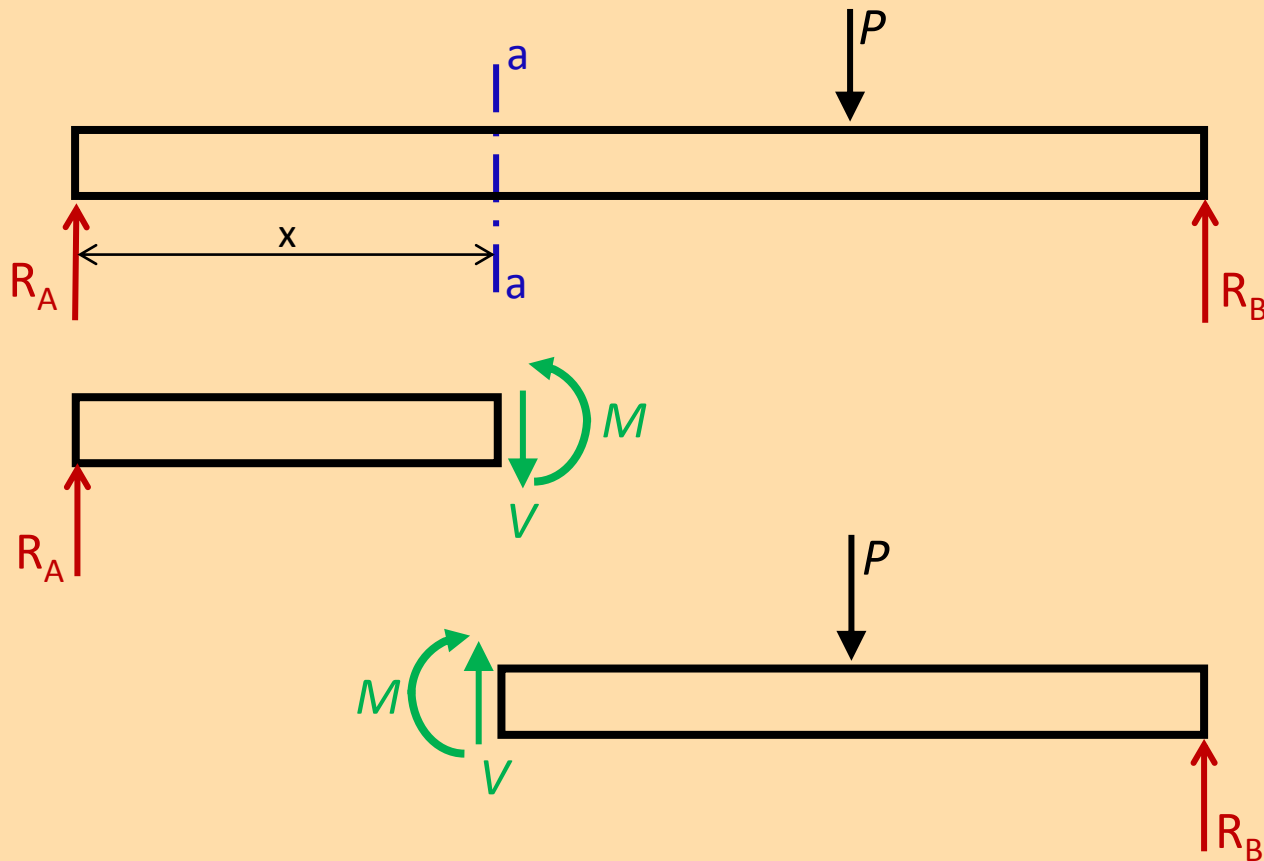
$$\Sigma M_A = 0$$

$$- M_A + 0.5(5)(2)(1/3)(2) + 20(3/5) (4) + 15 = 0$$

$$M_A = 3.3 + 48 + 15$$

$$M_A = 66.3 \text{ kNm}$$

# SHEAR FORCE & BENDING MOMENT DIAGRAM



# SHEAR FORCE & BENDING MOMENT DIAGRAM

**V** = shear force  
= the force that tends to separate the member  
= balances the reaction  $R_A$

**M** = bending moment  
= the reaction moment at a particular point  
(section)  
= balances the moment,  $R_A \cdot x$

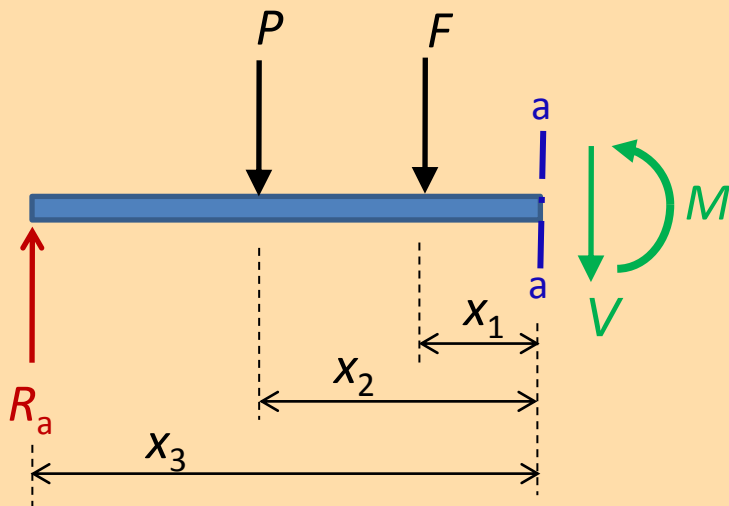
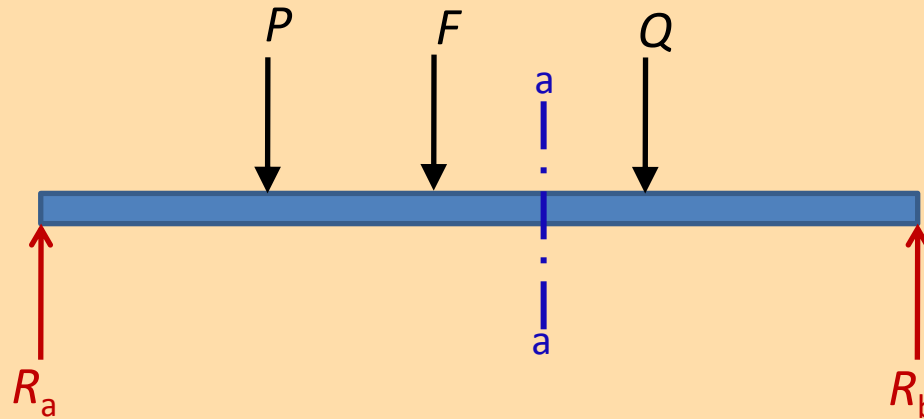
# SHEAR FORCE & BENDING MOMENT DIAGRAM

From the equilibrium equations of statics:

$$+\uparrow \Sigma F_y = 0; \quad R_A - V = 0 \quad \therefore V = R_A$$

$$+\curvearrowright \Sigma M_{a-a} = 0; \quad -M + R_A \cdot x = 0 \quad \therefore M = R_A \cdot x$$

# SHEAR FORCE & BENDING MOMENT DIAGRAM



$$\sum F_y = 0$$

$$R_a - P - F - V = 0$$

$$V = R_a - P - F$$

$$\sum M_{a-a} = 0$$

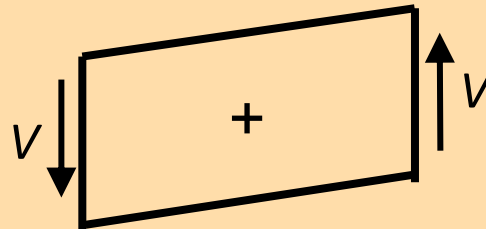
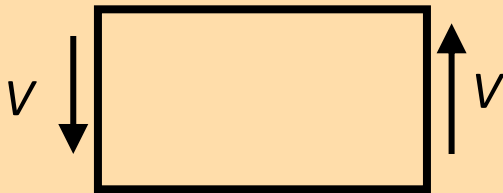
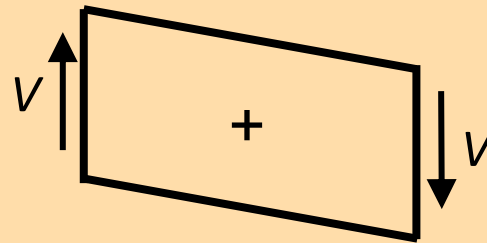
$$-M - F \cdot x_1 - P \cdot x_2 + R_a \cdot x_3 = 0$$

$$M = R_a \cdot x_3 - F \cdot x_1 - P \cdot x_2$$



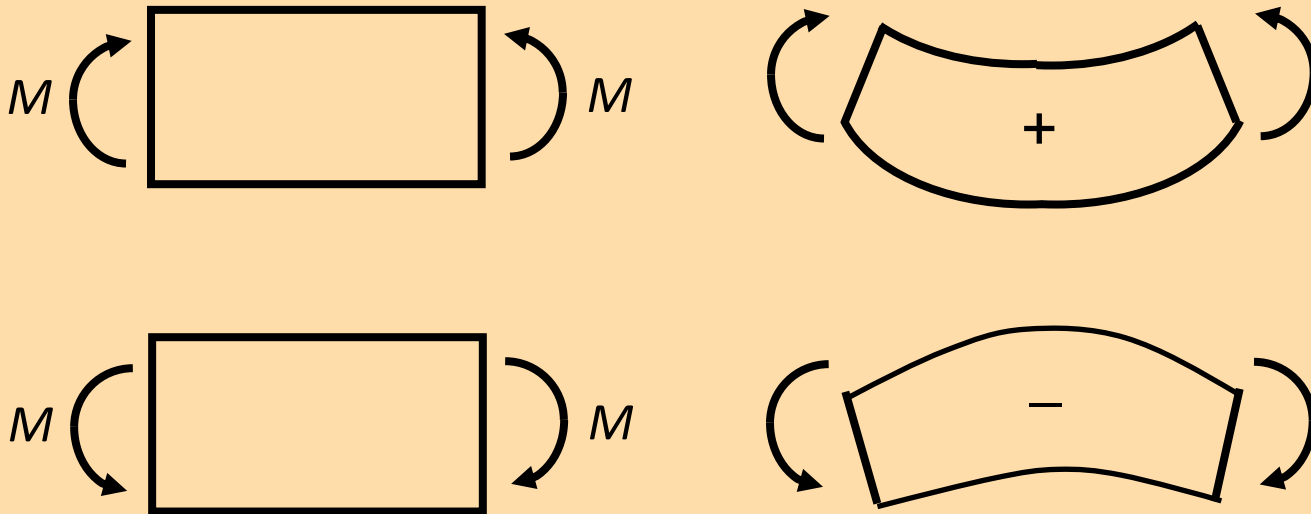
# SHEAR FORCE & BENDING MOMENT DIAGRAM

Shape deformation due to shear force:



# SHEAR FORCE & BENDING MOMENT DIAGRAM

Shape deformation due to bending moment:

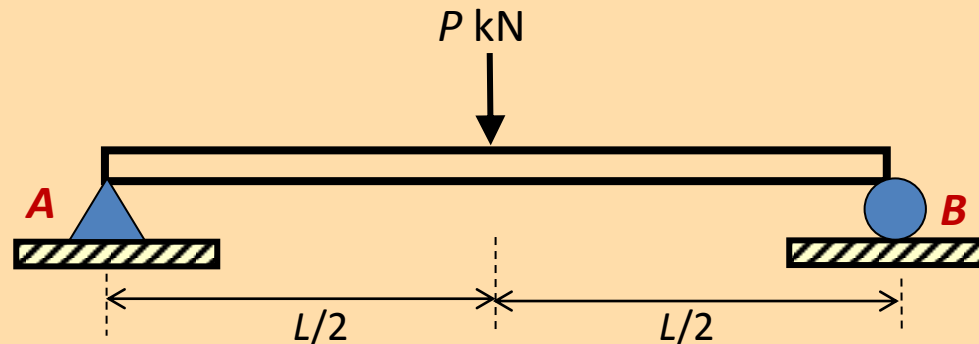


*Sign Convention:*

- **Positive shear force** diagram drawn **ABOVE** the beam
- **Positive bending moment** diagram drawn **BELOW** the beam

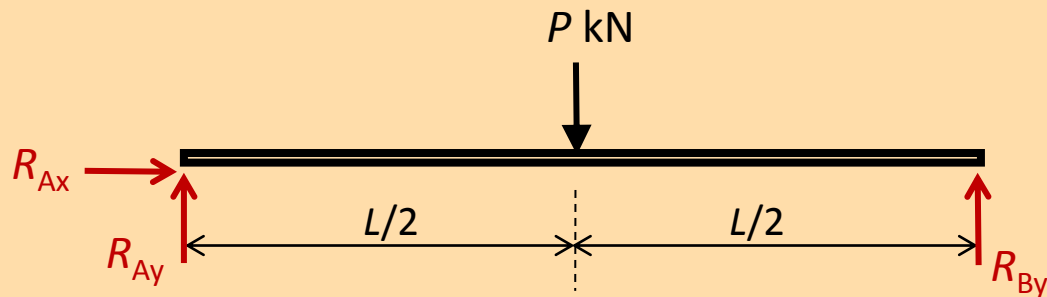
# EXAMPLE 4

- a) Calculate the shear force and bending moment for the beam subjected to a concentrated load as shown in the figure. Then, draw the shear force diagram (SFD) and bending moment diagram (BMD).
- b) If  $P = 20$  kN and  $L = 6$  m, draw the SFD and BMD for the beam.



