## Chapter 6 • Viscous Flow in Ducts

P6.1 An engineer claims that flow of SAE 30W oil, at $20^{\circ} \mathrm{C}$, through a 5-cm-diameter smooth pipe at 1 million N/h, is laminar. Do you agree? A million newtons is a lot, so this sounds like an awfully high flow rate.

Solution: For SAE 30 W oil at $20^{\circ} \mathrm{C}$ (Table A.3), take $\rho=891 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.29 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. Convert the weight flow rate to volume flow rate in SI units:

$$
\begin{aligned}
& Q=\frac{\dot{w}}{\rho g}=\frac{(1 E 6 \mathrm{~N} / \mathrm{h})(1 / 3600 \mathrm{~h} / \mathrm{s})}{\left(891 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}=0.0318 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}=\frac{\pi}{4}(0.05 \mathrm{~m})^{2} V \text {, solve } V=16.2 \frac{\mathrm{~m}}{\mathrm{~s}} \\
& \text { Calculate } \operatorname{Re}_{D}=\frac{\rho V D}{\mu}=\frac{\left(891 \mathrm{~kg} / \mathrm{m}^{3}\right)(16.2 \mathrm{~m} / \mathrm{s})(0.05 \mathrm{~m})}{0.29 \mathrm{~kg} / \mathrm{m}-\mathrm{s}} \approx 2500 \quad \text { (transitional) }
\end{aligned}
$$

This is not high, but not laminar. Ans. With careful inlet design, low disturbances, and a very smooth wall, it might still be laminar, but No, this is transitional, not definitely laminar.
6.2 Air at approximately 1 atm flows through a horizontal 4-cm-diameter pipe. (a) Find a formula for $Q \max$, the maximum volume flow for which the flow remains laminar, and plot $Q$ max versus temperature in the range $0^{\circ} \mathrm{C} \leq \mathrm{T} \leq 500^{\circ} \mathrm{C}$. (b) Is your plot linear? If not, explain.

Solution: (a) First convert the Reynolds number from a velocity form to a volume flow form:

$$
V=\frac{Q}{(\pi / 4) d^{2}}, \text { therefore } \operatorname{Re}_{d}=\frac{\rho V d}{\mu}=\frac{4 \rho Q}{\pi \mu d} \leq 2300 \text { for laminar flow }
$$

Maximum laminar volume flow is given by $\quad \mathbf{Q}_{\max }=\frac{2300 \pi d \mu}{4 \rho} \quad$ Ans. (a)
With $d=0.04 \mathrm{~m}=$ constant, get $\mu$ and $\rho$ for air from Table A-2 and plot $Q \max$ versus $T^{\circ} \mathrm{C}$ :


Fig. P6.2
The curve is not quite linear because $v=\mu / \rho$ is not quite linear with $T$ for air in this range. Ans. (b)
6.3 For a thin wing moving parallel to its chord line, transition to a turbulent boundary layer occurs at a "local" Reynolds number Rex, where $x$ is the distance from the leading edge of the wing. The critical Reynolds number depends upon the intensity of turbulent fluctuations in the stream and equals 2.8E6 if the stream is very quiet. A semiempirical correlation for this case [Ref. 3 of Ch. 6] is

$$
\operatorname{Re}_{x_{\text {crit }}}^{1 / 2} \approx \frac{-1+\left(1+13.25 \zeta^{2}\right)^{1 / 2}}{0.00392 \zeta^{2}}
$$

where $\zeta$ is the tunnel-turbulence intensity in percent. If $V=20 \mathrm{~m} / \mathrm{s}$ in air at $20^{\circ} \mathrm{C}$, use this formula to plot the transition position on the wing versus stream turbulence for $\zeta$ between 0 and 2 percent. At what value of $\zeta$ is $x$ crit decreased 50 percent from its value at $\zeta=0$ ?

Solution: This problem is merely to illustrate the strong effect of stream turbulence on the transition point. For air at $20^{\circ} \mathrm{C}$, take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Compute $\mathrm{Rex}_{\mathrm{x}, \text { crit }}$ from the correlation and plot $\mathrm{xtr}=\mu \mathrm{Rex}_{\mathrm{x}} /[\rho(20 \mathrm{~m} / \mathrm{s})]$ versus percent turbulence:


Fig. P6.3
The value of xcrit decreases by half (to 1.07 meters) at $\zeta \approx \mathbf{0 . 4 2 \%}$. Ans.
6.4 For flow of SAE 30 oil through a 5-cm-diameter pipe, from Fig. A.1, for what flow rate in $\mathrm{m}^{3} / \mathrm{h}$ would we expect transition to turbulence at (a) $20^{\circ} \mathrm{C}$ and (b) $100^{\circ} \mathrm{C}$ ?

Solution: For SAE 30 oil take $\rho=891 \mathrm{~kg} / \mathrm{m}^{3}$ and take $\mu=0.29 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$ at $20^{\circ} \mathrm{C}$ (Table A.3) and $0.01 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$ at $100^{\circ} \mathrm{C}$ (Fig A.1). Write the critical Reynolds number in terms of flow rate $Q$ :
(a) $\operatorname{Re}_{\text {crit }}=2300=\frac{\rho V D}{\mu}=\frac{4 \rho Q}{\pi \mu D}=\frac{4\left(891 \mathrm{~kg} / \mathrm{m}^{3}\right) Q}{\pi(0.29 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s})(0.05 \mathrm{~m})}$,
solve $Q=0.0293 \frac{m^{3}}{\mathrm{~s}}=\mathbf{1 0 6} \frac{\mathbf{m}^{3}}{\mathbf{h}}$ Ans. (a)
(b) $\operatorname{Re}_{\text {crit }}=2300=\frac{\rho V D}{\mu}=\frac{4 \rho Q}{\pi \mu D}=\frac{4\left(891 \mathrm{~kg} / \mathrm{m}^{3}\right) Q}{\pi(0.010 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s})(0.05 \mathrm{~m})}$,
solve $Q=0.00101 \frac{m^{3}}{s}=3.6 \frac{\mathbf{m}^{3}}{\mathbf{h}}$ Ans. (b)
6.5 In flow past a body or wall, early transition to turbulence can be induced by placing a trip wire on the wall across the flow, as in Fig. P6.5. If the trip wire in Fig. P6.5 is placed where the local velocity is $U$, it will trigger turbulence if $U d / v=850$, where $d$ is the wire diameter [Ref. 3 of Ch. 6]. If the sphere diameter is 20 cm and transition is observed at $\operatorname{Re} D=90,000$, what is the


Fig. P6.5 diameter of the trip wire in mm ?

Solution: For the same U and $v$,

$$
\begin{aligned}
\operatorname{Re}_{\mathrm{d}} & =\frac{\mathrm{Ud}}{v}=850 ; \quad \operatorname{Re}_{\mathrm{D}}=\frac{\mathrm{UD}}{v}=90000, \\
\text { or } \mathrm{d} & =\mathrm{D} \frac{\operatorname{Re}_{\mathrm{d}}}{\operatorname{Re}_{\mathrm{D}}}=(200 \mathrm{~mm})\left(\frac{850}{90000}\right) \approx \mathbf{1 . 9} \mathrm{mm}
\end{aligned}
$$

P6.6 For flow of a uniform stream parallel to a sharp flat plate, transition to a turbulent boundary layer on the plate may occur at $\mathrm{Re}_{x}=\rho U x / \mu \approx 1 \mathrm{E} 6$, where $U$ is the approach velocity and $x$ is distance along the plate. If $U=2.5 \mathrm{~m} / \mathrm{s}$, determine the distance $x$ for the following fluids at $20^{\circ} \mathrm{C}$ and 1 atm : (a) hydrogen; (b) air; (c) gasoline; ( $d$ ) water; (e) mercury; and ( $f$ ) glycerin.

Solution: We are to calculate $x=\left(\operatorname{Re}_{x}\right)(\mu) /(\rho U)=(1 \mathrm{E} 6)(\mu) /[\rho(2.5 \mathrm{~m} / \mathrm{s})]$. Make a table:

| FLUID | $\rho-\mathrm{kg} / \mathrm{m}^{3}$ | $\mu-\mathrm{kg} / \mathrm{m}-\mathrm{s}$ | $x-$ meters |
| :---: | :---: | :---: | :---: |
| Hydrogen | 0.00839 | $9.05 \mathrm{E}-5$ | 43. |
| Air | 1.205 | $1.80 \mathrm{E}-5$ | 6.0 |
| Gasoline | 680 | $2.92 \mathrm{E}-4$ | 0.17 |
| Water | 998 | 0.0010 | 0.40 |


| Mercury | 13,550 | $1.56 \mathrm{E}-3$ | 0.046 |
| :---: | :---: | :---: | :---: |
| Glycerin | 1260 | 1.49 | 470. |

Clearly there are vast differences between fluid properties and their effects on flows.
6.7 Cola, approximated as pure water at $20^{\circ} \mathrm{C}$, is to fill an $8-\mathrm{oz}$ container ( $1 \mathrm{U} . \mathrm{S}$. gal $=$ 128 fl oz ) through a 5-mm-diameter tube. Estimate the minimum filling time if the tube flow is to remain laminar. For what cola (water) temperature would this minimum time be 1 min ?

Solution: For cola "water", take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Convert 8 fluid ounces $=(8 / 128)\left(231 \mathrm{in}^{3}\right) \approx 2.37 \mathrm{E}-4 \mathrm{~m}^{3}$. Then, if we assume transition at $\operatorname{Re}=2300$,

$$
\mathrm{Re}_{\text {crit }}=2300=\frac{\rho \mathrm{VD}}{\mu}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{D}}, \quad \text { or: } \quad \mathrm{Q}_{\text {crit }}=\frac{2300 \pi(0.001)(0.005)}{4(998)} \approx 9.05 \mathrm{E}-6 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}
$$

$$
\text { Then } \quad \Delta \text { tfill }=v / \mathrm{Q}=2.37 \mathrm{E}-4 / 9.05 \mathrm{E}-6 \approx 26 \mathrm{~s} \quad \text { Ans. (a) }
$$

(b) We fill in exactly one minute if $\mathrm{Q}_{\text {crit }}=2.37 \mathrm{E}-4 / 60=3.94 \mathrm{E}-6 \mathrm{~m}^{3} / \mathrm{s}$. Then

$$
\mathrm{Q}_{\text {crit }}=3.94 \mathrm{E}-6 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}=\frac{2300 \pi \nu \mathrm{D}}{4} \text { if } v_{\text {water }} \approx 4.36 \mathrm{E}-7 \mathrm{~m}^{2} / \mathrm{s}
$$

From Table A-1, this kinematic viscosity occurs at $\mathbf{T} \approx \mathbf{6 6}^{\circ} \mathrm{C}$ Ans. (b)
6.8 When water at $20^{\circ} \mathrm{C}\left(\rho=998 \mathrm{~kg} / \mathrm{m}^{3}, \mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}\right)$ flows through an $8-\mathrm{cm}-$ diameter pipe, the wall shear stress is 72 Pa . What is the axial pressure gradient ( $\partial \mathrm{p} / \partial \mathrm{x}$ ) if the pipe is (a) horizontal; and (b) vertical with the flow up?
Solution: Equation (6.9b) applies in both cases, noting that $\tau_{\mathrm{w}}$ is negative:
(a) Horizontal: $\frac{d p}{d x}=\frac{2 \tau_{w}}{R}=\frac{2(-72 P a)}{0.04 m}=-3600 \frac{\mathbf{P a}}{\mathbf{m}} \quad$ Ans. (a)
(b) Vertical, up: $\frac{d p}{d x}=\frac{2 \tau_{w}}{R}-\rho g \frac{d z 1^{1}}{d x}=-3600-998(9.81)=-13,400 \frac{\text { Pa }}{\mathbf{m}} \quad$ Ans. (b)
6.9 A light liquid ( $\rho=950 \mathrm{~kg} / \mathrm{m}^{3}$ ) flows at an average velocity of $10 \mathrm{~m} / \mathrm{s}$ through a horizontal smooth tube of diameter 5 cm . The fluid pressure is measured at 1-m intervals along the pipe, as follows:

| $x, \mathrm{~m}:$ | 0 | 1 | 2 | 3 | 4 | 5 | 6 |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| $p, \mathrm{kPa}:$ | 304 | 273 | 255 | 240 | 226 | 213 | 200 |

Estimate (a) the total head loss, in meters; (b) the wall shear stress in the fully developed section of the pipe; and (c) the overall friction factor.

Solution: As sketched in Fig. 6.6 of the text, the pressure drops fast in the entrance region ( 31 kPa in the first meter) and levels off to a linear decrease in the "fully developed" region ( $13 \mathrm{kPa} / \mathrm{m}$ for this data).
(a) The overall head loss, for $\Delta z=0$, is defined by Eq. (6.8) of the text:

$$
h_{f}=\frac{\Delta p}{\rho g}=\frac{304,000-200,000 \mathrm{~Pa}}{\left(950 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}=\mathbf{1 1 . 2} \mathbf{~ m} \quad \text { Ans. (a) }
$$

(b) The wall shear stress in the fully-developed region is defined by Eq. (6.9b):

$$
\left.\frac{\Delta p}{\Delta L}\right|_{\text {fully developed }}=\frac{13000 \mathrm{~Pa}}{1 \mathrm{~m}}=\frac{4 \tau_{w}}{d}=\frac{4 \tau_{w}}{0.05 \mathrm{~m}} \text {, solve for } \tau_{w}=163 \text { Pa Ans. (b) }
$$

(c) The overall friction factor is defined by Eq. (6.10) of the text:

$$
f_{\text {overall }}=h_{f, \text { overall }} \frac{d}{L} \frac{2 g}{V^{2}}=(11.2 \mathrm{~m})\left(\frac{0.05 \mathrm{~m}}{6 m}\right) \frac{2\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}{(10 \mathrm{~m} / \mathrm{s})^{2}}=\mathbf{0 . 0 1 8 2} \quad \text { Ans. (c) }
$$

NOTE: The fully-developed friction factor is only 0.0137 .
6.10 Water at $20^{\circ} \mathrm{C}\left(\rho=998 \mathrm{~kg} / \mathrm{m}^{3}\right)$ flows through an inclined 8 -cm-diameter pipe. At sections A and $\mathrm{B}, \mathrm{pA}=186 \mathrm{kPa}, \mathrm{VA}=3.2 \mathrm{~m} / \mathrm{s}$, $\mathrm{zA}=24.5 \mathrm{~m}$, while $\mathrm{pB}=260 \mathrm{kPa}, \mathrm{VB}=3.2$ $\mathrm{m} / \mathrm{s}$, and $\mathrm{zB}=9.1 \mathrm{~m}$. Which way is the flow going? What is the head loss?

Solution: Guess that the flow is from A to B and write the steady flow energy equation:

$$
\frac{p_{A}}{\rho g}+\frac{V_{A}^{2}}{\not 2 g}+z_{A}=\frac{p_{B}}{\rho g}+\frac{V_{B}^{2}}{z g}+z_{B}+h_{f}, \quad \text { or: } \quad \frac{186000}{9790}+24.5=\frac{260000}{9790}+9.1+h_{f},
$$

or: $\quad 43.50=35.66+h_{f}, \quad$ solve: $\mathbf{h}_{\mathbf{f}}=+7.84 \mathbf{~ m}$ Yes, flow is from A to B. Ans. (a, b)
6.11 Water at $20^{\circ} \mathrm{C}$ flows upward at $4 \mathrm{~m} / \mathrm{s}$ in a 6 - cm -diameter pipe. The pipe length between points 1 and 2 is 5 m , and point 2 is 3 m higher. A mercury manometer, connected between 1 and 2 , has a reading $h=135 \mathrm{~mm}$, with $p 1$ higher. (a) What is the pressure change $(p 1-p 2)$ ? (b) What is the head loss, in meters? (c) Is the manometer reading proportional to head loss? Explain. (d) What is the friction factor of the flow?

Solution: A sketch of this situation is shown at right. By moving through the manometer, we obtain the pressure change between points 1 and 2, which we compare with Eq. (6.9b):


$$
\begin{aligned}
& p_{1}+\gamma_{w} h-\gamma_{m} h-\gamma_{w} \Delta z=p_{2} \\
& \text { or: } \quad p_{1}-p_{2}=\left(133100-9790 \frac{N}{m^{3}}\right)(0.135 m)+\left(9790 \frac{N}{m^{3}}\right)(3 \mathrm{~m}) \\
&=16650+29370=\mathbf{4 6 , 0 0 0} \mathbf{P a} \text { Ans. (a) }
\end{aligned}
$$

From Eq. (6.9b), $\quad h_{f}=\frac{\Delta p}{\gamma_{w}}-\Delta z=\frac{46000 P a}{9790 N / m^{3}}-3 m=4.7-3.0=1.7 \mathbf{m} \quad$ Ans. (b) The friction factor is $f=h_{f} \frac{d}{L} \frac{2 g}{V^{2}}=(1.7 \mathrm{~m})\left(\frac{0.06 \mathrm{~m}}{5 \mathrm{~m}}\right) \frac{2\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}{(4 \mathrm{~m} / \mathrm{s})^{2}}=\mathbf{0 . 0 2 5}$ Ans. (d)

By comparing the manometer relation to the head-loss relation above, we find that:
$h_{f}=\frac{\left(\gamma_{m}-\gamma_{w}\right)}{\gamma_{w}} h$ and thus head loss is proportional to manometer reading. Ans. (c)

## NOTE: IN PROBLEMS 6.12 TO 6.99, MINOR LOSSES ARE NEGLECTED.

6.12 A $5-\mathrm{mm}$-diameter capillary tube is used as a viscometer for oils. When the flow rate is $0.071 \mathrm{~m}^{3} / \mathrm{h}$, the measured pressure drop per unit length is $375 \mathrm{kPa} / \mathrm{m}$. Estimate the viscosity of the fluid. Is the flow laminar? Can you also estimate the density of the fluid?

Solution: Assume laminar flow and use the pressure drop formula (6.12):

$$
\frac{\Delta \mathrm{p}}{\mathrm{~L}} \stackrel{?}{=} \frac{8 \mathrm{Q} \mu}{\pi \mathrm{R}^{4}}, \quad \text { or: } \quad 375000 \frac{\mathrm{~Pa}}{\mathrm{~m}} \stackrel{?}{=} \frac{8(0.071 / 3600) \mu}{\pi(0.0025)^{4}}, \quad \text { solve } \mu \approx \mathbf{0 . 2 9 2} \frac{\mathbf{k g}}{\mathbf{m} \cdot \mathbf{s}} \text { Ans. }
$$

check $\operatorname{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{d}}=\frac{4(900)(0.071 / 3600)}{\pi(0.292)(0.005)} \approx 16 \quad$ OK, laminar Ans.
It is not possible to find density from this data, laminar pipe flow is independent of density.
6.13 A soda straw is 20 cm long and 2 mm in diameter. It delivers cold cola, approximated as water at $10^{\circ} \mathrm{C}$, at a rate of $3 \mathrm{~cm}^{3} / \mathrm{s}$. (a) What is the head loss through the straw? What is the axial pressure gradient $\partial p / \partial x$ if the flow is (b) vertically up or (c) horizontal? Can the human lung deliver this much flow?

Solution: For water at $10^{\circ} \mathrm{C}$, take $\rho=1000 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.307 \mathrm{E}-3 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Check Re:

$$
\left.\mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{~d}}=\frac{4(1000)\left(3 \mathrm{E}-6 \mathrm{~m}^{3} / \mathrm{s}\right)}{\pi(1.307 \mathrm{E}-3)(0.002)}=1460 \quad \text { (OK, laminar flow }\right)
$$

Then, from Eq. (6.12), $\mathrm{h}_{\mathrm{f}}=\frac{128 \mu \mathrm{LQ}}{\pi \rho \mathrm{gd}^{4}}=\frac{128(1.307 \mathrm{E}-3)(0.2)(3 \mathrm{E}-6)}{\pi(1000)(9.81)(0.002)^{4}} \approx \mathbf{0 . 2 0 4} \mathbf{~ m} \quad$ Ans. (a)
If the straw is horizontal, then the pressure gradient is simply due to the head loss:

$$
\left.\frac{\Delta \mathrm{p}}{\mathrm{~L}}\right|_{\text {horiz }}=\frac{\rho \mathrm{gh}_{\mathrm{f}}}{\mathrm{~L}}=\frac{1000(9.81)(0.204 \mathrm{~m})}{0.2 \mathrm{~m}} \approx \mathbf{9 9 8 0} \frac{\mathbf{P a}}{\mathbf{m}} \text { Ans. (c) }
$$

If the straw is vertical, with flow up, the head loss and elevation change add together:

$$
\left.\frac{\Delta \mathrm{p}}{\mathrm{~L}}\right|_{\text {vertical }}=\frac{\rho \mathrm{g}\left(\mathrm{~h}_{\mathrm{f}}+\Delta \mathrm{z}\right)}{\mathrm{L}}=\frac{1000(9.81)(0.204+0.2)}{0.2} \approx \mathbf{1 9 8 0 0} \frac{\mathbf{P a}}{\mathrm{~m}} \quad \text { Ans. (b) }
$$

The human lung can certainly deliver case (c) and strong lungs can develop case (b) also.
6.14 Water at $20^{\circ} \mathrm{C}$ is to be siphoned through a tube 1 m long and 2 mm in diameter, as in Fig. P6.14. Is there any height $H$ for which the flow might not be laminar? What is the flow rate if $H=50 \mathrm{~cm}$ ? Neglect the tube curvature.


Fig. P6.14

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Write the steady flow energy equation between points 1 and 2 above:

$$
\begin{equation*}
\frac{\mathrm{p}_{\mathrm{atm}}}{\rho \mathrm{~g}}+\frac{0^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}=\frac{\mathrm{p}_{\text {atm }}}{\rho \mathrm{g}}+\frac{\mathrm{V}_{\text {tube }}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}}, \quad \text { or: } \quad \mathrm{H}-\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}=\mathrm{h}_{\mathrm{f}}=\frac{32 \mu \mathrm{~L}}{\rho \mathrm{gd}^{2}} \mathrm{~V} \tag{1}
\end{equation*}
$$

Enter data in Eq. (1): $\quad 0.5-\frac{\mathrm{V}^{2}}{2(9.81)}=\frac{32(0.001)(1.0) \mathrm{V}}{(998)(9.81)(0.002)^{2}}$, solve $\mathrm{V} \approx 0.590 \frac{\mathrm{~m}}{\mathrm{~s}}$
Equation (1) is quadratic in V and has only one positive root. The siphon flow rate is

$$
\mathrm{Q}_{\mathrm{H}=50 \mathrm{~cm}}=\frac{\pi}{4}(0.002)^{2}(0.590)=1.85 \mathrm{E}-6 \frac{\mathrm{~m}^{3}}{\mathrm{~s}} \approx \mathbf{0 . 0 0 6 7} \frac{\mathbf{m}^{3}}{\mathbf{h}} \quad \text { if } \boldsymbol{H}=\mathbf{5 0} \mathbf{~ c m} \quad \text { Ans. }
$$

$$
\text { Check } \operatorname{Re}=(998)(0.590)(0.002) /(0.001) \approx 1180(\mathrm{OK}, \text { laminar flow })
$$

It is possible to approach $\mathrm{Re} \approx 2000$ (possible transition to turbulent flow) for $\mathrm{H}<1 \mathrm{~m}$, for the case of the siphon bent over nearly vertical. We obtain $\mathbf{R e}=\mathbf{2 0 0 0}$ at $\mathbf{H} \approx \mathbf{0 . 8 7} \mathbf{~ m}$.
6.15 Professor Gordon Holloway and his students at the University of New Brunswick went to a fast-food emporium and tried to drink chocolate shakes ( $\rho \approx 1200 \mathrm{~kg} / \mathrm{m}^{3}$, $\mu \approx 6 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$ ) through fat straws 8 mm in diameter and 30 cm long. (a) Verify that their human lungs, which can develop approximately 3000 Pa of vacuum pressure, would be unable to drink the milkshake through the vertical straw. (b) A student cut 15 cm from his straw and proceeded to drink happily. What rate of milkshake flow was produced by this strategy?

Solution: (a) Assume the straw is barely inserted into the milkshake. Then the energy equation predicts

$$
\begin{aligned}
\frac{p_{1}}{\rho g} & =\frac{V_{1}^{2}}{2 g}+z_{1}=\frac{p_{2}}{\rho g}=\frac{V_{2}^{2}}{2 g}+z_{2}+h_{f} \\
& =0+0+0=\frac{(-3000 P a)}{\left(1200 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}+\frac{V_{\text {tube }}^{2}}{2 g}+0.3 \mathrm{~m}+h_{f}
\end{aligned}
$$

Solve for $\boldsymbol{h}_{f}=0.255 m-0.3 m-\frac{V_{\text {tube }}^{2}}{2 g}<0 \quad$ which is impossible Ans. (a)
(b) By cutting off 15 cm of vertical length and assuming laminar flow, we obtain a new energy equation

$$
\begin{array}{r}
h_{f}=0.255-0.15-\frac{V^{2}}{2 g}=\frac{32 \mu L V}{\rho g d^{2}}=0.105 m-\frac{V^{2}}{2(9.81)}=\frac{32(6.0)(0.15) V}{(1200)(9.81)(0.008)^{2}}=38.23 V \\
\text { Solve for } V=0.00275 \mathrm{~m} / \mathrm{s}, \quad Q=A V=(\pi / 4)(0.008)^{2}(0.00275) \\
\\
Q=1.4 E-7 \frac{m^{3}}{s}=\mathbf{0 . 1 4} \frac{\mathbf{c m}^{3}}{\mathbf{s}} \quad \text { Ans. (b) }
\end{array}
$$

Check the Reynolds number: $\quad \operatorname{Red}=\rho V d / \mu=(1200)(0.00275)(0.008) /(6)=0.0044$ (Laminar).
6.16 Glycerin at $20^{\circ} \mathrm{C}$ is to be pumped through a horizontal smooth pipe at $3.1 \mathrm{~m}^{3} / \mathrm{s}$. It is desired that (1) the flow be laminar and (2) the pressure drop be no more than $100 \mathrm{~Pa} / \mathrm{m}$. What is the minimum pipe diameter allowable?

Solution: For glycerin at $20^{\circ} \mathrm{C}$, take $\rho=1260 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.49 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. We have two different constraints to satisfy, a pressure drop and a Reynolds number:

$$
\begin{aligned}
& \frac{\Delta \mathrm{p}}{\mathrm{~L}}=\frac{128 \mu \mathrm{Q}}{\pi \mathrm{~d}^{4}} \leq 100 \frac{\mathrm{~Pa}}{\mathrm{~m}} \quad(1) ; \quad \frac{128(1.49)(3.1)}{\pi \mathrm{d}^{4}} \leq 100, \quad \mathrm{~d} \geq 1.17 \mathrm{~m} \\
& \text { or: } \quad \mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{~d}} \leq 2000
\end{aligned}
$$

The second of these is more restrictive. Thus the proper diameter is $\mathbf{d} \geq \mathbf{1 . 6 7} \mathbf{~ m}$. Ans.
6.17 A capillary viscometer measures the time required for a specified volume $v$ of liquid to flow through a small-bore glass tube, as in Fig. P6.17. This transit time is then correlated with fluid viscosity. For the system shown, (a) derive an approximate formula for the time required, assuming laminar flow with no entrance and exit losses. (b) If $L=12 \mathrm{~cm}, l=2 \mathrm{~cm}, v=8 \mathrm{~cm}^{3}$, and the fluid is water at $20^{\circ} \mathrm{C}$, what capillary diameter $D$ will result in a transit time $t$ of 6 seconds?


Fig. P6.17

Solution: (a) Assume no pressure drop and neglect velocity heads. The energy equation reduces to:

$$
\frac{p_{1}}{\rho g}+\frac{V_{1}^{2}}{2 g}+z_{1}=0+0+(L+l)=\frac{p_{2}}{\rho g}+\frac{V_{2}^{2}}{2 g}+z_{2}+h_{f}=0+0+0+h_{f}, \quad \text { or: } \quad h_{f} \approx L+l
$$

For laminar flow, $h_{f}=\frac{128 \mu L Q}{\pi \rho g d^{4}}$ and, for uniform draining, $Q=\frac{v}{\Delta t}$

$$
\text { Solve for } \quad \Delta t=\frac{128 \mu L v}{\pi \rho g d^{4}(L+I)} \quad \text { Ans. (a) }
$$

(b) Apply to $\Delta t=6 \mathrm{~s}$. For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Formula (a) predicts:

$$
\Delta t=6 s=\frac{128(0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s})(0.12 \mathrm{~m})\left(8 E-6 \mathrm{~m}^{3}\right)}{\pi\left(998 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right) d^{4}(0.12+0.02 \mathrm{~m})}
$$

Solve for $\mathbf{d} \approx \mathbf{0 . 0 0 1 5} \mathbf{~ m}$ Ans. (b)
6.18 To determine the viscosity of a liquid of specific gravity 0.95 , you fill, to a depth of 12 cm , a large container which drains through a $30-\mathrm{cm}$-long vertical tube attached to the bottom. The tube diameter is 2 mm , and the rate of draining is found to be $1.9 \mathrm{~cm}^{3} / \mathrm{s}$. What is your estimate of the fluid viscosity? Is the tube flow laminar?


Fig. P6.18

Solution: The known flow rate and diameter enable us to find the velocity in the tube:

$$
V=\frac{Q}{A}=\frac{1.9 E-6 \mathrm{~m}^{3} / \mathrm{s}}{(\pi / 4)(0.002 \mathrm{~m})^{2}}=0.605 \frac{\mathrm{~m}}{\mathrm{~s}}
$$

Evaluate $\rho$ liquid $=0.95(998)=948 \mathrm{~kg} / \mathrm{m}^{3}$. Write the energy equation between the top surface and the tube exit:

$$
\begin{gathered}
\frac{p_{a} \uparrow}{\rho g}=\frac{V_{\text {top }}^{2}}{2 g}+z_{\text {top }}=\frac{p f a}{p g}+\frac{V^{2}}{2 g}+0+h_{f}, \\
\text { or: } \quad 0.42=\frac{V^{2}}{2 g}+\frac{32 \mu L V}{\rho g d^{2}}=\frac{(0.605)^{2}}{2(9.81)}+\frac{32 \mu(0.3)(0.605)}{948(9.81)(0.002)^{2}}
\end{gathered}
$$

Note that " $L$ " in this expression is the tube length only ( $L=30 \mathrm{~cm}$ ).

$$
\begin{aligned}
& \text { Solve for } \mu=\mathbf{0 . 0 0 2 5 7} \frac{\mathbf{k g}}{\mathbf{m} \cdot \mathbf{s}} \text { (laminar flow) Ans. } \\
& R e_{d}=\frac{\rho V d}{\mu}=\frac{948(0.605)(0.002)}{0.00257}=446 \text { (laminar) }
\end{aligned}
$$

6.19 An oil ( $\mathrm{SG}=0.9$ ) issues from the pipe in Fig. P6.19 at $Q=35 \mathrm{ft}^{3} / \mathrm{h}$. What is the kinematic viscosity of the oil in $\mathrm{ft}^{3} / \mathrm{s}$ ? Is the flow laminar?

Solution: Apply steady-flow energy:

$$
\frac{\mathrm{p}_{\mathrm{atm}}}{\rho \mathrm{~g}}+\frac{0^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}=\frac{\mathrm{p}_{\mathrm{atm}}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}},
$$



Fig. P6.19
where $\quad \mathrm{V}_{2}=\frac{\mathrm{Q}}{\mathrm{A}}=\frac{35 / 3600}{\pi(0.25 / 12)^{2}} \approx 7.13 \frac{\mathrm{ft}}{\mathrm{s}}$
Solve $h_{f}=z_{1}-z_{2}-\frac{V_{2}^{2}}{2 g}=10-\frac{(7.13)^{2}}{2(32.2)}=9.21 \mathrm{ft}$
Assuming laminar pipe flow, use Eq. (6.12) to relate head loss to viscosity:

$$
\mathrm{h}_{\mathrm{f}}=9.21 \mathrm{ft}=\frac{128 v \mathrm{LQ}}{\pi \mathrm{gd}^{4}}=\frac{128(6)(35 / 3600) v}{\pi(32.2)(0.5 / 12)^{4}}, \quad \text { solve } v=\frac{\mu}{\rho} \approx 3.76 \mathrm{E}-4 \frac{\mathrm{ft}^{2}}{\mathrm{~s}} \quad \text { Ans. }
$$

Check $\operatorname{Re}=4 \mathrm{Q} /(\pi \nu \mathrm{d})=4(35 / 3600) / \pi(3.76 \mathrm{E}-4)(0.5 / 12)] \approx 790(\mathrm{OK}$, laminar $)$

P6.20 The oil tanks in Tinyland are only 160 cm high, and they discharge to the Tinyland oil truck through a smooth tube 4 mm in diameter and 55 cm long. The tube exit is open to the atmosphere and 145 cm below the tank surface. The fluid is medium fuel oil, $\rho=850 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.11 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. Estimate the oil flow rate in $\mathrm{cm}^{3} / \mathrm{h}$.