# EXAMPLE 4 – Solution

a)



By taking the moment at A:

$$\Sigma M_A = 0$$

$$- R_{By} \times L + P \times L/2 = 0$$

$$R_{By} = P/2 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} = P$$

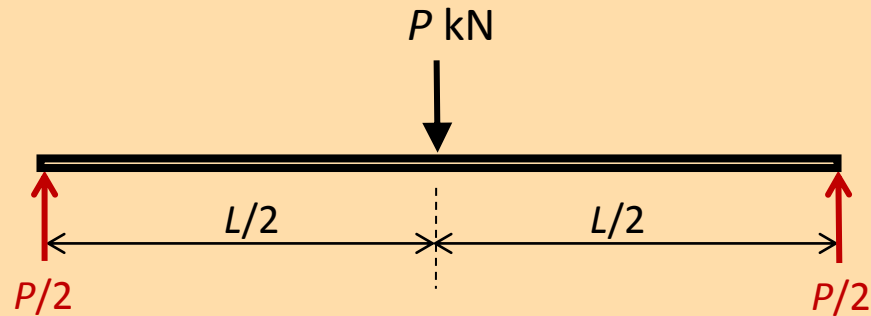
$$R_{Ay} = P - P/2$$

$$R_{Ay} = P/2 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

# EXAMPLE 4 – Solution



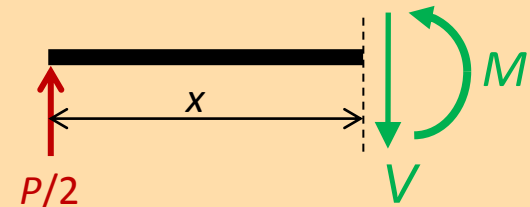
Between  $0 < x < L/2$ :

$$\sum F_y = 0, \quad -V + P/2 = 0$$

$$V = P/2 \text{ kN}$$

$$\sum M_{a-a} = 0, \quad -M + Px/2 = 0$$

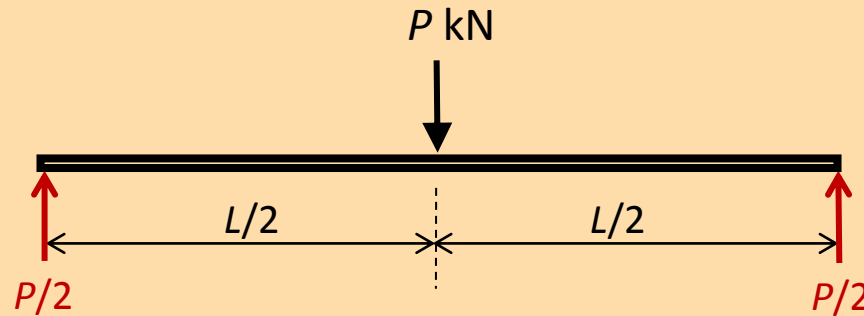
$$M = Px/2 \text{ kNm}$$



If  $x = 0$  m,  $V = P/2$  kN and  $M = 0$  kNm

If  $x = L/2$  m,  $V = P/2$  kN and  $M = PL/4$  kNm

# EXAMPLE 4 – Solution



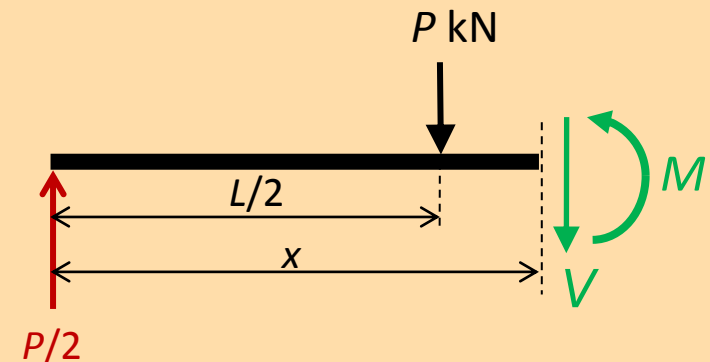
Between  $L/2 < x < L$ :

$$\sum F_y = 0, \quad -V + P/2 - P = 0$$

$$V = -P/2 \text{ kN}$$

$$\sum M_{a-a} = 0, \quad -M + Px/2 - P(x - L/2) = 0$$

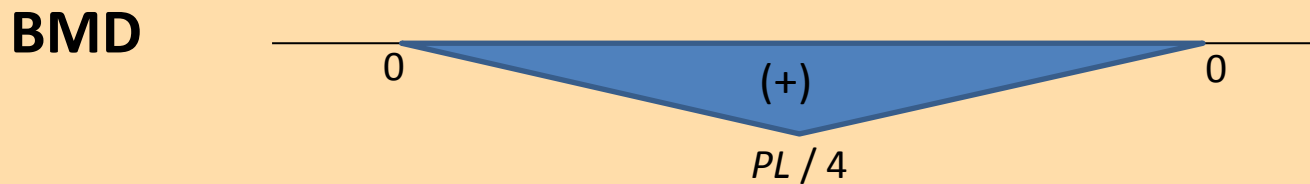
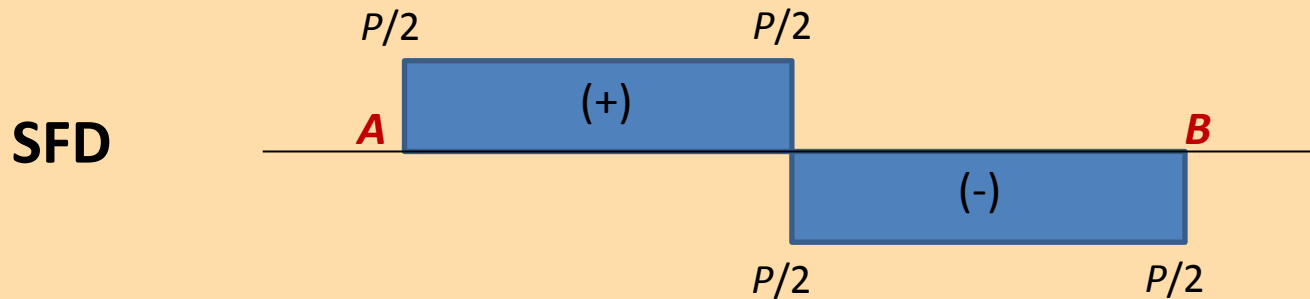
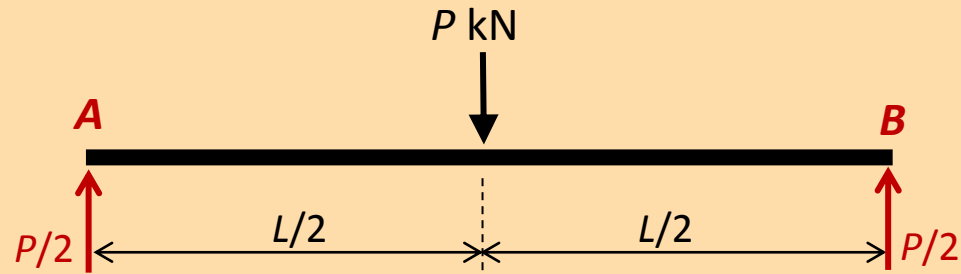
$$M = PL/2 - Px/2 \text{ kNm}$$



If  $x = L/2 \text{ m}$ ,  $V = -P/2 \text{ kN}$  and  $M = PL/4 \text{ kNm}$

If  $x = L \text{ m}$ ,  $V = -P/2 \text{ kN}$  and  $M = 0 \text{ kNm}$

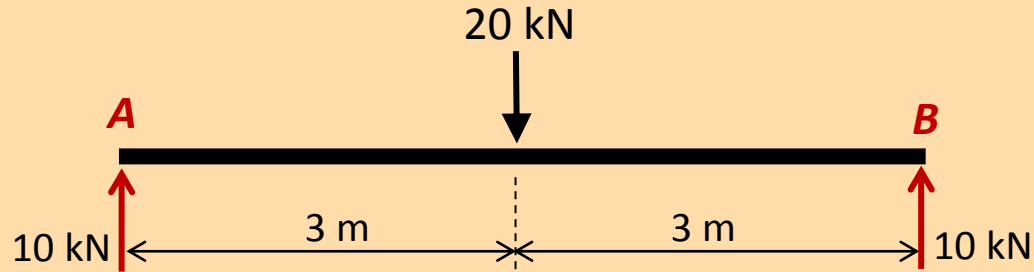
# EXAMPLE 4 – Solution



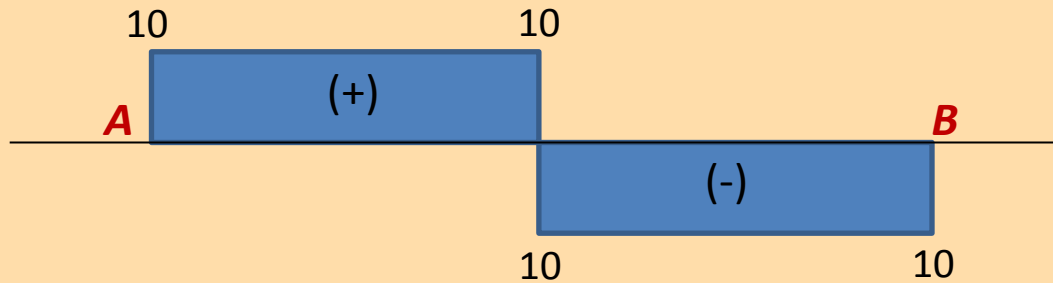


# EXAMPLE 4 – Solution

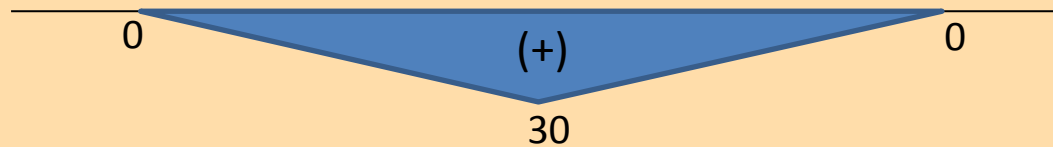
b)



SFD (kN)

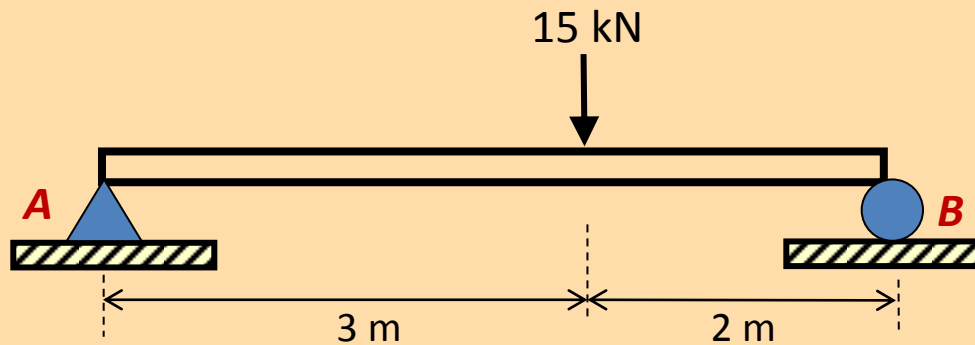


BMD (kNm)

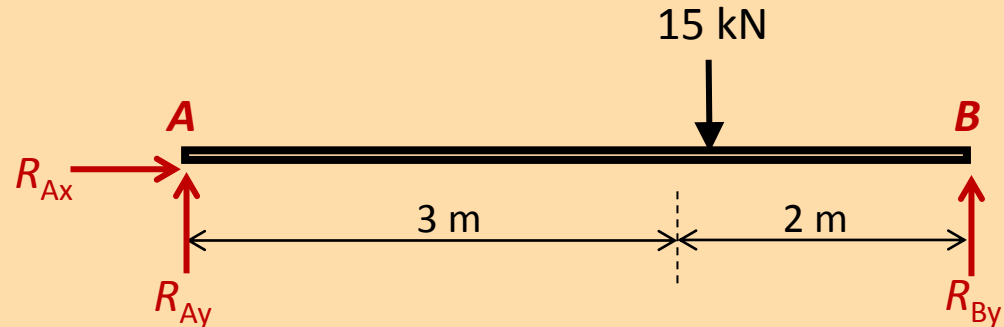


# EXAMPLE 5

Calculate the shear force and bending moment for the beam subjected to a concentrated load as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 5 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$- R_{By} \times 5 + 15 \times 3 = 0$$

$$R_{By} = 9 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} = 15$$

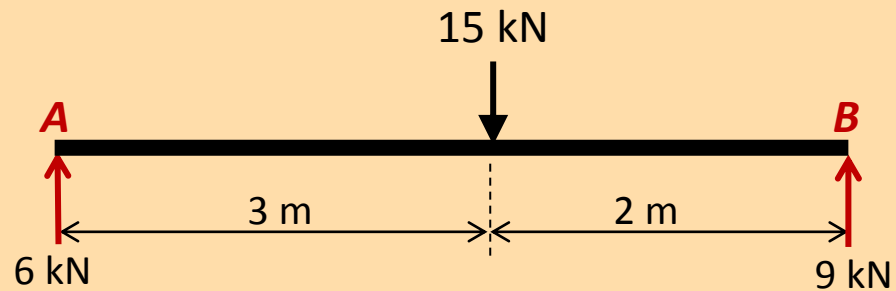
$$R_{Ay} = 15 - 9$$

$$R_{Ay} = 6 \text{ kN}$$

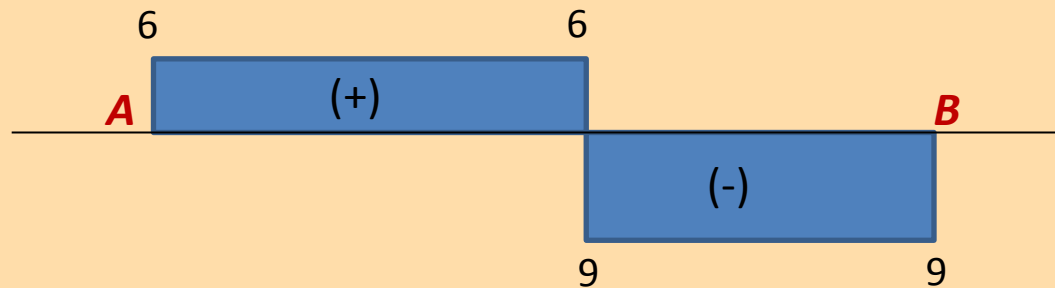
$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

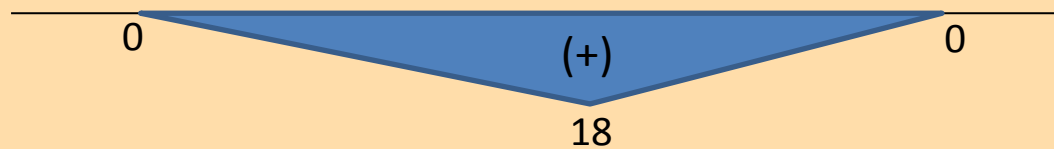
# EXAMPLE 5 – Solution



SFD (kN)

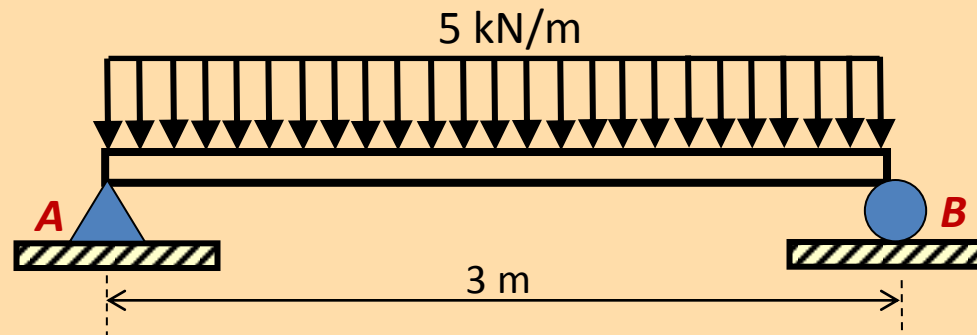


BMD (kNm)

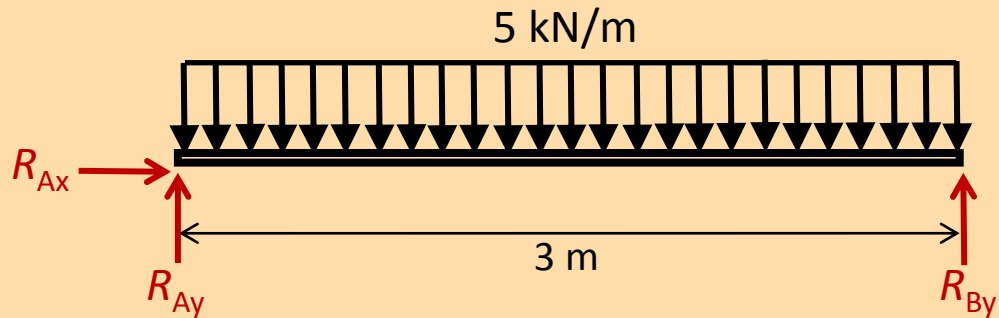


# EXAMPLE 6

Calculate the shear force and bending moment for the beam subjected to an uniformly distributed load as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 6 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$-R_{By} \times 3 + 5 \times 3 \times 3/2 = 0$$

$$R_{By} = 7.5 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} = 5 \times 3$$

$$R_{Ay} = 15 - 7.5$$

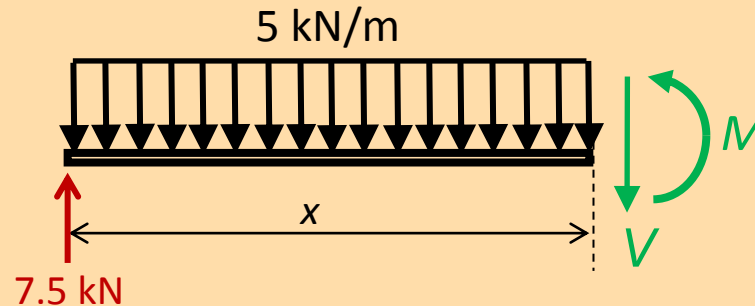
$$R_{Ay} = 7.5 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

# EXAMPLE 6 – Solution

These results for  $V$  and  $M$  can be checked by noting that  $dV/dx = -w$ . This is correct, since positive  $w$  acts downward. Also, notice that  $dM/dx = V$ . The maximum moments occurs when  $dM/dx = V = 0$ .



$$\Sigma M_{a-a} = 0,$$

$$-M + 7.5x - 5x(x/2) = 0$$

$$M = 7.5x - 5x^2/2$$

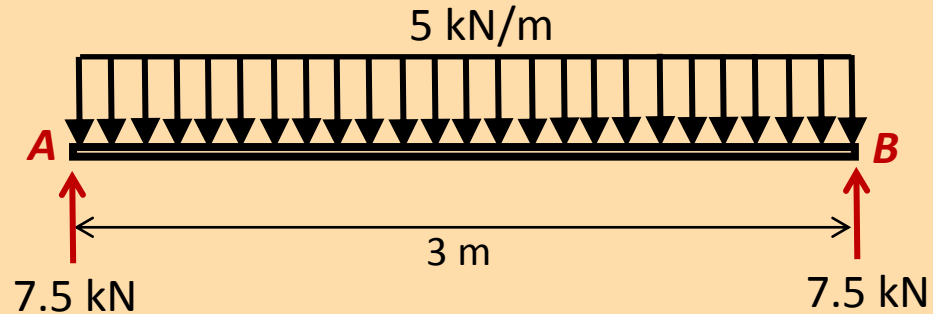
$$M = \text{maximum when } \frac{dM}{dx} = 0$$

$$\frac{dM}{dx} = 7.5 - 5x = 0$$

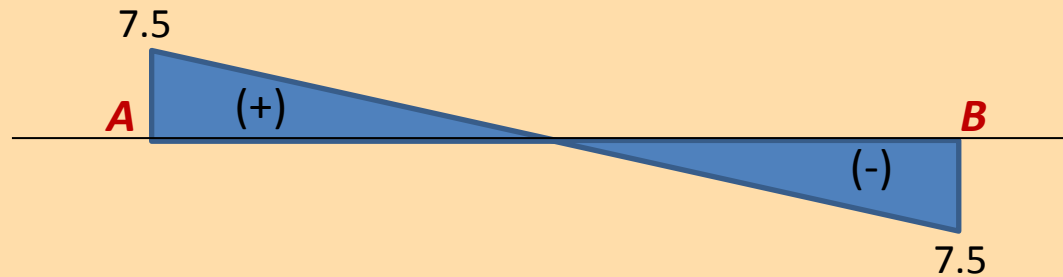
$$\therefore x = 1.5 \text{ m}$$

Therefore,  $M_{\max} = 5.625 \text{ kNm}$

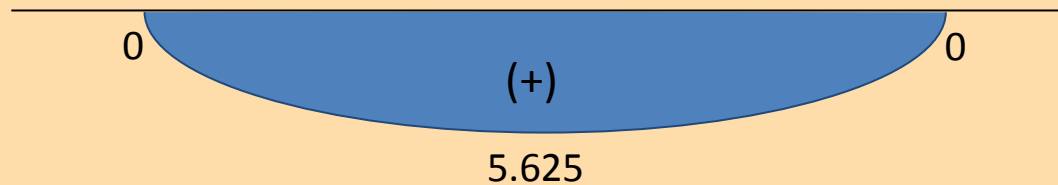
# EXAMPLE 6 – Solution



**SFD (kN)**



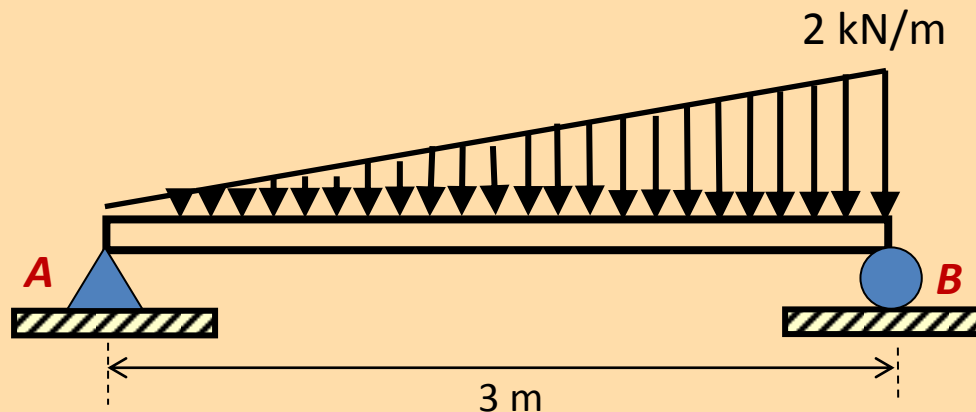
**BMD (kNm)**



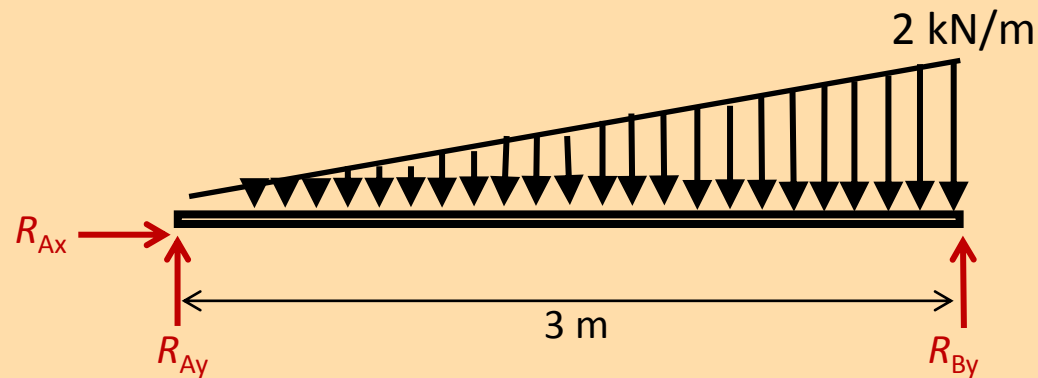


# EXAMPLE 7

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 7 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$2 \times 3/2 \times 3 \times 2/3 - R_{By} \times 3 = 0$$

$$R_{By} = 2 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} = 2 \times 3/2$$

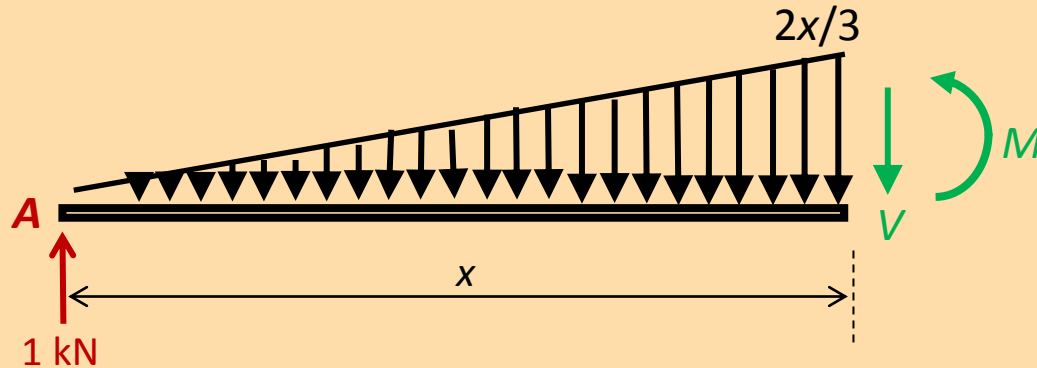
$$R_{Ay} = 3 - 2$$

$$R_{Ay} = 1 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

# EXAMPLE 7 – Solution



$$1 - 2x/3(x)(1/2) - V = 0$$

$$V = 1 - 2x^2/6$$

$$\text{If } x = 0, V = 1 \text{ kN and } x = 3, V = -2 \text{ kN}$$

$$-M + 1 \times x - 2x/3(x)(1/2)(x/3) = 0$$

$$M = x - x^3/9$$

$$M = \text{maximum when } \frac{dM}{dx} = 0$$

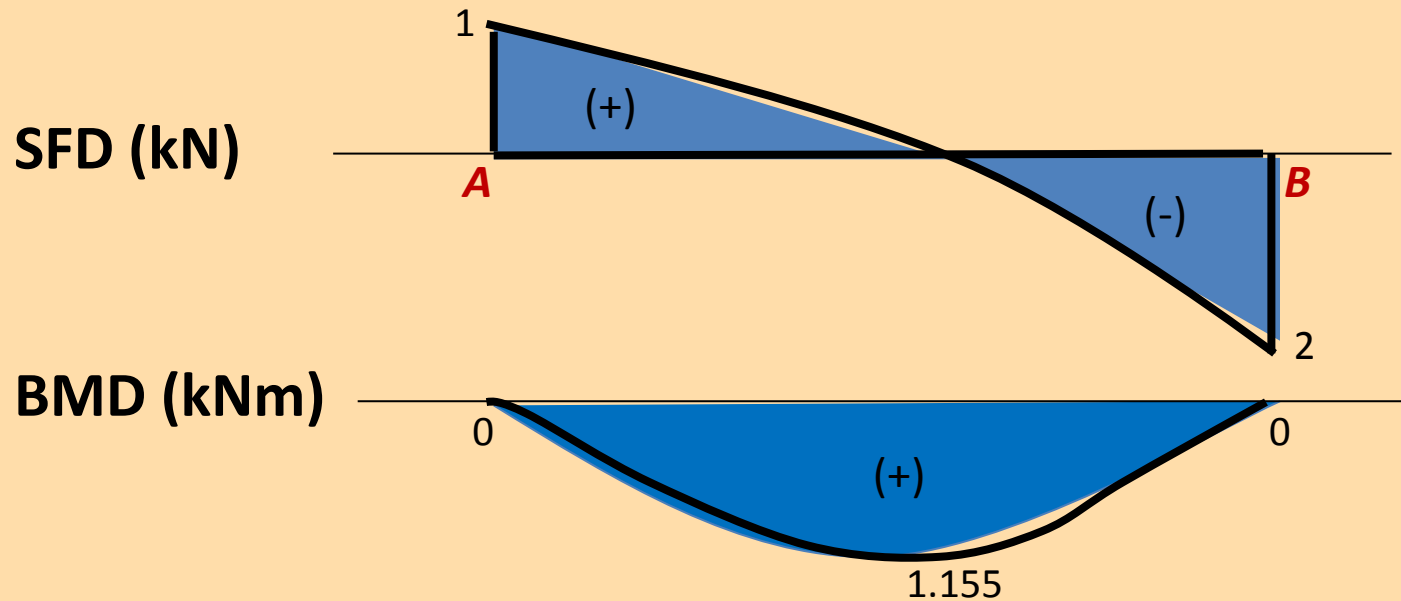
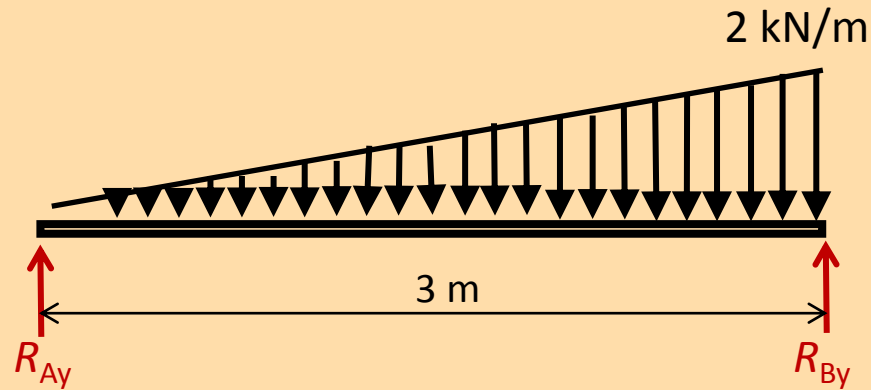
$$\frac{dM}{dx} = 1 - \frac{3x^2}{9} = 0$$