Solution: The steady flow energy equation, with 1 at the tank surface and 2 the exit, gives

$$
z_{1}=z_{2}+\frac{\alpha V^{2}}{2 g}+f \frac{L}{d} \frac{V^{2}}{2 g}, \text { or }: \Delta z=1.45 m=\frac{V^{2}}{2 g}\left(2.0+\frac{64}{\operatorname{Re}_{d}} \frac{0.55 m}{0.004 m}\right), \operatorname{Re}_{d}=\frac{850 V(0.004)}{0.11}
$$

We have taken the energy correction factor $\alpha=2.0$ for laminar pipe flow.
Solve for $V=0.10 \mathrm{~m} / \mathrm{s}, \mathrm{Re}_{d}=3.1$ (laminar), $Q=1.26 \mathrm{E}-6 \mathrm{~m}^{3} / \mathrm{s} \approx 4500 \mathrm{~cm}^{3} / \mathrm{h}$. Ans.
The exit jet energy $\alpha V^{2} / 2 g$ is properly included but is very small ( 0.001 m ).
6.21 In Tinyland, houses are less than a foot high! The rainfall is laminar! The drainpipe in Fig. P6.21 is only 2 mm in diameter. (a) When the gutter is full, what is the rate of draining? (b) The gutter is designed for a sudden rainstorm of up to 5 mm per hour. For this condition, what is the maximum roof area that can be drained successfully? (c) What is Red?

Solution: If the velocity at the gutter surface is neglected, the energy equation reduces to

$$
\Delta z=\frac{V^{2}}{2 g}+h_{f}, \quad \text { where } h_{f, \text { laminar }}=\frac{32 \mu L V}{\rho g d^{2}}
$$



Fig. P6.21

For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. (a) With $\Delta z$ known, this is a quadratic equation for the pipe velocity $V$ :

$$
\begin{gathered}
0.2 m=\frac{V^{2}}{2\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}+\frac{32(0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s})(0.2 \mathrm{~m}) V}{\left(998 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)(0.002 \mathrm{~m})^{2}}, \\
\text { or: } 0.051 V^{2}+0.1634 V-0.2=0, \quad \text { Solve for } V=0.945 \frac{\mathrm{~m}}{\mathrm{~s}}, \\
Q=\frac{\pi}{4}(0.002 \mathrm{~m})^{2}\left(0.945 \frac{\mathrm{~m}}{\mathrm{~s}}\right)=2.97 E-6 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}=\mathbf{0 . 0 1 0 7} \frac{\boldsymbol{m}^{3}}{\boldsymbol{h}} \quad \text { Ans. (a) }
\end{gathered}
$$

(b) The roof area needed for maximum rainfall is $0.0107 \mathrm{~m}^{3} / \mathrm{h} \div 0.005 \mathrm{~m} / \mathrm{h}=\mathbf{2 . 1 4} \mathbf{~ m}^{\mathbf{2}}$. Ans. (b)
(c) The Reynolds number of the gutter is Red $=(998)(0.945)(0.002) /(0.001)=\mathbf{1 8 9 0}$
laminar. Ans. (c)
6.22 A steady push on the piston in Fig. P6.22 causes a flow rate $Q=0.15 \mathrm{~cm}^{3} / \mathrm{s}$ through the needle. The fluid has $\rho=900 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.002 \mathrm{~kg} /(\mathrm{m} \cdot \mathrm{s})$. What force $F$ is required to maintain the flow?


Fig. P6.22
Solution: Determine the velocity of exit from the needle and then apply the steady-flow energy equation:

$$
V_{1}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{0.15}{(\pi / 4)(0.025)^{2}}=306 \mathrm{~cm} / \mathrm{s}
$$

Energy: $\frac{\mathrm{p}_{2}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{2}=\frac{\mathrm{p}_{1}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}+\mathrm{h}_{\mathrm{f} 1}+\mathrm{h}_{\mathrm{f} 2}$, with $\mathrm{z}_{1}=\mathrm{z}_{2}, \mathrm{~V}_{2} \approx 0, \mathrm{~h}_{\mathrm{f} 2} \approx 0$
Assume laminar flow for the head loss and compute the pressure difference on the piston:

$$
\begin{aligned}
& \frac{\mathrm{p}_{2}-\mathrm{p}_{1}}{\rho \mathrm{~g}}=\mathrm{h}_{\mathrm{f} 1}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}=\frac{32(0.002)(0.015)(3.06)}{(900)(9.81)(0.00025)^{2}}+\frac{(3.06)^{2}}{2(9.81)} \approx 5.79 \mathrm{~m} \\
& \text { Then } \quad \mathrm{F}=\Delta \mathrm{pA}_{\mathrm{p} \text { iston }}=(900)(9.81)(5.79) \frac{\pi}{4}(0.01)^{2} \approx 4.0 \mathrm{~N} \text { Ans. }
\end{aligned}
$$

6.23 SAE 10 oil at $20^{\circ} \mathrm{C}$ flows in a vertical pipe of diameter 2.5 cm . It is found that the pressure is constant throughout the fluid. What is the oil flow rate in $\mathrm{m}^{3} / \mathrm{h}$ ? Is the flow up or down?

Solution: For SAE 10 oil, take $\rho=870 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.104 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Write the energy equation between point 1 upstream and point 2 downstream:

$$
\frac{\mathrm{p}_{1}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}=\frac{\mathrm{p}_{2}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}} \text {, with } \mathrm{p}_{1}=\mathrm{p}_{2} \text { and } \mathrm{V}_{1}=\mathrm{V}_{2}
$$

Thus $\mathrm{h}_{\mathrm{f}}=\mathrm{z}_{1}-\mathrm{z}_{2}>0$ by definition. Therefore, flow is down. Ans.
While flowing down, the pressure drop due to friction exactly balances the pressure rise due to gravity. Assuming laminar flow and noting that $\Delta \mathrm{z}=\mathrm{L}$, the pipe length, we get

$$
\begin{gathered}
\mathrm{h}_{\mathrm{f}}=\frac{128 \mu \mathrm{LQ}}{\pi \rho \mathrm{gd}^{4}}=\Delta \mathrm{z}=\mathrm{L} \\
\text { or: } \mathrm{Q}=\frac{\pi(8.70)(9.81)(0.025)^{4}}{128(0.104)}=7.87 \mathrm{E}-4 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}=\mathbf{2 . 8 3} \frac{\mathbf{m}^{3}}{\mathbf{h}} \quad \text { Ans. }
\end{gathered}
$$

6.24 Two tanks of water at $20^{\circ} \mathrm{C}$ are connected by a capillary tube 4 mm in diameter and 3.5 m long. The surface of tank 1 is 30 cm higher than the surface of tank 2. (a) Estimate the flow rate in $\mathrm{m}^{3} / \mathrm{h}$. Is the flow laminar? (b) For what tube diameter will Red be 500?
Solution: For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. (a) Both tank surfaces are at atmospheric pressure and have negligible velocity. The energy equation, when neglecting minor losses, reduces to:

$$
\begin{gathered}
\Delta z=0.3 m=h_{f}=\frac{128 \mu L Q}{\pi \rho g d^{4}}=\frac{128(0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s})(3.5 \mathrm{~m}) Q}{\pi\left(998 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)(0.004 \mathrm{~m})^{4}} \\
\text { Solve for } Q=5.3 E-6 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}=\mathbf{0 . 0 1 9} \frac{\mathbf{m}^{3}}{\mathbf{h}} \text { Ans. (a) }
\end{gathered}
$$

Check $\left.\operatorname{Re}_{\mathrm{d}}=4 \rho Q /(\pi \mu d)=4(998)(5.3 \mathrm{E}-6) / \pi(0.001)(0.004)\right]$
$\mathbf{R e}_{\mathbf{d}}=1675$ laminar. Ans. (a)
(b) If Red $=500=4 \rho Q /(\pi \mu d)$ and $\Delta z=h f$, we can solve for both $Q$ and $d$ :

$$
\begin{gathered}
\operatorname{Re}_{d}=500=\frac{4\left(998 \mathrm{~kg} / \mathrm{m}^{3}\right) Q}{\pi(0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s}) \mathrm{d}}, \quad \text { or } \quad Q=0.000394 d \\
h_{f}=0.3 \mathrm{~m}=\frac{128(0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s})(3.5 \mathrm{~m}) Q}{\pi\left(998 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right) \mathrm{d}^{4}}, \quad \text { or } \quad Q=20600 \mathrm{~d}^{4}
\end{gathered}
$$

Combine these two to solve for $\quad Q=1.05 E-6 \mathrm{~m}^{3} / \mathrm{s}$ and $\quad \mathbf{d}=\mathbf{2 . 6 7} \mathbf{m m}$ Ans. (b)
6.25 For the configuration shown in Fig. P6.25, the fluid is ethyl alcohol at $20^{\circ} \mathrm{C}$, and the tanks are very wide. Find the flow rate which occurs in $\mathrm{m}^{3} / \mathrm{h}$. Is the flow laminar?

Solution: For ethanol, take $\rho=$ $789 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.0012 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Write the energy equation from upper free surface (1) to lower free surface (2):


Fig. P6.25

$$
\begin{aligned}
& \frac{\mathrm{p}_{1}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}=\frac{\mathrm{p}_{2}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}}, \quad \text { with } \mathrm{p}_{1}=\mathrm{p}_{2} \text { and } \mathrm{V}_{1} \approx \mathrm{~V}_{2} \approx 0 \\
& \text { Then } \mathrm{h}_{\mathrm{f}}=\mathrm{z}_{1}-\mathrm{z}_{2}=0.9 \mathrm{~m}=\frac{128 \mu \mathrm{LQ}}{\pi \rho \mathrm{gd}^{4}}=\frac{128(0.0012)(1.2 \mathrm{~m}) \mathrm{Q}}{\pi(789)(9.81)(0.002)^{4}}
\end{aligned}
$$

Solve for $\mathrm{Q} \approx 1.90 \mathrm{E}-6 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{0 . 0 0 6 8 4} \mathbf{m}^{\mathbf{3}} / \mathbf{h}$. Ans.
Check the Reynolds number $\mathrm{Re}=4 \rho \mathrm{Q} /(\pi \mu \mathrm{d}) \approx 795-\mathbf{O K}$, laminar flow.

$$
Z_{\mathrm{a}}=22 \mathrm{~m}
$$

P6.26 Two oil tanks are connected by
two 9-m-long pipes, as in Fig. P6.26.

Pipe 1 is 5 cm in diameter and is 6 m higher than pipe 2. It is found that the flow rate in pipe 2 is twice as large as the flow in pipe 1. (a) What is the diameter


Fig. P6.26
of pipe 2? (b) Are both pipe flows laminar?
(c) What is the flow rate in pipe $2\left(\mathrm{~m}^{3} / \mathrm{s}\right)$ ?

Neglect minor losses.

Solution: (a) If we know the flows are laminar, and $(L, \rho, \mu)$ are constant, then $Q \propto D^{4}$ :

From Eq.(6.12), $\frac{Q_{2}}{Q_{1}}=2.0=\left(\frac{D_{2}}{D_{1}}\right)^{4}$, hence $D_{2}=(5 \mathrm{~cm})(2.0)^{1 / 4}=5.95 \mathrm{~cm} \quad$ Ans.(a) We will check later in part (b) to be sure the flows are laminar. [Placing pipe 1 six meters higher was meant to be a confusing trick, since both pipes have exactly the same head loss and $\Delta z$.] (c) Find the flow rate first and then backtrack to the Reynolds numbers. For SAE 30 W oil at $20^{\circ} \mathrm{C}$ (Table A.3), take $\rho=891 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.29 \mathrm{~kg} / \mathrm{m}-$ s. From the energy equation, with $V_{1}=V_{2}=0$, and Eq. (6.12) for the laminar head loss,

$$
\begin{array}{r}
\Delta z=22-15=7 m=h_{f}=\frac{128 \mu L Q}{\pi \rho g D_{2}^{4}}=\frac{128(0.29 \mathrm{~kg} / \mathrm{m}-\mathrm{s})(9 \mathrm{~m}) Q_{2}}{\pi\left(891 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)(0.0595 \mathrm{~m})^{4}} \\
\text { Solve for } \quad Q_{2}=\mathbf{0 . 0 0 7 2} \mathbf{m}^{\mathbf{3}} / \mathbf{s} \quad \text { Ans.(c) }
\end{array}
$$

In a similar manner, insert $D_{1}=0.05 \mathrm{~m}$ and compute $Q_{1}=\mathbf{0 . 0 0 3 6} \mathrm{m}^{3} / \mathrm{s}=(1 / 2) Q_{1}$.
(b) Now go back and compute the Reynolds numbers:
$\operatorname{Re}_{1}=\frac{4 \rho Q_{1}}{\pi \mu D_{1}}=\frac{4(891)(0.0036)}{\pi(0.29)(0.050)}=281 ; \operatorname{Re}_{2}=\frac{4 \rho Q_{2}}{\pi \mu D_{2}}=\frac{4(891)(0.0072)}{\pi(0.29)(0.0595)}=473$ Ans. $(b)$

Both flows are laminar, which verifies our flashy calculation in part (a).
6.27 Let us attack Prob. 6.25 in symbolic fashion, using Fig. P6.27. All parameters are constant except the upper tank depth $Z(t)$. Find an expression for the flow rate $Q(t)$ as a function of $Z(t)$. Set up a differential equation, and solve for the time $t 0$ to drain the upper tank completely. Assume quasisteady laminar flow.

Solution: The energy equation of Prob. 6.25, using symbols only, is combined with a control-volume mass balance for the tank to give the basic differential equation for $\mathrm{Z}(\mathrm{t})$ :


Fig. P6.27

$$
\begin{gathered}
\text { energy: } \quad \mathrm{h}_{\mathrm{f}}=\frac{32 \mu \mathrm{LV}}{\rho \mathrm{gd}^{2}}=\mathrm{h}+\mathrm{Z} ; \quad \text { mass balance: } \frac{\mathrm{d}}{\mathrm{dt}}\left[\frac{\pi}{4} \mathrm{D}^{2} \mathrm{Z}+\frac{\pi}{4} \mathrm{~d}^{2} \mathrm{~L}\right]=-\mathrm{Q}=-\frac{\pi}{4} \mathrm{~d}^{2} \mathrm{~V} \text {, } \\
\text { or: } \frac{\pi}{4} \mathrm{D}^{2} \frac{\mathrm{dZ}}{\mathrm{dt}}=-\frac{\pi}{4} \mathrm{~d}^{2} \mathrm{~V} \text {, where } \mathrm{V}=\frac{\rho \mathrm{gd}^{2}}{32 \mu \mathrm{~L}}(\mathrm{~h}+\mathrm{Z})
\end{gathered}
$$

Separate the variables and integrate, combining all the constants into a single "C":

$$
\int_{\mathrm{Z}_{0}}^{\mathrm{Z}} \frac{\mathrm{dZ}}{\mathrm{~h}+\mathrm{Z}}=-\mathrm{C} \int_{0}^{\mathrm{t}} \mathrm{dt}, \quad \text { or: } \quad \mathbf{Z}=\left(\mathbf{h}+\mathbf{Z}_{\mathbf{0}}\right) \mathbf{e}^{-\mathrm{Ct}}-\mathbf{h}, \quad \text { where } \mathbf{C}=\frac{\rho \mathbf{g d}^{4}}{32 \mu \mathbf{L D}^{2}} \quad \text { Ans. }
$$

Tank drains completely when $\mathrm{Z}=0$, at $\mathbf{t}_{\mathbf{0}}=\frac{\mathbf{1}}{\mathbf{C}} \ln \left(\mathbf{1}+\frac{\mathbf{Z}_{\mathbf{0}}}{\mathbf{h}}\right) \quad$ Ans.
6.28 For straightening and smoothing an airflow in a 50 -cm-diameter duct, the duct is packed with a "honeycomb" of thin straws of length 30 cm and diameter 4 mm , as in Fig. P6.28. The inlet flow is air at 110 kPa and $20^{\circ} \mathrm{C}$, moving at an average velocity of $6 \mathrm{~m} / \mathrm{s}$. Estimate the pressure drop across the honeycomb.

Solution: For air at $20^{\circ} \mathrm{C}$, take $\mu \approx$ $1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$ and $\rho=1.31 \mathrm{~kg} / \mathrm{m}^{3}$. There would be approximately 12000 straws, but each one would see the average velocity of $6 \mathrm{~m} / \mathrm{s}$. Thus


Fig. P6. 28

$$
\Delta \mathrm{p}_{\text {laminar }}=\frac{32 \mu \mathrm{LV}}{\mathrm{~d}^{2}}=\frac{32(1.8 \mathrm{E}-5)(0.3)(6.0)}{(0.004)^{2}} \approx 65 \text { Pa Ans. }
$$

Check $\operatorname{Re}=\rho \mathrm{Vd} / \mu=(1.31)(6.0)(0.004) /(1.8 \mathrm{E}-5) \approx 1750 \quad$ OK, laminar flow.
6.29 Oil, with $\rho=890 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.07 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$, flows through a horizontal pipe 15 m long. The power delivered to the flow is 1 hp . (a) What is the appropriate pipe diameter if the flow is at the laminar transition point? For this condition, what are (b) $Q$ in $\mathrm{m}^{3} / \mathrm{h}$; and (c) $\tau_{\mathrm{w}}$ in kPa ?

Solution: (a, b) Set the Reynolds number equal to 2300 and the (laminar) power equal to 1 hp :

$$
\begin{gathered}
\mathrm{Re}_{d}=2300=\frac{\left(890 \mathrm{~kg} / \mathrm{m}^{3}\right) \mathrm{Vd}}{0.07 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s}} \text { or } \quad V d=0.181 \mathrm{~m}^{2} / \mathrm{s} \\
\text { Power }=1 \mathrm{hp}=745.7 \mathrm{~W}=Q \Delta p_{\text {laminar }}=\left(\frac{\pi}{4} d^{2} V\right)\left(\frac{32 \mu L V}{d^{2}}\right)=\left(\frac{\pi}{4}\right) 32(0.07)(15) V^{2} \\
\text { Solve for } V=5.32 \frac{\mathrm{~m}}{\mathrm{~s}} \text { and } \mathbf{d}=\mathbf{0 . 0 3 4} \mathbf{~ m} \text { Ans. (a) }
\end{gathered}
$$

It follows that $Q=(\pi / 4) d^{2} V=(\pi / 4)(0.034 \mathrm{~m})^{2}(5.32 \mathrm{~m} / \mathrm{s})=0.00484 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{1 7 . 4} \mathbf{~ m}^{\mathbf{3}} / \mathbf{h} \quad$ Ans. (b) (c) From Eq. (6.12), the wall shear stress is

$$
\tau_{w}=\frac{8 \mu V}{d}=\frac{8(0.07 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{~s})(5.32 \mathrm{~m} / \mathrm{s})}{(0.034 \mathrm{~m})}=88 P a=\mathbf{0 . 0 8 8} \mathbf{~ k P a} \quad \text { Ans. (c) }
$$

6.30 SAE 10 oil at $20^{\circ} \mathrm{C}$ flows through the 4 -cm-diameter vertical pipe of Fig. P6.30. For the mercury manometer reading $h=42 \mathrm{~cm}$ shown, (a) calculate the volume flow rate in $\mathrm{m}^{3} / \mathrm{h}$, and (b) state the direction of flow.

Solution: For SAE 10 oil, take $\rho=$ $870 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.104 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The pressure at the lower point (1) is considerably higher than p2 according to the


Fig. P6.30 manometer reading:

$$
\begin{gathered}
\mathrm{p}_{1}-\mathrm{p}_{2}=\left(\rho_{\mathrm{Hg}}-\rho_{\text {oil }}\right) \mathrm{g} \Delta \mathrm{~h}=(13550-870)(9.81)(0.42) \approx 52200 \mathrm{~Pa} \\
\left.\Delta \mathrm{p} /\left(\rho_{\text {oil }} \mathrm{g}\right)=52200 / 870(9.81)\right] \approx 6.12 \mathrm{~m}
\end{gathered}
$$

This is more than 3 m of oil, therefore it must include a friction loss: flow is up. Ans. (b) The energy equation between (1) and (2), with $\mathrm{V} 1=\mathrm{V} 2$, gives

$$
\begin{aligned}
& \frac{\mathrm{p}_{1}-\mathrm{p}_{2}}{\rho \mathrm{~g}}=\mathrm{z}_{2}-\mathrm{z}_{1}+\mathrm{h}_{\mathrm{f}}, \quad \text { or } \quad 6.12 \mathrm{~m}=3 \mathrm{~m}+\mathrm{h}_{\mathrm{f}}, \quad \text { or: } \mathrm{h}_{\mathrm{f}} \approx 3.12 \mathrm{~m}=\frac{128 \mu \mathrm{LQ}}{\pi \rho \mathrm{gd}^{4}} \\
& \text { Compute } \quad \mathrm{Q}=\frac{(6.12-3) \pi(870)(9.81)(0.04)^{4}}{128(0.104)(3.0)}=0.00536 \frac{\mathrm{~m}^{3}}{\mathrm{~s}} \approx \mathbf{1 9 . 3} \frac{\mathbf{m}^{3}}{\mathrm{~h}} \quad \text { Ans. (a) }
\end{aligned}
$$

Check $\operatorname{Re}=4 \rho \mathrm{Q} /(\pi \mu \mathrm{d})=4(870)(0.00536) / \pi(0.104)(0.04)] \approx 1430(\mathrm{OK}$, laminar flow $)$.

P6.31 A laminar flow element or LFE (Meriam Instrument Co.) measures low gas-flow rates with a bundle of capillary tubes packed inside a large outer tube. Consider oxygen at $20^{\circ} \mathrm{C}$ and 1 atm flowing at $84 \mathrm{ft}^{3} / \mathrm{min}$ in a 4 -in-diameter pipe. (a) Is the flow approaching the element turbulent? (b) If there are 1000 capillary tubes, $L=4$ in, select a tube diameter to keep $\operatorname{Re}_{d}$ below 1500 and also to keep the tube pressure drop no greater than $0.5 \mathrm{lbf} / \mathrm{in}^{2}$. (c) Do the tubes selected in part (b) fit nicely within the approach pipe?

Solution: For oxygen at $20^{\circ} \mathrm{C}$ and 1 atm (Table A.4), take $R=260 \mathrm{~m}^{2} /\left(\mathrm{s}^{2} \mathrm{~K}\right)$, hence $\rho=p / R T=$ $(101350 \mathrm{~Pa}) /[260(293 \mathrm{~K})]=1.33 \mathrm{~kg} / \mathrm{m}^{3}=0.00258 \mathrm{slug} / \mathrm{ft}^{3}$. Also read $\mu=2.0 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m}-\mathrm{s}=4.18 \mathrm{E}-$ 7 slug/ft-s. Convert $Q=84 \mathrm{ft}^{3} / \mathrm{min}=1.4 \mathrm{ft}^{3} / \mathrm{s}$. Then the entry pipe Reynolds number is
$\operatorname{Re}_{D}=\frac{\rho V D}{\mu}=\frac{4 \rho Q}{\pi \mu D}=\frac{4\left(0.00258 s \text { slug } / f t^{3}\right)\left(1.4 \mathrm{ft}^{3} / s\right)}{\pi(4.18 E-7 \text { slug } / f t-s)(4 / 12 f t)}=33,000$ (turbulent) Ans.(a)
(b) To keep $\mathrm{Re}_{d}$ below 1500 and keep the (laminar) pressure drop no more than $72 \mathrm{psf}(0.5 \mathrm{psi})$,

$$
\operatorname{Re}_{d}=\frac{\rho V d}{\mu} \leq 1500 \text { and } \quad \Delta p=\frac{32 \mu L V}{d^{2}} \leq 72 \frac{l b f}{f t^{2}}, \quad \text { where } V=\frac{Q / 1000}{(\pi / 4) d^{2}}
$$

Select values of $d$ and iterate, or use EES. The upper limit on Reynolds number gives

$$
\mathrm{Re}_{d}=1500 \text { if } \quad d=0.00734 \mathrm{ft}=\mathbf{0 . 0 8 8} \mathbf{i n} ; \quad \Delta p=2.74 \mathrm{lbf} / \mathrm{ft}^{2} \text { Ans.(b) }
$$

This is a satisfactory answer, since the pressure drop is no problem, quite small. One thousand of these tubes would have an area about one-half of the pipe area, so would fit nicely. Ans.(c)

Increasing the tube diameter would lower $\mathrm{Re}_{d}$ and have even smaller pressure drop. Example: $d=0.01 \mathrm{ft}, \mathrm{Re}_{d}=1100, \Delta p=0.8 \mathrm{psf}$. These 0.01 - ft -diameter tubes would just barely fit into the larger pipe. One disadvantage, however, is that these tubes are short: the entrance length is longer than the tube length, and thus $\Delta p$ will be larger than calculated by "fully-developed" formulas.
6.32 SAE 30 oil at $20^{\circ} \mathrm{C}$ flows in the 3-cm-diameter pipe in Fig. P6.32, which slopes at $37^{\circ}$. For the pressure measurements shown, determine (a) whether the flow is up or down and (b) the flow rate in $\mathrm{m}^{3} / \mathrm{h}$.

Solution: For SAE 30 oil, take $\rho=891$ $\mathrm{kg} / \mathrm{m}^{3}$ and $\mu=0.29 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Evaluate the hydraulic grade lines:


Fig. P6.32

$$
\mathrm{HGL}_{\mathrm{B}}=\frac{\mathrm{P}_{\mathrm{B}}}{\rho \mathrm{~g}}+\mathrm{z}_{\mathrm{B}}=\frac{180000}{891(9.81)}+15=35.6 \mathrm{~m} ; \quad \mathrm{HGL}_{\mathrm{A}}=\frac{500000}{891(9.81)}+0=57.2 \mathrm{~m}
$$

Since $\mathrm{HGL}_{\mathrm{A}}>\mathrm{HGL}_{\mathrm{B}}$ the flow is up Ans. (a)
The head loss is the difference between hydraulic grade levels:

$$
\mathrm{h}_{\mathrm{f}}=57.2-35.6=21.6 \mathrm{~m}=\frac{128 \mu \mathrm{LQ}}{\pi \rho \mathrm{gd}^{4}}=\frac{128(0.29)(25) \mathrm{Q}}{\pi(891)(9.81)(0.03)^{4}}
$$

Solve for $\mathrm{Q}=0.000518 \mathrm{~m}^{3} / \mathrm{s} \approx \mathbf{1 . 8 6} \mathbf{~ m}^{\mathbf{3}} / \mathrm{h}$ Ans. (b)

$$
\text { Finally, check } \operatorname{Re}=4 \rho \mathrm{Q} /(\pi \mu \mathrm{d}) \approx 68 \text { (OK, laminar flow). }
$$

6.33 In Problem 6.32, suppose it is desired to add a pump between A and B to drive the oil upward from A to B at a rate of $3 \mathrm{~kg} / \mathrm{s}$. At $100 \%$ efficiency, what pump power is required?


Fig. P6.33
Solution: For SAE 30 oil at $20^{\circ} \mathrm{C}, \rho=891 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.29 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. With mass flow known, we can evaluate the pipe velocity:

$$
\begin{gathered}
V=\frac{\dot{m}}{\rho A}=\frac{3 \mathrm{~kg} / \mathrm{s}}{891 \pi(0.015)^{2}}=4.76 \frac{\mathrm{~m}}{\mathrm{~s}} \\
\text { Check } R e_{d}=\frac{891(4.76)(0.03)}{0.29}=439(\text { OK, laminar })
\end{gathered}
$$

Apply the steady flow energy equation between $A$ and $B$ :

$$
\frac{p_{A}}{\rho g}+\frac{V_{A}^{2}}{2 g}+z_{A}=\frac{p_{B}}{\rho g}+\frac{V_{B}^{2}}{2 g}+z_{B}+h_{f}-h_{p}, \quad \text { or }: \quad \frac{500000}{891(9.81)}=\frac{180000}{891(9.81)}+15+h_{f}-h_{p}
$$

where $h_{f}=\frac{32 \mu L V}{\rho g d^{2}}=\frac{32(0.29)(25)(4.76)}{891(9.81)(0.03)^{2}}=140.5 \mathrm{~m}$, Solve for $h_{\text {pump }}=118.9 \mathrm{~m}$
The pump power is then given by

$$
\text { Power }=\rho g Q h_{p}=\dot{m} g h_{p}=\left(3 \frac{\mathrm{~kg}}{\mathrm{~s}}\right)\left(9.81 \frac{\mathrm{~m}}{\mathrm{~s}^{2}}\right)(118.9 \mathrm{~m})=\mathbf{3 5 0 0} \text { watts Ans. }
$$

6.34 Derive the time-averaged $x$-momentum equation (6.21) by direct substitution of Eqs. (6.19) into the momentum equation (6.14). It is convenient to write the convective acceleration as

$$
\frac{d \mathrm{u}}{d \mathrm{t}}=\frac{\partial}{\partial \mathrm{x}}\left(\mathrm{u}^{2}\right)+\frac{\partial}{\partial \mathrm{y}}(\mathrm{uv})+\frac{\partial}{\partial \mathrm{z}}(\mathrm{uw})
$$

which is valid because of the continuity relation, Eq. (6.14).

Solution: Into the $x$-momentum eqn. substitute $u=u+u^{\prime}, v=v+v^{\prime}$, etc., to obtain

$$
\begin{gathered}
\rho\left[\frac{\partial}{\partial \mathrm{x}}\left(\overline{\mathrm{u}}^{2}+2 \overline{\mathrm{u}} \mathrm{u}^{\prime}+\mathrm{u}^{\prime 2}\right)+\frac{\partial}{\partial \mathrm{y}}\left(\overline{\mathrm{v}} \overline{\mathrm{u}}+\overline{\mathrm{v}} \mathrm{u}^{\prime}+\mathrm{v}^{\prime} \overline{\mathrm{u}}+\mathrm{v}^{\prime} u^{\prime}\right)+\frac{\partial}{\partial \mathrm{z}}\left(\overline{\mathrm{w}} \overline{\mathrm{u}}+\overline{\mathrm{w}} \mathrm{u}^{\prime}+\mathrm{w}^{\prime} \overline{\mathrm{u}}+\mathrm{w}^{\prime} \mathrm{u}^{\prime}\right)\right] \\
\\
\quad=-\frac{\partial}{\partial \mathrm{x}}\left(\overline{\mathrm{p}}+\mathrm{p}^{\prime}\right)+\rho \mathrm{g}_{\mathrm{x}}+\mu\left[\nabla^{2}\left(\overline{\mathrm{u}}+\mathrm{u}^{\prime}\right)\right]
\end{gathered}
$$

Now take the time-average of the entire equation to obtain Eq. (6.21) of the text:

$$
\rho\left[\frac{\mathbf{d} \overline{\mathbf{u}}}{\mathbf{d t}}+\frac{\partial}{\partial \mathbf{x}}\left(\overline{\mathbf{u}^{\prime 2}}\right)+\frac{\partial}{\partial \mathbf{y}}\left(\overline{\mathbf{u}^{\prime} \mathbf{v}^{\prime}}\right)+\frac{\partial}{\partial \mathbf{z}}\left(\overline{\mathbf{u}^{\prime} \mathbf{w}^{\prime}}\right)\right]=-\frac{\partial \overline{\mathbf{p}}}{\partial \mathbf{x}}+\rho \mathbf{g}_{\mathbf{x}}+\mu \nabla^{2}(\overline{\mathbf{u}}) \quad \text { Ans. }
$$

6.35 By analogy with Eq. (6.21) write the turbulent mean-momentum differential equation for (a) the $y$ direction and (b) the $z$ direction. How many turbulent stress terms appear in each equation? How many unique turbulent stresses are there for the total of three directions?