$$x^2 = \frac{9}{3}$$

$$x = \frac{3}{\sqrt{3}} = 1.732m$$

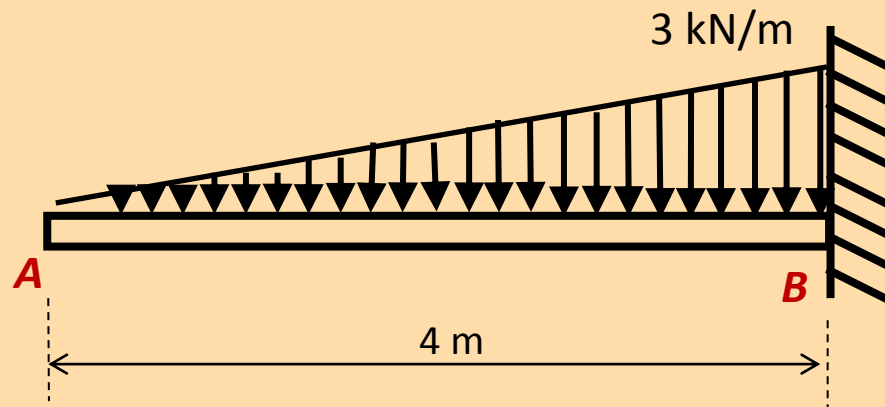
Therefore,  $M_{\max} = 1.155 \text{ kNm}$

# EXAMPLE 7 – Solution

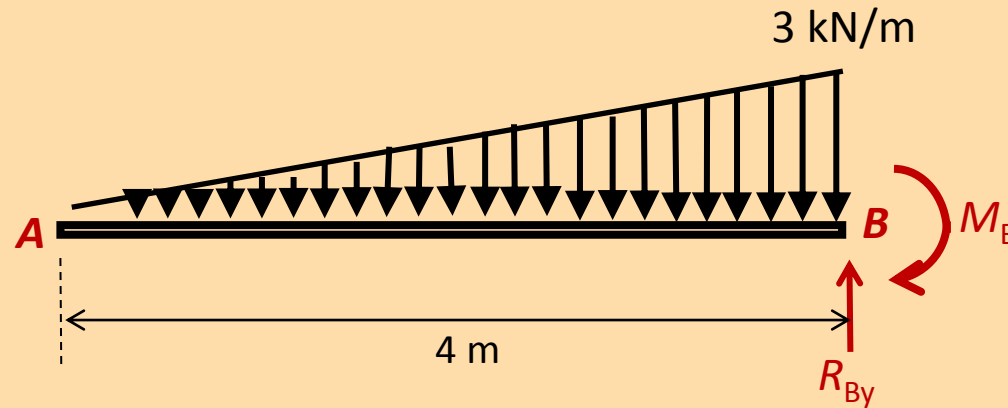


# EXAMPLE 8

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 8 – Solution



By taking the moment at B:

$$\Sigma M_B = 0$$

$$M_B = 3 \times 4/2 \times 4/3$$

$$M_B = 8 \text{ kNm}$$

$$\Sigma F_y = 0$$

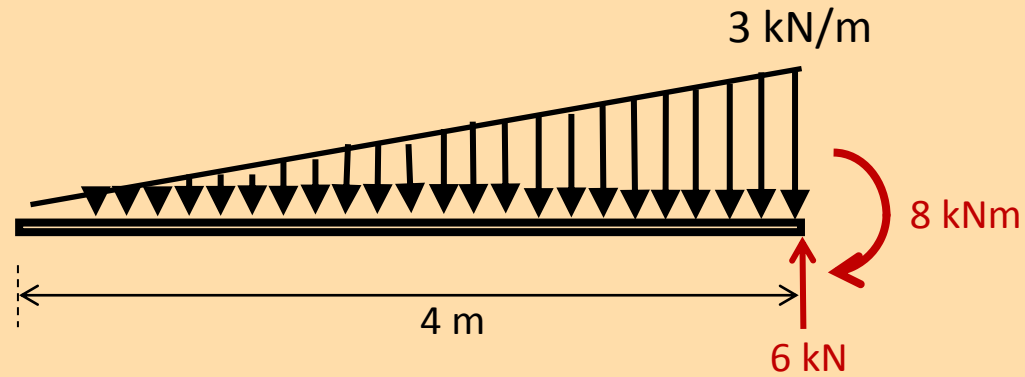
$$R_{By} = 3 \times 4/2$$

$$R_{By} = 6 \text{ kN}$$

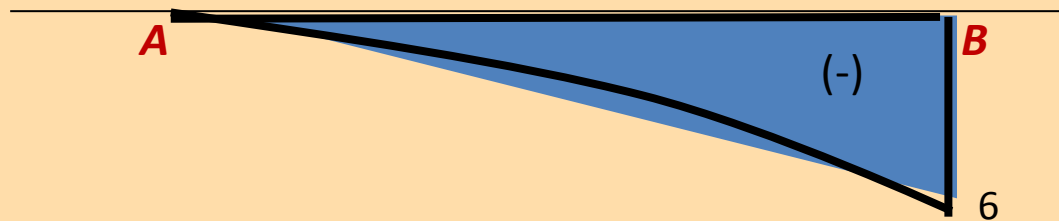
$$\Sigma F_x = 0$$

$$R_{Bx} = 0$$

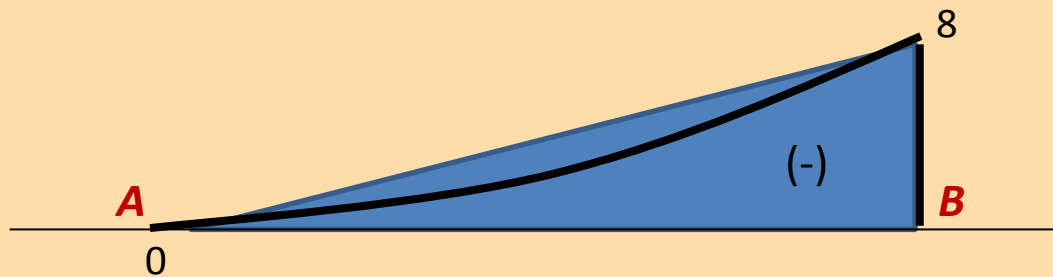
# EXAMPLE 8 – Solution



SFD (kN)

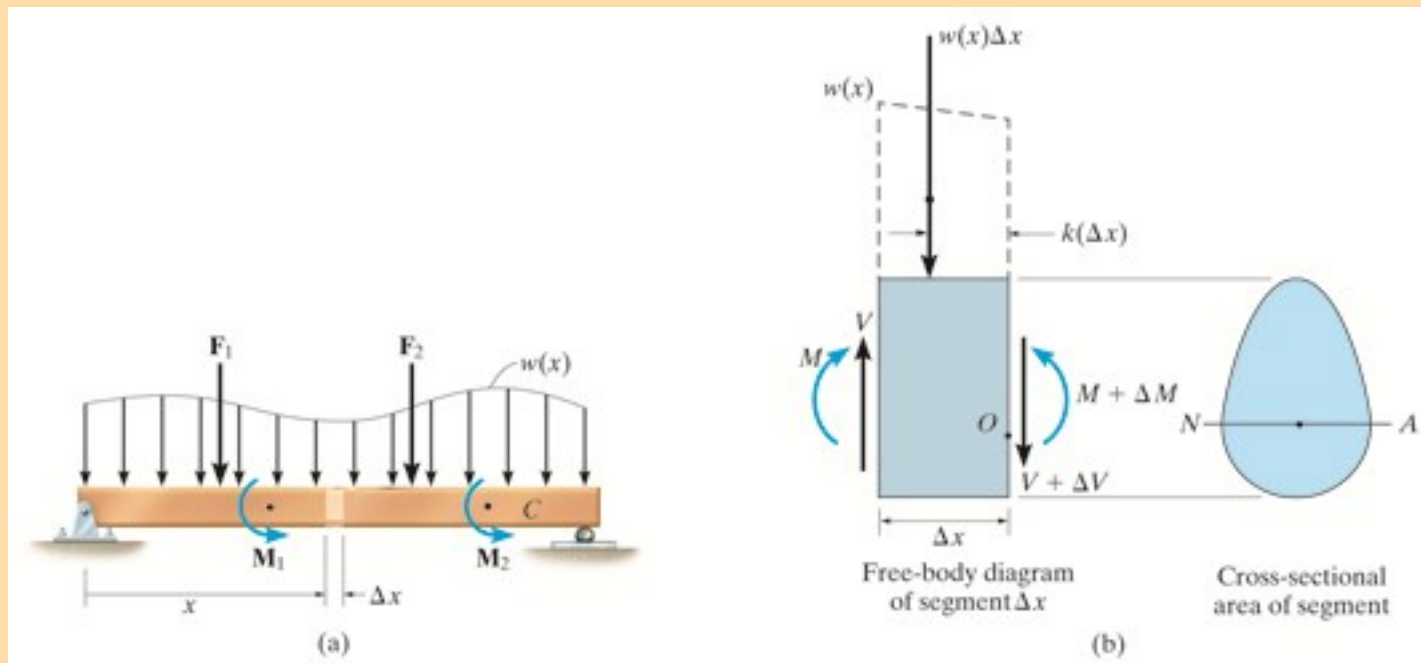


BMD (kNm)



# RELATIONSHIP BETWEEN LOAD, SHEAR FORCE & BENDING MOMENT

When a beam is subjected to two or more concentrated or distributed load, the way to calculate and draw the SFD and BMD may not be the same as in the previous situation.





# REGION OF DISTRIBUTED LOAD

$$\Sigma F_y = 0; \quad V - w(x)\Delta x - (V + \Delta V) = 0$$

$$\Delta V = w(x)\Delta x$$

$$\Sigma M_o = 0;$$

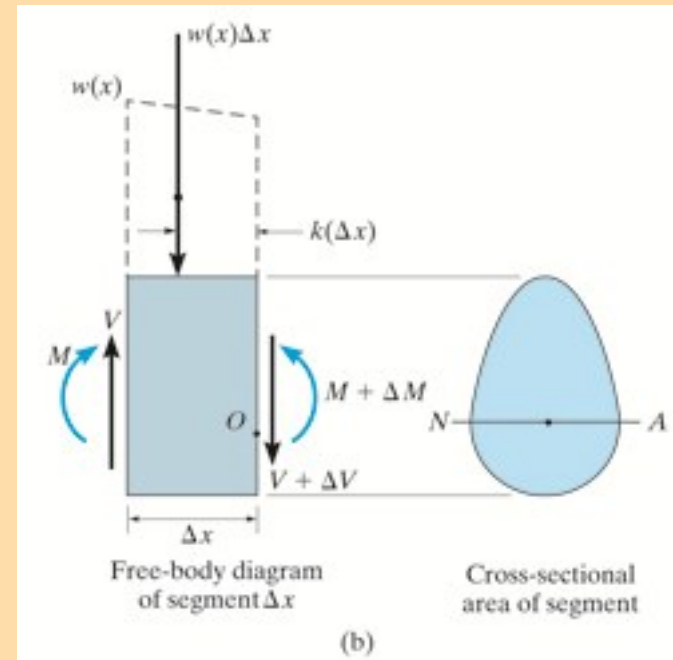
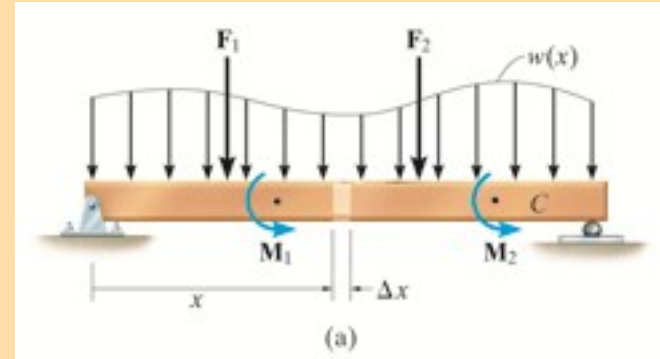
$$-V \Delta x - M + w(x)\Delta x[k \Delta x] + (M + \Delta M) = 0$$

$$\Delta M = V \Delta x - w(x)k\Delta x^2$$

Dividing by  $\Delta x$  and taking the limit as  $\Delta x = 0$ , the above two equations become:

Slope of the shear diagram at each point  $\frac{dV}{dx} = -w(x)$  distributed load intensity at each point

Slope of moment diagram at each point  $\frac{dM}{dx} = V$  Shear at each point



# REGION OF DISTRIBUTED LOAD

- We can integrate these areas between any two points to get change in shear and moment.