Solution: You can re-derive, as in Prob. 6.34, or just permute the axes:
(a) $\mathbf{y}: \rho \frac{\mathrm{d} \overline{\mathrm{v}}}{\mathrm{dt}}=-\frac{\partial \overline{\mathrm{p}}}{\partial \mathrm{y}}+\rho \mathbf{g}_{\mathrm{y}}+\frac{\partial}{\partial \mathrm{x}}\left(\mu \frac{\partial \overline{\mathrm{v}}}{\partial \mathrm{x}}-\rho \mathrm{u}^{\prime} \mathrm{v}^{\prime}\right)+\frac{\partial}{\partial \mathrm{y}}\left(\mu \frac{\partial \overline{\mathrm{v}}}{\partial \mathrm{y}}-\rho \mathrm{v}^{\prime} \mathrm{v}^{\prime}\right)$

$$
+\frac{\partial}{\partial \mathrm{z}}\left(\mu \frac{\partial \overline{\mathrm{v}}}{\partial \mathrm{z}}-\rho \mathrm{v}^{\prime} \mathrm{w}^{\prime}\right)
$$



$$
+\frac{\partial}{\partial \mathrm{z}}\left(\mu \frac{\partial \overline{\mathrm{w}}}{\partial \mathrm{z}}-\rho \mathrm{w}^{\prime} \mathrm{w}^{\prime}\right)
$$

6.36 The following turbulent-flow velocity data $u(y)$, for air at $75^{\circ} \mathrm{F}$ and 1 atm near a smooth flat wall, were taken in the University of Rhode Island wind tunnel:

| $\mathrm{y}, \mathrm{in}:$ | 0.025 | 0.035 | 0.047 | 0.055 | 0.065 |
| :--- | :--- | :--- | :--- | :--- | :--- |
| u, ft/s: | 51.2 | 54.2 | 56.8 | 57.6 | 59.1 |

Estimate (a) the wall shear stress and (b) the velocity $u$ at $y=0.22$ in.
Solution: For air at $75^{\circ} \mathrm{F}$ and 1 atm , take $\rho=0.00230$ slug $/ \mathrm{ft}^{3}$ and $\mu=3.80 \mathrm{E}-7 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. We fit each data point to the logarithmic-overlap law, Eq. (6.28):

$$
\frac{\mathrm{u}}{\mathrm{u}^{*}} \approx \frac{1}{\kappa} \ln \frac{\rho \mathrm{u}^{*} \mathrm{y}}{\mu}+\mathrm{B} \approx \frac{1}{0.41} \ln \left[\frac{0.0023 \mathrm{u}^{*} \mathrm{y}}{3.80 \mathrm{E}-7}\right]+5.0, \quad \mathrm{u}^{*}=\sqrt{\tau_{\mathrm{w}} / \rho}
$$

Enter each value of $u$ and $y$ from the data and estimate the friction velocity $u *$ :

| $\mathrm{y}, \mathrm{in}:$ | 0.025 | 0.035 | 0.047 | 0.055 | 0.065 |
| :--- | :--- | :--- | :--- | :--- | :--- |
| $\mathrm{u}^{*}, \mathrm{ft} / \mathrm{s}:$ | 3.58 | 3.58 | 3.59 | $\mathbf{3 . 5 6}$ | $\mathbf{3 . 5 6}$ |
| $\mathrm{yu}^{*} / \nu$ (approx): | 45 | 63 | 85 | 99 | 117 |

Each point gives a good estimate of $u^{*}$, because each point is within the logarithmic layer in Fig. 6.10 of the text. The overall average friction velocity is

$$
\begin{equation*}
\mathrm{u}_{\mathrm{avg}}^{*} \approx 3.57 \frac{\mathrm{ft}}{\mathrm{~s}} \pm 1 \%, \quad \tau_{\mathrm{w}, \mathrm{avg}}=\rho \mathrm{u}^{*^{2}}=(0.0023)(3.57)^{2} \approx \mathbf{0 . 0 2 9 3} \frac{\mathbf{l b f}}{\mathbf{f t}^{2}} \tag{a}
\end{equation*}
$$

Out at $\mathrm{y}=0.22$ inches, we may estimate that the log-law still holds:

$$
\begin{gathered}
\frac{\rho \mathrm{u}^{*} \mathrm{y}}{\mu}=\frac{0.0023(3.57)(0.22 / 12)}{3.80 \mathrm{E}-7} \approx 396, \quad \mathrm{u} \approx \mathrm{u}^{*}\left[\frac{1}{0.41} \ln (396)+5.0\right] \\
\text { or: } \mathrm{u} \approx(3.57)(19.59) \approx \mathbf{7 0} \frac{\mathbf{f t}}{\mathbf{s}} \text { Ans. (b) }
\end{gathered}
$$

Figure 6.10 shows that this point $\left(y^{+} \approx 396\right)$ seems also to be within the logarithmic layer.
6.37 Two infinite plates a distance $h$ apart are parallel to the $x z$ plane with the upper plate moving at speed $V$, as in Fig. P6.37. There is a fluid of viscosity $\mu$ and constant pressure between the plates. Neglecting gravity and assuming incompressible turbulent flow $u(y)$ between the plates, use the logarithmic law and appropriate


Fig. P6.37 boundary conditions to derive a formula for dimensionless wall shear stress versus dimensionless plate velocity. Sketch a typical shape of the profile $u(y)$.

Solution: The shear stress between parallel plates is constant, so the centerline velocity must be exactly $u=V / 2$ at $y=h / 2$. Anti-symmetric log-laws form, one with increasing velocity for $0<y<h / 2$, and one with decreasing velocity for $h / 2<y<h$, as shown below:


The match-point at the center gives us a log-law estimate of the shear stress:

$$
\frac{V}{2 u^{*}} \approx \frac{1}{\kappa} \ln \left(\frac{h u^{*}}{2 v}\right)+\mathrm{B}, \quad \kappa \approx 0.41, \mathrm{~B} \approx 5.0, \boldsymbol{u}^{*}=\left(\tau_{\mathrm{w}} / \rho\right)^{1 / 2} \quad \text { Ans. }
$$

This is one form of "dimensionless shear stress." The more normal form is friction coefficient versus Reynolds number. Calculations from the log-law fit a Power-law curve-fit expression in the range $2000<\operatorname{Reh}<1$ E5:

$$
\mathbf{C}_{\mathbf{f}}=\frac{\tau_{\mathrm{w}}}{(1 / 2) \rho V^{2}} \approx \frac{0.018}{(\rho V h / v)^{1 / 4}}=\frac{\mathbf{0 . 0 1 8}}{\mathbf{R e}_{\mathbf{h}}^{1 / 4}} \text { Ans. }
$$

6.38 Suppose in Fig. P6.37 that $h=3 \mathrm{~cm}$, the fluid is water at $20^{\circ} \mathrm{C}\left(\rho=998 \mathrm{~kg} / \mathrm{m}^{3}, \mu=\right.$ $0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$ ), and the flow is turbulent, so that the logarithmic law is valid. If the shear stress in the fluid is 15 Pa , estimate $V$ in $\mathrm{m} / \mathrm{s}$.

Solution: Just as in Prob. 6.37, apply the log-law at the center between the wall, that is, $y=h / 2, u=V / 2$. With $\tau_{\mathrm{w}}$ known, we can evaluate $u^{*}$ immediately:

$$
\begin{gathered}
u^{*}=\sqrt{\frac{\tau_{w}}{\rho}}=\sqrt{\frac{15}{998}}=0.123 \frac{m}{s}, \quad \frac{V / 2}{u^{*}} \approx \frac{1}{\kappa} \ln \left(\frac{u^{*} h / 2}{v}\right)+B, \\
\text { or: } \frac{V / 2}{0.123}=\frac{1}{0.41} \ln \left[\frac{0.123(0.03 / 2)}{0.001 / 998}\right]+5.0=23.3, \text { Solve for } \mathbf{V} \approx 5.72 \frac{\mathbf{m}}{\mathbf{s}} \text { Ans. }
\end{gathered}
$$

6.39 By analogy with laminar shear, $\tau=\mu d u / d y$. T. V. Boussinesq in 1877 postulated that turbulent shear could also be related to the mean-velocity gradient $\tau$ urb $=\varepsilon d u / d y$, where $\varepsilon$ is called the eddy viscosity and is much larger than $\mu$. If the logarithmic-overlap law, Eq. (6.28), is valid with $\tau \approx \tau w$, show that $\varepsilon \approx \kappa \rho u^{*} y$.

Solution: Differentiate the log-law, Eq. (6.28), to find du/dy, then introduce the eddy viscosity into the turbulent stress relation:

$$
\text { If } \frac{u}{u^{*}}=\frac{1}{\kappa} \ln \left(\frac{y u^{*}}{v}\right)+B, \quad \text { then } \frac{d u}{d y}=\frac{u^{*}}{\kappa y}
$$

Then, if $\tau \approx \tau_{w} \equiv \rho u^{*^{2}}=\varepsilon \frac{d u}{d y}=\varepsilon \frac{u^{*}}{\kappa y}$, solve for $\varepsilon=\kappa \rho \mathbf{u} * \mathbf{y}$ Ans.
6.40 Theodore von Kármán in 1930 theorized that turbulent shear could be represented by $\tau$ turb $=\varepsilon$ du/dy where $\varepsilon=\rho \kappa^{2} \mathrm{y}^{2} \mid$ du/dy $\mid$ is called the mixing-length eddy viscosity and $\kappa$ $\approx 0.41$ is Kármán's dimensionless mixing-length constant [2,3]. Assuming that $\tau$ turb $\approx \tau_{w}$ near the wall, show that this expression can be integrated to yield the logarithmic-overlap law, Eq. (6.28).

Solution: This is accomplished by straight substitution:

$$
\begin{aligned}
& \tau_{\text {turb }} \approx \tau_{\mathrm{w}}=\rho \mathrm{u}^{*^{2}}=\varepsilon \frac{\mathrm{du}}{\mathrm{dy}}=\left[\rho \kappa^{2} \mathrm{y}^{2}\left|\frac{\mathrm{du}}{\mathrm{dy}}\right|\right] \frac{\mathrm{du}}{\mathrm{dy}}, \quad \text { solve for } \frac{\mathrm{du}}{\mathrm{dy}}=\frac{\mathrm{u}^{*}}{\kappa \mathrm{y}} \\
& \text { Integrate: } \quad \int \mathrm{du}=\frac{\mathrm{u}^{*}}{\kappa} \int \frac{\mathrm{dy}}{\mathrm{y}}, \quad \text { or: } \quad \mathbf{u}=\frac{\mathbf{u}^{*}}{\kappa} \ln (\mathrm{y})+\text { constant Ans. }
\end{aligned}
$$

To convert this to the exact form of Eq. (6.28) requires fitting to experimental data.

P6.41 Two reservoirs, which differ in surface elevation by 40 m , are connected by 350 m of new pipe of diameter 8 cm . If the desired flow rate is at least $130 \mathrm{~N} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$, may the pipe material be (a) galvanized iron, (b) commercial steel, or (c) cast iron? Neglect minor losses.

Solution: Applying the extended Bernoulli equation between reservoir surfaces yields

$$
\Delta z=40 m=f \frac{L}{D} \frac{V^{2}}{2 g}=f\left(\frac{350 m}{0.08 m}\right) \frac{V^{2}}{2\left(9.81 m / s^{2}\right)}
$$

where $f$ and $V$ are related by the friction factor relation:

$$
\frac{1}{\sqrt{f}} \approx-2.0 \log _{10}\left(\frac{\varepsilon / D}{3.7}+\frac{2.51}{\operatorname{Re}_{D} \sqrt{f}}\right) \quad \text { where } \quad \operatorname{Re}_{D}=\frac{\rho V D}{\mu}
$$

When $V$ is found, the weight flow rate is given by $w=\rho g Q$ where $Q=\mathrm{AV}=\left(\pi D^{2} / 4\right) V$. For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. Given the desired $w=130 \mathrm{~N} / \mathrm{s}$, solve this system of equations by EES to yield the wall roughness. The results are:

$$
f=0.0257 ; V=2.64 \mathrm{~m} / \mathrm{s} ; \quad \mathrm{Re}_{\mathrm{D}}=211,000 ; \quad \varepsilon_{\max }=0.000203 \mathrm{~m}=\mathbf{0} .203 \mathrm{~mm}
$$

Any less roughness is OK. From Table 6-1, the three pipe materials have
(a) galvanized: $\varepsilon=0.15 \mathrm{~mm}$; (b) commercial steel: $\varepsilon=0.046 \mathrm{~mm}$; cast iron: $\varepsilon=0.26 \mathrm{~mm}$

Galvanized and steel are fine, but cast iron is too rough.. Ans. Actual flow rates are
(a) galvanized: $135 \mathrm{~N} / \mathrm{s}$;
(b) steel: $152 \mathrm{~N} / \mathrm{s}$;
(c) cast iron: $126 \mathrm{~N} / \mathrm{s}$ (not enough)
6.42 It is clear by comparing Figs. $6.12 b$ and 6.13 that the effects of sand roughness and commercial (manufactured) roughness are not quite the same. Take the special case of commercial roughness ratio $\varepsilon / d=0.001$ in Fig. 6.13, and replot in the form of the wall-law shift $\Delta B$ (Fig. 6.12a) versus the logarithm of $\varepsilon^{+}=\varepsilon u^{*} / v$. Compare your plot with Eq. (6.45).

Solution: To make this plot we must relate $\Delta \mathrm{B}$ to the Moody-chart friction factor. We use Eq. (6.33) of the text, which is valid for any $B$, in this case, $B=B_{o}-\Delta B$, where $B_{o} \approx 5.0$ :

$$
\begin{equation*}
\frac{\mathrm{V}}{\mathrm{u}^{*}} \approx \frac{1}{\kappa} \ln \left(\frac{\mathrm{Ru}^{*}}{v}\right)+\mathrm{B}_{0}-\Delta \mathrm{B}-\frac{3}{2 \kappa} \text {, where } \frac{\mathrm{V}}{\mathrm{u}^{*}}=\sqrt{\frac{8}{\mathrm{f}}} \text { and } \frac{\mathrm{Ru}^{*}}{v}=\frac{1}{2} \mathrm{Re}_{\mathrm{d}} \sqrt{\frac{\mathrm{f}}{8}} \tag{1}
\end{equation*}
$$

Combine Eq. (1) with the Colebrook friction formula (6.48) and the definition of $\varepsilon^{+}$:

$$
\begin{gather*}
\frac{1}{\sqrt{ } \mathrm{f}} \approx-2.0 \log _{10}\left(\frac{\varepsilon / d}{3.7}+\frac{2.51}{\operatorname{Re} \sqrt{ } \mathrm{f}}\right)  \tag{2}\\
\text { and } \quad \varepsilon^{+}=\frac{\varepsilon \mathrm{u}^{*}}{v}=\frac{\varepsilon}{d} d^{+}=\frac{\varepsilon}{d} \operatorname{Re} \sqrt{\frac{\mathrm{f}}{8}} \tag{3}
\end{gather*}
$$

Equations ( $1,2,3$ ) enable us to make the plot below of "commercial" log-shift $\Delta \mathrm{B}$, which is similar to the 'sand-grain’ shift predicted by Eq. (6.45): $\Delta \mathrm{B}$ sand $\approx(1 / \kappa) \ln \left(\varepsilon^{+}\right)-3.5$.


Ans.

Fig. P6.42

P6.43 A reservoir supplies water through

100 m of 30 -cm-diameter cast iron pipe to a turbine that extracts 80 hp from the flow.

The water then exhausts to the atmosphere.

| $z_{1}=35 \mathrm{~m}$ |  |
| :---: | :---: |
|  | water <br> at $20^{\circ} \mathrm{C}$ |
| $\longleftarrow=5 \mathrm{~m}$ |  |

Neglect minor losses. (a) Assuming that
Fig. P6.43
$f \approx 0.019$, find the flow rate (there is a cubic
polynomial). Explain why there are two solutions.
(b) For extra credit, solve for the flow rate using the actual friction factors.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. The energy equation yields a relation between elevation, friction, and turbine power:

$$
\begin{aligned}
& \frac{p_{1}}{\rho g}+\frac{V_{1}^{2}}{2 g}+z_{1}=\frac{p_{f}^{4}}{p g}+\frac{V_{2}^{2}}{2 g}+z_{2}+h_{t u r b}+h_{f} \\
& z_{1}-z_{2}=35-5 m=30 m=h_{t u r b}+h_{f}=\frac{\text { Power }}{\rho g Q}+\left(1+f \frac{L}{D}\right) \frac{V_{2}^{2}}{2 g}, Q=\frac{\pi}{4} D^{2} V_{2} \\
& \left.30 m=\frac{(80 h p)(745.7 \mathrm{~W} / h p)}{\left(9790 \mathrm{~N} / \mathrm{m}^{3}\right)(\pi / 4)(0.3 m)^{2} V_{2}}+\left[1+(0.019) \frac{100 m}{0.3 m}\right] \frac{V_{2}^{2}}{2\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)}\right]
\end{aligned}
$$

Clean this up into a cubic polynomial:

$$
30=\frac{86.2}{V}+0.373 V^{2}, \quad \text { or }: \quad V^{3}-80.3 V+231=0
$$

Three roots: $V=3.34 \mathrm{~m} / \mathrm{s} ; 6.81 \mathrm{~m} / \mathrm{s} ;-10.15 \mathrm{~m} / \mathrm{s}$

The third (negative) root is meaningless. The other two are correct. Either

$$
\begin{aligned}
& \mathbf{Q}=\mathbf{0 . 4 8 1} \mathbf{m}^{3} / \mathbf{s}, \quad h_{\text {turbine }}=12.7 \mathrm{~m}, \quad h_{\mathrm{f}}=17.3 \mathrm{~m} \\
& \mathbf{Q}=\mathbf{0 . 2 3 6} \mathbf{~ m}^{3} / \mathbf{s}, \quad h_{\text {turbine }}=25.8 \mathrm{~m}, \quad h_{\mathrm{f}}=4.2 \mathrm{~m} \quad \text { Ans. }(a) \text { ) }
\end{aligned}
$$

Both solutions are valid. The higher flow rate wastes a lot of water and creates 17 meters of friction loss. The lower rate uses $51 \%$ less water and has proportionately much less friction.
(b) The actual friction factors are very close to the problem's "Guess". Thus we obtain $\operatorname{Re}=2.04 \mathrm{E} 6, f=0.0191 ; \quad \mathbf{Q}=\mathbf{0 . 4 7 9} \mathbf{~ m}^{3} / \mathbf{s}, \quad h_{\text {turbine }}=12.7 \mathrm{~m}, \quad h_{\mathrm{f}}=17.3 \mathrm{~m}$ $\operatorname{Re}=1.01 \mathrm{E} 6, f=0.0193 ; \quad \mathbf{Q}=\mathbf{0 . 2 3 7} \mathbf{m}^{3} / \mathbf{s}, \quad h_{\text {turbine }}=25.7 \mathrm{~m}, \quad \mathrm{~h}_{\mathrm{f}}=4.3 \mathrm{~m}$ Ans.(b)

The same remarks apply: The lower flow rate is better, less friction, less water used.
6.44 Mercury at $20^{\circ} \mathrm{C}$ flows through 4 meters of $7-\mathrm{mm}$-diameter glass tubing at an average velocity of $5 \mathrm{~m} / \mathrm{s}$. Estimate the head loss in meters and the pressure drop in kPa .

Solution: For mercury at $20^{\circ} \mathrm{C}$, take $\rho=13550 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.00156 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Glass tubing is considered hydraulically "smooth," $\varepsilon / d=0$. Compute the Reynolds number:

$$
\begin{aligned}
R e_{d}=\frac{\rho V d}{\mu} & =\frac{13550(5)(0.007)}{0.00156}=304,000 ; \quad \text { Moody chart smooth: } f \approx 0.0143 \\
h_{f} & =f \frac{L}{d} \frac{V^{2}}{2 g}=0.0143\left(\frac{4.0}{0.007}\right) \frac{5^{2}}{2(9.81)}=\mathbf{1 0 . 4} \mathbf{~ m} \quad \text { Ans. (a) } \\
\Delta p=\rho g h_{f} & =(13550)(9.81)(10.4)=1,380,000 \mathrm{~Pa}=\mathbf{1 3 8 0} \mathbf{~ k P a} \quad \text { Ans. (b) }
\end{aligned}
$$

6.45 Oil, SG $=0.88$ and $v=4 \mathrm{E}-5 \mathrm{~m}^{2} / \mathrm{s}$, flows at $400 \mathrm{gal} / \mathrm{min}$ through a 6 -inch asphalted cast-iron pipe. The pipe is 0.5 miles long ( 2640 ft ) and slopes upward at $8^{\circ}$ in the flow direction. Compute the head loss in feet and the pressure change.

Solution: First convert $400 \mathrm{gal} / \mathrm{min}=0.891 \mathrm{ft}^{3} / \mathrm{s}$ and $v=0.000431 \mathrm{ft}^{2} / \mathrm{s}$. For asphalted cast-iron, $\varepsilon=0.0004 \mathrm{ft}$, hence $\varepsilon / d=0.0004 / 0.5=0.0008$. Compute $V$, Red, and $f$ :

$$
\begin{gathered}
V=\frac{0.891}{\pi(0.25)^{2}}=4.54 \frac{f t}{s} ; \quad R e_{d}=\frac{4.54(0.5)}{0.000431}=5271 ; \quad \text { calculate } \quad f_{\text {Moody }}=0.0377 \\
\text { then } \quad h_{f}=f \frac{L}{d} \frac{V^{2}}{2 g}=0.0377\left(\frac{2640}{0.5}\right) \frac{(4.54)^{2}}{2(32.2)}=\mathbf{6 3 . 8} \mathbf{f t} \quad \text { Ans. (a) }
\end{gathered}
$$

If the pipe slopes upward at $8^{\circ}$, the pressure drop must balance both friction and gravity:

$$
\Delta p=\rho g\left(h_{f}+\Delta z\right)=0.88(62.4)\left[63.8+2640 \sin 8^{\circ}\right]=23700 \frac{\mathbf{l b f}}{\mathbf{f t}^{2}} \quad \text { Ans. (b) }
$$

6.46 Kerosene at $20^{\circ} \mathrm{C}$ is pumped at $0.15 \mathrm{~m}^{3} / \mathrm{s}$ through 20 km of 16 -cm-diameter castiron horizontal pipe. Compute the input power in kW required if the pumps are 85 percent efficient.

Solution: For kerosene at $20^{\circ} \mathrm{C}$, take $\rho=804 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.92 \mathrm{E}-3 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast iron take $\varepsilon \approx 0.26 \mathrm{~mm}$, hence $\varepsilon / d=0.26 / 160 \approx 0.001625$. Compute $\mathrm{V}, \mathrm{Re}$, and f :

$$
\begin{aligned}
\mathrm{V}=\frac{0.15}{(\pi / 4)(0.16)^{2}} & =7.46 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{~d}}=\frac{4(804)(0.15)}{\pi(0.00192)(0.16)} \approx 500,000 \\
\varepsilon / d & \approx 0.001625: \quad \text { Moody chart: } \mathrm{f} \approx 0.0226 \\
\text { Then } \quad \mathrm{h}_{\mathrm{f}} & =\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=(0.0226)\left(\frac{20000}{0.16}\right) \frac{(7.46)^{2}}{2(9.81)} \approx 8020 \mathrm{~m}
\end{aligned}
$$

At $85 \%$ efficiency, the pumping power required is:

$$
\mathrm{P}=\frac{\rho \mathrm{gQh}_{\mathrm{f}}}{\eta}=\frac{804(9.81)(0.15)(8020)}{0.85} \approx 11.2 \mathrm{E}+6 \mathrm{~W}=11.2 \mathbf{M W} \quad \text { Ans. }
$$

6.47 The gutter and smooth drainpipe in Fig. P6.47 remove rainwater from the roof of a building. The smooth drainpipe is 7 cm in diameter. (a) When the gutter is full, estimate the rate of draining. (b) The gutter is designed for a sudden rainstorm of up to 5 inches per hour. For this condition, what is the maximum roof area that can be drained successfully?

Solution: If the velocity at the gutter surface is neglected, the energy equation reduces to


Fig. P6.47

$$
\Delta z=\frac{V^{2}}{2 g}+h_{f}, \quad h_{f}=f \frac{L}{d} \frac{V^{2}}{2 g}, \quad \text { solve } V^{2}=\frac{2 g \Delta z}{1+f L / d}=\frac{2(9.81)(4.2)}{1+f(4.2 / 0.07)}
$$

For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Guess $f \approx 0.02$ to obtain the velocity estimate $\mathrm{V} \approx 6 \mathrm{~m} / \mathrm{s}$ above. Then Red $\approx \rho V d / \mu \approx(998)(6)(0.07) /(0.001) \approx 428,000$ (turbulent). Then, for a smooth pipe, $f \approx 0.0135$, and V is changed slightly to $6.74 \mathrm{~m} / \mathrm{s}$. After convergence, we obtain

$$
V=6.77 \mathrm{~m} / \mathrm{s}, \quad Q=V(\pi / 4)(0.07)^{2}=\mathbf{0 . 0 2 6} \mathbf{~ m}^{3} / \mathbf{s} \quad \text { Ans. (a) }
$$

A rainfall of $5 \mathrm{in} / \mathrm{h}=(5 / 12 \mathrm{ft} / \mathrm{h})(0.3048 \mathrm{~m} / \mathrm{ft}) /(3600 \mathrm{~s} / \mathrm{h})=0.0000353 \mathrm{~m} / \mathrm{s}$. The required roof area is

$$
A_{\text {roof }}=Q_{\text {drain }} / V_{\text {rain }}=\left(0.026 \mathrm{~m}^{3} / \mathrm{s}\right) / 0.0000353 \mathrm{~m} / \mathrm{s} \approx 740 \mathrm{~m}^{2} \text { Ans. (b) }
$$

6.48 Show that if Eq. (6.33) is accurate, the position in a turbulent pipe flow where local velocity $u$ equals average velocity $V$ occurs exactly at $r=0.777 R$, independent of the Reynolds number.

Solution: Simply find the log-law position $\mathrm{y}^{+}$where $\mathrm{u}^{+}$exactly equals $\mathrm{V} / \mathrm{u}^{*}$ :

$$
\mathrm{V}=\mathrm{u}^{*}\left[\frac{1}{\kappa} \ln \frac{\mathrm{Ru}^{*}}{v}+\mathrm{B}-\frac{3}{2 \kappa}\right] \stackrel{?}{=} \mathrm{u}^{*}\left[\frac{1}{\kappa} \ln \frac{\mathrm{yu}^{*}}{v}+\mathrm{B}\right] \quad \text { if } \frac{1}{\kappa} \ln \frac{\mathrm{y}}{\mathrm{R}}=-\frac{3}{2 \kappa}
$$

Since $\mathrm{y}=\mathrm{R}-\mathrm{r}$, this is equivalent to $\frac{\mathrm{r}}{\mathrm{R}}=1-\mathrm{e}^{-3 / 2}=1-0.223 \approx \mathbf{0 . 7 7 7}$ Ans.
6.49 The tank-pipe system of Fig. P6.49 is to deliver at least $11 \mathrm{~m}^{3} / \mathrm{h}$ of water at $20^{\circ} \mathrm{C}$ to the reservoir. What is the maximum roughness height $\varepsilon$ allowable for the pipe?


Fig. P6. 49

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Evaluate V and Re for the expected flow rate:

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{11 / 3600}{(\pi / 4)(0.03)^{2}}=4.32 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \mathrm{Re}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{998(4.32)(0.03)}{0.001}=129000
$$

The energy equation yields the value of the head loss:

$$
\frac{\mathrm{p}_{\mathrm{atm}}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}=\frac{\mathrm{p}_{\mathrm{atm}}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}} \quad \text { or } \quad \mathrm{h}_{\mathrm{f}}=4-\frac{(4.32)^{2}}{2(9.81)}=3.05 \mathrm{~m}
$$

But also $\quad h_{f}=\mathrm{f} \frac{\mathrm{L}}{\mathrm{d}} \frac{\mathrm{V}^{2}}{2 \mathrm{~g}}, \quad$ or: $\quad 3.05=\mathrm{f}\left(\frac{5.0}{0.03}\right) \frac{(4.32)^{2}}{2(9.81)}, \quad$ solve for $\mathrm{f} \approx 0.0192$
With f and Re known, we can find $\varepsilon / d$ from the Moody chart or from Eq. (6.48):

$$
\frac{1}{(0.0192)^{1 / 2}}=-2.0 \log _{10}\left[\frac{\varepsilon / d}{3.7}+\frac{2.51}{129000(0.0192)^{1 / 2}}\right], \text { solve for } \frac{\varepsilon}{d} \approx 0.000394
$$

Then $\varepsilon=0.000394(0.03) \approx 1.2 \mathrm{E}-5 \mathrm{~m} \approx \mathbf{0 . 0 1 2} \mathbf{~ m m}$ (very smooth) Ans.
6.50 Ethanol at $20^{\circ} \mathrm{C}$ flows at 125 U.S. gal/min through a horizontal cast-iron pipe with $\mathrm{L}=12 \mathrm{~m}$ and $\mathrm{d}=5 \mathrm{~cm}$. Neglecting entrance effects, estimate (a) the pressure gradient, $\mathrm{dp} / \mathrm{dx}$; (b) the wall shear stress, $\tau_{\mathrm{w}}$; and (c) the percent reduction in friction factor if the pipe walls are polished to a smooth surface.

Solution: For ethanol (Table A-3) take $\rho=789 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.0012 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Convert $125 \mathrm{gal} / \mathrm{min}$ to $0.00789 \mathrm{~m}^{3} / \mathrm{s}$. Evaluate $\mathrm{V}=\mathrm{Q} / \mathrm{A}=0.00789 /\left[\pi(0.05)^{2} / 4\right]=4.02 \mathrm{~m} / \mathrm{s}$.

$$
\begin{aligned}
& R e_{d}=\frac{\rho V d}{\mu}=\frac{789(4.02)(0.05)}{0.0012}=132,000, \quad \frac{\varepsilon}{d}=\frac{0.26 \mathrm{~mm}}{50 \mathrm{~mm}}=0.0052 \text { Then } f_{\text {Moody }} \approx 0.0314 \\
& \text { (b) } \tau_{w}=\frac{f}{8} \rho V^{2}=\frac{0.0314}{8}(789)(4.02)^{2}=50 \mathbf{P a} \text { Ans. (b) } \\
& \text { (a) } \frac{d p}{d x}=-\frac{4 \tau_{w}}{d}=\frac{-4(50)}{0.05}=-\mathbf{4 0 0 0} \frac{\mathbf{P a}}{\mathbf{m}} \text { Ans. (a) }
\end{aligned}
$$

(c) $R e=132000, \quad f_{\text {smooth }}=0.0170$, hence the reduction in f is

$$
\left(1-\frac{0.0170}{0.0314}\right)=\mathbf{4 6 \%} \quad \text { Ans. (c) }
$$

6.51 The viscous sublayer (Fig. 6.10) is normally less than 1 percent of the pipe diameter and therefore very difficult to probe with a finite-sized instrument. In an effort to generate a thick sublayer for probing, Pennsylvania State University in 1964 built a pipe with a flow of glycerin. Assume a smooth 12 -in-diameter pipe with $V=60 \mathrm{ft} / \mathrm{s}$ and glycerin at $20^{\circ} \mathrm{C}$. Compute the sublayer thickness in inches and the pumping horsepower required at 75 percent efficiency if $L=40 \mathrm{ft}$.

Solution: For glycerin at $20^{\circ} \mathrm{C}$, take $\rho=2.44 \mathrm{slug} / \mathrm{ft}^{3}$ and $\mu=0.0311 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. Then

$$
\begin{gathered}
\mathrm{Re}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{2.44(60)(1 \mathrm{ft})}{0.0311}=4710 \text { (barely turbulent!) Smooth: } \mathrm{f}_{\text {Moody }} \approx 0.0380 \\
\text { Then } \mathrm{u}^{*}=\mathrm{V}(\mathrm{f} / 8)^{1 / 2}=60\left(\frac{0.0380}{8}\right)^{1 / 2} \approx 4.13 \frac{\mathrm{ft}}{\mathrm{~s}}
\end{gathered}
$$

The sublayer thickness is defined by $\mathrm{y}^{+} \approx 5.0=\rho \mathrm{yu}^{*} / \mu$. Thus

$$
\mathrm{y}_{\text {sublayer }} \approx \frac{5 \mu}{\rho \mathrm{u}^{*}}=\frac{5(0.0311)}{(2.44)(4.13)}=0.0154 \mathrm{ft} \approx \mathbf{0 . 1 8 5} \text { inches } \quad \text { Ans. }
$$

With $f$ known, the head loss and the power required can be computed:

$$
\begin{gathered}
\mathrm{h}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=(0.0380)\left(\frac{40}{1}\right) \frac{(60)^{2}}{2(32.2)} \approx 85 \mathrm{ft} \\
\mathrm{P}=\frac{\rho \mathrm{gQh}_{\mathrm{f}}}{\eta}=\frac{1}{0.75}(2.44)(32.2)\left[\frac{\pi}{4}(1)^{2}(60)\right](85)=419000 \div 550 \approx 760 \mathrm{hp} \quad \text { Ans. }
\end{gathered}
$$

6.52 The pipe flow in Fig. P6.52 is driven by pressurized air in the tank. What gage pressure $p 1$ is needed to provide a $20^{\circ} \mathrm{C}$ water flow rate $Q=60 \mathrm{~m}^{3} / \mathrm{h}$ ?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Get V, Re, f:

$$
\mathrm{V}=\frac{60 / 3600}{(\pi / 4)(0.05)^{2}}=8.49 \frac{\mathrm{~m}}{\mathrm{~s}}
$$



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Fig. P6.52

$$
\operatorname{Re}=\frac{998(8.49)(0.05)}{0.001} \approx 424000 ; \quad \mathrm{f}_{\mathrm{smooth}} \approx \mathbf{0 . 0 1 3 6}
$$

Write the energy equation between points (1) (the tank) and (2) (the open jet):

$$
\begin{aligned}
\frac{\mathrm{p}_{1}}{\rho \mathrm{~g}}+\frac{0^{2}}{2 \mathrm{~g}}+10=\frac{0}{\rho \mathrm{~g}} & +\frac{\mathrm{V}_{\text {pipe }}^{2}}{2 \mathrm{~g}}+80+\mathrm{h}_{\mathrm{f}}, \quad \text { where } \mathrm{h}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}} \text { and } \mathrm{V}_{\text {pipe }}=8.49 \frac{\mathrm{~m}}{\mathrm{~s}} \\
\text { Solve } \mathrm{p}_{1} & =(998)(9.81)\left[80-10+\frac{(8.49)^{2}}{2(9.81)}\left\{1+0.0136\left(\frac{170}{0.05}\right)\right\}\right] \\
& \approx 2.38 \mathrm{E} 6 \text { Pa Ans. }
\end{aligned}
$$

[This is a gage pressure (relative to the pressure surrounding the open jet.)]
6.53 In Fig. P6.52 suppose $\mathrm{p} 1=700 \mathrm{kPa}$ and the fluid specific gravity is 0.68 . If the flow rate is $27 \mathrm{~m}^{3} / \mathrm{h}$, estimate the viscosity of the fluid. What fluid in Table A-5 is the likely suspect?
Solution: Evaluate $\rho=0.68(998)=679 \mathrm{~kg} / \mathrm{m}^{3}$. Evaluate $\mathrm{V}=\mathrm{Q} / \mathrm{A}=(27 / 3600) /\left[\pi(0.025)^{2}\right]=$ $3.82 \mathrm{~m} / \mathrm{s}$. The energy analysis of the previous problem now has $f$ as the unknown:

$$
\frac{p_{1}}{\rho g}=\frac{700000}{679(9.81)}=\Delta z+\frac{V^{2}}{2 g}+f \frac{L}{d} \frac{V^{2}}{2 g}=70+\frac{(3.82)^{2}}{2(9.81)}\left[1+f \frac{170}{0.05}\right], \text { solve } f=0.0136
$$

Smooth pipe: $f=0.0136, R e_{d}=416000=\frac{679(3.82)(0.05)}{\mu}$,

$$
\text { Solve } \quad \mu=\mathbf{0 . 0 0 0 3 1} \frac{\mathbf{k g}}{\mathbf{m} \cdot \mathbf{s}} \quad \text { Ans. }
$$

The density and viscosity are close to the likely suspect, gasoline. Ans.
6.54* A swimming pool W by Y by $h$ deep is to be emptied by gravity through the long pipe shown in Fig. P6.54. Assuming an average pipe friction factor $f$ av and neglecting minor losses, derive a formula for the time to empty the tank from an initial level $h_{o}$.


Fig. P6.54
Solution: With no driving pressure and negligible tank surface velocity, the energy equation can be combined with a control-volume mass conservation:

$$
h(t)=\frac{V^{2}}{2 g}+f_{a v} \frac{L}{D} \frac{V^{2}}{2 g}, \quad \text { or: } \quad Q_{\text {out }}=A_{p i p e} V=\frac{\pi}{4} D^{2} \sqrt{\frac{2 g h}{1+f_{a v} L / D}}=-W Y \frac{d h}{d t}
$$

We can separate the variables and integrate for time to drain:

$$
\begin{aligned}
& \quad \frac{\pi}{4} D^{2} \sqrt{\frac{2 g}{1+f_{a v} L / D}} \int_{0}^{t} d t=-W Y \int_{h_{o}}^{0} \frac{d h}{\sqrt{h}}=-W Y\left(0-2 \sqrt{h_{o}}\right) \\
& \text { Clean this up to obtain: } \quad \boldsymbol{t}_{\text {drain }} \approx \frac{\mathbf{4 W Y}}{\boldsymbol{\pi} \boldsymbol{D}^{2}}{\sqrt{\frac{\mathbf{2 \boldsymbol { h } _ { o } ( \mathbf { 1 } + \boldsymbol { f } _ { a v } \mathbf { L } / \boldsymbol { D } )}}{\boldsymbol{g}}}}^{\mathbf{1 / 2}} \text { Ans. }
\end{aligned}
$$

6.55 The reservoirs in Fig. P6.55 contain water at $20^{\circ} \mathrm{C}$. If the pipe is smooth with $L=$ 4500 m and $d=4 \mathrm{~cm}$, what will the flow rate in $\mathrm{m}^{3} / \mathrm{h}$ be for $\Delta z=100 \mathrm{~m}$ ?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The energy equation from surface 1 to surface 2 gives

$$
\mathrm{p}_{1}=\mathrm{p}_{2} \quad \text { and } \quad \mathrm{V}_{1}=\mathrm{V}_{2},
$$



Fig. P6.55
thus $\mathrm{h}_{\mathrm{f}}=\mathrm{z}_{1}-\mathrm{z}_{2}=100 \mathrm{~m}$

$$
\text { Then } 100 \mathrm{~m}=\mathrm{f}\left(\frac{4500}{0.04}\right) \frac{\mathrm{V}^{2}}{2(9.81)}, \quad \text { or } \quad \mathrm{fV}^{2} \approx 0.01744
$$

Iterate with an initial guess of $\mathrm{f} \approx 0.02$, calculating V and Re and improving the guess:

$$
\begin{aligned}
\mathrm{V} & \approx\left(\frac{0.01744}{0.02}\right)^{1 / 2} \approx 0.934 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re} \approx \frac{998(0.934)(0.04)}{0.001} \approx 37300, \quad \mathrm{f}_{\text {smooth }} \approx 0.0224 \\
& \mathrm{~V}_{\text {better }} \approx\left(\frac{0.01744}{0.0224}\right)^{1 / 2} \approx 0.883 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \operatorname{Re}_{\text {better }} \approx 35300, \quad \mathrm{f}_{\text {better }} \approx 0.0226, \text { etc...... }
\end{aligned}
$$

This process converges to

$$
\mathrm{f}=0.0227, \mathrm{Re}=35000, \mathrm{~V}=0.877 \mathrm{~m} / \mathrm{s}, \mathrm{Q} \approx 0.0011 \mathrm{~m}^{3} / \mathrm{s} \approx 4.0 \mathrm{~m}^{3} / \mathbf{h} . \quad \text { Ans. }
$$

6.56 Consider a horizontal 4-ft-diameter galvanized-iron pipe simulating the Alaska Pipeline. The oil flow is 70 million U.S. gallons per day, at a density of $910 \mathrm{~kg} / \mathrm{m}^{3}$ and viscosity of $0.01 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$ (see Fig. A. 1 for SAE 30 oil at $100^{\circ} \mathrm{C}$ ). Each pump along the line raises the oil pressure to 8 MPa , which then drops, due to head loss, to 400 kPa at the entrance to the next pump. Estimate (a) the appropriate distance between pumping stations; and (b) the power required if the pumps are $88 \%$ efficient.

Solution: For galvanized iron take $\varepsilon=0.15 \mathrm{~mm}$. Convert $d=4 \mathrm{ft}=1.22 \mathrm{~m}$. Convert $Q=$ $7 \mathrm{E} 7 \mathrm{gal} /$ day $=3.07 \mathrm{~m}^{3} / \mathrm{s}$. The flow rate gives the velocity and Reynolds number:

$$
\begin{gathered}
V=\frac{Q}{A}=\frac{3.07}{\pi(1.22)^{2} / 4}=2.63 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \operatorname{Re} e_{d}=\frac{\rho V d}{\mu}=\frac{910(2.63)(1.22)}{0.01}=292,500 \\
\frac{\varepsilon}{d}=\frac{0.15 \mathrm{~mm}}{1220 \mathrm{~mm}}=0.000123, \quad f_{\text {Moody }} \approx 0.0157
\end{gathered}
$$

Relating the known pressure drop to friction factor yields the unknown pipe length:

$$
\begin{gathered}
\Delta p=8,000,00-400,000 P a=f \frac{L}{d} \frac{\rho}{2} V^{2}=0.0157 \frac{L}{1.22}\left(\frac{910}{2}\right)(2.63)^{2}, \\
\text { Solve } L=\mathbf{1 8 8 , 0 0 0} \mathbf{m}=117 \text { miles Ans. (a) }
\end{gathered}
$$

The pumping power required follows from the pressure drop and flow rate:

$$
\begin{aligned}
\text { Power } & =\frac{Q \Delta p}{\text { Efficiency }}=\frac{3.07(8 E 6-4 E 5)}{0.88}=2.65 E 7 \text { watts } \\
& =\mathbf{2 6 . 5} \mathbf{~ M W}(35,500 \mathrm{hp}) \text { Ans. (b) }
\end{aligned}
$$

6.57 Apply the analysis of Prob. 6.54 to the following data. Let $W=5 \mathrm{~m}, Y=8 \mathrm{~m}, h_{0}=$ $2 \mathrm{~m}, L=15 \mathrm{~m}, D=5 \mathrm{~cm}$, and $\varepsilon=0$. (a) By letting $h=1.5 \mathrm{~m}$ and 0.5 m as representative depths, estimate the average friction factor. Then (b) estimate the time to drain the pool.

Solution: For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The velocity in Prob. 6.54 is calculated from the energy equation:

$$
V=\sqrt{\frac{2 g h}{1+f L / D}} \text { with } f=f c n\left(\operatorname{Re}_{D}\right)_{\text {smooth pipe }} \text { and } \operatorname{Re}_{D}=\frac{\rho V D}{\mu}, L / D=300
$$

(a) With a bit of iteration for the Moody chart, we obtain ReD $=108,000$ and $f \approx$ 0.0177 at $h=1.5 \mathrm{~m}$, and $\operatorname{ReD}=59,000$ and $f \approx .0202$ at $h=0.5 \mathrm{~m}$; thus the average value $\boldsymbol{f} \mathbf{a v} \approx \mathbf{0 . 0 1 9}$. Ans. (a)

The drain formula from Prob. 6.54 then predicts:

$$
\begin{aligned}
t_{\text {drain }} & \approx \frac{4 W Y}{\pi D^{2}} \sqrt{\frac{2 h_{o}\left(1+f_{a v} L / D\right)}{g}} \approx \frac{4(5)(8)}{\pi(0.05)^{2}} \sqrt{\frac{2(2)[1+0.019(300)]}{9.81}} \\
& =33700 \mathrm{~s}=\mathbf{9 . 4} \mathbf{~ h} \quad \text { Ans. (b) }
\end{aligned}
$$

6.58 In Fig. P6.55 assume that the pipe is cast iron with $L=550 \mathrm{~m}, d=7 \mathrm{~cm}$, and $\Delta z=$ 100 m . If an 80 percent efficient pump is placed at point $B$, what input power is required to deliver $160 \mathrm{~m}^{3} / \mathrm{h}$ of water upward from reservoir 2 to 1 ?


Fig. P6.55

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Compute V, Re:

$$
\begin{aligned}
\mathrm{V}= & \frac{\mathrm{Q}}{\mathrm{~A}}=\frac{160 / 3600}{(\pi / 4)(0.07)^{2}} \approx 11.55 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \operatorname{Re}=\frac{998(11.55)(0.07)}{0.001} \approx 807000 \\
& \left.\frac{\varepsilon}{d}\right|_{\text {cast i ron }}=\frac{0.26 \mathrm{~mm}}{70 \mathrm{~mm}} \approx 0.00371 ; \quad \text { Moody chart: } \mathrm{f} \approx 0.00280
\end{aligned}
$$

The energy equation from surface 1 to surface 2 , with a pump at $B$, gives

$$
\begin{gathered}
\mathrm{h}_{\text {pump }}=\Delta \mathrm{z}+\mathrm{h}_{\mathrm{f}}=100+(0.0280)\left(\frac{550}{0.07}\right) \frac{(11.55)^{2}}{2(9.81)}=100+1494 \approx 1594 \mathrm{~m} \\
\text { Power }=\frac{\rho \mathrm{gQh}_{\mathrm{p}}}{\eta}=\frac{(998)(9.81)(160 / 3600)(1594)}{0.80}=8.67 \mathrm{E} 5 \mathrm{~W} \approx \mathbf{8 6 7} \mathbf{~ k W} \text { Ans. }
\end{gathered}
$$

6.59 The following data were obtained for flow of $20^{\circ} \mathrm{C}$ water at $20 \mathrm{~m}^{3} / \mathrm{hr}$ through a badly corroded 5 -cm-diameter pipe which slopes downward at an angle of $8^{\circ}: \mathrm{p} 1=420 \mathrm{kPa}$, $\mathrm{z} 1=12 \mathrm{~m}, \mathrm{p} 2=250 \mathrm{kPa}, \mathrm{z} 2=3 \mathrm{~m}$. Estimate (a) the roughness ratio of the pipe; and (b) the percent change in head loss if the pipe were smooth and the flow rate the same.

Solution: The pipe length is given indirectly as $L=\Delta z / \sin \theta=(9 \mathrm{~m}) / \sin 8^{\circ}=64.7 \mathrm{~m}$. The steady flow energy equation then gives the head loss:

$$
\begin{gathered}
\frac{p_{1}}{\rho g}+\frac{V_{1}^{2}}{2 g}+z_{1}=\frac{p_{2}}{\rho g}+\frac{V_{2}^{2}}{2 g}+z_{2}+h_{f}, \quad \text { or: } \quad \frac{420000}{9790}+12=\frac{250000}{9790}+3+h_{f}, \\
\text { Solve } h_{f}=26.4 \mathrm{~m}
\end{gathered}
$$

Now relate the head loss to the Moody friction factor:

$$
h_{f}=26.4=f \frac{L}{d} \frac{V^{2}}{2 g}=f \frac{64.7}{0.05} \frac{(2.83)^{2}}{2(9.81)}, \text { Solve } f=0.050, R e=141000, \text { Read } \frac{\varepsilon}{d} \approx 0.0211
$$

The estimated (and uncertain) pipe roughness is thus $\varepsilon=0.0211 d \approx \mathbf{1 . 0 6} \mathbf{~ m m ~ A n s . ~ ( a ) ~}$
(b) At the same Red $=141000, f_{\text {smooth }}=0.0168$, or $\mathbf{6 6 \%}$ less head loss. Ans. (b)

P6.60 In the spirit of Haaland's explicit pipe friction factor approximation, Eq. (6.49), Jeppson [20] proposed the following explicit formula:

$$
\frac{1}{\sqrt{f}} \approx-2.0 \log _{10}\left(\frac{\varepsilon / d}{3.7}+\frac{5.74}{\mathrm{Re}_{d}^{0.9}}\right)
$$

(a) Is this identical to Haaland's formula and just a simple rearrangement? Explain.
(b) Compare Jeppson to Haaland for a few representative values of (turbulent) $\mathrm{Re}_{\mathrm{d}}$ and $\varepsilon / d$ and their deviations compared to the Colebrook formula (6.48).

Solution: (a) No, it looks like a rearrangement of Haaland's formula, but it is not. Haaland started with Colebrook's smooth-wall formula and added just enough $\varepsilon / d$ effect for accuracy. Jeppson started with the rough-wall formula and added just enough $\operatorname{Re}_{d}$ effect for accuracy. Both are excellent approximations over the full (turbulent) range of $\mathrm{Re}_{d}$ and $\varepsilon / d$. Their predicted values of $f$ are nearly the same and very close to the implicit Colebrook formula. Here is a table of their standard deviations of their values when subtracted from Colebrook:

| $1 \mathrm{E} 4<\mathrm{Re} d<1 \mathrm{e} 8$ | $\varepsilon / d=0.03$ | 0.01 | 0.001 | 0.0001 | 0.00001 |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Jeppson rms error | 0.000398 | 0.000328 | 0.000195 | 0.000067 | 0.000088 |
| Haaland rms error | 0.000034 | 0.000043 | 0.000129 | 0.000113 | 0.000083 |

As expected, Jeppson is slightly better for smooth walls, Haaland for rough walls. Both are within $\pm 2 \%$ of the Colebrook formula over the entire range of $\mathrm{Re}_{d}$ and $\varepsilon / d$.
6.61 What level $h$ must be maintained in

Fig. P6.61 to deliver a flow rate of $0.015 \mathrm{ft}^{3} / \mathrm{s}$ through the $\frac{1}{2}$-in commercial-steel pipe?


Fig. P6.61

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. For commercial steel, take $\varepsilon \approx 0.00015 \mathrm{ft}$, or $\varepsilon / d=0.00015 /(0.5 / 12) \approx 0.0036$. Compute

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{0.015}{(\pi / 4)(0.5 / 12)^{2}}=11.0 \frac{\mathrm{ft}}{\mathrm{~s}}
$$

$$
\operatorname{Re}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{1.94(11.0)(0.5 / 12)}{2.09 \mathrm{E}-5} \approx 42500 \quad \varepsilon / d=0.0036, \quad \mathrm{f}_{\mathrm{Moody}} \approx 0.0301
$$

The energy equation, with $\mathrm{p} 1=\mathrm{p} 2$ and $\mathrm{V} 1 \approx 0$, yields an expression for surface elevation:

$$
\mathrm{h}=\mathrm{h}_{\mathrm{f}}+\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}=\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}\left(1+\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}\right)=\frac{(11.0)^{2}}{2(32.2)}\left[1+0.0301\left(\frac{80}{0.5 / 12}\right)\right] \approx \mathbf{1 1 1} \mathbf{f t} \quad \text { Ans. }
$$

6.62 Water at $20^{\circ} \mathrm{C}$ is to be pumped through 2000 ft of pipe from reservoir 1 to 2 at a rate of $3 \mathrm{ft}^{3} / \mathrm{s}$, as shown in Fig. P6.62. If the pipe is cast iron of diameter 6 in and the pump is 75 percent efficient, what horsepower pump is needed?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.94 slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft.s. For cast iron, take $\varepsilon \approx 0.00085 \mathrm{ft}$, or $\varepsilon / d=$ $0.00085 /(6 / 12) \approx 0.0017$. Compute $\mathrm{V}, \mathrm{Re}$,


Fig. P6.62 and $f$ :

$$
\begin{gathered}
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{3}{(\pi / 4)(6 / 12)^{2}}=15.3 \frac{\mathrm{ft}}{\mathrm{~s}} ; \\
\operatorname{Re}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{1.94(15.3)(6 / 12)}{2.09 \mathrm{E}-5} \approx 709000 \quad \varepsilon / d=0.0017, \quad \mathrm{f}_{\mathrm{Moody}} \approx 0.0227
\end{gathered}
$$

The energy equation, with $\mathrm{p} 1=\mathrm{p} 2$ and $\mathrm{V} 1 \approx \mathrm{~V} 2 \approx 0$, yields an expression for pump head:

$$
\begin{aligned}
& \mathrm{h}_{\text {pump }}=\Delta \mathrm{z}+\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=120 \mathrm{ft}+0.0227\left(\frac{2000}{6 / 12}\right) \frac{(15.3)^{2}}{2(32.2)}=120+330 \approx 450 \mathrm{ft} \\
& \text { Power: } \quad \mathrm{P}=\frac{\rho \mathrm{gQh}_{\mathrm{p}}}{\eta}=\frac{1.94(32.2)(3.0)(450)}{0.75}=112200 \div 550 \approx \mathbf{2 0 4} \mathbf{~ h p} \quad \text { Ans. }
\end{aligned}
$$

6.63 A tank contains $1 \mathrm{~m}^{3}$ of water at $20^{\circ} \mathrm{C}$ and has a drawn-capillary outlet tube at the bottom, as in Fig. P6.63. Find the outlet volume flux $Q$ in $\mathrm{m}^{3} / \mathrm{h}$ at this instant.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For drawn tubing, take $\varepsilon \approx 0.0015 \mathrm{~mm}$, or $\varepsilon / d=$ $0.0015 / 40 \approx 0.0000375$. The steady-flow energy equation, with $\mathrm{p} 1=\mathrm{p} 2$ and $\mathrm{V} 1 \approx 0$, gives


Fig. P6.63

$$
\mathrm{h}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=\Delta \mathrm{z}-\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}, \quad \text { or: } \quad \frac{\mathrm{V}^{2}}{2 \mathrm{~g}}\left(1+\frac{0.8}{0.04} \mathrm{f}\right) \approx 1.8 \mathrm{~m}, \mathrm{~V}^{2} \approx \frac{35.32}{1+20 \mathrm{f}}
$$

Guess $\quad \mathrm{f} \approx 0.015, \quad \mathrm{~V}=\left[\frac{35.32}{1+20(0.015)}\right]^{1 / 2} \approx 5.21 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{998(5.21)(0.04)}{0.001} \approx 208000$

$$
\mathrm{f}_{\text {better }} \approx 0.0158, \quad \mathrm{~V}_{\text {better }} \approx 5.18 \mathrm{~m} / \mathrm{s}, \quad \mathrm{Re} \approx 207000(\text { converged })
$$

Thus $\quad \mathrm{V} \approx 5.18 \mathrm{~m} / \mathrm{s}, \quad \mathrm{Q}=(\pi / 4)(0.04)^{2}(5.18)=0.00651 \mathrm{~m}^{3} / \mathrm{s} \approx \mathbf{2 3 . 4} \mathbf{m}^{3} / \mathbf{h}$. Ans.
6.64 Repeat Prob. 6.63 to find the flow rate if the fluid is SAE 10 oil. Is the flow laminar or turbulent?

Solution: For SAE 10 oil at $20^{\circ} \mathrm{C}$, take $\rho=870 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.104 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For drawn tubing, take $\varepsilon \approx 0.0015 \mathrm{~mm}$, or $\varepsilon / d=0.0015 / 40 \approx 0.0000375$. Guess laminar flow:

$$
\mathrm{h}_{\mathrm{f}}=1.8 \mathrm{~m}-\frac{\mathrm{V}^{2}}{2 \mathrm{~g}} \stackrel{?}{=} \frac{32 \mu \mathrm{LV}}{\rho \mathrm{gd}^{2}}, \quad \text { or: } \quad 1.8-\frac{\mathrm{V}^{2}}{2(9.81)}=\frac{32(0.104)(0.8) \mathrm{V}}{870(9.81)(0.04)^{2}}=0.195 \mathrm{~V}
$$

Quadratic equation: $\mathrm{V}^{2}+3.83 \mathrm{~V}-35.32=0$, solve $\mathrm{V}=4.33 \mathrm{~m} / \mathrm{s}$
Check $\operatorname{Re}=(870)(4.33)(0.04) /(0.104) \approx \mathbf{1 4 5 0}$ (OK, laminar)
So it is laminar flow, and $\mathrm{Q}=(\pi / 4)(0.04)^{2}(4.33)=0.00544 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{1 9 . 6} \mathbf{m}^{\mathbf{3}} / \mathbf{h}$. Ans.
6.65 In Prob. 6.63 the initial flow is turbulent. As the water drains out of the tank, will the flow revert to laminar motion as the tank becomes nearly empty? If so, at what tank depth? Estimate the time, in h, to drain the tank completely.

Solution: Recall that $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}, \mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$, and $\varepsilon / d \approx 0.0000375$. Let Z be the depth of water in the $\operatorname{tank}(\mathrm{Z}=1 \mathrm{~m}$ in Fig. P6.63). When $\mathrm{Z}=0$, find the flow rate:

$$
\begin{aligned}
\mathrm{Z}=0, \mathrm{~h}_{\mathrm{f}}=0.8 \mathrm{~m}, \quad \mathrm{~V}^{2} & \approx \frac{2(9.81)(0.8)}{1+20 \mathrm{f}} \text { converges to } \mathrm{f}=0.0171, \mathrm{Re}=136000 \\
\mathrm{~V} & \approx 3.42 \mathrm{~m} / \mathrm{s}, \quad \mathrm{Q} \approx 12.2 \mathrm{~m}^{3} / \mathrm{h}(\mathrm{Z}=0)
\end{aligned}
$$

So even when the tank is empty, the flow is still turbulent. Ans.
The time to drain the tank is $\frac{d}{d t}\left(v_{\operatorname{tank}}\right)=-Q=\frac{d}{d t}\left(A_{\operatorname{tank}} Z\right)=\left(1 \mathrm{~m}^{2}\right) \frac{\mathrm{dZ}}{\mathrm{dt}}=-Q$,

$$
\text { or } \quad \mathrm{t}_{\text {drain }}=-\int_{1 \mathrm{~m}}^{0 \mathrm{~m}} \frac{\mathrm{dZ}}{\mathrm{Q}}=\left(\frac{1}{\mathrm{Q}}\right)_{\text {avg }}(1 \mathrm{~m})
$$

So all we need is the average value of $(1 / \mathrm{Q})$ during the draining period. We know Q at $\mathrm{Z}=0$ and $Z=1 \mathrm{~m}$, let's check it also at $Z=0.5 \mathrm{~m}$ : Calculate Qmidway $\approx 19.8 \mathrm{~m}^{3} / \mathrm{h}$. Then

$$
\frac{1}{\mathrm{Q}} \mathrm{avg}_{\mathrm{avg}} \approx \frac{1}{6}\left[\frac{1}{23.4}+\frac{4}{19.8}+\frac{1}{12.2}\right] \approx 0.0544 \frac{\mathrm{~h}}{\mathrm{~m}^{3}}, \quad \mathrm{t}_{\mathrm{drain}}=\mathbf{0 . 0 5 4 4} \mathbf{h} \approx \mathbf{3 . 3} \mathbf{~ m i n} \quad \text { Ans. }
$$

6.66 Ethyl alcohol at $20^{\circ} \mathrm{C}$ flows through a $10-\mathrm{cm}$ horizontal drawn tube 100 m long. The fully developed wall shear stress is 14 Pa . Estimate (a) the pressure drop, (b) the volume flow rate, and (c) the velocity u at $r=1 \mathrm{~cm}$.

Solution: For ethyl alcohol at $20^{\circ} \mathrm{C}, \rho=789 \mathrm{~kg} / \mathrm{m}^{3}, \mu=0.0012 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For drawn tubing, take $\varepsilon \approx 0.0015 \mathrm{~mm}$, or $\varepsilon / d=0.0015 / 100 \approx 0.000015$. From Eq. (6.12),

$$
\Delta \mathrm{p}=4 \tau_{\mathrm{w}} \frac{\mathrm{~L}}{\mathrm{~d}}=4(14)\left(\frac{100}{0.1}\right) \approx 56000 \text { Pa Ans. (a) }
$$

The wall shear is directly related to $f$, and we may iterate to find V and Q :

$$
\begin{gathered}
\tau_{\mathrm{w}}=\frac{\mathrm{f}}{8} \rho \mathrm{~V}^{2}, \quad \text { or: } \quad \mathrm{fV}^{2}=\frac{8(14)}{789}=0.142 \quad \text { with } \frac{\varepsilon}{d}=0.000015 \\
\text { Guess } \mathrm{f} \approx 0.015, \quad \mathrm{~V}=\left[\frac{0.142}{0.015}\right]^{1 / 2} \approx 3.08 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{789(3.08)(0.1)}{0.0012} \approx 202000 \\
\mathrm{f}_{\text {better }} \approx 0.0158, \quad \mathrm{~V}_{\text {better }} \approx 3.00 \mathrm{~m} / \mathrm{s}, \quad \mathrm{Re}_{\text {better }} \approx 197000 \text { (converged) }
\end{gathered}
$$

Then $\mathrm{V} \approx \mathbf{3 . 0 0} \mathbf{~ m} / \mathbf{s}$, and $\mathrm{Q}=(\pi / 4)(0.1)^{2}(3.00)=0.0236 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{8 5} \mathbf{~ m}^{\mathbf{3}} / \mathbf{h}$. Ans. (b)
Finally, the log-law Eq. (6.28) can estimate the velocity at $\mathrm{r}=1 \mathrm{~cm}, " y "=R-r=4 \mathrm{~cm}$ :

$$
\begin{gathered}
\mathrm{u}^{*}=\left(\frac{\tau_{\mathrm{w}}}{\rho}\right)^{1 / 2}=\left(\frac{14}{789}\right)^{1 / 2}=0.133 \frac{\mathrm{~m}}{\mathrm{~s}} ; \\
\frac{\mathrm{u}}{\mathrm{u}^{*}} \approx \frac{1}{\kappa} \ln \left[\frac{\rho \mathrm{u}^{*} \mathrm{y}}{\mu}\right]+\mathrm{B}=\frac{1}{0.41} \ln \left[\frac{789(0.133)(0.04)}{0.0012}\right]+5.0=24.9 \\
\text { Then } \mathrm{u} \approx 24.9(0.133) \approx 3.3 \mathrm{~m} / \mathrm{s} \quad \text { at } \mathrm{r}=1 \mathrm{~cm} . \text { Ans. (c) }
\end{gathered}
$$

6.67 A straight $10-\mathrm{cm}$ commercial-steel pipe is 1 km long and is laid on a constant slope of $5^{\circ}$. Water at $20^{\circ} \mathrm{C}$ flows downward, due to gravity only. Estimate the flow rate in $\mathrm{m}^{3} / \mathrm{h}$. What happens if the pipe length is 2 km ?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. If the flow is due to gravity only, then the head loss exactly balances the elevation change:

$$
\mathrm{h}_{\mathrm{f}}=\Delta \mathrm{z}=\mathrm{L} \sin \theta=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}, \quad \text { or } \quad \mathrm{fV}^{2}=2 \mathrm{gd} \sin \theta=2(9.81)(0.1) \sin 5^{\circ} \approx 0.171
$$

Thus the flow rate is independent of the pipe length L if laid on a constant slope. Ans. For commercial steel, take $\varepsilon \approx 0.046 \mathrm{~mm}$, or $\varepsilon / d \approx 0.00046$. Begin by guessing fullyrough flow for the friction factor, and iterate V and Re and f :

$$
\begin{gathered}
\mathrm{f} \approx 0.0164, \quad \mathrm{~V} \approx\left(\frac{0.171}{0.0164}\right)^{1 / 2} \approx 3.23 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{998(3.23)(0.1)}{0.001} \approx 322000 \\
\mathrm{f}_{\text {better }} \approx 0.0179, \quad \mathrm{~V}_{\text {better }} \approx 3.09 \mathrm{~m} / \mathrm{s}, \quad \mathrm{Re} \approx 308000 \text { (converged) } \\
\text { Then } \mathrm{Q} \approx(\pi / 4)(0.1)^{2}(3.09) \approx 0.0243 \mathrm{~m}^{3} / \mathrm{s} \approx \mathbf{8 7} \mathbf{~ m}^{3} / \mathbf{h} \text {. Ans. }
\end{gathered}
$$

6.68 The Moody chart, Fig. 6.13, is best for finding head loss (or $\Delta p$ ) when $Q, V$, d, and L are known. It is awkward for the "2nd" type of problem, finding Q when hf or $\Delta \mathrm{p}$ are known (see Ex. 6.9). Prepare a modified Moody chart whose abscissa is independent of Q and V , using $\varepsilon / d$ as a parameter, from which one can immediately read the ordinate to find (dimensionless) Q or V. Use your chart to solve Example 6.9.