Change in shear

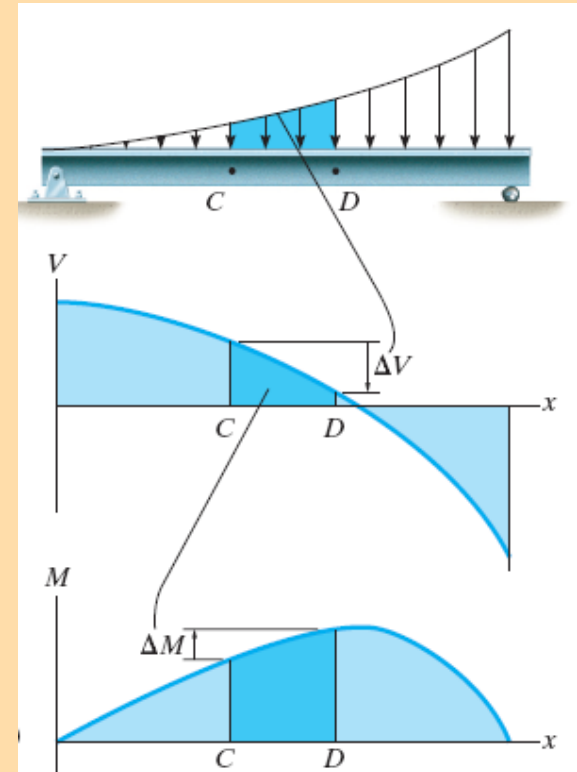
$$\Delta V = -\int w(x)dx$$

Area under distributed loading

Change in moment

$$\Delta M = \int V(x)dx$$

Area under shear diagram

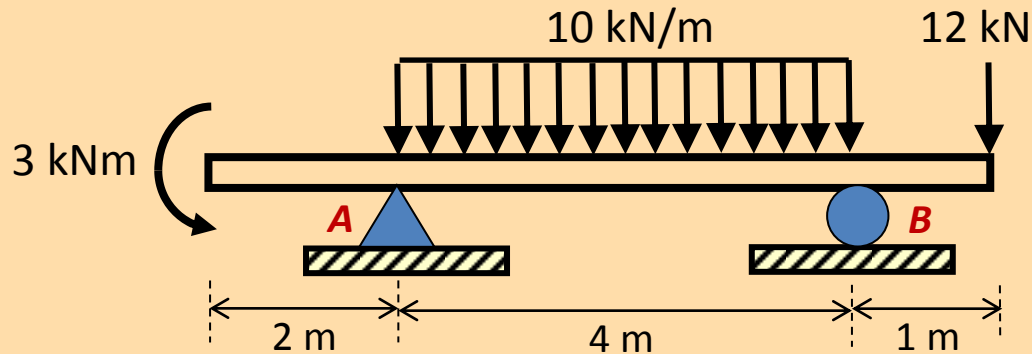


# USEFUL TIPS...

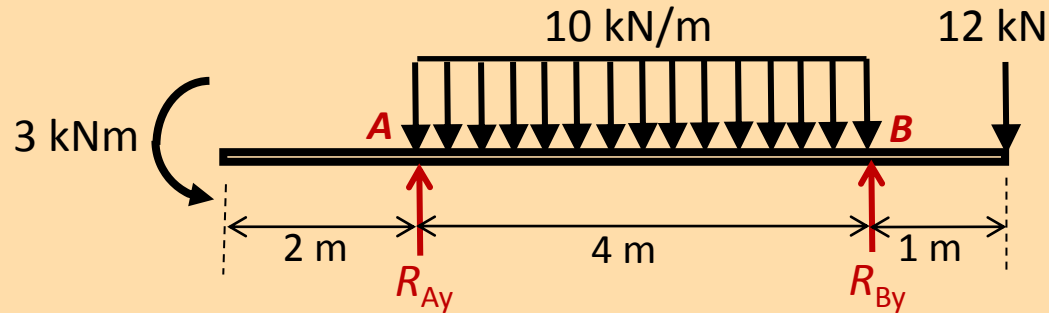
- Slope of bending moment always determined by the shape of shear force lines. The changes in slope (sagging or hogging also depends on the changes in shear force values)
- When shear force intersects BMD axis, there is a maximum moment
- When SF maximum, BM minimum and vice versa
- SFD and BMD always start and end with zero values (unless at the point where there is a moment/couple)
- When a moment/couple acting:
  - Clockwise ( $\downarrow$ ) (+), Anticlockwise ( $\uparrow$ ) (-)

# EXAMPLE 9

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 9 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$-R_{By} \times 4 - 3 + 10 \times 4 \times 4/2 + 12 \times 5 = 0$$

$$R_{By} = 34.25 \text{ kN}$$

$$\Sigma F_y = 0$$

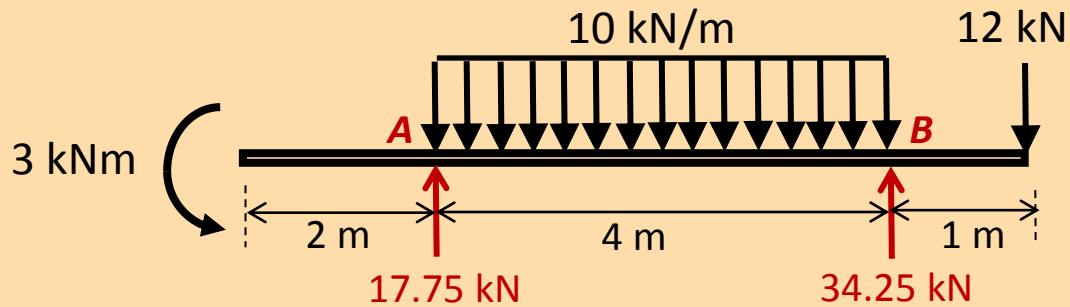
$$R_{Ay} + R_{By} = 10 \times 4 + 12$$

$$R_{Ay} = 17.75 \text{ kN}$$

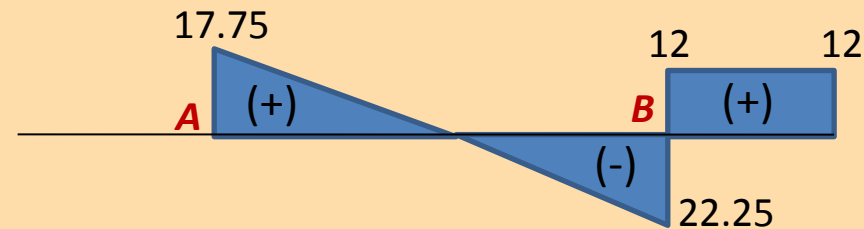
$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

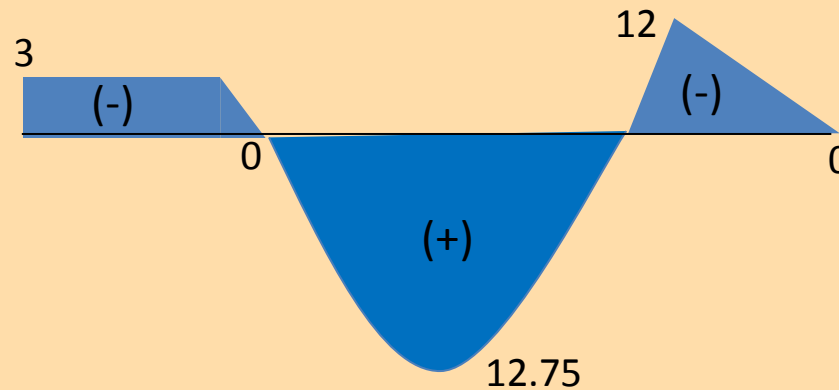
# EXAMPLE 9 – Solution



**SFD (kN)**

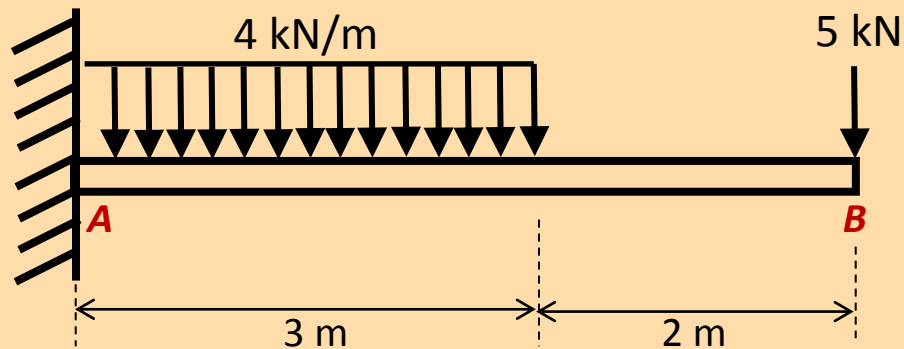


**BMD (kNm)**

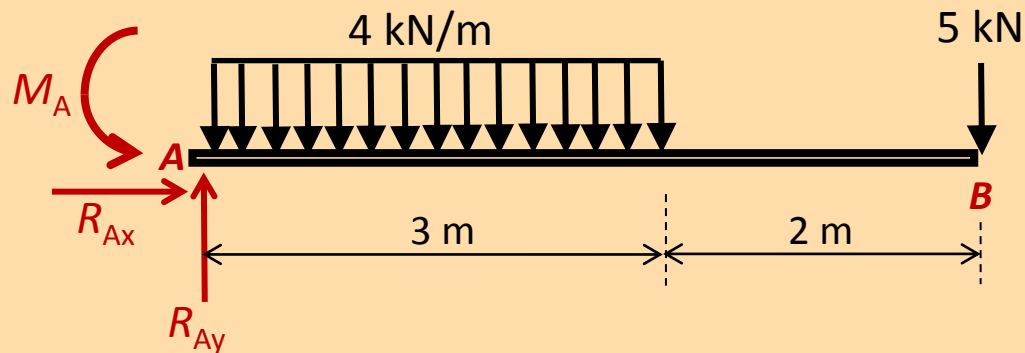


# EXAMPLE 10

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 10 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$-M_A + 4 \times 3 \times 3/2 + 5 \times 5 = 0$$

$$M_A = 43 \text{ kNm}$$

$$\Sigma F_y = 0$$

$$R_{Ay} = 4 \times 3 + 5$$

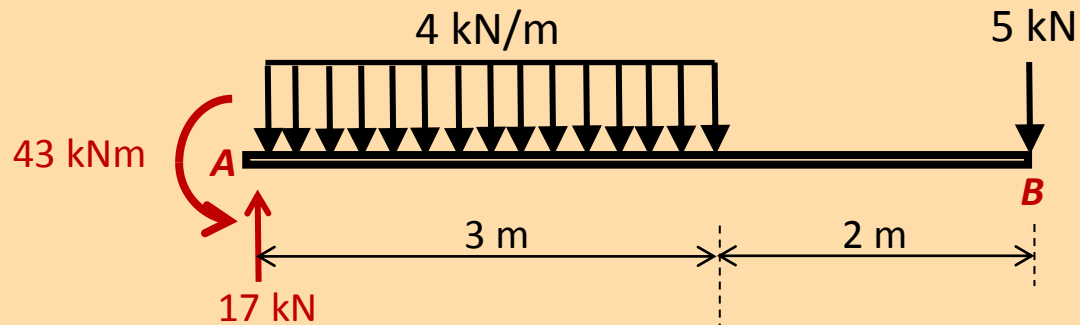
$$R_{Ay} = 17 \text{ kN}$$

$$\Sigma F_x = 0$$

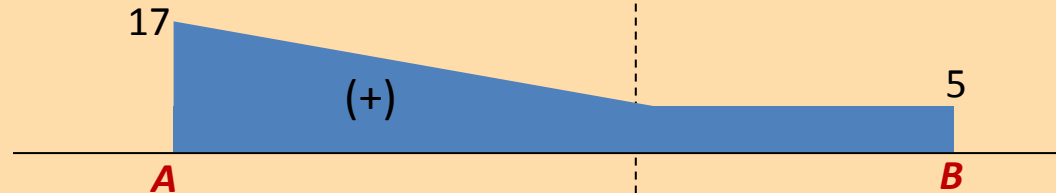
$$R_{Ax} = 0$$



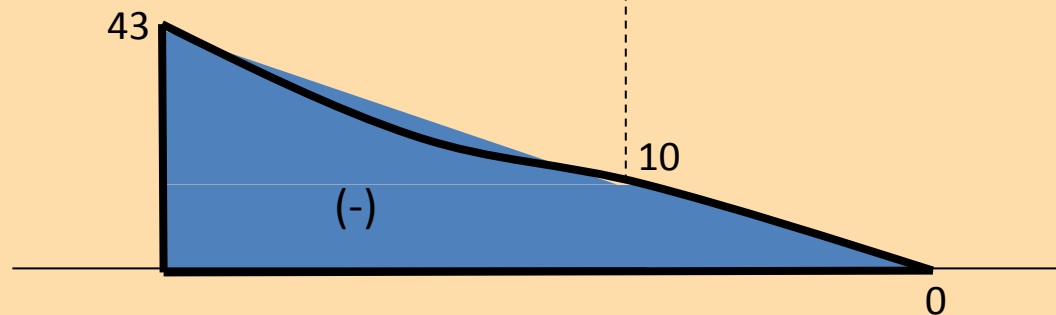
# EXAMPLE 10 – Solution



SFD (kN)

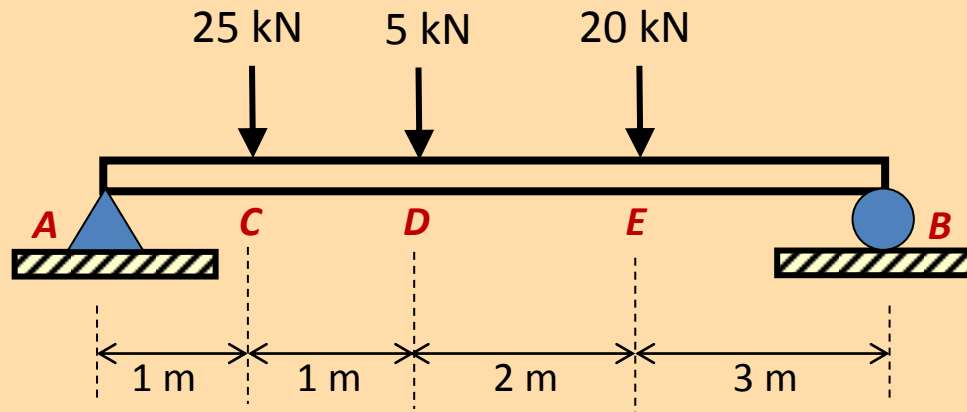


BMD (kNm)

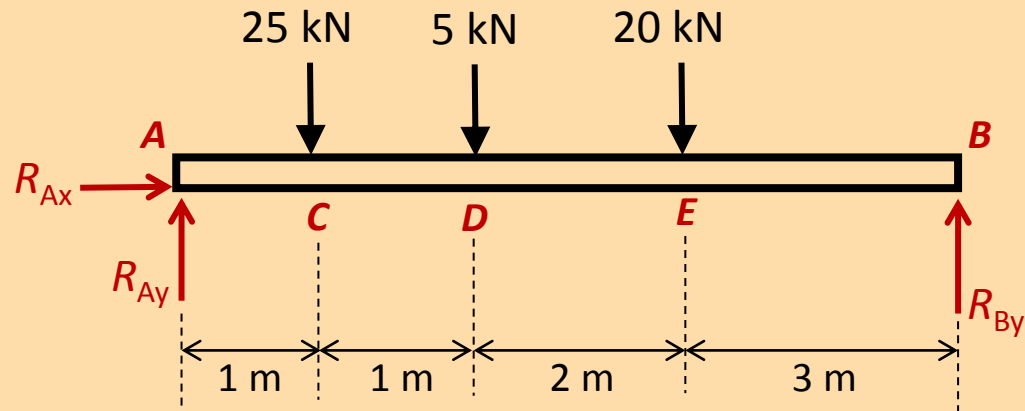


# EXAMPLE 11

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 11 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$25 \times 1 + 5 \times 2 + 20 \times 4 - R_{By} \times 7 = 0$$

$$R_{By} = 16.43 \text{ kN}$$

$$\Sigma F_y = 0$$

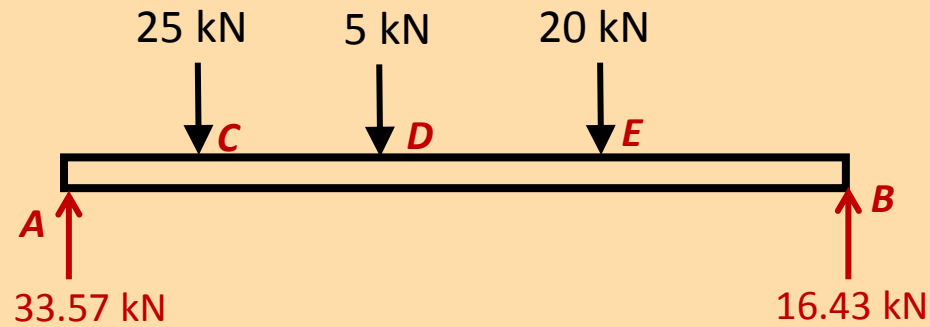
$$R_{Ay} + R_{By} = 25 + 5 + 20$$

$$R_{Ay} = 33.57 \text{ kN}$$

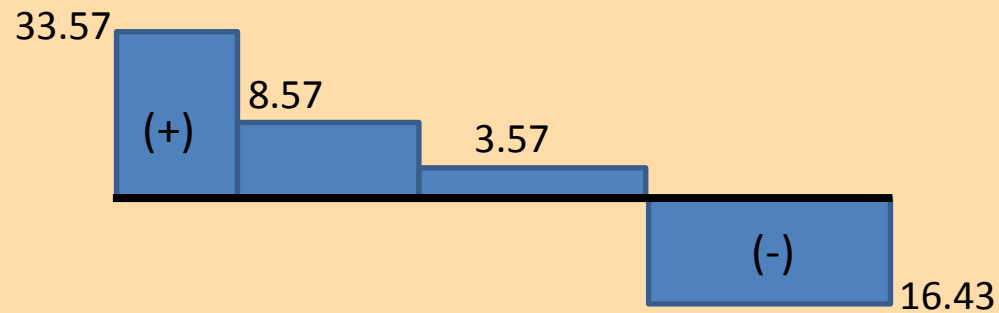
$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

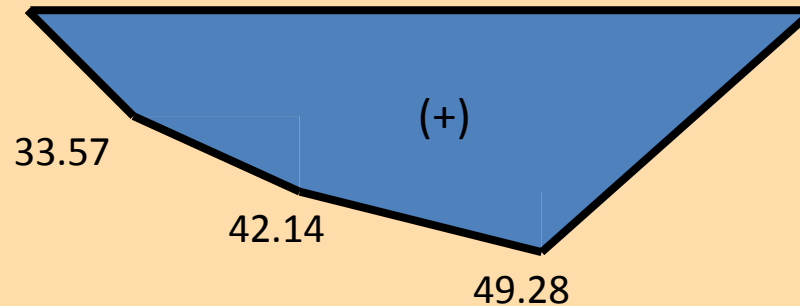
# EXAMPLE 11 – Solution



**SFD (kN)**

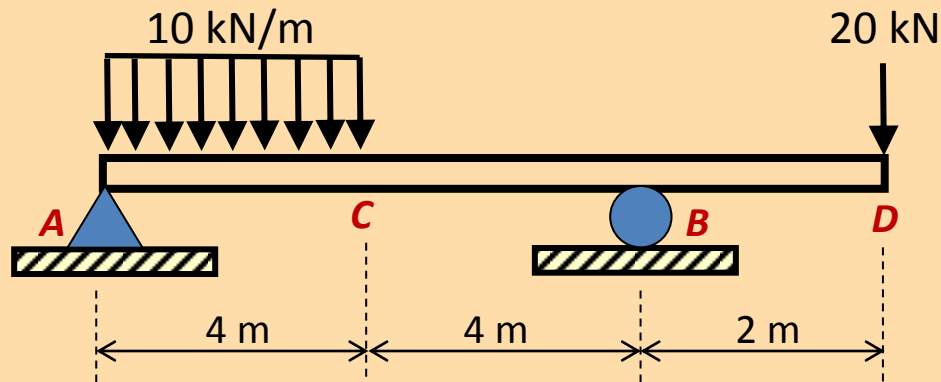


**BMD (kNm)**

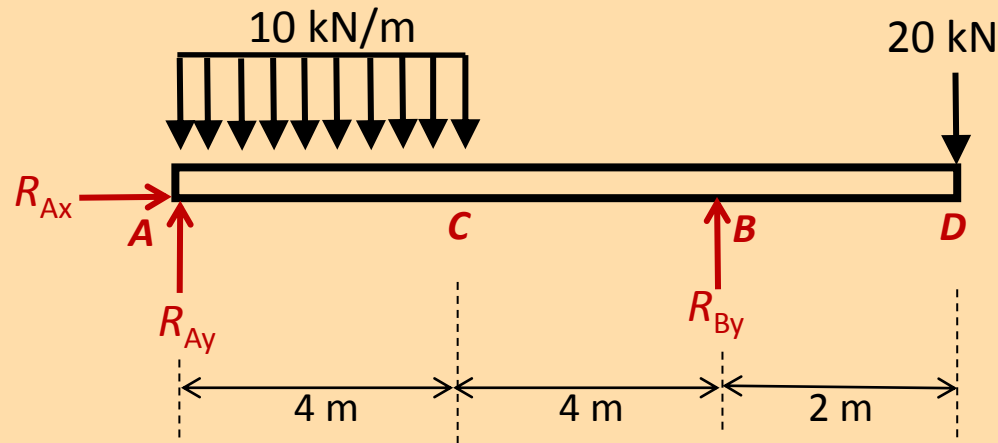


# EXAMPLE 12

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 12 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$10 \times 4 \times 2 + 20 \times 10 - R_{By} \times 8 = 0$$

$$R_{By} = 35 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} = 10 \times 4 + 20$$

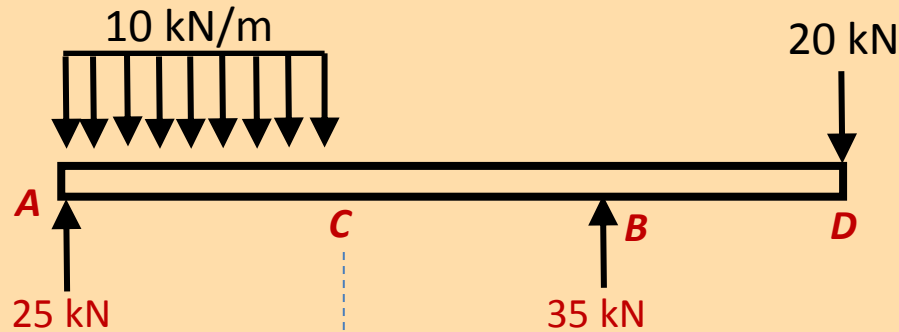
$$R_{Ay} = 60 - 35$$

$$R_{Ay} = 25 \text{ kN}$$

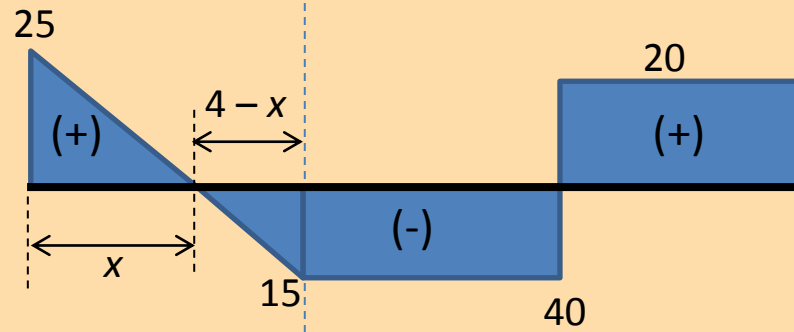
$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

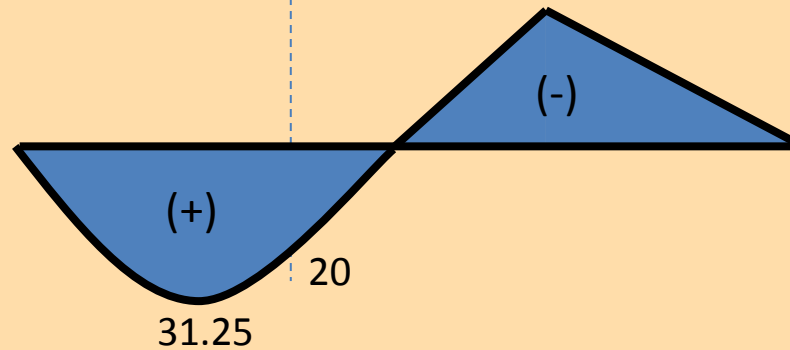
# EXAMPLE 12 – Solution



**SFD (kN)**



**BMD (kNm)**



$$\frac{x}{25} = \frac{4-x}{15}$$

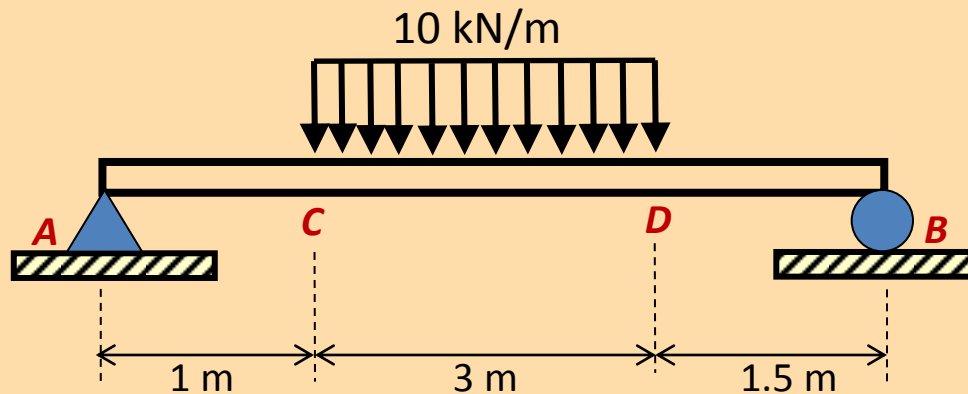
$$15x = 100 - 25x$$

$$40x = 100$$

$$x = 2.5$$

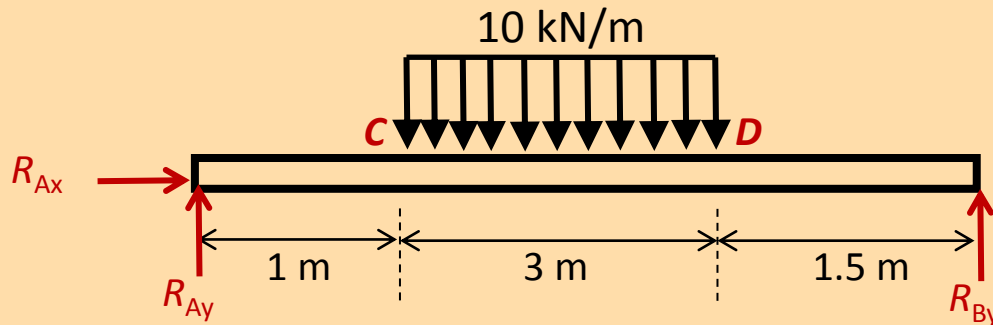
# EXAMPLE 13

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).