Solution: This problem was mentioned analytically in the text as Eq. (6.51). The proper parameter which contains head loss only, and not flow rate, is $\zeta$ :

$$
\begin{equation*}
\zeta=\frac{g d^{3} h_{f}}{L v^{2}} \quad R e_{d}=-(8 \zeta)^{1 / 2} \log \left(\frac{\varepsilon / d}{3.7}+\frac{1.775}{\sqrt{\zeta}}\right) \tag{6.51}
\end{equation*}
$$

We simply plot Reynolds number versus $\zeta$ for various $\varepsilon / d$, as shown below:


To solve Example 6.9, a 100 -m-long, 30 -cm-diameter pipe with a head loss of 8 m and $\varepsilon / d=0.0002$, we use that data to compute $\zeta=5.3 E 7$. The oil properties are $\rho=950$ $\mathrm{kg} / \mathrm{m}^{3}$ and $v=2 \mathrm{E}-5 \mathrm{~m}^{2} / \mathrm{s}$. Enter the chart above: let's face it, the scale is very hard to read, but we estimate, at $\zeta=5.3 \mathrm{E} 7$, that $6 \mathrm{E} 4<\mathrm{Red}<9 \mathrm{E} 4$, which translates to a flow rate of
$\mathbf{0 . 2 8}<\mathbf{Q}<\mathbf{0 . 4 2} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s}$. Ans. (Example 6.9 gave $\mathrm{Q}=0.342 \mathrm{~m}^{3} / \mathrm{s}$.)
6.69 For Prob. 6.62 suppose the only pump available can deliver only 80 hp to the fluid. What is the proper pipe size in inches to maintain the $3 \mathrm{ft}^{3} / \mathrm{s}$ flow rate?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. For cast iron, take $\varepsilon \approx 0.00085 \mathrm{ft}$. We can't specify $\varepsilon / d$ because we don't know d . The energy analysis above is correct and should be modified to replace V by Q :

$$
\mathrm{h}_{\mathrm{p}}=120+\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\left(4 \mathrm{Q} / \pi \mathrm{d}^{2}\right)^{2}}{2 \mathrm{~g}}=120+\mathrm{f} \frac{2000}{\mathrm{~d}} \frac{\left[4(3.0) / \pi \mathrm{d}^{2}\right]^{2}}{2(32.2)}=120+453 \frac{\mathrm{f}}{\mathrm{~d}^{5}}
$$

But also $\quad h_{p}=\frac{\text { Power }}{\rho g Q}=\frac{80(550)}{62.4(3.0)}=235=120+\frac{453 \mathrm{f}}{\mathrm{d}^{5}}, \quad$ or: $\quad \mathrm{d}^{5} \approx 3.94 \mathrm{f}$

Guess $\mathrm{f} \approx 0.02$, calculate $\mathrm{d}, \varepsilon / d$ and Re and get a better $f$ and iterate:

$$
\begin{aligned}
& \mathrm{f} \approx 0.020, \mathrm{~d} \approx[3.94(0.02)]^{1 / 5} \approx 0.602 \mathrm{ft}, \quad \mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{~d}}=\frac{4(1.94)(3.0)}{\pi(2.09 \mathrm{E}-5)(0.602)}, \\
& \text { or } \mathrm{Re} \approx 589000, \quad \frac{\varepsilon}{d}=\frac{0.00085}{0.602} \approx 0.00141, \quad \text { Moody chart: } \mathrm{f}_{\text {better }} \approx 0.0218 \text { (repeat) }
\end{aligned}
$$

We are nearly converged. The final solution is $\mathrm{f} \approx 0.0217, \mathrm{~d} \approx 0.612 \mathrm{ft} \approx 7.3$ in Ans.

P6.70 Water at $68^{\circ} \mathrm{F}$ flows through 200 ft of a horizontal 6-in-diameter asphalted cast iron pipe. (a) If the head loss is 4.5 ft , find the average velocity and the flow rate, using the rescaled variable $\zeta$ discussed as a "Type 2 " problem. (b) Does this input data seem familiar to you?

Solution: For water in BG units, take $v=1.1 \mathrm{E}-5 \mathrm{ft}^{2} / \mathrm{s}$. For asphalted cast iron, $\varepsilon=$ 0.0004 ft , hence $\varepsilon / d=0.0004 \mathrm{ft} / 0.5 \mathrm{ft}=0.0008$. Calculate the velocity-free group $\zeta$ :

$$
\zeta=\frac{g d^{3} h_{f}}{L v^{2}}=\frac{\left(32.2 \mathrm{ft} / \mathrm{s}^{2}\right)(0.5 f t)^{3}(4.5 f t)}{(200 f t)\left(1.1 E-5 f t^{2} / \mathrm{s}\right)}=7.485 E 8
$$

Now get the Reynolds number from the modified Colebrook formula, Eq. (6.51):

$$
\begin{gathered}
\operatorname{Re}_{d}=-(8 \zeta)^{1 / 2} \log _{10}\left(\frac{\varepsilon / d}{3.7}+\frac{1.775}{\sqrt{\zeta}}\right)=-[8 \sqrt{7.485 E 8}] \log _{10}\left[\frac{0.0008}{3.7}+\frac{1.775}{\sqrt{7.485 E 8}}\right]=274,800 \\
\text { Then } V=\frac{v \operatorname{Re}_{d}}{d}=\frac{(1.1 E-5)(274800)}{0.5}=6.05 \mathrm{ft} / \mathrm{s} \quad \text { Ans.(a) } \\
Q=\frac{\pi}{4} d^{2} V=\frac{\pi}{4}(0.5 \mathrm{ft})^{2}(6.05 \mathrm{ft} / \mathrm{s})=1.19 \mathrm{ft}^{3} / \mathrm{s} \quad \text { Ans.(a) }
\end{gathered}
$$

(b) These are the numbers for L. F. Moody's classic example, which was introduced in Ex. 6.6 of the text. We did not get $V=6.00 \mathrm{ft} / \mathrm{s}$ because $h_{f}$ was rounded off from 4.47 ft .
6.71 It is desired to solve Prob. 6.62 for the most economical pump and cast-iron pipe system. If the pump costs $\$ 125$ per horsepower delivered to the fluid and the pipe costs $\$ 7000$ per inch of diameter, what are the minimum cost and the pipe and pump size to maintain the $3 \mathrm{ft}^{3} / \mathrm{s}$ flow rate? Make some simplifying assumptions.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft.s. For cast iron, take $\varepsilon \approx 0.00085 \mathrm{ft}$. Write the energy equation (from Prob. 6.62) in terms of Q and d:

$$
\begin{aligned}
& \mathrm{P}_{\text {in hp }}=\frac{\rho \mathrm{gQ}}{550}\left(\Delta \mathrm{z}+\mathrm{h}_{\mathrm{f}}\right)=\frac{62.4(3.0)}{550}\left\{120+\mathrm{f}\left(\frac{2000}{\mathrm{~d}}\right) \frac{\left[4(3.0) / \pi \mathrm{d}^{2}\right]^{2}}{2(32.2)}\right\}=40.84+\frac{154.2 \mathrm{f}}{\mathrm{~d}^{5}} \\
& \text { Cost }=\$ 125 \mathrm{P}_{\mathrm{hp}}+\$ 7000 \mathrm{~d}_{\text {inches }}=125\left(40.84+154.2 \mathrm{f} / \mathrm{d}^{5}\right)+7000(12 \mathrm{~d}), \text { with } \mathrm{d} \text { in } \mathrm{ft} .
\end{aligned}
$$

$$
\text { Clean up: Cost } \approx \$ 5105+19278 \mathrm{f} / \mathrm{d}^{5}+84000 \mathrm{~d}
$$

Regardless of the (unknown) value of $f$, this Cost relation does show a minimum. If we assume for simplicity that $f$ is constant, we may use the differential calculus:

$$
\left.\frac{\mathrm{d}(\text { Cost })}{\mathrm{d}(\mathrm{~d})}\right|_{\mathrm{f} \approx \text { const }}=\frac{-5(19278) \mathrm{f}}{\mathrm{~d}^{6}}+84000, \quad \text { or } \quad \mathrm{d}_{\text {best }} \approx(1.148 \mathrm{f})^{1 / 6}
$$

Guess $\mathrm{f} \approx 0.02, \mathrm{~d} \approx[1.148(0.02)]^{1 / 6} \approx 0.533 \mathrm{ft}, \quad \mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{d}} \approx 665000, \quad \frac{\varepsilon}{\mathrm{~d}} \approx 0.00159$

$$
\text { Then } \mathrm{f}_{\text {better }} \approx 0.0224, \quad \mathrm{~d}_{\text {better }} \approx 0.543 \mathrm{ft} \text { (converged) }
$$

Result: dbest $\approx 0.543 \mathrm{ft} \approx \mathbf{6 . 5} \mathbf{~ i n}, \quad$ Costmin $\approx \$ 14300$ pump $+\$ 45600_{\text {pipe }} \approx \mathbf{\$ 6 0 0 0 0}$. Ans.
6.72 Modify Prob. P6.57 by letting the diameter be unknown. Find the proper pipe diameter for which the pool will drain in about 2 hours flat.

Solution: Recall the data: Let $W=5 \mathrm{~m}, Y=8 \mathrm{~m}, h_{0}=2 \mathrm{~m}, L=15 \mathrm{~m}$, and $\varepsilon=0$, with water, $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. We apply the same theory as Prob. 6.57:

$$
V=\sqrt{\frac{2 g h}{1+f L / D}}, \quad t_{d r a i n} \approx \frac{4 W Y}{\pi D^{2}} \sqrt{\frac{2 h_{o}\left(1+f_{a v} L / D\right)}{g}}, \quad f_{a v}=f c n\left(\operatorname{Re}_{D}\right) \quad \text { for a smooth pipe. }
$$

For the present problem, $t$ drain $=2$ hours and $D$ is the unknown. Use an average value $h=$ 1 m to find fav. Enter these equations on EES (or you can iterate by hand) and the final results are

$$
V=2.36 \mathrm{~m} / \mathrm{s} ; \quad \operatorname{Re}_{D}=217,000 ; \quad f_{\mathrm{av}} \approx 0.0154 ; \quad D=0.092 \mathrm{~m} \approx 9.2 \mathrm{~cm} \quad \text { Ans. }
$$

6.73 The Moody chart, Fig. 6.13, is best for finding head loss (or $\Delta \mathrm{p}$ ) when $Q, V, d$, and $L$ are known. It is awkward for the "3rd" type of problem, finding $d$ when $h f($ or $\Delta p$ ) and $Q$ are known (see Ex. 6.11). Prepare a modified Moody chart whose abscissa is independent of $d$, using as a parameter $\varepsilon$ non-dimensionalized without $d$, from which one can immediately read the (dimensionless) ordinate to find $d$. Use your chart to solve Ex. 6.11.

Solution: An appropriate Pi group which does not contain $d$ is $\beta=\left(\mathrm{ghfQ}^{3}\right) /\left(\mathrm{L} v^{5}\right)$.
Similarly, an appropriate roughness parameter without $d$ is $\sigma=(\varepsilon v / \mathrm{Q})$. After a lot of algebra, the Colebrook friction factor formula (6.48) becomes

$$
R e_{d}^{5 / 2}=-2.0\left(\frac{128 \beta}{\pi^{3}}\right)^{1 / 2} \log _{10}\left[\frac{\pi \sigma R e_{d}}{14.8}+\frac{2.51 R e_{d}^{3 / 2}}{\left(128 \beta / \pi^{3}\right)^{1 / 2}}\right]
$$

A plot of this messy relation is given below.


To solve Example 6.11, a 100-m-long, unknown-diameter pipe with a head loss of 8 m , flow rate of $0.342 \mathrm{~m}^{3} / \mathrm{s}$, and $\varepsilon=0.06 \mathrm{~mm}$, we use that data to compute $\beta=\mathbf{9 . 8 E 2 1}$ and $\sigma=3.5 \mathrm{E}-6$. The oil properties are $\rho=950 \mathrm{~kg} / \mathrm{m}^{3}$ and $v=2 \mathrm{E}-5 \mathrm{~m}^{2} / \mathrm{s}$. Enter the chart above: let's face it, the scale is very hard to read, but we estimate, at $\beta=9.8 \mathrm{E} 21$ and $\sigma=$ $3.5 \mathrm{E}-6$, that $6 \mathrm{E} 4<\mathrm{Red}<8 \mathrm{E} 4$, which translates to a diameter of $\mathbf{0 . 2 7}<\boldsymbol{d}<\mathbf{0 . 3 6} \mathbf{~ m}$. Ans. (Example 6.11 gave $d=0.3 \mathrm{~m}$.)

P6.74 Two reservoirs, which differ in surface elevation by 40 m , are connected by a new commercial steel pipe of diameter 8 cm . If the desired weight flow rate is $200 \mathrm{~N} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$, what is the proper length of the pipe? Neglect minor losses.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. For commercial steel, $\varepsilon=0.046 \mathrm{~mm}$, thus $\varepsilon / d=0.046 \mathrm{~mm} / 80 \mathrm{~mm}=0.000575$. Find the velocity and the friction factor:

$$
\begin{gathered}
V=\frac{\dot{w} /(\rho g)}{(\pi / 4) D^{2}}=\frac{200 /[998(9.81)]}{(\pi / 4)(0.08)^{2}}=4.06 \frac{m}{s}, \quad \operatorname{Re}_{D}=\frac{\rho V D}{\mu}=\frac{998(4.06)(0.08)}{0.001}=324,000 \\
\frac{1}{\sqrt{f}} \approx-2.0 \log _{10}\left(\frac{\varepsilon / D}{3.7}+\frac{2.51}{\operatorname{Re}_{D} \sqrt{f}}\right) \text { yields } \quad f=0.0185
\end{gathered}
$$

Then we find the pipe length from the energy equation, which is simple in this case:

$$
\Delta z=40 m=f \frac{L}{D} \frac{V^{2}}{2 g}=(0.0185) \frac{L}{(0.08 m)} \frac{(4.06)^{2}}{2(9.81)}, \text { Solve } L \approx 205 m \quad \text { Ans. }
$$

6.75 You wish to water your garden with 100 ft of $\frac{5}{8}$-in-diameter hose whose roughness is 0.011 in . What will be the delivery, in $\mathrm{ft}^{3} / \mathrm{s}$, if the gage pressure at the faucet is $60 \mathrm{lbf} / \mathrm{in}^{2}$ ? If there is no nozzle (just an open hose exit), what is the maximum horizontal distance the exit jet will carry?


Fig. P6.75

Solution: For water, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. We are given $\varepsilon / d=$ $0.011 /(5 / 8) \approx 0.0176$. For constant area hose, $\mathrm{V} 1=\mathrm{V} 2$ and energy yields

$$
\begin{aligned}
& \quad \frac{\mathrm{p}_{\text {faucet }}}{\rho \mathrm{g}}=\mathrm{h}_{\mathrm{f}}, \quad \text { or: } \quad \frac{60 \times 144 \mathrm{psf}}{1.94(32.2)}=138 \mathrm{ft}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=\mathrm{f} \frac{100}{(5 / 8) / 12} \frac{\mathrm{~V}^{2}}{2(32.2)}, \\
& \text { or } \mathrm{fV}^{2} \approx 4.64 . \quad \text { Guess } \mathrm{f} \approx \mathrm{f}_{\text {fully rough }}=0.0463, \quad \mathrm{~V} \approx 10.0 \frac{\mathrm{ft}}{\mathrm{~s}}, \quad \mathrm{Re} \approx 48400 \\
& \quad \text { then } \quad \mathrm{f}_{\text {better }} \approx 0.0472, \quad \mathrm{~V}_{\text {final }} \approx \mathbf{9 . 9 1} \mathbf{f t} / \mathbf{s} \text { (converged) }
\end{aligned}
$$

The hose delivery then is $\mathrm{Q}=(\pi / 4)(5 / 8 / 12)^{2}(9.91)=\mathbf{0 . 0 2 1 1} \mathbf{f t}^{3} / \mathbf{s}$. Ans. (a)
From elementary particle-trajectory theory, the maximum horizontal distance X travelled by the jet occurs at $\theta=45^{\circ}$ (see figure) and is $\mathbf{X}=\mathrm{V}^{2} / \mathrm{g}=(9.91)^{2} /(32.2) \approx 3.05 \mathrm{ft}$ Ans. (b), which is pitiful. You need a nozzle on the hose to increase the exit velocity.
6.76 The small turbine in Fig. P6.76 extracts 400 W of power from the water flow. Both pipes are wrought iron. Compute the flow rate $Q \mathrm{~m}^{3} / \mathrm{h}$. Sketch the EGL and HGL accurately.

Solution: For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For wrought iron, take $\varepsilon \approx 0.046 \mathrm{~mm}$, hence $\varepsilon / d_{1}=0.046 / 60$ $\approx 0.000767$ and $\varepsilon / d 2=0.046 / 40 \approx 0.00115$.


Fig. P6.76 The energy equation, with $\mathrm{V} 1 \approx 0$ and $\mathrm{p} 1=$ p2, gives

$$
\begin{gathered}
\mathrm{z}_{1}-\mathrm{z}_{2}=20 \mathrm{~m}=\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}+\mathrm{h}_{\mathrm{f} 2}+\mathrm{h}_{\mathrm{f} 1}+\mathrm{h}_{\text {turbine }}, \quad \mathrm{h}_{\mathrm{f} 1}=\mathrm{f}_{1} \frac{\mathrm{~L}_{1}}{\mathrm{~d}_{1}} \frac{\mathrm{~V}_{1}^{2}}{2 \mathrm{~g}} \quad \text { and } \mathrm{h}_{\mathrm{f} 2}=\mathrm{f}_{2} \frac{\mathrm{~L}_{2}}{\mathrm{~d}_{2}} \frac{\mathrm{~V}_{2}^{2}}{2 \mathrm{~g}} \\
\text { Also, } \quad \mathrm{h}_{\text {turbine }}=\frac{\mathrm{P}}{\rho \mathrm{gQ}}=\frac{400 \mathrm{~W}}{998(9.81) \mathrm{Q}} \quad \text { and } \mathrm{Q}=\frac{\pi}{4} \mathrm{~d}_{1}^{2} \mathrm{~V}_{1}=\frac{\pi}{4} \mathrm{~d}_{2}^{2} \mathrm{~V}_{2}
\end{gathered}
$$

The only unknown is Q , which we may determine by iteration after an initial guess:

$$
\begin{gathered}
\mathrm{h}_{\text {turb }}=\frac{400}{998(9.81) \mathrm{Q}}=20-\frac{8 \mathrm{f}_{1} \mathrm{~L}_{1} \mathrm{Q}^{2}}{\pi^{2} \operatorname{gd}_{1}^{5}}-\frac{8 \mathrm{f}_{2} \mathrm{~L}_{2} \mathrm{Q}^{2}}{\pi^{2} \mathrm{gd}_{2}^{5}}-\frac{8 \mathrm{Q}^{2}}{\pi^{2} \operatorname{gd}_{2}^{4}} \\
\text { Guess } \mathrm{Q}=0.003 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}, \text { then } \mathrm{Re}_{1}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{~d}_{1}}=63500, \mathrm{f}_{1, \text { Moody }} \approx 0.0226, \\
\operatorname{Re}_{2}=95300, \quad \mathrm{f}_{2} \approx 0.0228
\end{gathered}
$$

But, for this guess, hturb(left hand side) $\approx 13.62 \mathrm{~m}$, hturb(right hand side) $\approx 14.53 \mathrm{~m}$ (wrong). Other guesses converge to hturb $\approx 9.9$ meters. For $\mathrm{Q} \approx 0.00413 \mathrm{~m}^{3} / \mathrm{s} \approx \mathbf{1 5} \mathbf{m}^{\mathbf{3}} / \mathbf{h}$. Ans.
6.77 Modify Prob. 6.76 into an economic analysis, as follows. Let the 40 m of wroughtiron pipe have a uniform diameter $d$. Let the steady water flow available be $Q=30 \mathrm{~m}^{3} / \mathrm{h}$. The cost of the turbine is $\$ 4$ per watt developed, and the cost of the piping is $\$ 75$ per centimeter of diameter. The power generated may be sold for $\$ 0.08$ per kilowatt hour. Find the proper pipe diameter for minimum payback time, i.e., minimum time for which the power sales will equal the initial cost of the system.

Solution: With flow rate known, we need only guess a diameter and compute power from the energy equation similar to Prob. 6.76:

$$
\mathrm{P}=\rho \mathrm{gQh}_{\mathrm{t}}, \quad \text { where } \mathrm{h}_{\mathrm{t}}=20 \mathrm{~m}-\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}\left(1+\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}\right)=20-\frac{8 \mathrm{Q}^{2}}{\pi^{2} \mathrm{gd}^{4}}\left(1+\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}\right)
$$

Then Cost $=\$ 4^{*} \mathrm{P}+\$ 75(100 \mathrm{~d})$ and Annual income $=\$ 0.08\left(\frac{\mathrm{P}}{1000}\right)(24)(365)$
The Moody friction factor is computed from $\mathrm{Re}=4 \rho \mathrm{Q} /(\pi \mu \mathrm{d})$ and $\varepsilon / d=0.066 / d(\mathrm{~mm})$. The payback time, in years, is then the cost divided by the annual income. For example,

$$
\begin{gathered}
\text { If } \mathrm{d}=0.1 \mathrm{~m}, \quad \mathrm{Re} \approx 106000, \quad \mathrm{f} \approx 0.0200, \quad \mathrm{ht} \approx 19.48 \mathrm{~m}, \quad \mathrm{P}=1589.3 \mathrm{~W} \\
\text { Cost } \approx \$ 7107 \quad \text { Income }=\$ 1,114 / \text { year } \quad \text { Payback } \approx \mathbf{6 . 3 8} \text { years }
\end{gathered}
$$

Since the piping cost is very small ( $<\$ 1000$ ), both cost and income are nearly proportional to power, hence the payback will be nearly the same ( 6.38 years) regardless of diameter. There is an almost invisible minimum at $\mathbf{d} \approx 7 \mathbf{c m}, \operatorname{Re} \approx 151000$, $\mathrm{f} \approx 0.0201$, $h t \approx 17.0 \mathrm{~m}$, Cost $\approx \$ 6078$, Income $\approx \$ 973$, Payback $\approx 6.25$ years. However, as diameter d decreases, we generate less power and gain little in payback time.
6.78 In Fig. P6.78 the connecting pipe is commercial steel 6 cm in diameter. Estimate the flow rate, in $\mathrm{m}^{3} / \mathrm{h}$, if the fluid is water at $20^{\circ} \mathrm{C}$. Which way is the flow?

Solution: For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For commercial steel, take $\varepsilon \approx 0.046 \mathrm{~mm}$, hence $\varepsilon / d=$ $0.046 / 60 \approx 0.000767$. With p1, V1, and V2 all $\approx 0$, the energy equation between


Fig. P6.78 surfaces (1) and (2) yields

$$
\begin{aligned}
& 0+0+\mathrm{z}_{1} \approx \frac{\mathrm{p}_{2}}{\rho \mathrm{~g}}+0+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}}, \quad \text { or } \quad \mathrm{h}_{\mathrm{f}}=15-\frac{200000}{998(9.81)} \approx-5.43 \mathrm{~m} \text { (flow to left) } \leftarrow \\
& \text { Guess turbulent flow: } \mathrm{h}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=\mathrm{f} \frac{50}{0.06} \frac{\mathrm{~V}^{2}}{2(9.81)}=5.43, \quad \text { or: } \mathrm{fV}^{2} \approx 0.1278 \\
& \frac{\varepsilon}{d}=0.00767, \quad \text { guess } \mathrm{f}_{\text {fully rough }} \approx 0.0184, \quad \mathrm{~V} \approx\left(\frac{0.1278}{0.0184}\right)^{1 / 2} \approx 2.64 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=158000 \\
& \mathrm{f}_{\text {better }} \approx 0.0204, \quad \mathrm{~V}_{\text {better }}=2.50 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}_{\text {better }} \approx 149700, \quad \mathrm{f}_{\text {3rd iteration }} \approx 0.0205 \text { (converged) }
\end{aligned}
$$

The iteration converges to

$$
\mathrm{f} \approx 0.0205, \mathrm{~V} \approx 2.49 \mathrm{~m} / \mathrm{s}, \mathrm{Q}=(\pi / 4)(0.06)^{2}(2.49)=0.00705 \mathrm{~m}^{3} / \mathrm{s}=25 \mathrm{~m}^{3} / \mathrm{h} \leftarrow \quad \text { Ans. }
$$

6.79 A garden hose is used as the return line in a waterfall display at the mall. In order to select the proper pump, you need to know the hose wall roughness, which is not supplied by the manufacturer. You devise a simple experiment: attach the hose to the drain of an above-ground pool whose surface is 3 m above the hose outlet. You estimate the minor loss coefficient in the entrance region as 0.5 , and the drain valve has a minorloss equivalent length of 200 diameters when fully open. Using a bucket and stopwatch, you open the valve and measure a flow rate of $2.0 \mathrm{E}-4 \mathrm{~m}^{3} / \mathrm{s}$ for a hose of inside diameter 1.5 cm and length 10 m . Estimate the roughness height of the hose inside surface.

Solution: First evaluate the average velocity in the hose and its Reynolds number:

$$
V=\frac{Q}{A}=\frac{2.0 E-4}{(\pi / 4)(0.015)^{2}}=1.13 \frac{m}{s}, \quad R e_{d}=\frac{\rho V d}{\mu}=\frac{998(1.13)(0.015)}{0.001}=16940 \text { (turbulent) }
$$

Write the energy equation from surface (point 1) to outlet (point 2), assuming an energy correction factor $\alpha=1.05$ :

$$
\frac{p_{1}}{\rho g}+\frac{\alpha_{1} V_{1}^{2}}{2 g}+z_{1}=\frac{p_{2}}{\rho g}+\frac{\alpha_{2} V_{2}^{2}}{2 g}+z_{2}+h_{f}+\sum h_{\text {loss }}, \quad \text { where } \sum h_{\text {loss }}=\left(K_{e}+f \frac{L_{e q}}{d}\right) \frac{\alpha_{2} V_{2}^{2}}{2 g}
$$

The unknown is the friction factor:

$$
f=\frac{\frac{z_{1}-z_{2}}{V^{2} / 2 g}-\alpha_{2}-K_{e}}{\left(L+L_{e q}\right) / d}=\frac{\frac{3 m}{(1.13)^{2} / 2(9.81)}-1.05-0.5}{(10 / 0.015+200)}=0.0514
$$

For $f=0.0514$ and Red $=16940$, the Moody chart (Eq. 6.48) predicts $\varepsilon / d \approx 0.0206$. Therefore the estimated hose-wall roughness is $\varepsilon=0.0206(1.5 \mathrm{~cm})=\mathbf{0 . 0 3 1} \mathbf{~ c m ~ A n s}$.
6.80 The head-versus-flow-rate characteristics of a centrifugal pump are shown in Fig. P6.80. If this pump drives water at $20^{\circ} \mathrm{C}$ through 120 m of 30 -cm-diameter cast-iron pipe, what will be the resulting flow rate, in $\mathrm{m}^{3} / \mathrm{s}$ ?

Solution: For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast iron, take $\varepsilon \approx$ 0.26 mm , hence $\varepsilon / d=0.26 / 300 \approx 0.000867$.


Fig. P6.80

The head loss must match the pump head:

$$
\mathrm{h}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=\frac{8 \mathrm{fLQ}^{2}}{\pi^{2} \mathrm{gd}^{5}}=\mathrm{h}_{\mathrm{pump}} \approx 80-20 \mathrm{Q}^{2} \text {, with } \mathrm{Q} \text { in } \mathrm{m}^{3} / \mathrm{s}
$$

Evaluate $\mathrm{h}_{\mathrm{f}}=\frac{8 \mathrm{f}(120) \mathrm{Q}^{2}}{\pi^{2}(9.81)(0.3)^{5}}=80-20 \mathrm{Q}^{2}, \quad$ or: $\quad \mathrm{Q}^{2} \approx \frac{80}{20+4080 \mathrm{f}}$
Guess $\mathrm{f} \approx 0.02, \mathrm{Q}=\left[\frac{80}{20+4080(0.02)}\right]^{1 / 2} \approx 0.887 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{d}} \approx 3.76 \mathrm{E} 6$
$\frac{\varepsilon}{d}=0.000867, \mathrm{f}_{\text {better }} \approx 0.0191, \mathrm{Re}_{\text {better }} \approx 3.83 \mathrm{E} 6$, converges to $\mathbf{Q} \approx \mathbf{0 . 9 0 5} \frac{\mathbf{m}^{3}}{\mathbf{s}}$ Ans.
6.81 The pump in Fig. P6.80 is used to deliver gasoline at $20^{\circ} \mathrm{C}$ through 350 m of $30-\mathrm{cm}$-diameter galvanized iron pipe. Estimate the resulting flow rate, in $\mathrm{m}^{3} / \mathrm{s}$. (Note that the pump head is now in meters of gasoline.)

Solution: For gasoline, take $\rho=680 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=2.92 \mathrm{E}-4 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For galvanized iron, take $\varepsilon \approx 0.15 \mathrm{~mm}$, hence $\varepsilon / d=0.15 / 300 \approx 0.0005$. Head loss matches pump head:

$$
\begin{gathered}
\mathrm{h}_{\mathrm{f}}=\frac{8 \mathrm{fLQ}{ }^{2}}{\pi^{2} \mathrm{gd}^{5}}=\frac{8 \mathrm{f}(350) \mathrm{Q}^{2}}{\pi^{2}(9.81)(0.3)^{5}}=11901 \mathrm{fQ}^{2}=\mathrm{h}_{\text {pump }} \approx 80-20 \mathrm{Q}^{2}, \quad \mathrm{Q}^{2}=\frac{80}{20+11901 \mathrm{f}} \\
\text { Guess } \mathrm{f}_{\text {rough }} \approx 0.017, \quad \mathrm{Q} \approx 0.600 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}, \\
\operatorname{Re}_{\text {better }} \approx 5.93 \mathrm{E} 6, \quad \frac{\varepsilon}{d}=0.0005, \quad \mathrm{f}_{\text {better }} \approx 0.0168 \\
\text { This converges to } \mathrm{f} \approx 0.0168, \quad \mathrm{Re} \approx 5.96 \mathrm{E} 6, \quad \mathbf{Q} \approx \mathbf{0 . 6 0 3} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s} . \quad \text { Ans. }
\end{gathered}
$$

6.82 The pump in Fig. P6.80 has its maximum efficiency at a head of 45 m . If it is used to pump ethanol at $20^{\circ} \mathrm{C}$ through 200 m of commercial-steel pipe, what is the proper pipe diameter for maximum pump efficiency?

Solution: For ethanol, take $\rho=789 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.2 \mathrm{E}-3 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For commercial steel, take $\varepsilon \approx 0.046 \mathrm{~mm}$, hence $\varepsilon / d=0.046 /(1000 d)$. We know the head and flow rate:

$$
\mathrm{h}_{\text {pump }}=45 \mathrm{~m} \approx 80-20 \mathrm{Q}^{2}, \text { solve for } \mathrm{Q} \approx 1.323 \mathrm{~m}^{3} / \mathrm{s}
$$

Then $h_{p}=h_{f}=\frac{8 f^{2} Q^{2}}{\pi^{2} g^{5}}=\frac{8 f(200)(1.323)^{2}}{\pi^{2}(9.81) \mathrm{d}^{5}}=\frac{28.92 \mathrm{f}}{\mathrm{d}^{5}}=45 \mathrm{~m}, \quad$ or: $\quad \mathrm{d} \approx 0.915 \mathrm{f}^{1 / 5}$

$$
\text { Guess } \quad \mathrm{f} \approx 0.02, \quad \mathrm{~d} \approx 0.915(0.02)^{1 / 5} \approx 0.419 \mathrm{~m}
$$

$$
\mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{~d}} \approx 2.6 \mathrm{E} 6, \quad \frac{\varepsilon}{d} \approx 0.000110
$$

Then $\quad \mathrm{f}_{\text {better }} \approx 0.0130, \quad \mathrm{~d}_{\text {better }} \approx 0.384 \mathrm{~m}, \quad \mathrm{Re}_{\text {better }} \approx 2.89 \mathrm{E} 6,\left.\quad \frac{\varepsilon}{d}\right|_{\text {better }} \approx 0.000120$
This converges to $\mathrm{f} \approx 0.0129, \quad \mathrm{Re} \approx 2.89 \mathrm{E} 6, \quad \mathbf{d} \approx \mathbf{0 . 3 8 4} \mathbf{m} . \quad$ Ans.
6.83 For the system of Fig. P6.55, let $\Delta z=$ 80 m and $L=185 \mathrm{~m}$ of cast-iron pipe. What is the pipe diameter for which the flow rate will be $7 \mathrm{~m}^{3} / \mathrm{h}$ ?

Solution: For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast iron, take $\varepsilon \approx$ 0.26 mm , but d is unknown. The energy equation is simply


Fig. P6.55

$$
\Delta \mathrm{z}=80 \mathrm{~m}=\mathrm{h}_{\mathrm{f}}=\frac{8 \mathrm{fLQ}^{2}}{\pi^{2} \mathrm{gd}^{5}}=\frac{8 \mathrm{f}(185)(7 / 3600)^{2}}{\pi^{2}(9.81) \mathrm{d}^{5}}=\frac{5.78 \mathrm{E}-5 \mathrm{f}}{\mathrm{~d}^{5}}, \quad \text { or } \quad \mathrm{d} \approx 0.0591 \mathrm{f}^{1 / 5}
$$

Guess $\quad \mathrm{f} \approx 0.03, \quad \mathrm{~d}=0.0591(0.03)^{1 / 5} \approx 0.0293 \mathrm{~m}, \quad \mathrm{Re}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{d}} \approx 84300, \quad \frac{\varepsilon}{\mathrm{~d}} \approx 0.00887$
Iterate: fbetter $\approx 0.0372$, dbetter $\approx 0.0306 \mathrm{~m}$, Rebetter $\approx 80700, \varepsilon / d \mid$ better $\approx 0.00850$, etc. The process converges to $\mathrm{f} \approx 0.0367, \mathbf{d} \approx \mathbf{0 . 0 3 0 5} \mathbf{~ m}$. Ans.
6.84 It is desired to deliver $60 \mathrm{~m}^{3} / \mathrm{h}$ of water ( $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}, \mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$ ) at $20^{\circ} \mathrm{C}$ through a horizontal asphalted cast-iron pipe. Estimate the pipe diameter which will cause the pressure drop to be exactly 40 kPa per 100 meters of pipe length.

Solution: Write out the relation between $\Delta \mathrm{p}$ and friction factor, taking " L " $=100 \mathrm{~m}$ :

$$
\Delta p=f \frac{L}{d} \frac{\rho}{2} V^{2}=f \frac{100}{d} \frac{(998)}{2}\left[\frac{60 / 3600}{(\pi / 4) d^{2}}\right]^{2}=40,000=22.48 \frac{f}{d^{5}}, \quad \text { or: } \quad d^{5}=0.00562 f
$$

Knowing $\varepsilon=0.12 \mathrm{~mm}$, then $\varepsilon / d=0.00012 / d$ and $\operatorname{Red}=4 \rho \mathrm{Q} /(\pi \mu d)=21178 / d$. Use EES, or guess $f \approx 0.02$ and iterate until the proper diameter and friction factor are found.
Final convergence: $f \approx 0.0216$; Red $\approx 204,000 ; \boldsymbol{d}=\mathbf{0 . 1 0 4} \mathbf{~ m}$. Ans.

P6.85 Repeat Prob. 6.26 if the fluid is water at $20^{\circ} \mathrm{C}$. This is more laborious than the earlier problem, but the basic concepts are just the same.
Them $Z_{a}=22 \mathrm{~m}$
The system sketch is repeated here for convenience.

Solution: The problem has three parts.
(a) What is $D_{2}$ if its flow rate is twice that of pipe 1 ?

If $\mathrm{Re}_{D}$ is not too large, $10^{4}-10^{5}$, we use Eq. (6.41) for a quick estimate. If head loss and $(L, \rho, \mu)$ are constant, then $Q^{1.75}$ is proportional to $D^{4.75}$, or

Fig. P6.26
$Q_{\text {turbulent }} \propto D^{19 / 7}$. If the flow rate doubles, then

$$
\frac{Q_{2}}{Q_{1}}=2.0 \approx\left(\frac{D_{2}}{D_{1}}\right)^{19 / 7}, \text { hence } D_{2} \approx(5 \mathrm{~cm})(2.0)^{7 / 19} \approx 0.0645 \mathrm{~cm} \quad \text { Ans. }(a)
$$

$$
\text { More accurate result from part(c): } D_{2}=\mathbf{0 . 0 6 4 9 5} \mathbf{m}
$$

(b) Are both pipe flows turbulent? Well, we think so and used Eq. (6.41). Check this later.
(c) What is the flow rate in pipe $2\left(\mathrm{~m}^{3} / \mathrm{s}\right)$ ? Neglect minor losses. For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. The energy equation yields the same for both pipes:

$$
\Delta z=22 m-15 m=7 m=f_{2} \frac{L}{D_{2}} \frac{V_{2}^{2}}{2 g}=f_{1} \frac{L}{D_{1}} \frac{V_{1}^{2}}{2 g}, \quad \text { smooth tube }
$$

Since we don't know $\mathrm{Re}_{D}$, or even know the exact $D_{2}$, this requires either iteration or EES. The results (obtained by EES) are:

$$
\begin{aligned}
& \operatorname{Re}_{1}=370,000 ; f_{1}=0.01391 ; D_{1}(\text { given })=0.05 \mathrm{~m} ; Q_{1}=\mathbf{0 . 0 1 4 5 4} \mathbf{m}^{\mathbf{3}} / \mathbf{s} \\
& \operatorname{Re}_{2}=569,000 ; f_{2}=0.01285 ; D_{2}=\mathbf{0 . 0 6 4 9 5} \mathbf{m} ; Q_{2}=0.02908 \mathrm{~m}^{3} / \mathrm{s}=2 Q_{1}
\end{aligned}
$$

The Reynolds numbers are too high for the power-law, Eq. (6.41), but the error is only 0.7\%.
6.86 SAE 10 oil at $20^{\circ} \mathrm{C}$ flows at an average velocity of $2 \mathrm{~m} / \mathrm{s}$ between two smooth parallel horizontal plates 3 cm apart. Estimate (a) the centerline velocity, (b) the head loss per meter, and (c) the pressure drop per meter.

Solution: For SAE 10 oil, take $\rho=870 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.104 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The half-distance between plates is called " $h$ " (see Fig. 6.37). Check Dh and Re:

$$
\begin{gathered}
\mathrm{D}_{\mathrm{h}}=\frac{4 \mathrm{~A}}{\mathrm{P}}=4 \mathrm{~h}=6 \mathrm{~cm}, \quad \operatorname{Re}_{\mathrm{D}_{\mathrm{h}}}=\frac{\rho \mathrm{VD}_{\mathrm{h}}}{\mu}=\frac{870(2.0)(0.06)}{0.104} \approx 1004 \text { (laminar) } \\
\text { Then } \mathrm{u}_{\mathrm{CL}}=\mathrm{u}_{\max }=\frac{3}{2} \mathrm{~V}=\frac{3}{2}(2.0) \approx 3.0 \mathrm{~m} / \mathrm{s} \text { Ans. (a) }
\end{gathered}
$$

The head loss and pressure drop per meter folow from laminar theory, Eq. (6.63):

$$
\begin{gathered}
\Delta \mathrm{p}=\frac{3 \mu \mathrm{VL}}{\mathrm{~h}^{2}}=\frac{3(0.104)(2.0)(1.0)}{(0.015 \mathrm{~m})^{2}} \approx 2770 \mathrm{~Pa} / \mathrm{m} \quad \text { Ans. (c) } \\
\mathrm{h}_{\mathrm{f}}=\frac{\Delta \mathrm{p}}{\rho \mathrm{~g}}=\frac{2770}{870(9.81)} \approx \mathbf{0 . 3 2 5} \mathbf{~ m} / \mathrm{m} \text { Ans. (b) }
\end{gathered}
$$

6.87 A commercial-steel annulus 40 ft long, with $a=1$ in and $\mathrm{b}=\frac{1}{2}$ in, connects two reservoirs which differ in surface height by 20 ft . Compute the flow rate in $\mathrm{ft}^{3} / \mathrm{s}$ through the annulus if the fluid is water at $20^{\circ} \mathrm{C}$.

Solution: For water, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. For commercial steel, take $\varepsilon \approx 0.00015 \mathrm{ft}$. Compute the hydraulic diameter of the annulus:

$$
\begin{gathered}
\mathrm{D}_{\mathrm{h}}=\frac{4 \mathrm{~A}}{\mathrm{P}}=2(\mathrm{a}-\mathrm{b})=1 \text { inch; } \\
\mathrm{h}_{\mathrm{f}}=20 \mathrm{ft}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{D}_{\mathrm{h}}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=\mathrm{f}\left(\frac{40}{1 / 12}\right) \frac{\mathrm{V}^{2}}{2(32.2)}, \quad \text { or: } \quad \mathrm{fV}^{2} \approx 2.683
\end{gathered}
$$

We can make a reasonable estimate by simply relating the Moody chart to $D h$, rather than the more complicated "effective diameter" method of Eq. (6.77). Thus

$$
\begin{aligned}
& \frac{\varepsilon}{\mathrm{D}_{\mathrm{h}}}=\frac{0.00015}{1 / 12} \approx 0.0018, \quad \text { Guess } \mathrm{f}_{\text {rough }} \approx 0.023, \quad \mathrm{~V}=(2.683 / 0.023)^{1 / 2} \approx 10.8 \frac{\mathrm{ft}}{\mathrm{~s}} \\
& \mathrm{Re}=\frac{\rho \mathrm{VD}_{\mathrm{h}}}{\mu}=\frac{1.94(10.8)(1 / 12)}{2.09 \mathrm{E}-5} \approx 83550, \quad \mathrm{f}_{\text {better }} \approx 0.0249, \quad \mathrm{~V}_{\text {better }} \approx 10.4 \frac{\mathrm{ft}}{\mathrm{~s}}
\end{aligned}
$$

This converges to $\mathrm{f} \approx 0.0250, \mathrm{~V} \approx 10.37 \mathrm{ft} / \mathrm{s}, \mathrm{Q}=\pi\left(\mathrm{a}^{2}-\mathrm{b}^{2}\right) \mathrm{V}=\mathbf{0 . 1 7} \mathbf{f t}^{\mathbf{3}} / \mathbf{s} . \quad$ Ans.
6.88 An oil cooler consists of multiple parallel-plate passages, as shown in Fig. P6.88. The available pressure drop is 6 kPa , and the fluid is SAE 10 W oil at $20^{\circ} \mathrm{C}$. If the desired total flow rate is $900 \mathrm{~m}^{3} / \mathrm{h}$, estimate the appropriate number of passages. The plate walls are hydraulically smooth.


Fig. P6.88
Solution: For SAE 10W oil, $\rho=870 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.104 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The pressure drop remains 6 kPa no matter how many passages there are (ducts in parallel). Guess laminar flow, Eq. (6.63),

$$
Q_{\text {one passage }}=\frac{b h^{3}}{3 \mu} \frac{\Delta p}{L}
$$

where $h$ is the half-thickness between plates. If there are $N$ passages, then $b=50 \mathrm{~cm}$ for all and $h=0.5 \mathrm{~m} /(2 N)$. We find $h$ and $N$ such that $N Q=900 \mathrm{~m}^{3} / \mathrm{h}$ for the full set of passages. The problem is ideal for EES, but one can iterate with a calculator also. We find that 18 passages are one too many- Q only equals $835 \mathrm{~m}^{3} / \mathrm{h}$. The better solution is:

$$
N=17 \text { passages, } \quad Q_{N}=935 \mathrm{~m}^{3} / \mathrm{h}, \quad h=1.47 \mathrm{~cm}, \quad \operatorname{Re}_{\mathrm{D}_{\mathrm{h}}}=512 \text { (laminar flow) }
$$

6.89 An annulus of narrow clearance causes a very large pressure drop and is useful as an accurate measurement of viscosity. If a smooth annulus 1 m long with $a=50 \mathrm{~mm}$ and $b=49 \mathrm{~mm}$ carries an oil flow at $0.001 \mathrm{~m}^{3} / \mathrm{s}$, what is the oil viscosity if the pressure drop is 250 kPa ?

Solution: Assuming laminar flow, use Eq. (6.73) for the pressure drop and flow rate:

$$
\begin{gathered}
\mathrm{Q}=\frac{\pi}{8 \mu} \frac{\Delta \mathrm{p}}{\mathrm{~L}}\left[\mathrm{a}^{4}-\mathrm{b}^{4}-\frac{\left(\mathrm{a}^{2}-\mathrm{b}^{2}\right)^{2}}{\ln (\mathrm{a} / \mathrm{b})}\right], \quad \text { or, for the given data: } \\
0.001 \mathrm{~m}^{3} / \mathrm{s}=\frac{\pi}{8 \mu}\left(\frac{250000}{1 \mathrm{~m}}\right)\left[(0.05)^{4}-(0.049)^{4}-\frac{\left\{(0.05)^{2}-(0.049)^{2}\right\}^{2}}{\ln (0.05 / 0.049)}\right] \\
\text { Solve for } \mu \approx \mathbf{0 . 0 0 6 5} \mathbf{~ k g} / \mathbf{m} \cdot \mathbf{s} \text { Ans. }
\end{gathered}
$$

6.90 A 90-ft-long sheet-steel duct carries air at approximately $20^{\circ} \mathrm{C}$ and 1 atm . The duct cross section is an equilateral triangle whose side measures 9 in. If a blower can supply 1 hp to the flow, what flow rate, in $\mathrm{ft}^{3} / \mathrm{s}$, will result?


Fig. P6.90

Solution: For air at $20^{\circ} \mathrm{C}$ and 1 atm , take $\rho \approx 0.00234$ slug/ft ${ }^{3}$ and $\mu=3.76 \mathrm{E}-7$ slug/ft•s. Compute the hydraulic diameter, and express the head loss in terms of Q :

$$
\begin{gathered}
D_{h}=\frac{4 A}{P}=\frac{4(1 / 2)(9)\left(9 \sin 60^{\circ}\right)}{3(9)}=5.2^{\prime \prime}=0.433 \mathrm{ft} \\
h_{f}=f \frac{L}{D_{h}} \frac{(Q / A)^{2}}{2 g}=f\left(\frac{90}{0.433}\right) \frac{\left\{Q /\left[0.5(9 / 12)^{2} \sin 60^{\circ}\right]\right\}^{2}}{2(32.2)} \approx 54.4 \mathrm{fQ}^{2}
\end{gathered}
$$

For sheet steel, take $\varepsilon \approx 0.00015 \mathrm{ft}$, hence $\varepsilon / \mathrm{Dh} \approx 0.000346$. Now relate everything to the input power:

$$
\begin{gathered}
\text { Power }=1 \mathrm{hp}=550 \frac{\mathrm{ft} \cdot \mathrm{lbf}}{\mathrm{~s}}=\rho \mathrm{gQh}_{\mathrm{f}}=(0.00234)(32.2) \mathrm{Q}\left[54.4 \mathrm{fQ}^{2}\right] \\
\text { or: } \mathrm{fQ}^{3} \approx 134 \text { with } \mathrm{Q} \text { in } \mathrm{ft}^{3} / \mathrm{s}
\end{gathered}
$$

$$
\text { Guess } \mathrm{f} \approx 0.02, \quad \mathrm{Q}=(134 / 0.02)^{1 / 3} \approx 18.9 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{\rho(\mathrm{Q} / \mathrm{A}) \mathrm{D}_{\mathrm{h}}}{\mu} \approx 209000
$$

Iterate: fbetter $\approx 0.0179$, Qbetter $\approx 19.6 \mathrm{ft}^{3} / \mathrm{s}$, Rebetter $\approx 216500$. The process converges to

$$
\mathrm{f} \approx 0.01784, \mathrm{~V} \approx 80.4 \mathrm{ft} / \mathrm{s}, \mathbf{Q} \approx 19.6 \mathrm{ft}^{3} / \mathrm{s} . \quad \text { Ans. }
$$

6.91 Heat exchangers often consist of many triangular passages. Typical is Fig. P6.91, with $L=60 \mathrm{~cm}$ and an isoscelestriangle cross section of side length $a=$ 2 cm and included angle $\beta=80^{\circ}$. If the average velocity is $V=2 \mathrm{~m} / \mathrm{s}$ and the fluid is SAE 10 oil at $20^{\circ} \mathrm{C}$, estimate the pressure drop.


Fig. P6.91

Solution: For SAE 10 oil, take $\rho=870 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.104 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The Reynolds number based on side length $a$ is $\operatorname{Re}=\rho \mathrm{Va} / \mu \approx 335$, so the flow is laminar. The bottom side of the triangle is $2(2 \mathrm{~cm}) \sin 40^{\circ} \approx 2.57 \mathrm{~cm}$. Calculate hydraulic diameter:

$$
\begin{aligned}
& A=\frac{1}{2}(2.57)\left(2 \cos 40^{\circ}\right) \approx 1.97 \mathrm{~cm}^{2} ; \quad \mathrm{P}=6.57 \mathrm{~cm} ; \quad \mathrm{D}_{\mathrm{h}}=\frac{4 \mathrm{~A}}{\mathrm{P}} \approx 1.20 \mathrm{~cm} \\
& \mathrm{Re}_{\mathrm{D}_{\mathrm{h}}}=\frac{\rho \mathrm{VD}_{\mathrm{h}}}{\mu}=\frac{870(2.0)(0.0120)}{0.104} \approx 201 ; \text { from Table 6.4, } \theta=40^{\circ}, \quad \mathrm{fRe} \approx 52.9 \\
& \text { Then } \mathrm{f}=\frac{52.9}{201} \approx 0.263, \quad \Delta \mathrm{p}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{D}_{\mathrm{h}}} \frac{\rho}{2} \mathrm{~V}^{2}=(0.263)\left(\frac{0.6}{0.012}\right)\left(\frac{870}{2}\right)(2)^{2} \\
& \approx \mathbf{2 3 0 0 0} \mathbf{P a} \text { Ans. }
\end{aligned}
$$

6.92 A large room uses a fan to draw in atmospheric air at $20^{\circ} \mathrm{C}$ through a 30 cm by 30 cm commercial-steel duct 12 m long, as in Fig. P6.92. Estimate (a) the air flow rate in $\mathrm{m}^{3} / \mathrm{hr}$ if the room pressure is 10 Pa vacuum; and (b) the room pressure if the flow rate is $1200 \mathrm{~m}^{3} / \mathrm{hr}$. Neglect minor losses.


Fig. P6.92

Solution: For air, take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For commercial steel, $\varepsilon=$ 0.046 mm . For a square duct, $\mathrm{Dh}=$ side-length $=30 \mathrm{~cm}$, hence $\varepsilon / d=0.046 / 300=0.000153$. The (b) part is easier, with flow rate known we can evaluate velocity, Reynolds number, and friction factor:

$$
V=\frac{Q}{A}=\frac{1200 / 3600}{(0.3)(0.3)}=3.70 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad R e_{D_{h}}=\frac{1.2(3.70)(0.3)}{1.8 E-5}=74100, \quad \text { thus } f_{M o o d y} \approx 0.0198
$$

Then the pressure drop follows immediately:

$$
\begin{gathered}
\Delta p=f \frac{L}{D_{h}} \frac{\rho}{2} V^{2}=0.0198\left(\frac{12}{0.3}\right)\left(\frac{1.2}{2}\right)(3.70)^{2}=6.53 \mathrm{~Pa}, \\
\text { or: } \mathbf{P}_{\text {room }}=\mathbf{6 . 5} \mathbf{P a}(\text { vacuum }) \text { Ans. (b) }
\end{gathered}
$$

(a) If $\Delta \mathrm{p}=10 \mathrm{~Pa}$ (vacuum) is known, we must iterate to find friction factor:

$$
\Delta p=10 P a=f\left(\frac{12}{0.3}\right)\left(\frac{1.2}{2}\right) V^{2}, \quad V=\frac{Q}{(0.3)^{2}}, \quad f=f c n\left(\frac{1.2 V(0.3)}{1.8 E-5}, \frac{\varepsilon}{D_{h}}=0.000153\right)
$$

After iteration, the results converge to:

$$
V=4.69 \mathrm{~m} / \mathrm{s} ; \quad \operatorname{Red}=93800 ; \quad f=0.0190 ; \quad \boldsymbol{Q}=0.422 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{1 5 2 0} \mathbf{~ m}^{3} / \mathbf{h} \quad \text { Ans. (a) }
$$

P6.93 In Moody’s Example 6.6, the 6-inch diameter, 200-ft-long asphalted cast iron pipe has a pressure drop of about $280 \mathrm{lbf} / \mathrm{ft}^{2}$ when the average water velocity is $6 \mathrm{ft} / \mathrm{s}$. Compare this to an annular cast iron pipe with an inner diameter of 6 in and the same annular average velocity of 6 $\mathrm{ft} / \mathrm{s}$. (a) What outer diameter would cause the flow to have the same pressure drop of $280 \mathrm{lbf} / \mathrm{ft}^{2}$ ? (b) How do the cross-section areas compare, and why? Use the hydraulic diameter approximation.

Solution: Recall the Ex. 6.6 data, $\varepsilon=0.0004 \mathrm{ft}$. For water at $68^{\circ} \mathrm{F}$, take $\rho=1.94 \mathrm{slug} / \mathrm{ft}^{3}$ and $\mu=$ $2.09 \mathrm{E}-5$ slug/ft-sec. The hydraulic diameter of an annulus is $D_{\mathrm{h}}=2\left(R_{\mathrm{o}}-R_{\mathrm{i}}\right)$, where $R_{\mathrm{i}}=0.25 \mathrm{ft}$. We know the pressure drop, hence the head loss is

$$
h_{f}=f \frac{L}{D_{h}} \frac{V^{2}}{2 g}=f \frac{200 \mathrm{ft}}{2\left(R_{o}-0.25 f t\right)} \frac{(6 \mathrm{ft} / \mathrm{s})^{2}}{32.2 \mathrm{ft} / \mathrm{s}^{2}}=\frac{\Delta p}{\rho g}=\frac{280 \mathrm{lbf} / \mathrm{ft}^{2}}{62.4 \mathrm{lbf} / \mathrm{ft}^{3}}=4.49 \mathrm{ft}
$$

We do not know $f$ or $R_{0}$. The additional relation is the Moody friction factor correlation:
$\frac{1}{\sqrt{f}} \approx-2.0 \log _{10}\left(\frac{\varepsilon / D_{h}}{3.7}+\frac{2.51}{\operatorname{Re}_{D_{h}} \sqrt{f}}\right) \quad$ where $\quad \operatorname{Re}_{D_{h}}=\frac{\rho V D_{h}}{\mu}=\frac{(1.94)(6.0)\left[2\left(R_{o}-0.25\right)\right]}{2.09 E-5}$
(a) For $\varepsilon=0.0004 \mathrm{ft}$, solve these two simultaneously, using EES or Excel, to obtain

$$
f=0.0199 ; \operatorname{Re}_{D_{\text {ho }}}=276,000 ; R_{o}=\mathbf{0 . 4 9 8 f t} \quad \text { Ans. }(a)
$$

(b) The annular gap is $0.498-0.25=0.248 \mathrm{ft}$, just about equal to the inner radius. However, the annular area is three times the area of Moody's pipe! Ans .(b) The annular pipe has much more wall area than a hollow pipe, more friction, so more area is needed to match the pressure drop.
6.94 As shown in Fig. P6.94, a multiduct cross section consists of seven $2-\mathrm{cm}-$ diameter smooth thin tubes packed tightly in a hexagonal "bundle" within a single 6 -cm-diameter tube. Air, at about $20^{\circ} \mathrm{C}$ and 1 atm, flows through this system at 150 $\mathrm{m}^{3} / \mathrm{h}$. Estimate the pressure drop per meter.


Fig. P6.94

Solution: For air, take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. A separate analysis would show that the small triangular cusped passages have fifty times more resistance to the flow than the 2 -cm-diameter tubes. Therefore we assume all the flow goes through the seven $2-\mathrm{cm}$ tubes. Thus each tube takes one-seventh of the flow rate:

$$
\begin{aligned}
& \mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}_{7 \text { tubes }}}=\frac{150 / 3600}{7 \pi(0.01)^{2}} \approx 18.95 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{1.2(18.95)(0.02)}{1.8 \mathrm{E}-5} \approx 25300 \\
& \text { Turbulent: } \quad \mathrm{f}_{\text {smooth }} \approx 0.0245, \quad \Delta \mathrm{p}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\rho}{2} \mathrm{~V}^{2}=0.0245\left(\frac{1.0}{0.02}\right) \frac{1.2}{2}(18.95)^{2} \\
& \Delta \mathrm{p}
\end{aligned} \begin{aligned}
& \approx \mathbf{2 6 0} \mathbf{P a} \text { Ans. }
\end{aligned}
$$

6.95 A wind tunnel is made of wood and is 28 m long, with a rectangular section 50 cm by 80 cm . It draws in sea-level standard air with a fan. If the fan delivers 7 kW of power to the air, estimate (a) the average velocity; and (b) the pressure drop in the wind tunnel.

Solution: For sea-level air, $\rho=1.22 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.81 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The hydraulic diameter is:

$$
D_{h}=\frac{4 A}{P}=\frac{4(50 \mathrm{~cm})(80 \mathrm{~cm})}{2(50+80 \mathrm{~cm})}=61.54 \mathrm{~cm}=0.6154 \mathrm{~m}
$$

(a, b) The known power is related to both the flow rate and the pressure drop:

$$
\begin{gathered}
\text { Power }=Q \Delta p=[H W V]\left[f \frac{L}{D_{h}} \frac{\rho}{2} V^{2}\right] \\
=[(0.5 \mathrm{~m})(0.8 \mathrm{~m}) \mathrm{V}]\left[f \frac{28 \mathrm{~m}}{0.6154 \mathrm{~m}} \frac{1.22 \mathrm{~kg} / \mathrm{m}^{3}}{2} \mathrm{~V}^{2}\right]=11.1 \mathrm{f} \mathrm{~V}^{3}=7000 \mathrm{~W}
\end{gathered}
$$

Thus we need to find V such that $f V^{3}=631 \mathrm{~m}^{3} / \mathrm{s}^{3}$. For wood, take roughness $\varepsilon=0.5 \mathrm{~mm}$. Then $\varepsilon / D \mathrm{~h}=0.0005 \mathrm{~m} / 0.6154 \mathrm{~m}=0.000813$. Use the Moody chart to find V and the Reynolds number. Guess $f \approx 0.02$ to start, or use EES. The iteration converges to:

$$
f=0.0189, \quad \operatorname{Re}_{D_{h}}=1.33 E 6, \quad \boldsymbol{V}=32 \mathrm{~m} / \mathrm{s}, \quad \Delta \boldsymbol{p}=540 \mathrm{~Pa} \quad \text { Ans. }(\mathrm{a}, \mathrm{~b})
$$

6.96 Water at $20^{\circ} \mathrm{C}$ is flowing through a 20-cm-square smooth duct at a (turbulent) Reynolds number of 100,000. For a "laminar
flow element" measurement, it is desired to pack the pipe with a honeycomb array of small square passages (see Fig. P6.28 for

hXh passages
Fig. P6.96 an example). What passage width $h$ will ensure that the flow in each tube will be laminar (Reynolds number less than 2000)?

Solution: The hydraulic diameter of a square is the side length $h$ (or $a$ ). For water, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The Reynolds number establishes flow velocity:

$$
R e_{D_{h}}=100,000=\frac{\rho V d}{\mu}=\frac{998 V(0.2)}{0.001}, \quad \text { Solve for } V=0.501 \frac{\mathrm{~m}}{\mathrm{~s}}
$$

This velocity is the same when we introduce small passages, if we neglect the blockage of the thin passage walls. Thus we merely set the passage Reynolds number $=2000$ :

$$
R e_{h} \frac{\rho V h}{\mu}=\frac{998(0.501) h}{0.001} \leq 2000 \quad \text { if } h \leq \mathbf{0 . 0 0 4} \mathbf{~ m}=\mathbf{4} \mathbf{~ m m} \quad \text { Ans. }
$$

6.97 A heat exchanger consists of multiple parallel-plate passages, as shown in Fig. P6.97. The available pressure drop is 2 kPa , and the fluid is water at $20^{\circ} \mathrm{C}$. If the desired total flow rate is $900 \mathrm{~m}^{3} / \mathrm{h}$, estimate the appropriate number of passages. The plate walls are hydraulically smooth.


Fig. P6.97
Solution: For water, $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Unlike Prob. 6.88, here we expect turbulent flow. If there are $N$ passages, then $\mathrm{b}=50 \mathrm{~cm}$ for all $N$ and the passage thickness is $H=0.5 \mathrm{~m} / N$. The hydraulic diameter is $D \mathrm{~h}=2 H$. The velocity in each passage is related to the pressure drop by Eq. (6.58):

$$
\Delta p=f \frac{L}{D_{h}} \frac{\rho}{2} V^{2} \quad \text { where } f=f_{\text {smooth }}=f c n\left(\frac{\rho V D_{h}}{\mu}\right)
$$

For the given data, $2000 \mathrm{~Pa}=f \frac{2.0 \mathrm{~m}}{2(0.5 \mathrm{~m} / \mathrm{N})} \frac{998 \mathrm{~kg} / \mathrm{m}^{3}}{2} V^{2}$

Select $N$, find $H$ and $V$ and $Q$ total $=A V=b^{2} V$ and compare to the desired flow of 900 $\mathrm{m}^{3} / \mathrm{h}$. For example, guess $N=20$, calculate $f=0.0173$ and $Q$ total $=2165 \mathrm{~m}^{3} / \mathrm{h}$. The converged result is

$$
\begin{gathered}
Q_{\text {total }}=908 \mathrm{~m}^{3} / \mathrm{h}, \quad f=0.028, \\
\operatorname{Re}_{D_{h}}=14400, \quad H=7.14 \mathrm{~mm}, \quad N=70 \text { passages } \quad \text { Ans. }
\end{gathered}
$$

6.98 A rectangular heat exchanger is to be divided into smaller sections using sheets of commercial steel 0.4 mm thick, as sketched in Fig. P6.98. The flow rate is $20 \mathrm{~kg} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$. Basic dimensions are $L=1 \mathrm{~m}, W=20 \mathrm{~cm}$, and $H=10 \mathrm{~cm}$. What is the proper number of square sections if the overall pressure drop is to be no more than 1600 Pa ?


Fig. P6.98

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For commercial steel, $\varepsilon \approx 0.046 \mathrm{~mm}$. Let the short side $(10 \mathrm{~cm})$ be divided into " J " squares. Then the long $(20 \mathrm{~cm})$ side divides into " 2 J " squares and altogether there are $\mathrm{N}=2 \mathrm{~J}^{2}$ squares. Denote the side length of the square as "a," which equals $(10 \mathrm{~cm}) / \mathrm{J}$ minus the wall thickness. The hydraulic diameter of a square exactly equals its side length, $\mathrm{Dh}=a$. Then the pressure drop relation becomes
$\Delta \mathrm{p}=\mathrm{f} \frac{\mathrm{L}}{\mathrm{D}_{\mathrm{h}}} \frac{\rho}{2} \mathrm{~V}^{2}=\mathrm{f} \frac{1.0}{\mathrm{a}}\left(\frac{998}{2}\right)\left(\frac{\mathrm{Q}}{\mathrm{Na}^{2}}\right)^{2} \leq 1600 \mathrm{~Pa}$, where $\mathrm{N}=2 \mathrm{~J}^{2}$ and $\mathrm{a}=\frac{0.1}{\mathrm{~J}}-0.0004$
As a first estimate, neglect the $0.4-\mathrm{mm}$ wall thickness, so $a \approx 0.1 / \mathrm{J}$. Then the relation for $\Delta \mathrm{p}$ above reduces to $\mathrm{fJ} \approx 0.32$. Since $\mathrm{f} \approx 0.036$ for this turbulent Reynolds number ( $\mathrm{Re} \approx$ 1 E 4 ) we estimate that $\mathrm{J} \approx 9$ and in fact this is not bad even including wall thickness:

$$
\begin{gathered}
\mathrm{J}=9, \quad \mathrm{~N}=2(9)^{2}=162, \quad \mathrm{a}=\frac{0.1}{9}-0.0004=0.0107 \mathrm{~m}, \quad \mathrm{~V}=\frac{20 / 998}{162(0.0107)^{2}} \approx 1.078 \frac{\mathrm{~m}}{\mathrm{~s}} \\
\mathrm{Re}=\frac{\rho \mathrm{Va}}{\mu}=\frac{998(1.078)(0.0107)}{0.001} \approx 11526, \quad \frac{\varepsilon}{\mathrm{a}}=\frac{0.046}{10.7} \approx 0.00429, \quad \mathrm{f}_{\text {Moody }} \approx 0.0360 \\
\text { Then } \quad \Delta \mathrm{p}=(0.036)\left(\frac{1.0}{0.0107}\right)\left(\frac{998}{2}\right)(1.078)^{2} \approx 1950 \mathrm{~Pa}
\end{gathered}
$$

So the wall thickness increases $V$ and decreases $a$ so $\Delta \mathrm{p}$ is too large. Try $\mathrm{J}=8$ :

$$
\begin{gathered}
\mathrm{J}=8, \quad \mathrm{~N}=128, \quad \mathrm{a}=0.0121 \mathrm{~m}, \quad \mathrm{~V}=1.069 \frac{\mathrm{~m}}{\mathrm{~s}}, \\
\operatorname{Re}=12913, \quad \frac{\varepsilon}{\mathrm{a}}=0.0038, \quad \mathrm{f} \approx 0.0347
\end{gathered}
$$

Then $\Delta \mathrm{p}=\mathrm{f}(\mathrm{L} / \mathrm{a})(\rho / 2) \mathrm{V}^{2} \approx \mathbf{1 6 3 6}$ Pa. Close enough, $\mathrm{J}=8, \mathbf{N}=128$ Ans.
[I suppose a practical person would specify $\mathrm{J}=7, \mathrm{~N}=98$, to keep $\Delta \mathrm{p}<1600 \mathrm{~Pa}$.]