# EXAMPLE 13 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$10 \times 3 \times 2.5 - R_{By} \times 5.5 = 0$$

$$R_{By} = 13.64 \text{ kN}$$

$$\Sigma F_y = 0$$

$$R_{Ay} + R_{By} = 10 \times 3$$

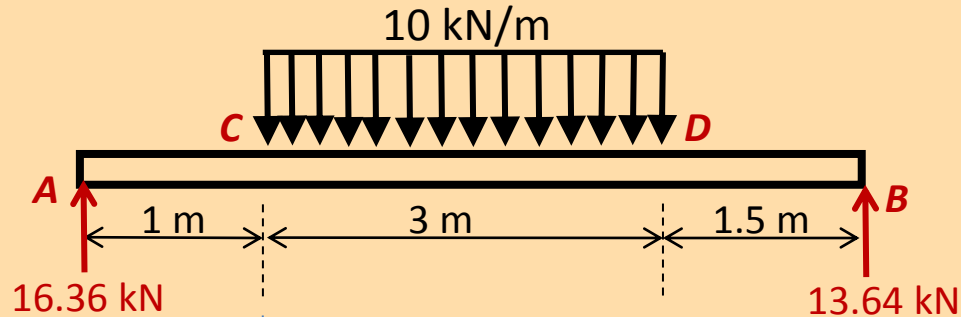
$$R_{Ay} = 30 - 13.64$$

$$R_{Ay} = 16.36 \text{ kN}$$

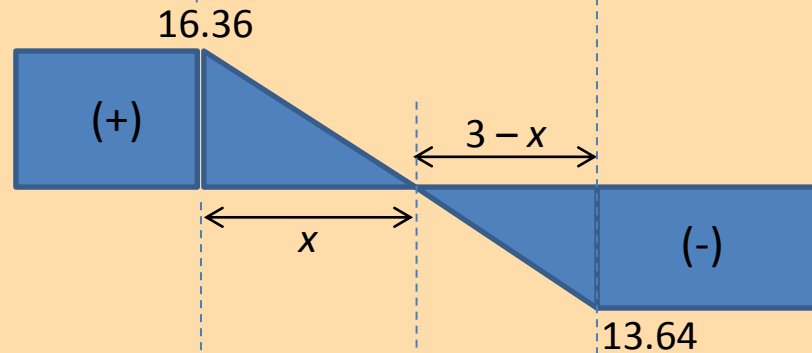
$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

# EXAMPLE 13 – Solution



**SFD (kN)**



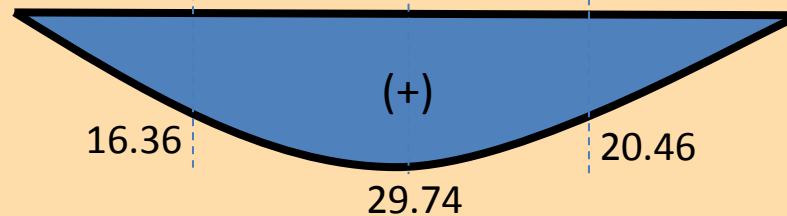
$$\frac{x}{16.36} = \frac{3-x}{13.64}$$

$$13.64x = 49.08 - 16.36x$$

$$30x = 49.08$$

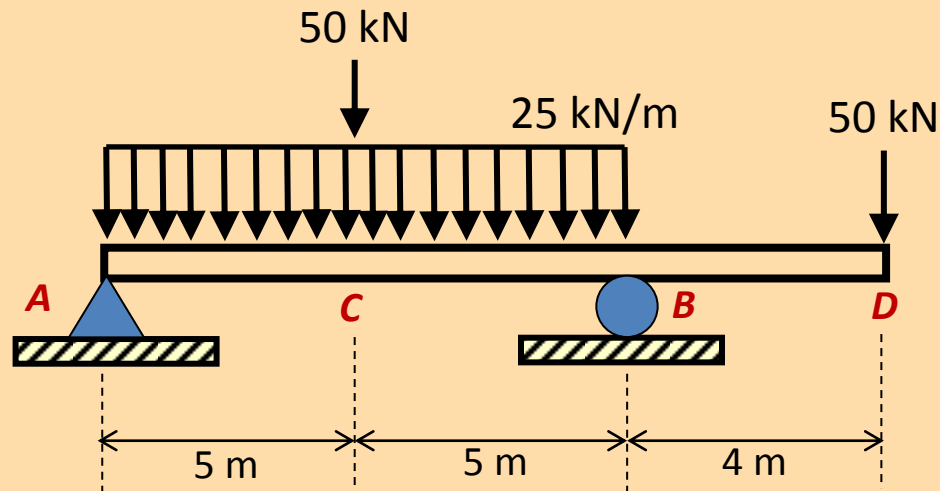
$$x = 1.636$$

**BMD (kNm)**

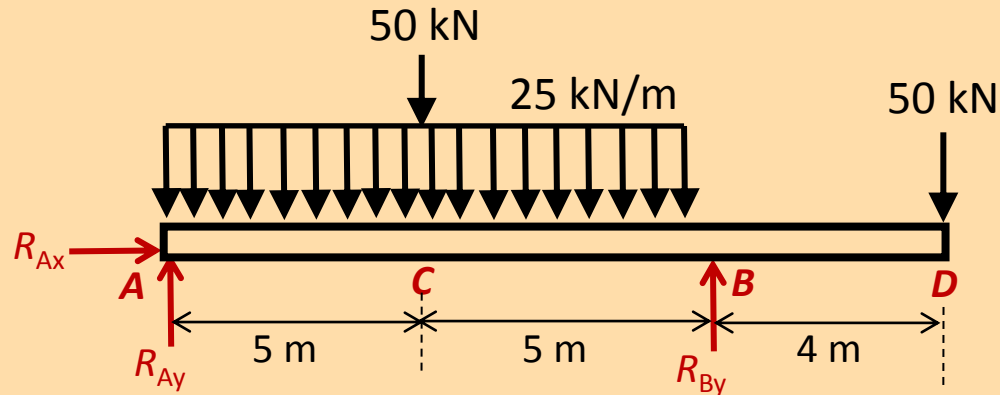


# EXAMPLE 14

Calculate the shear force and bending moment for the beam subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 14 – Solution



By taking the moment at A:

$$\Sigma M_A = 0$$

$$25 \times 10 \times 5 + 50 \times 5 + 50 \times 14 - R_{By} \times 10 = 0$$

$$R_{By} = 220 \text{ kN}$$

$$\Sigma F_y = 0$$

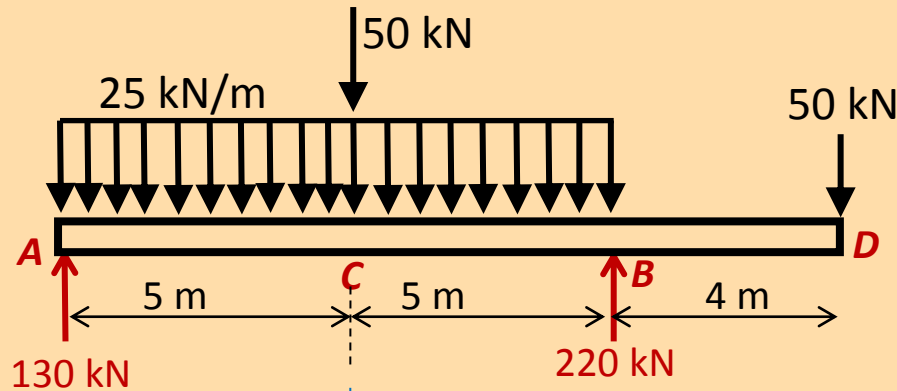
$$R_{Ay} + R_{By} = 25 \times 10 + 50 + 50$$

$$R_{Ay} = 130 \text{ kN}$$

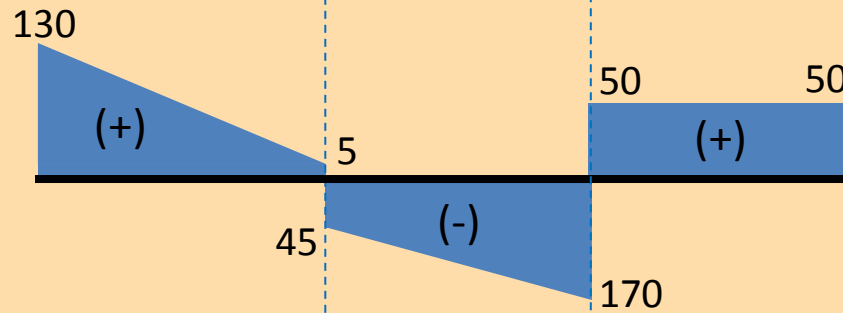
$$\Sigma F_x = 0$$

$$R_{Ax} = 0$$

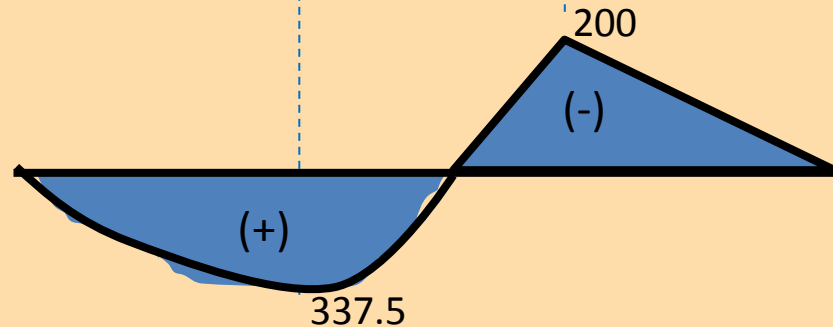
# EXAMPLE 14 – Solution



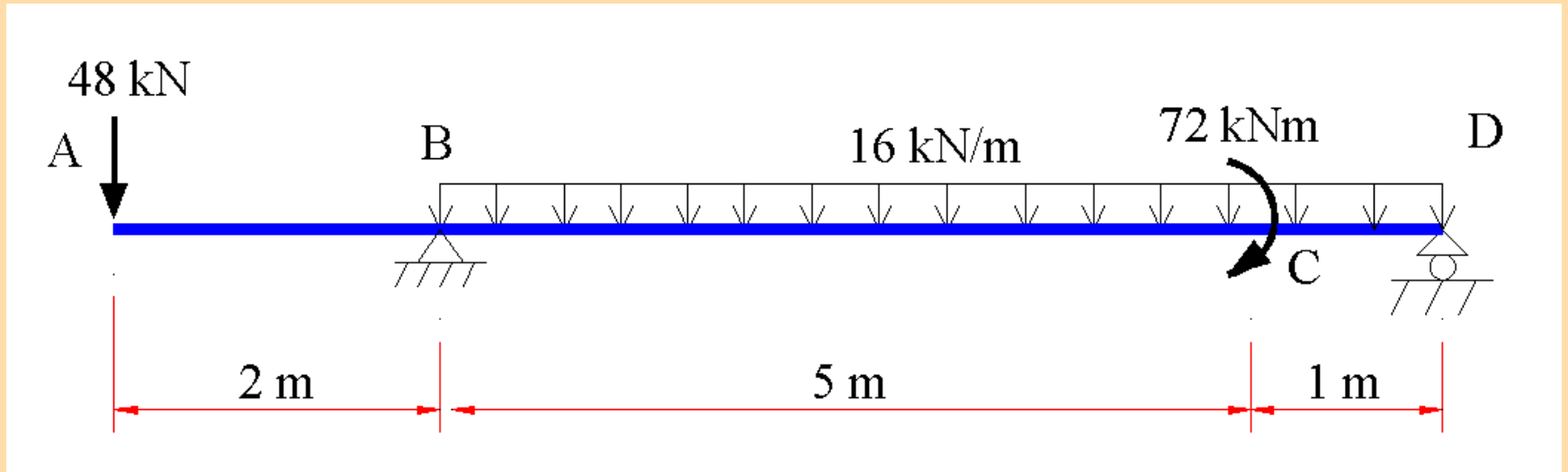
**SFD (kN)**



**BMD (kNm)**



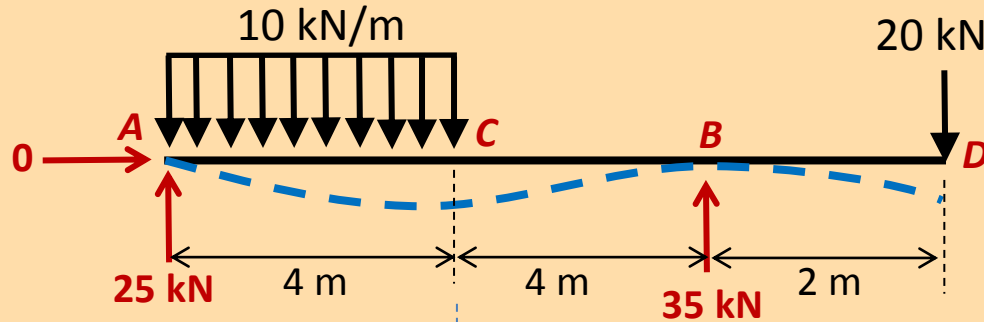
# CLASS EXERCISE



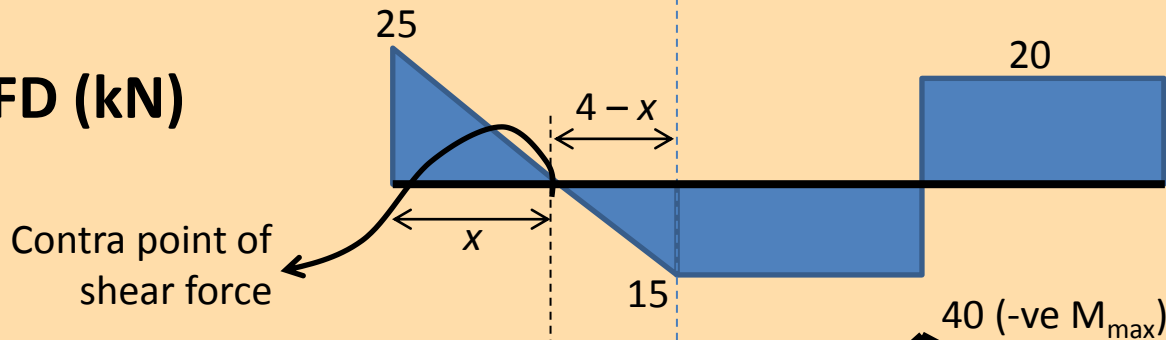
# CONTRA POINT OF SHEAR FORCE & BENDING MOMENT

- Contra point is a place where positive shear force/bending moment shifting to the negative region or vice-versa.
- Contra point for shear:  $V = 0$
- Contra point for moment:  $M = 0$
- When shear force is *zero*, the moment is *maximum*.
- *Maximum shear force* usually occur at the *support / concentrated load*.