P6.99 In Sec. 6.11 it was mentioned
that Roman aqueduct customers obtained extra water by attaching a diffuser to their
pipe exits. Fig. P6.99 shows a simulation:
a smooth inlet pipe, with and without a $15^{\circ}$
diffuser expanding to a 5-cm-diameter exit.


Fig. P6.99

The pipe entrance is sharp-edged.
Calculate the flow rate $(a)$ without, and $(b)$ with the diffuser.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. The energy equation between the aqueduct surface and the pipe exit yields

$$
z_{\text {surf }}=z_{2}+\frac{V_{2}^{2}}{2 g}+h_{f}+\Sigma h_{m}=z_{2}+\frac{V_{2}^{2}}{2 g}+\frac{V_{1}^{2}}{2 g}\left(f \frac{L}{D_{1}}+K_{\text {entrance }}+K_{\text {diffuser }}\right)
$$

(a) Without the diffuser, $K_{\text {diff }}=0$, and $V_{1}=V_{2}$. For a sharp edge, take $K_{\text {ent }}=0.5$. We obtain
$2 m=\frac{V_{1}^{2}}{2 g}\left(1+f \frac{2 m}{0.03 m}+0.5\right)$, with $f=f c n\left(\operatorname{Re}=\rho V_{1} D_{1} / \mu\right)$
Solve: $\operatorname{Re}=115,000 ; f=0.0175 ; V_{1}=4.48 \mathrm{~m} / \mathrm{s} ; Q_{\text {without }}=0.00271 \mathrm{~m}^{3} / \mathrm{s}$ Ans. $(a)$
(b) With the diffuser, from Fig. 6.23, for $D_{1} / D_{2}=3 / 5=0.6$ and $2 \theta=15^{\circ}$, read $K_{\text {diffuser }} \approx 0.2$. From one-dimensional continuity, $V_{2}=V_{1}(3 / 5)^{2}=0.36 V_{1}$. The energy equation becomes

$$
2 m=\frac{\left(0.36 V_{1}\right)^{2}}{2 g}+\frac{V_{1}^{2}}{2 g}\left(1+f \frac{2 m}{0.03 m}+0.5+0.2\right)
$$

Solve: $\operatorname{Re}=134,000 ; f=0.0169 ; V_{1}=3.84 \mathrm{~m} / \mathrm{s} ; Q_{\text {with }}=0.00316 \mathrm{~m}^{3} / \mathrm{s}$ Ans.(b)

Adding the diffuser increases the flow rate by $\mathbf{1 7 \%}$. [NOTE: Don't know if the Romans did this, but a well-rounded entrance, $K_{\text {ent }}=0.05$, would increase the flow rate by another $15 \%$.]
6.100 Repeat Prob. 6.92 by including minor losses due to a sharp-edged entrance, the exit into the room, and an open gate valve. If the room pressure is 10 Pa (vacuum), by what percentage is the flow rate decreased from part (a) of Prob. 6.92?

Solution: For air, take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For commercial steel, $\varepsilon=$


Fig. P6.100 0.046 mm . For a square duct, $\mathrm{Dh}=$ side-
length $=30 \mathrm{~cm}$, hence $\varepsilon / d=0.046 / 300=0.000153$. Now add Kentrance $=0.5$, Kexit $=1.0$, and Kvalve $=0.03$ to the energy equation:

$$
\Delta p=10 P a=\frac{\rho}{2} V^{2}\left(f \frac{L}{D_{h}}+\sum K_{\text {minor }}\right)=\left(\frac{1.2}{2}\right)\left[\frac{Q}{(0.3)^{2}}\right]^{2}\left(f \frac{12}{0.3}+0.5+1.0+0.03\right)
$$

where we compute $f$ based on $\mathrm{Re}_{\mathrm{D}_{\mathrm{h}}}=(1.2) V(0.3) / 1.8 \mathrm{E}-5$ and $\varepsilon / \mathrm{Dh}=0.000153$. The iteration converges to

$$
V=2.65 \mathrm{~m} / \mathrm{s} ; \quad \operatorname{Red}=53000 ; \quad f=0.0212 ; \quad \boldsymbol{Q}=0.238 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{8 6 0} \mathbf{~ m}^{\mathbf{3}} / \mathbf{h}
$$

Moral: Don't forget minor losses! The flow rate is $\mathbf{4 3 \%}$ less than Prob. 6.92! Ans.

NOTE: IN PROBLEMS 6.100-6.110, MINOR LOSSES ARE INCLUDED.
6.101 In Fig. P6.101 a thick filter is being tested for losses. The flow rate in the pipe is $7 \mathrm{~m}^{3} / \mathrm{min}$, and the upstream pressure is 120 kPa . The fluid is air at $20^{\circ} \mathrm{C}$. Using the water-manometer reading, estimate the loss coefficient $K$ of the filter.


Fig. P6. 101
Solution: The upstream density is $\rho_{\text {air }}=p /(R T)=120000 /[287(293)]=1.43 \mathrm{~kg} / \mathrm{m}^{3}$. The average velocity $V$ (which is used to correlate loss coefficient) follows from the flow rate:

$$
V=\frac{Q}{A_{\text {pipe }}}=\frac{7 / 60 \mathrm{~m}^{3} / \mathrm{s}}{(\pi / 4)(0.1 \mathrm{~m})^{2}}=14.85 \mathrm{~m} / \mathrm{s}
$$

The manometer measures the pressure drop across the filter:

$$
\Delta p_{\text {mano }}=\left(\rho_{w}-\rho_{a}\right) g h_{\text {mano }}=\left(998-1.43 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)(0.04 \mathrm{~m})=391 \mathrm{~Pa}
$$

This pressure is correlated as a loss coefficient using Eq. (6.78):

$$
K_{\text {filter }}=\frac{\Delta p_{\text {filter }}}{(1 / 2) \rho V^{2}}=\frac{391 \mathrm{~Pa}}{(1 / 2)\left(1.43 \mathrm{~kg} / \mathrm{m}^{3}\right)(14.85 \mathrm{~m} / \mathrm{s})^{2}} \approx 2.5 \quad \text { Ans. }
$$

6.102 A 70 percent efficient pump delivers water at $20^{\circ} \mathrm{C}$ from one reservoir to another 20 ft higher, as in Fig. P6.102. The piping system consists of 60 ft of galvanizediron 2 -in pipe, a reentrant entrance, two screwed $90^{\circ}$ long-radius elbows, a screwedopen gate valve, and a sharp exit. What is the input power required in horsepower with and without a $6^{\circ}$ well-designed conical expansion added to the exit? The flow rate is $0.4 \mathrm{ft}^{3} / \mathrm{s}$.


## Fig. P6.102

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. For galvanized iron, $\varepsilon \approx 0.0005 \mathrm{ft}$, whence $\varepsilon / d=0.0005 /(2 / 12 \mathrm{ft}) \approx 0.003$. Without the $6^{\circ}$ cone, the minor losses are:

$$
\mathrm{K}_{\text {reentrant }} \approx 1.0 ; \quad \mathrm{K}_{\text {elbows }} \approx 2(0.41) ; \quad \mathrm{K}_{\text {gate valve }} \approx 0.16 ; \quad \mathrm{K}_{\text {sharp exit }} \approx 1.0
$$

Evaluate $\quad \mathrm{V}=\frac{\mathrm{Q}}{\mathrm{A}}=\frac{0.4}{\pi(2 / 12)^{2} / 4}=18.3 \frac{\mathrm{ft}}{\mathrm{s}} ; \quad \mathrm{Re}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{1.94(18.3)(2 / 12)}{2.09 \mathrm{E}-5} \approx 284000$
At this Re and roughness ratio, we find from the Moody chart that $\mathrm{f} \approx 0.0266$. Then
(a) $\mathrm{h}_{\text {pump }}=\Delta \mathrm{z}+\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}\left(\mathrm{f} \frac{\mathrm{L}}{\mathrm{d}}+\sum \mathrm{K}\right)=20+\frac{(18.3)^{2}}{2(32.2)}\left[0.0266\left(\frac{60}{2 / 12}\right)+1.0+0.82+0.16+1.0\right]$

$$
\text { or } \begin{aligned}
\mathrm{h}_{\text {pump }} \approx 85.6 \mathrm{ft}, \quad \text { Power } & =\frac{\rho \mathrm{gQh}_{\mathrm{p}}}{\eta}=\frac{(62.4)(0.4)(85.6)}{0.70} \\
& =3052 \div 550 \approx \mathbf{5 . 5 5} \mathbf{~ h p} \text { Ans. (a) }
\end{aligned}
$$

(b) If we replace the sharp exit by a $6^{\circ}$ conical diffuser, from Fig. 6.23 , Kexit $\approx 0.3$. Then

$$
\mathrm{h}_{\mathrm{p}}=20+\frac{(18.3)^{2}}{2(32.2)}\left[0.0266\left(\frac{60}{2 / 12}\right)+1.0+.82+.16+0.3\right]=81.95 \mathrm{ft}
$$

then Power $=(62.4)(0.4)(81.95) / 0.7 \div 550 \approx 5.31 \mathbf{h p}(4 \%$ less $) \quad$ Ans. (b)
6.103 The reservoirs in Fig. P6.103 are connected by cast-iron pipes joined abruptly, with sharp-edged entrance and exit. Including minor losses, estimate the flow of water at $20^{\circ} \mathrm{C}$ if the surface of reservoir 1 is 45 ft higher than that of reservoir 2.


Fig. P6.103

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. Let "a" be the small pipe and "b" the larger. For wrought iron, $\varepsilon \approx 0.00015 \mathrm{ft}$, whence $\varepsilon / d a=$ 0.0018 and $\varepsilon / d b=0.0009$. From the continuity relation,

$$
\mathrm{Q}=\mathrm{V}_{\mathrm{a}} \frac{\pi}{4} \mathrm{~d}_{\mathrm{a}}^{2}=\mathrm{V}_{\mathrm{b}} \frac{\pi}{4} \mathrm{~d}_{\mathrm{b}}^{2} \quad \text { or, since } \mathrm{d}_{\mathrm{b}}=2 \mathrm{~d}_{\mathrm{a}} \text {, we obtain } \mathrm{V}_{\mathrm{b}}=\frac{1}{4} \mathrm{~V}_{\mathrm{a}}
$$

For pipe "a" there are two minor losses: a sharp entrance, $\mathrm{K} 1=0.5$, and a sudden expansion, Fig. 6.22, Eq. (6.101), $\mathrm{K} 2=\left[1-(1 / 2)^{2}\right]^{2} \approx 0.56$. For pipe "b" there is one minor loss, the submerged exit, $\mathrm{K} 3 \approx 1.0$. The energy equation, with equal pressures at (1) and (2) and near zero velocities at (1) and (2), yields

$$
\begin{gathered}
\Delta \mathrm{z}=\mathrm{h}_{\mathrm{f}-\mathrm{a}}+\sum \mathrm{h}_{\mathrm{m}-\mathrm{a}}+\mathrm{h}_{\mathrm{f}-\mathrm{b}}+\sum \mathrm{h}_{\mathrm{m}-\mathrm{b}}=\frac{\mathrm{V}_{\mathrm{a}}^{2}}{2 \mathrm{~g}}\left(\mathrm{f}_{\mathrm{a}} \frac{\mathrm{~L}_{\mathrm{a}}}{\mathrm{~d}_{\mathrm{a}}}+0.5+0.56\right)+\frac{\mathrm{V}_{\mathrm{b}}^{2}}{2 \mathrm{~g}}\left(\mathrm{f}_{\mathrm{b}} \frac{\mathrm{~L}_{\mathrm{b}}}{\mathrm{~d}_{\mathrm{b}}}+1.0\right), \\
\text { or, since } \quad \mathrm{V}_{\mathrm{b}}=\mathrm{V}_{\mathrm{a}} / 4, \quad \Delta \mathrm{z}=45 \mathrm{ft}=\frac{\mathrm{V}_{\mathrm{a}}^{2}}{2(32.2)}\left[240 \mathrm{f}_{\mathrm{a}}+1.06+\frac{120}{16} \mathrm{f}_{\mathrm{b}}+\frac{1.0}{16}\right]
\end{gathered}
$$

where fa and fb are separately related to different values of Re and $\varepsilon / d$. Guess to start:

$$
\begin{gathered}
\mathrm{f}_{\mathrm{a}} \approx \mathrm{f}_{\mathrm{b}} \approx 0.02 \text { : then } \mathrm{V}_{\mathrm{a}}=21.85 \mathrm{ft} / \mathrm{s}, \quad \mathrm{Re}_{\mathrm{a}} \approx 169000, \quad \varepsilon / \mathrm{d}_{\mathrm{a}}=0.0018, \quad \mathrm{f}_{\mathrm{a}-2} \approx 0.0239 \\
\mathrm{~V}_{\mathrm{b}}=5.46 \mathrm{ft} / \mathrm{s}, \quad \mathrm{Re}_{\mathrm{b}} \approx 84500, \quad \varepsilon / \mathrm{d}_{\mathrm{b}}=0.0009, \quad \mathrm{f}_{\mathrm{b}-2} \approx 0.0222 \\
\text { Converges to: } \mathrm{f}_{\mathrm{a}}=0.024, \quad \mathrm{f}_{\mathrm{b}}=0.0224, \quad \mathrm{~V}_{\mathrm{a}} \approx 20.3 \mathrm{ft} / \mathrm{s}, \\
\mathrm{Q}=\mathrm{V}_{\mathrm{a}} \mathrm{~A}_{\mathrm{a}} \approx \mathbf{0 . 1 1 1} \mathbf{f t}^{3} / \mathbf{s} . \text { Ans. }
\end{gathered}
$$

6.104 Reconsider the air hockey table of Problem 3.162, but with inclusion of minor losses. The table is 3 ft by 6 ft in area, with 1/16-in-diameter holes spaced every inch in a rectangular grid ( 2592 holes total). The required jet speed from each hole is $50 \mathrm{ft} / \mathrm{s}$. Your job is to select an appropriate blower to meet the requirements.


Hint: Assume that the air is stagnant in the manifold under the table surface, and assume sharp-edge inlets at each hole. (a) Estimate the pressure rise (in lbf/in ${ }^{2}$ ) required of the blower. (b) Compare your answer to the previous calculation in Prob. 3.162, where minor losses were ignored. Are minor losses significant?

Solution: Write the energy equation between manifold and atmosphere:

$$
\frac{p_{1}}{\rho g}+\alpha_{1} \frac{V_{1}^{2}}{2 g}+z_{1}=\frac{p_{2}}{\rho g}+\alpha_{2} \frac{V_{2}^{2}}{2 g}+z_{2}+h_{\text {losses }}, \quad \text { where } h_{\text {losses }} \approx K_{\text {inlet }} \frac{V_{\text {jet }}^{2}}{2 g}
$$

Neglect $\mathrm{V} 1 \approx 0$ and $\mathrm{z} 1 \approx \mathrm{z} 2$, assume $\alpha 1,2=1.0$, and solve for

$$
\Delta p=p_{1}-p_{2}=\frac{\rho}{2} V_{j e t}^{2}\left(1+K_{\text {inlet }}\right), \quad \text { where } K_{\text {sharp-edge-inlet }} \approx 0.5
$$

Clearly, the pressure drop is about $\mathbf{5 0 \%}$ greater due to the minor loss. Ans. (b) Work out $\Delta \mathrm{p}$, assuming $\rho$ air $\approx 0.00234$ slug $/ \mathrm{ft}^{3}$ :

$$
\Delta p=\frac{0.00234}{2}(50)^{2}(1+0.5)=4.39 \frac{\mathrm{lbf}}{{f t^{2}}_{2}} \div 144=\mathbf{0 . 0 3 0 5} \frac{\mathbf{l b f}}{\mathbf{i n}^{2}} \quad \text { Ans. (a) }
$$

(Again, this is 50\% higher than Prob. 3.162.)
6.105 The system in Fig. P6.105 consists of 1200 m of 5 cm cast-iron pipe, two $45^{\circ}$ and four $90^{\circ}$ flanged long-radius elbows, a fully open flanged globe valve, and a sharp exit into a reservoir. If the elevation at point 1 is 400 m , what gage pressure is required at point 1 to deliver $0.005 \mathrm{~m}^{3} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$ into the reservoir?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast iron, take $\varepsilon \approx 0.26 \mathrm{~mm}$, hence $\varepsilon / d=0.0052$.


Fig. P6.105 With the flow rate known, we can compute V, Re:

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{0.005}{(\pi / 4)(0.05)^{2}}=2.55 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \mathrm{Re}=\frac{998(2.55)(0.05)}{0.001} \approx 127000, \quad \mathrm{f}_{\text {Moody }} \approx 0.0315
$$

The minor losses may be listed as follows:
$45^{\circ}$ long-radius elbow: $K \approx 0.2 ; 90^{\circ}$ long-radius elbow: $\mathrm{K} \approx 0.3$
Open flanged globe valve: $K \approx 8.5$; submerged exit: $K \approx 1.0$
Then the energy equation between (1) and (2-the reservoir surface) yields

$$
\begin{gathered}
\frac{\mathrm{p}_{1}}{\rho \mathrm{~g}}+\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}+\mathrm{z}_{1}=0+0+\mathrm{z}_{2}+\mathrm{h}_{\mathrm{f}}+\sum \mathrm{h}_{\mathrm{m}}, \\
\text { or: } \quad \mathrm{p}_{1} /(\rho \mathrm{g})=500-400+\frac{(2.55)^{2}}{2(9.81)}\left[0.0315\left(\frac{1200}{0.05}\right)+0.5+2(0.2)+4(0.3)+8.5+1-1\right] \\
=100+253=353 \mathrm{~m}, \quad \text { or: } \quad \mathbf{p}_{1}=(998)(9.81)(353) \approx 3.46 \mathbf{~ M P a \quad \text { Ans. }}
\end{gathered}
$$

6.106 The water pipe in Fig. 6.106 slopes upward at $30^{\circ}$. The pipe is 1 -inch diameter and smooth. The flanged globe valve is fully open. If the mercury manometer shows a 7-inch deflection, what is the flow rate in cubic feet per sec?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.94 slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. The pipe length and elevation change are


Fig. P6. 106

$$
\mathrm{L}=\frac{10 \mathrm{ft}}{\cos 30^{\circ}}=11.55 \mathrm{ft} ; \quad \mathrm{z}_{2}-\mathrm{z}_{1}=10 \tan 30^{\circ}=5.77 \mathrm{ft}, \quad \text { Open } 1^{\prime \prime} \text { globe valve: } \mathrm{K} \approx 13
$$

The manometer indicates the total pressure change between (1) and (2):

$$
\mathrm{p}_{1}-\mathrm{p}_{2}=\left(\rho_{\mathrm{Merc}}-\rho_{\mathrm{w}}\right) \mathrm{gh}+\rho_{\mathrm{w}} \mathrm{~g} \Delta \mathrm{z}=(13.6-1)(62.4)\left(\frac{7}{12}\right)+62.4(5.77) \approx 819 \mathrm{psf}
$$

The energy equation yields

$$
\frac{\mathrm{p}_{1}-\mathrm{p}_{2}}{\rho \mathrm{~g}}=\Delta \mathrm{z}+\mathrm{h}_{\mathrm{f}}+\mathrm{h}_{\mathrm{m}}=5.77+\frac{\mathrm{V}^{2}}{2(32.2)}\left[\mathrm{f} \frac{11.55}{1 / 12}+13\right] \approx \frac{819 \mathrm{lbf} / \mathrm{ft}^{2}}{62.4 \mathrm{lbf} / \mathrm{ft}^{3}}
$$

or: $\mathrm{V}^{2} \approx \frac{2(32.2)(7.35)}{(139 \mathrm{f}+13)}$. Guess $\mathrm{f} \approx 0.02, \mathrm{~V} \approx 5.48 \frac{\mathrm{ft}}{\mathrm{s}}, \mathrm{Re} \approx 42400, \mathrm{f}_{\text {new }} \approx 0.0217$
Rapid convergence to $\mathrm{f} \approx 0.0217, \mathrm{~V} \approx 5.44 \mathrm{ft} / \mathrm{s}, \mathrm{Q}=\mathrm{V}(\pi / 4)(1 / 12)^{2} \approx \mathbf{0 . 0 2 9 6} \mathbf{f t}^{\mathbf{3}} / \mathbf{s} . \quad$ Ans. [NOTE that the manometer reading of 7 inches exactly balances the friction losses, and the hydrostatic pressure change $\rho \mathrm{g} \Delta \mathrm{z}$ cancels out of the energy equation.]
6.107 In Fig. P6.107 the pipe is galvanized iron. Estimate the percentage increase in the flow rate (a) if the pipe entrance is cut off flush with the wall and (b) if the butterfly valve is opened wide.


Fig. P6.107

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For galvanized iron, take $\varepsilon \approx 0.15 \mathrm{~mm}$, hence $\varepsilon / d=0.003$. First establish minor losses as shown:

$$
\text { Protruding entrance (Fig. 6.21a), } \frac{\mathrm{L}}{\mathrm{~d}} \approx 1.2, \mathrm{~K} \approx 1 \text {; }
$$

$$
\text { Butterfly @ 30º (Fig 6.19) K } \approx 80 \pm 20
$$

The energy equation, with $\mathrm{p} 1=\mathrm{p} 2$, yields:

$$
\begin{gathered}
\Delta \mathrm{z}=\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}+\mathrm{h}_{\mathrm{f}}+\sum \mathrm{h}_{\mathrm{m}}=\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}\left[1+\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}+\sum \mathrm{K}\right]=\frac{\mathrm{V}^{2}}{2(9.81)}\left[1+\mathrm{f}\left(\frac{2}{0.05}\right)+1.0+80 \pm 20\right]=5 \mathrm{~m} \\
\text { Guess } \mathrm{f} \approx 0.02, \quad \mathrm{~V} \approx 1.09 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re} \approx 54300, \quad \frac{\varepsilon}{d}=0.003, \\
\mathrm{f}_{\text {new }} \approx 0.0284, \quad \mathrm{~V}_{\text {new }} \approx 1.086 \frac{\mathrm{~m}}{\mathrm{~s}}
\end{gathered}
$$

Thus the "base" flow, for our comparison, is $\mathrm{V}_{\mathrm{o}} \approx 1.086 \mathrm{~m} / \mathrm{s}$, $\mathrm{Qo}_{\mathrm{o}} \approx 0.00213 \mathrm{~m}^{3} / \mathrm{s}$.
If we cut off the entrance flush, we reduce Kent from 1.0 to $\underline{\mathbf{0 . 5}}$; hardly a significant reduction in view of the huge butterfly valve loss Kvalve $\approx 80$. The energy equation is

$$
\begin{gathered}
5 \mathrm{~m}=\frac{\mathrm{V}^{2}}{2(9.81)}[1+40 \mathrm{f}+0.5+80 \pm 20], \text { solve } \mathrm{V} \approx 1.090 \frac{\mathrm{~m}}{\mathrm{~s}}, \\
\mathrm{Q}=\mathbf{0 . 0 0 2 1 4} \frac{\mathbf{m}^{3}}{\mathbf{s}}(0.3 \% \text { more }) \text { Ans. (a) }
\end{gathered}
$$

If we open the butterfly wide, Kvalve decreases from 80 to only $\mathbf{0 . 3}$, a huge reduction:

$$
\begin{gathered}
5 \mathrm{~m}=\frac{\mathrm{V}^{2}}{2(9.81)}[1+40 \mathrm{f}+1.0+0.3], \text { solve } \mathrm{V} \approx 5.4 \frac{\mathrm{~m}}{\mathrm{~s}}, \\
\mathrm{Q}=\mathbf{0 . 0 1 0 6} \frac{\mathbf{m}^{3}}{\mathbf{s}}(5 \text { times more }) \text { Ans. (b) }
\end{gathered}
$$

Obviously opening the valve has a dominant effect for this system.
6.108 The water pump in Fig. P6.108 maintains a pressure of 6.5 psig at point 1. There is a filter, a half-open disk valve, and two regular screwed elbows. There are 80 ft of 4-inch diameter commercial steel pipe. (a) If the flow rate is $0.4 \mathrm{ft}^{3} / \mathrm{s}$, what is the loss coefficient of the filter? (b) If the disk valve is wide open and Kfilter $=7$, what is the resulting flow rate?


Fig. P6. 108
Solution: For water, take $\rho=1.94$ slug/ $\mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. The energy equation is written from point 1 to the surface of the tank:

$$
\frac{p_{1}}{\rho g}+\frac{V_{1}^{2}}{2 g}+z_{1}=\frac{p_{2}}{\rho g}+\frac{V_{2}^{2}}{2 g}+z_{2}+h_{f}+K_{\text {valve }}+K_{\text {filter }}+2 K_{\text {elbow }}+K_{\text {exit }}
$$

(a) From the flow rate, $V_{1}=\mathrm{Q} / \mathrm{A}=\left(0.4 \mathrm{ft}^{3} / \mathrm{s}\right) /\left[(\pi / 4)(4 / 12 \mathrm{ft})^{2}\right]=4.58 \mathrm{ft} / \mathrm{s}$. Look up minor losses and enter into the energy equation:

$$
\begin{aligned}
& \frac{(6.5)(144) \mathrm{lbf} / \mathrm{ft}^{2}}{62.4 \mathrm{lbf} / \mathrm{ft}^{3}}+\frac{(4.58 \mathrm{ft} / \mathrm{s})^{2}}{2\left(32.2 \mathrm{ft} / \mathrm{s}^{2}\right)}+0 \\
& \quad=0+0+9 \mathrm{ft}+\frac{(4.58)^{2}}{2(32.2)}\left[f \frac{80 \mathrm{ft}}{(4 / 12 \mathrm{ft})}+2.8+K_{\text {filter }}+2(0.64)+1\right]
\end{aligned}
$$

We can solve for Kfilter if we evaluate $f$. Compute $\operatorname{ReD}=(1.94)(4.58)(4 / 12) /(2.09 \mathrm{E}-5)=$ 141,700 . For commercial steel, $\varepsilon / D=0.00015 \mathrm{ft} / 0.333 \mathrm{ft}=0.00045$. From the Moody chart, $f \approx 0.0193$, and $f L / D=4.62$. The energy equation above becomes:

$$
\begin{gathered}
15.0 \mathrm{ft}+0.326 \mathrm{ft}=9 \mathrm{ft}+0.326\left(4.62+2.8+K_{\text {filter }}+1.28+1\right) \mathrm{ft}, \\
\text { Solve } \quad \boldsymbol{K}_{\text {filter }} \approx \mathbf{9 . 7} \text { Ans. (a) }
\end{gathered}
$$

(b) If $K$ filter $=7.0$ and $V$ is unknown, we must iterate for the velocity and flow rate. The energy equation becomes, with the disk valve wide open (KValve $\approx 0$ ):

$$
15.0 \mathrm{ft}+\frac{V^{2}}{2(32.2)}=9 \mathrm{ft}+\frac{V^{2}}{2(32.2)}\left(f \frac{80}{1 / 3}+0+7.0+1.28+1\right)
$$

Iterate to find $f \approx 0.0189, \quad \operatorname{Re}_{D}=169,000, \quad V=5.49 \mathrm{ft} / \mathrm{s}$,

$$
\mathbf{Q}=\mathbf{A V}=0.48 \mathrm{ft}^{3} / \mathrm{s} \quad \text { Ans. (b) }
$$

6.109 In Fig. P6.109 there are 125 ft of 2 -in pipe, 75 ft of 6 -in pipe, and 150 ft of 3-in pipe, all cast iron. There are three $90^{\circ}$ elbows and an open globe valve, all flanged. If the exit elevation is zero, what horsepower is extracted by the turbine when the flow rate is $0.16 \mathrm{ft}^{3} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$ ?


Fig. P6.109

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft.s. For cast iron, $\varepsilon \approx 0.00085 \mathrm{ft}$. The $2^{\prime \prime}, 6^{\prime \prime}$, and $3^{\prime \prime}$ pipes have, respectively,
(a) $\mathrm{L} / \mathrm{d}=750, \varepsilon / d=0.0051$;
(b) $L / d=150, \varepsilon / d=0.0017$;
(c) $\mathrm{L} / \mathrm{d}=600, \varepsilon / d=0.0034$

The flow rate is known, so each velocity, Reynolds number, and $f$ can be calculated:

$$
\mathrm{V}_{\mathrm{a}}=\frac{0.16}{\pi(2 / 12)^{2} / 4}=7.33 \frac{\mathrm{ft}}{\mathrm{~s}} ; \quad \operatorname{Re}_{\mathrm{a}}=\frac{1.94(7.33)(2 / 12)}{2.09 \mathrm{E}-5}=113500, \quad \mathrm{f}_{\mathrm{a}} \approx 0.0314
$$

Also, $\mathrm{V}_{\mathrm{b}}=0.82 \mathrm{ft} / \mathrm{s}, \mathrm{Re}_{\mathrm{b}}=37800, \mathrm{f}_{\mathrm{c}} \approx 0.0266 ; \mathrm{V}_{\mathrm{c}}=3.26, \mathrm{Re}_{\mathrm{c}}=75600, \mathrm{f}_{\mathrm{c}} \approx 0.0287$
Finally, the minor loss coefficients may be tabulated:
sharp $2^{\prime \prime}$ entrance: $K=0.5$; three $2^{\prime \prime} 90^{\circ}$ elbows: $K=3(0.95)$
$2^{\prime \prime}$ sudden expansion: $\mathrm{K} \approx 0.79 ; \quad 3^{\prime \prime}$ open globe valve: $\mathrm{K} \approx 6.3$
The turbine head equals the elevation difference minus losses and the exit velocity head:

$$
\begin{aligned}
\mathrm{h}_{\mathrm{t}}= & \Delta \mathrm{z}-\sum \mathrm{h}_{\mathrm{f}}-\sum \mathrm{h}_{\mathrm{m}}-\mathrm{V}_{\mathrm{c}}^{2} /(2 \mathrm{~g}) \\
= & 100-\frac{(7.33)^{2}}{2(32.2)}[0.0314(750)+0.5+3(0.95)+0.79] \\
& -\frac{(0.82)^{2}}{2(32.2)}(0.0266)(150)-\frac{(3.26)^{2}}{2(32.2)}[0.0287(600)+6.3+1] \approx 72.8 \mathrm{ft}
\end{aligned}
$$

The resulting turbine power $=\rho \mathrm{gQht}=(62.4)(0.16)(72.8) \div 550 \approx \mathbf{1 . 3 2} \mathbf{h p}$. Ans.
6.110 In Fig. P6.110 the pipe entrance is sharp-edged. If the flow rate is $0.004 \mathrm{~m}^{3} / \mathrm{s}$, what power, in W , is extracted by the turbine?