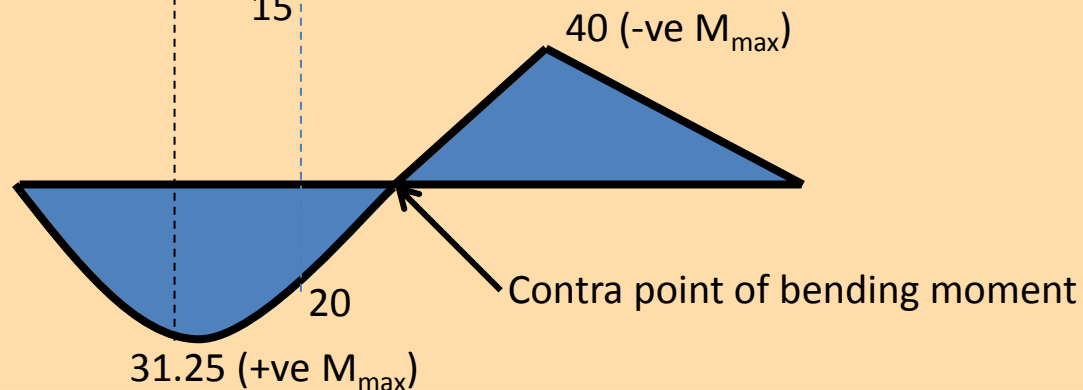
# CONTRA POINT OF SHEAR FORCE & BENDING MOMENT



**SFD (kN)**



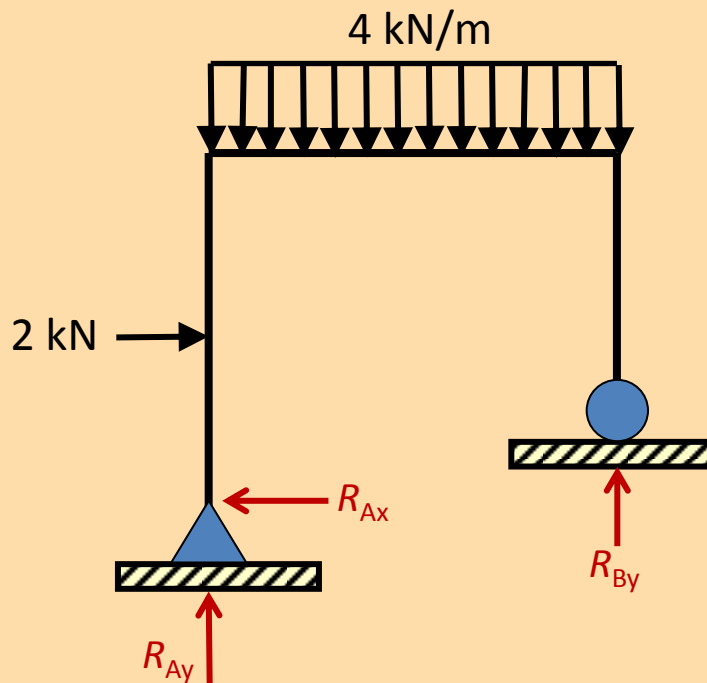
**BMD (kNm)**





# STATICALLY DETERMINATE FRAMES

- For a frame to be statically determinate, the number of unknown (reactions) must be able to be solved using the equations of equilibrium.



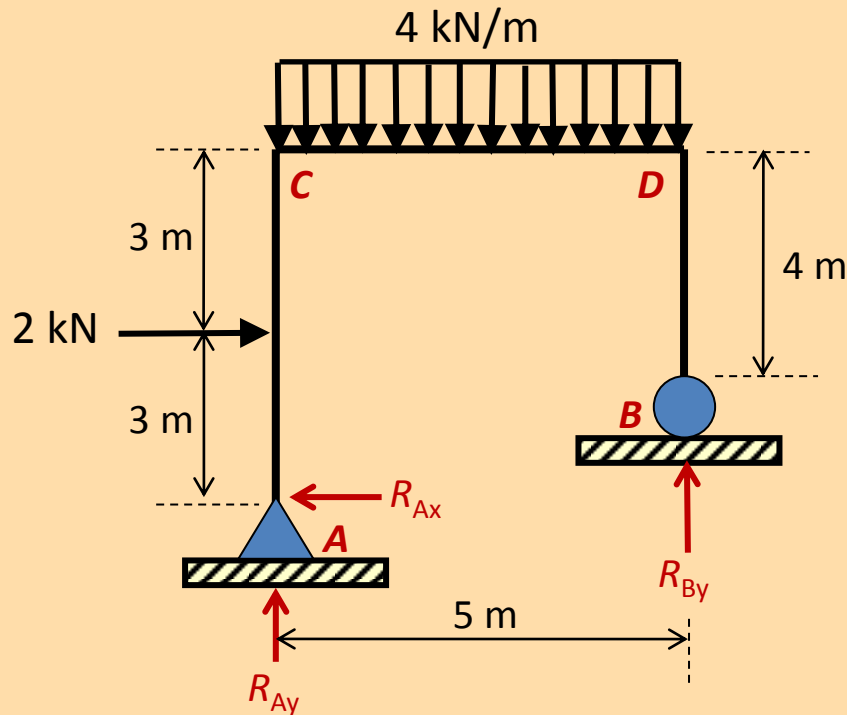
$$\Sigma M_A = 0$$

$$\Sigma F_y = 0$$

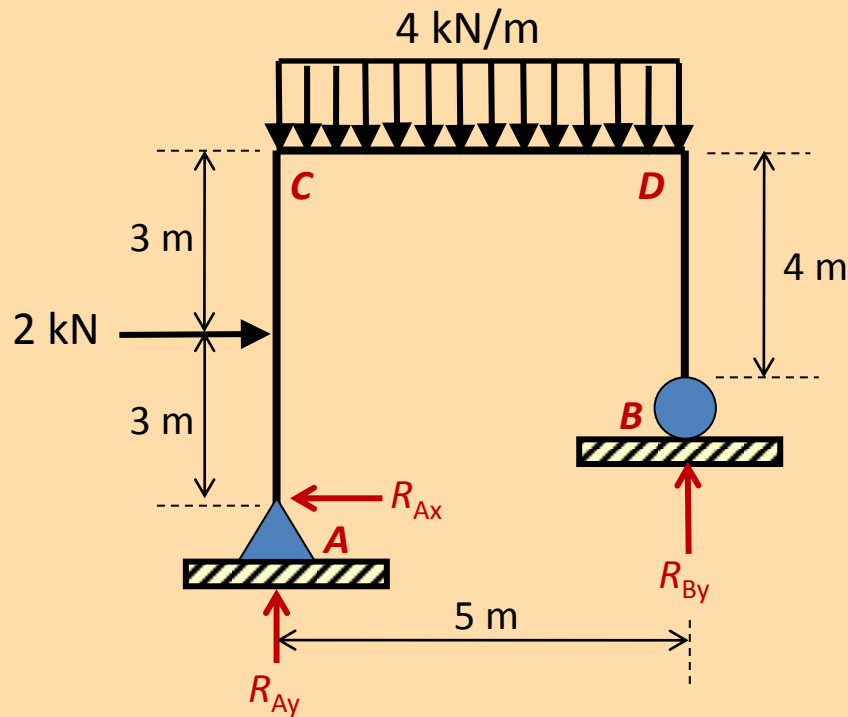
$$\Sigma F_x = 0$$

# EXAMPLE 15

Calculate the shear force and bending moment for the frame subjected to the loads as shown in the figure, then draw the shear force diagram (SFD) and bending moment diagram (BMD).



# EXAMPLE 15 – Solution



$$\Sigma M_A = 0$$

$$4 \times 5 \times 2.5 + 2 \times 3 - R_{By} \times 5 = 0$$

$$R_{By} = 11.2 \text{ kN}$$

$$\Sigma F_y = 0$$

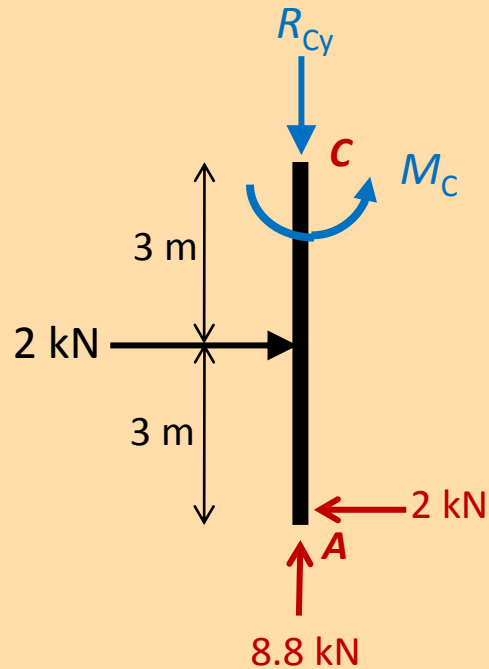
$$R_{Ay} + R_{By} = 4 \times 5$$

$$R_{Ay} = 8.8 \text{ kN}$$

$$\Sigma F_x = 0$$

$$R_{Ax} = 2 \text{ kN}$$

# EXAMPLE 15 – Solution



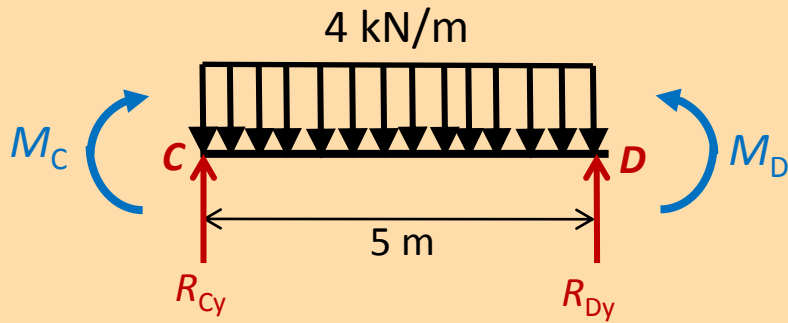
$$\Sigma M_A = 0: 2 \times 3 - M_C = 0$$

$$\therefore M_C = 6 \text{ kNm}$$

$$\Sigma F_y = 0$$

$$\therefore R_{cy} = 8.8 \text{ kN}$$

# EXAMPLE 15 – Solution



$$\Sigma F_y = 0: R_{Cy} + R_{Dy} = 4 \times 5$$

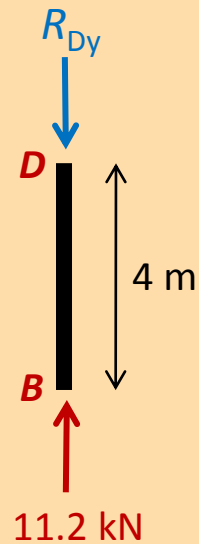
$$R_{Dy} = 20 - 8.8 = 11.2 \text{ kN}$$

$$\Sigma M_C = 0:$$

$$M_C + 4 \times 5 \times 2.5 - R_{Dy} \times 5 - M_D = 0$$

$$M_D = 0 \text{ kNm}$$

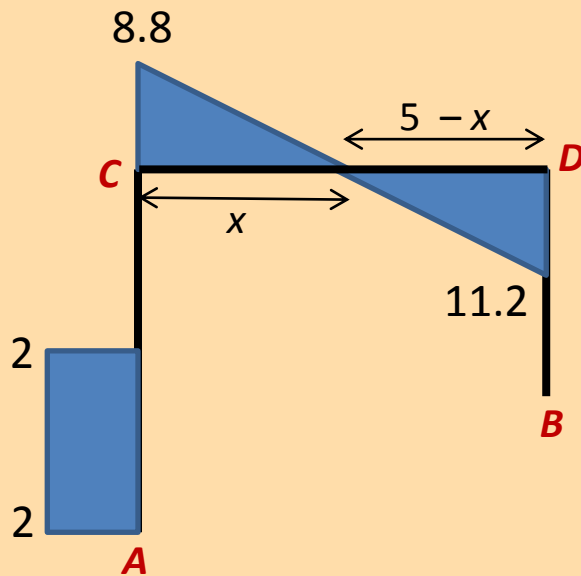
# EXAMPLE 15 – Solution



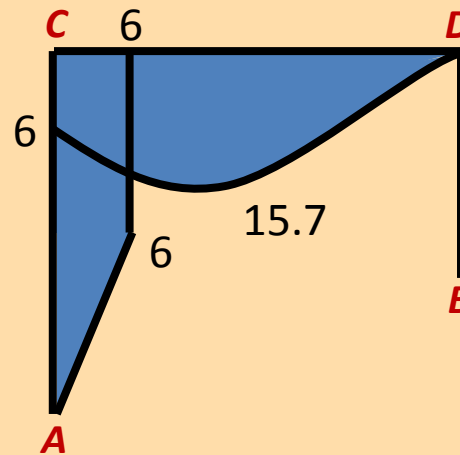
$$\Sigma F_y = 0:$$

$$R_{Dy} = 11.2 \text{ kN}$$

# EXAMPLE 15 – Solution



SFD (kN)



BMD (kNm)

$$\frac{x}{8.8} = \frac{5-x}{11.2}$$

$$11.2x = 44 - 8.8x$$

$$20x = 44$$

$$x = 2.2$$

$$M_{\max} = 8.8 \times 2.2 \times 0.5 + 6 = 15.7 \text{ kNm}$$



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