Fig. P6.110 Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$
$998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast
iron, $\varepsilon \approx 0.26 \mathrm{~mm}$, hence $\varepsilon / d=0.26 / 50 \approx 0.0052$. The minor loss coefficients are Entrance: $\mathrm{K} \approx 0.5$; $5-\mathrm{cm}\left(\approx 2^{\prime \prime}\right)$ open globe valve: $\mathrm{K} \approx 6.9$.

The flow rate is known, hence we can compute $\mathrm{V}, \mathrm{Re}$, and f :

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{0.004}{(\pi / 4)(0.05)^{2}}=2.04 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{998(2.04)(0.05)}{0.001} \approx 102000, \quad \mathrm{f} \approx 0.0316
$$

The turbine head equals the elevation difference minus losses and exit velocity head:

$$
\begin{gathered}
\mathrm{h}_{\mathrm{t}}=\Delta \mathrm{z}-\mathrm{h}_{\mathrm{f}}-\sum \mathrm{h}_{\mathrm{m}}-\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}=40-\frac{(2.04)^{2}}{2(9.81)}\left[(0.0316)\left(\frac{125}{0.05}\right)+0.5+6.9+1\right] \approx 21.5 \mathrm{~m} \\
\text { Power }=\rho \mathrm{gQh}_{\mathrm{t}}=(998)(9.81)(0.004)(21.5) \approx \mathbf{8 4 0} \mathbf{~ W} \text { Ans. }
\end{gathered}
$$

6.111 For the parallel-pipe system of Fig. P6.111, each pipe is cast iron, and the pressure drop $p 1-p 2=3 \mathrm{lbf} / \mathrm{in}^{2}$. Compute the total flow rate between 1 and 2 if the fluid is SAE 10 oil at $20^{\circ} \mathrm{C}$.


Fig. P6.111

Solution: For SAE 10 oil at $20^{\circ} \mathrm{C}$, take $\rho=1.69$ slug/ft ${ }^{3}$ and $\mu=0.00217$ slug/ft $\cdot \mathrm{s}$. For cast iron, $\varepsilon \approx 0.00085 \mathrm{ft}$. Convert $\Delta \mathrm{p}=3 \mathrm{psi}=432 \mathrm{psf}$ and guess laminar flow in each:

$$
\begin{gathered}
\Delta \mathrm{p}_{\mathrm{a}} \stackrel{?}{=} \frac{128 \mu \mathrm{~L}_{\mathrm{a}} \mathrm{Q}_{\mathrm{a}}}{\pi \mathrm{~d}_{\mathrm{a}}^{4}}=432=\frac{128(0.00217)(250) \mathrm{Q}_{\mathrm{a}}}{\pi(3 / 12)^{4}}, \\
\mathrm{Q}_{\mathrm{a}} \approx 0.0763 \frac{\mathrm{ft}^{3}}{\mathrm{~s}} \cdot \quad \text { Check Re } \approx 300(\mathrm{OK}) \\
\Delta \mathrm{p}_{\mathrm{b}} \stackrel{?}{=} \frac{128 \mu \mathrm{~L}_{\mathrm{b}} \mathrm{Q}_{\mathrm{b}}}{\pi \mathrm{~d}_{\mathrm{b}}^{4}}=432=\frac{128(0.00217)(200) \mathrm{Q}_{\mathrm{b}}}{\pi(2 / 12)^{4}}, \\
\mathrm{Q}_{\mathrm{b}} \approx 0.0188 \frac{\mathrm{ft}^{3}}{\mathrm{~s}} . \quad \text { Check } \mathrm{Re} \approx 112(\mathrm{OK})
\end{gathered}
$$

The total flow rate is $\mathbf{Q}=\mathrm{Q}_{\mathrm{a}}+\mathrm{Q}_{\mathrm{b}}=0.0763+0.0188 \approx \mathbf{0 . 0 9 5} \mathbf{f t}^{\mathbf{3}} / \mathbf{s}$. Ans.
6.112 If the two pipes in Fig. P6.111 are instead laid in series with the same total pressure drop of 3 psi, what will the flow rate be? The fluid is SAE 10 oil at $20^{\circ} \mathrm{C}$.

Solution: For SAE 10 oil at $20^{\circ} \mathrm{C}$, take $\rho=1.69$ slug/ft ${ }^{3}$ and $\mu=0.00217 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. Again guess laminar flow. Now, instead of $\Delta \mathrm{p}$ being the same, $\mathrm{Qa}=\mathrm{Qb}=\mathrm{Q}$ :

$$
\Delta \mathrm{p}_{\mathrm{a}}+\Delta \mathrm{p}_{\mathrm{b}}=432 \operatorname{psf}=\frac{128 \mu \mathrm{~L}_{\mathrm{a}} \mathrm{Q}}{\pi \mathrm{~d}_{\mathrm{a}}^{4}}+\frac{128 \mu \mathrm{~L}_{\mathrm{b}} \mathrm{Q}}{\pi \mathrm{~d}_{\mathrm{b}}^{4}}=\frac{128(0.00217)}{\pi} \mathrm{Q}\left[\frac{250}{(3 / 12)^{4}}+\frac{200}{(2 / 12)^{4}}\right]
$$

Solve for $\mathbf{Q} \approx \mathbf{0 . 0 1 5 1} \mathbf{f t}^{\mathbf{3}} / \mathbf{s}$ Ans. Check $\mathrm{Re}_{\mathrm{a}} \approx 60(\mathrm{OK})$ and $\mathrm{Re}_{\mathrm{b}} \approx 90(\mathrm{OK})$ In series, the flow rate is six times less than when the pipes are in parallel.
6.113 The parallel galvanized-iron pipe system of Fig. P6.113 delivers water at $20^{\circ} \mathrm{C}$ with a total flow rate of $0.036 \mathrm{~m}^{3} / \mathrm{s}$. If the pump is wide open and not running, with a loss coefficient $K=1.5$, determine (a) the flow rate in each pipe and (b) the overall pressure drop.


Fig. P6.113

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For galvanized iron, $\varepsilon=0.15 \mathrm{~mm}$. Assume turbulent flow, with $\Delta \mathrm{p}$ the same for each leg:

$$
\mathrm{h}_{\mathrm{f} 1}=\mathrm{f}_{1} \frac{\mathrm{~L}_{1}}{\mathrm{~d}_{1}} \frac{\mathrm{~V}_{1}^{2}}{2 \mathrm{~g}}=\mathrm{h}_{\mathrm{f} 2}+\mathrm{h}_{\mathrm{m} 2}=\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}\left(\mathrm{f}_{2} \frac{\mathrm{~L}_{2}}{\mathrm{~d}_{2}}+1.5\right),
$$

$$
\text { and } \mathrm{Q}_{1}+\mathrm{Q}_{2}=(\pi / 4) \mathrm{d}_{1}^{2} \mathrm{~V}_{1}+(\pi / 4) \mathrm{d}_{2}^{2} \mathrm{~V}_{2}=\mathrm{Q}_{\text {total }}=0.036 \mathrm{~m}^{3} / \mathrm{s}
$$

When the friction factors are correctly found from the Moody chart, these two equations may be solved for the two velocities (or flow rates). Begin by guessing $\mathrm{f} \approx 0.020$ :

$$
(0.02)\left(\frac{60}{0.05}\right) \frac{\mathrm{V}_{1}^{2}}{2(9.81)}=\frac{\mathrm{V}_{2}^{2}}{2(9.81)}\left[(0.02)\left(\frac{55}{0.04}\right)+1.5\right], \text { solve for } \mathrm{V}_{1} \approx 1.10 \mathrm{~V}_{2}
$$

then $\frac{\pi}{4}(0.05)^{2}\left(1.10 V_{2}\right)+\frac{\pi}{4}(0.04)^{2} V_{2}=0.036$. Solve $V_{2} \approx 10.54 \frac{\mathrm{~m}}{\mathrm{~s}}, \mathrm{~V}_{1} \approx 11.59 \frac{\mathrm{~m}}{\mathrm{~s}}$
Correct $\mathrm{Re}_{1} \approx 578000, \mathrm{f}_{1} \approx 0.0264, \mathrm{Re}_{2} \approx 421000, \mathrm{f}_{2} \approx 0.0282$, repeat.

The 2nd iteration converges: $\mathrm{f} 1 \approx 0.0264, \mathrm{~V} 1=11.69 \mathrm{~m} / \mathrm{s}, \mathrm{f} 2 \approx 0.0282, \mathrm{~V} 2=10.37 \mathrm{~m} / \mathrm{s}$,

$$
\mathrm{Q}_{1}=\mathrm{A}_{1} \mathrm{~V}_{1}=\mathbf{0 . 0 2 3} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s}, \quad \mathrm{Q}_{2}=\mathrm{A}_{2} \mathrm{~V}_{2}=\mathbf{0 . 0 1 3} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s} . \quad \text { Ans. (a) }
$$

The pressure drop is the same in either leg:

$$
\Delta \mathrm{p}=\mathrm{f}_{1} \frac{\mathrm{~L}_{1}}{\mathrm{~d}_{1}} \frac{\rho \mathrm{~V}_{1}^{2}}{2}=\left(\mathrm{f}_{2} \frac{\mathrm{~L}_{2}}{\mathrm{~d}_{2}}+1.5\right) \frac{\rho \mathrm{V}_{2}^{2}}{2} \approx 2.16 \mathrm{E} 6 \text { Pa Ans. (b) }
$$

*P6.114 A blower supplies standard air to a plenum that feeds two horizontal square sheetmetal ducts with sharp-edged entrances. One duct is 100 ft long, with a cross-section 6 in by 6 in. The second duct is 200 ft long. Each duct exhausts to the atmosphere. When the plenum pressure is $5.0 \mathrm{lbf} / \mathrm{ft}^{2}$ gage, the volume flow in the longer duct is three times the flow in the shorter duct. Estimate both volume flows and the cross-section size of the longer duct.

Solution: For standard air, in BG units, take $\rho=0.00238$ slug $/ \mathrm{ft}^{3}$ and $\mu=3.78 \mathrm{E}-7$ slug/ftsec. For sheet-metal, take $\varepsilon=0.00016 \mathrm{ft}$. The energy equation for this case is

$$
\begin{gathered}
\frac{p_{1}}{\rho g}+\frac{V_{\gamma}^{2}}{2 g}+z_{1}=\frac{p_{2}}{\rho g}+\frac{V_{2}^{2}}{2 g}+z_{2}+h_{f}+h_{\text {entrance }}, \text { or }: \\
\Delta p=\frac{1}{2} \rho V^{2}\left(1+f \frac{L}{D_{h}}+K_{\text {ent }}\right) \quad \text { where } K_{\text {sharp-edged }} \approx 0.5
\end{gathered}
$$

We have abbreviated the duct velocity to $V$, without a subscript. For a square duct, the hydraulic diameter is the side length of the square. First compute the flow rate in the short duct:
$5.0 \frac{\mathrm{lbf}}{f t^{2}}=\frac{0.00238 \text { slug } / f t^{3}}{2}\left\{1+f \frac{100 f t}{0.5 f t}+0.5\right), f=f c n\left(\operatorname{Re}_{D_{h}}, \frac{\varepsilon}{D_{h}}\right)$

The Reynolds number for the short duct is $\quad \operatorname{Re}=(0.00238) \mathrm{V}(0.5) /(3.78 \mathrm{E}-7)=3148 \mathrm{~V}$, and $\varepsilon / D_{\mathrm{h}}=0.00016 \mathrm{ft} / 0.5 \mathrm{ft}=0.00032$. The solution is

$$
L=100 \mathrm{ft}: \operatorname{Re}_{D_{h}}=87,000 ; f=0.0200 ; V=27.7 \mathrm{ft} / \mathrm{s} ; Q_{\text {short }}=\mathbf{6 . 9 2} \mathbf{f t}^{\mathbf{3}} / \mathbf{s}
$$

For the longer duct, $\mathrm{Re}=(0.00238) \mathrm{V} D_{\mathrm{h}} /(3.78 \mathrm{E}-7)$, and $\varepsilon / D_{\mathrm{h}}=0.00016 \mathrm{ft} / D_{\mathrm{h}}$. We don't know $D_{\mathrm{h}}$ and must solve to make $Q_{\text {long }}=3 \mathrm{Q}_{\text {short. }}$. The solution is

$$
\begin{aligned}
L=200 \mathrm{ft}: \operatorname{Re}_{D_{h}}=150,000 ; f & =0.0177 ; V=27.5 \mathrm{ft} / \mathrm{s} ; Q_{\text {long }}=20.75 \mathrm{ft}^{3} / \mathrm{s} \\
& \text { Solve for } \quad D_{h, l o n g}=\mathbf{0 . 8 7} \mathbf{f t}
\end{aligned} \text { Ans. }
$$

NOTE: It is an interesting numerical quirk that, for these duct parameters, the velocities in each duct are almost identical, regardless of the magnitude of the pressure drop.
6.115 In Fig. P6.115 all pipes are 8-cm-diameter cast iron. Determine the flow rate from reservoir (1) if valve C is (a) closed; and (b) open, with Kvalve $=0.5$.


Fig. P6.115
Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast iron, $\varepsilon \approx$ 0.26 mm , hence $\varepsilon / d=0.26 / 80 \approx 0.00325$ for all three pipes. Note $\mathrm{p} 1=\mathrm{p} 2, \mathrm{~V} 1=\mathrm{V} 2 \approx 0$. These are long pipes, but we might wish to account for minor losses anyway:
sharp entrance at $\mathrm{A}: \mathrm{K} 1 \approx 0.5$; line junction from A to $\mathrm{B}: \mathrm{K} 2 \approx 0.9$ (Table 6.5)
branch junction from A to $\mathrm{C}: \mathrm{K} 3 \approx 1.3$; two submerged exits: $\mathrm{KB}=\mathrm{KC} \approx 1.0$
If valve $C$ is closed, we have a straight series path through $A$ and $B$, with the same flow rate Q , velocity V , and friction factor $f$ in each. The energy equation yields

$$
\begin{gathered}
\mathrm{z}_{1}-\mathrm{z}_{2}=\mathrm{h}_{\mathrm{fA}}+\sum \mathrm{h}_{\mathrm{mA}}+\mathrm{h}_{\mathrm{fB}}+\sum \mathrm{h}_{\mathrm{mB}}, \\
\text { or: } 25 \mathrm{~m}=\frac{\mathrm{V}^{2}}{2(9.81)}\left[\mathrm{f} \frac{100}{0.08}+0.5+0.9+\mathrm{f} \frac{50}{0.08}+1.0\right], \text { where } f=\mathrm{fcn}\left(\operatorname{Re}, \frac{\varepsilon}{d}\right)
\end{gathered}
$$

Guess $\mathrm{f} \approx \mathrm{ffully}$ rough $\approx 0.027$, then $\mathrm{V} \approx 3.04 \mathrm{~m} / \mathrm{s}, \mathrm{Re} \approx 998(3.04)(0.08) /(0.001) \approx 243000$, $\varepsilon / d=0.00325$, then $\mathrm{f} \approx 0.0273$ (converged). Then the velocity through A and B is $\mathrm{V}=$ $3.03 \mathrm{~m} / \mathrm{s}$, and $\mathrm{Q}=(\pi / 4)(0.08)^{2}(3.03) \approx \mathbf{0 . 0 1 5 2} \mathbf{~ m}^{\mathbf{3}} / \mathrm{s}$. Ans. (a).
If valve $C$ is open, we have parallel flow through $B$ and $C$, with $Q A=Q B+Q C$ and, with $d$ constant, $\mathrm{VA}=\mathrm{VB}+\mathrm{VC}$. The total head loss is the same for paths $\mathrm{A}-\mathrm{B}$ and $\mathrm{A}-\mathrm{C}$ :

$$
\begin{aligned}
\mathrm{z}_{1}-\mathrm{z}_{2} & =\mathrm{h}_{\mathrm{fA}}+\sum \mathrm{h}_{\mathrm{mA}-\mathrm{B}}+\mathrm{h}_{\mathrm{fB}}+\sum \mathrm{h}_{\mathrm{mB}}=\mathrm{h}_{\mathrm{fA}}+\sum \mathrm{h}_{\mathrm{mA}-\mathrm{C}}+\mathrm{h}_{\mathrm{fC}}+\sum \mathrm{h}_{\mathrm{mC}}, \\
\text { or: } 25 & =\frac{\mathrm{V}_{\mathrm{A}}^{2}}{2(9.81)}\left[\mathrm{f}_{\mathrm{A}} \frac{100}{0.08}+0.5+0.9\right]+\frac{\mathrm{V}_{\mathrm{B}}^{2}}{2(9.81)}\left[\mathrm{f}_{\mathrm{B}} \frac{50}{0.08}+1.0\right] \\
& =\frac{\mathrm{V}_{\mathrm{A}}^{2}}{2(9.81)}\left[\mathrm{f}_{\mathrm{A}} \frac{100}{0.08}+0.5+1.3\right]+\frac{\mathrm{V}_{\mathrm{C}}^{2}}{2(9.81)}\left[\mathrm{f}_{\mathrm{C}} \frac{70}{0.08}+1.0\right]
\end{aligned}
$$

plus the additional relation $\mathrm{VA}=\mathrm{VB}+\mathrm{VC}$. Guess $\mathrm{f} \approx$ ffully rough $\approx 0.027$ for all three pipes and begin. The initial numbers work out to

$$
2 \mathrm{~g}(25)=490.5=\mathrm{V}_{\mathrm{A}}^{2}\left(1250 \mathrm{f}_{\mathrm{A}}+1.4\right)+\mathrm{V}_{\mathrm{B}}^{2}\left(625 \mathrm{f}_{\mathrm{B}}+1\right)=\mathrm{V}_{\mathrm{A}}^{2}\left(1250 \mathrm{f}_{\mathrm{A}}+1.8\right)+\mathrm{V}_{\mathrm{C}}^{2}\left(875 \mathrm{f}_{\mathrm{C}}+1\right)
$$

If $\mathrm{f} \approx 0.027$, solve (laboriously) $\mathrm{V}_{\mathrm{A}} \approx 3.48 \mathrm{~m} / \mathrm{s}, \mathrm{V}_{\mathrm{B}} \approx 1.91 \mathrm{~m} / \mathrm{s}, \mathrm{V}_{\mathrm{C}} \approx 1.57 \mathrm{~m} / \mathrm{s}$.
Compute $\operatorname{Re}_{\mathrm{A}}=278000, \mathrm{f}_{\mathrm{A}} \approx 0.0272, \mathrm{Re}_{\mathrm{B}}=153000, \mathrm{f}_{\mathrm{B}}=0.0276$,

$$
\operatorname{Re}_{\mathrm{C}}=125000, \quad \mathrm{f}_{\mathrm{C}}=0.0278
$$

Repeat once for convergence: $\mathrm{VA} \approx 3.46 \mathrm{~m} / \mathrm{s}, \mathrm{VB} \approx 1.90 \mathrm{~m} / \mathrm{s}, \mathrm{VC} \approx 1.56 \mathrm{~m} / \mathrm{s}$. The flow rate from reservoir $(1)$ is $\mathbf{Q A}=(\pi / 4)(0.08)^{2}(3.46) \approx \mathbf{0 . 0 1 7 4} \mathbf{m}^{3} / \mathrm{s}$. ( $14 \%$ more) Ans. (b)
6.116 For the series-parallel system of Fig. P6.116, all pipes are 8 -cm-diameter asphalted cast iron. If the total pressure drop $p 1-p 2=750 \mathrm{kPa}$, find the resulting flow rate $Q \mathrm{~m}^{3} / \mathrm{h}$ for water at $20^{\circ} \mathrm{C}$. Neglect minor losses.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For
asphalted cast iron, $\varepsilon \approx 0.12 \mathrm{~mm}$, hence $\varepsilon / d=$ $0.12 / 80 \approx 0.0015$ for all three pipes. The head loss is the same through AC and BC :


Fig. P6.116

$$
\frac{\Delta \mathrm{p}}{\rho \mathrm{~g}}=\mathrm{h}_{\mathrm{fA}}+\mathrm{h}_{\mathrm{fC}}=\mathrm{h}_{\mathrm{fB}}+\mathrm{h}_{\mathrm{fC}}=\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}\right)_{\mathrm{A}}+\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}\right)_{\mathrm{C}}=\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}\right)_{\mathrm{B}}+\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}\right)_{\mathrm{C}}
$$

Since $d$ is the same, $\mathrm{VA}+\mathrm{VB}=\mathrm{VC}$ and $\mathrm{fA}, \mathrm{fB}$, fc are found from the Moody chart. Cancel $g$ and introduce the given data:

$$
\frac{750000}{998}=\mathrm{f}_{\mathrm{A}} \frac{250}{0.08} \frac{\mathrm{~V}_{\mathrm{A}}^{2}}{2}+\mathrm{f}_{\mathrm{C}} \frac{150}{0.08} \frac{\mathrm{~V}_{\mathrm{C}}^{2}}{2}=\mathrm{f}_{\mathrm{B}} \frac{100}{0.08} \frac{\mathrm{~V}_{\mathrm{B}}^{2}}{2}+\mathrm{f}_{\mathrm{C}} \frac{150}{0.08} \frac{\mathrm{~V}_{\mathrm{C}}^{2}}{2}, \quad \mathrm{~V}_{\mathrm{A}}+\mathrm{V}_{\mathrm{B}}=\mathrm{V}_{\mathrm{C}}
$$

Guess $\mathrm{f}_{\text {rough }} \approx 0.022$ and solve laboriously: $\mathrm{V}_{\mathrm{A}} \approx 2.09 \frac{\mathrm{~m}}{\mathrm{~s}}, \mathrm{~V}_{\mathrm{B}} \approx 3.31 \frac{\mathrm{~m}}{\mathrm{~s}}, \mathrm{~V}_{\mathrm{C}} \approx 5.40 \frac{\mathrm{~m}}{\mathrm{~s}}$
Now compute $\mathrm{ReA} \approx 167000, \mathrm{fA} \approx 0.0230$, $\mathrm{ReB} \approx 264000$, $\mathrm{fB} \approx 0.0226, \mathrm{ReC} \approx 431000$, and $\mathrm{fC} \approx 0.0222$. Repeat the head loss iteration and we converge: $\mathrm{VA} \approx 2.06 \mathrm{~m} / \mathrm{s}$, $\mathrm{VB} \approx$ $3.29 \mathrm{~m} / \mathrm{s}, \mathrm{VC} \approx 5.35 \mathrm{~m} / \mathrm{s}, \mathrm{Q}=(\pi / 4)(0.08)^{2}(5.35) \approx \mathbf{0 . 0 2 6 9} \mathbf{m}^{3} / \mathbf{s} . \quad$ Ans.
6.117 A blower delivers air at $3000 \mathrm{~m}^{3} / \mathrm{h}$ to the duct circuit in Fig. P6.117. Each duct is commercial steel and of square crosssection, with side lengths $a 1=a 3=20 \mathrm{~cm}$ and $a 2=a 4=12 \mathrm{~cm}$. Assuming sea-level air conditions, estimate the power required if the blower has an efficiency of $75 \%$. Neglect minor losses.

Solution: For air take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Establish conditions in each duct:


Fig. P6.117

$$
\begin{gathered}
Q=\frac{3000}{3600}=0.833 \frac{\mathrm{~m}^{3}}{\mathrm{~s}} ; V_{1 \& 3}=\frac{0.833 \mathrm{~m}^{3} / \mathrm{s}}{(0.2 \mathrm{~m})^{2}}=20.8 \mathrm{~m} / \mathrm{s} ; \quad \operatorname{Re}_{1 \& 3}=\frac{1.2(20.8)(0.2)}{1.8 E-5}=278,000 \\
V_{2 \& 4}=\frac{0.833 \mathrm{~m}^{3} / \mathrm{s}}{(0.12 \mathrm{~m})^{2}}=57.8 \mathrm{~m} / \mathrm{s} ; \quad \operatorname{Re}_{2 \& 4}=\frac{1.2(57.8)(0.12)}{1.8 E-5}=463,000
\end{gathered}
$$

For commercial steel (Table 6.1) $\varepsilon=0.046 \mathrm{~mm}$. Then we can find the two friction factors:

$$
\left.\frac{\varepsilon}{D}\right|_{1 \& 3}=\frac{0.046}{200}=0.00023 ; \quad \operatorname{Re}_{1 \& 3}=278000 ; \quad \text { Moody chart: } f_{1 \& 3} \approx 0.0166
$$

$$
\begin{aligned}
\left.\frac{\varepsilon}{D}\right|_{2 \& 4}= & \frac{0.046}{120}=0.000383 ; \quad \operatorname{Re}_{2 \& 4}=463000 ; \quad \text { Moody chart: } f_{1 \& 3} \approx 0.0170 \\
\text { Then } \quad \Delta p_{1 \& 3} & =\left(f \frac{L}{D} \frac{\rho V^{2}}{2}\right)_{1 \& 3}=(0.0166)\left(\frac{80}{0.2}\right) \frac{(1.2)(20.8)^{2}}{2}=1730 \mathrm{~Pa} \\
\text { and } \quad \Delta p_{2 \& 4} & =\left(f \frac{L}{D} \frac{\rho V^{2}}{2}\right)_{1 \& 3}=(0.0170)\left(\frac{60}{0.12}\right) \frac{(1.2)(57.8)^{2}}{2}=17050 \mathrm{~Pa}
\end{aligned}
$$

The total power required, at 75\% efficiency, is thus:

$$
\text { Power }=\frac{Q \Delta p}{\eta}=\frac{\left(0.833 \mathrm{~m}^{3} / \mathrm{s}\right)(1730+17050 \mathrm{~Pa})}{0.75}=20900 \mathrm{~W} \text { Ans. }
$$

6.118 For the piping system of Fig. P6.118, all pipes are concrete with a roughness of 0.04 inch. Neglecting minor losses, compute the overall pressure drop p1 - p2 in $\mathrm{lbf} / \mathrm{in}^{2}$. The flow rate is $20 \mathrm{ft}^{3} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.94 slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ $\mathrm{ft} \cdot \mathrm{s}$. Since the pipes are all different make a little table of their respective $L / d$ and $\varepsilon / d$ :


Fig. P6.118

| (a) | $\mathrm{L}=1000 \mathrm{ft}$, | $\mathrm{d}=12 \mathrm{in}$, | $\mathrm{L} / \mathrm{d}=1000$, | $\varepsilon / d=\mathbf{0 . 0 0 3 3 3}$ |
| :--- | ---: | ---: | ---: | ---: |
| (b) | 1500 ft | 8 in | 2250 | $\mathbf{0 . 0 0 5 0 0}$ |
| (c) | 800 ft | 12 in | 800 | $\mathbf{0 . 0 0 3 3 3}$ |
| (d) | 1200 ft | 15 in | 960 | $\mathbf{0 . 0 0 2 6 7}$ |

With the flow rate known, we can find everything in pipe (a):

$$
\mathrm{V}_{\mathrm{a}}=\frac{\mathrm{Q}_{\mathrm{a}}}{\mathrm{~A}_{\mathrm{a}}}=\frac{20}{(\pi / 4)(1 \mathrm{ft})^{2}}=25.5 \frac{\mathrm{ft}}{\mathrm{~s}}, \quad \operatorname{Re}_{\mathrm{a}}=\frac{1.94(25.5)(1)}{2.09 \mathrm{E}-5}=2.36 \mathrm{E} 6, \quad \mathrm{f}_{\mathrm{a}} \approx 0.0270
$$

Then pipes (b,c,d) are in parallel, each having the same head loss and with flow rates which must add up to the total of $20 \mathrm{ft}^{3} / \mathrm{s}$ :

$$
\mathrm{h}_{\mathrm{fb}}=\frac{8 \mathrm{f}_{\mathrm{b}} \mathrm{~L}_{\mathrm{b}} \mathrm{Q}_{\mathrm{b}}^{2}}{\pi^{2} \mathrm{gd}_{\mathrm{b}}^{5}}=\mathrm{h}_{\mathrm{fc}}=\frac{8 \mathrm{f}_{\mathrm{c}} \mathrm{~L}_{\mathrm{c}} \mathrm{Q}_{\mathrm{c}}^{2}}{\pi^{2} \mathrm{gd}_{\mathrm{c}}^{5}}=\mathrm{h}_{\mathrm{fd}}=\frac{8 \mathrm{f}_{\mathrm{d}} \mathrm{~L}_{\mathrm{d}} \mathrm{Q}_{\mathrm{d}}^{2}}{\pi^{2} \mathrm{gd}_{\mathrm{d}}^{5}}, \quad \text { and } \quad \mathrm{Q}_{\mathrm{b}}+\mathrm{Q}_{\mathrm{c}}+\mathrm{Q}_{\mathrm{d}}=20 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}
$$

Introduce $\mathrm{Lb}, \mathrm{db}$, etc. to find that $\mathrm{Qc}=3.77 \mathrm{Qb}(\mathrm{fb} / \mathrm{fc})^{1 / 2}$ and $\mathrm{Qd}=5.38 \mathrm{Qb}(\mathrm{fb} / \mathrm{fd})^{1 / 2}$ Then the flow rates are iterated from the relation

$$
\sum \mathrm{Q}=20 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}=\mathrm{Q}_{\mathrm{b}}\left[1+3.77\left(\mathrm{f}_{\mathrm{b}} / \mathrm{f}_{\mathrm{c}}\right)^{1 / 2}+5.38\left(\mathrm{f}_{\mathrm{b}} / \mathrm{f}_{\mathrm{d}}\right)^{1 / 2}\right]
$$

First guess: $f_{b}=f_{c}=f_{d}: \quad Q_{b} \approx 1.97 \mathrm{ft}^{3} / \mathrm{s} ; \quad \mathrm{Q}_{\mathrm{c}} \approx 7.43 \mathrm{ft}^{3} / \mathrm{s} ; \quad \mathrm{Q}_{\mathrm{d}} \approx 10.6 \mathrm{ft}^{3} / \mathrm{s}$ Improve by computing $\mathrm{Reb} \approx 349000$, $\mathrm{fb} \approx 0.0306$, Rec $\approx 878000$, $\mathrm{fc}_{c} \approx 0.0271$, Red $\approx$ $1002000, \mathrm{fd} \approx 0.0255$. Repeat to find $\mathrm{Qb} \approx 1.835 \mathrm{ft}^{3} / \mathrm{s}, \mathrm{Qc} \approx 7.351 \mathrm{ft}^{3} / \mathrm{s}, \mathrm{Qd} \approx 10.814 \mathrm{ft}^{3} / \mathrm{s}$. Repeat once more and quit: $\mathrm{Qb} \approx 1.833 \mathrm{ft}^{3} / \mathrm{s}$, $\mathrm{Qc} \approx 7.349 \mathrm{ft}^{3} / \mathrm{s}$, $\mathrm{Qd} \approx 10.819 \mathrm{ft}^{3} / \mathrm{s}$, from which $\mathrm{Vb} \approx 5.25 \mathrm{ft} / \mathrm{s}, \mathrm{Vc}_{\mathrm{c}} \approx 9.36 \mathrm{ft} / \mathrm{s}, \mathrm{Vd} \approx 8.82 \mathrm{ft} / \mathrm{s}$. The pressure drop is

$$
\begin{aligned}
\mathrm{p}_{1}-\mathrm{p}_{2} & =\Delta \mathrm{p}_{\mathrm{a}}+\Delta \mathrm{p}_{\mathrm{b}}=\mathrm{f}_{\mathrm{a}} \frac{\mathrm{~L}_{\mathrm{a}}}{\mathrm{~d}_{\mathrm{a}}} \frac{\rho \mathrm{~V}_{\mathrm{a}}^{2}}{2}+\mathrm{f}_{\mathrm{b}} \frac{\mathrm{~L}_{\mathrm{b}}}{\mathrm{~d}_{\mathrm{b}}} \frac{\rho \mathrm{~V}_{\mathrm{b}}^{2}}{2} \\
& =17000+1800 \approx 18800 \text { psf } \approx \mathbf{1 3 1} \frac{\mathbf{l b f}}{\mathbf{i n}^{2}} \text { Ans. }
\end{aligned}
$$

6.119 Modify Prob. 6.118 as follows. Let the pressure drop ( $\mathrm{p} 1-\mathrm{p} 2$ ) be $98 \mathrm{lbf} / \mathrm{in}^{2}$. Neglecting minor losses, determine the flow rate in $\mathrm{ft}^{3} / \mathrm{s}$.

Solution: From the solution just above for $\Delta \mathrm{p} \approx 131 \mathrm{psi}$, we can see that $\Delta \mathrm{pa}$ is about $90.2 \%$ of the total drop. Therefore our first guess can be that

$$
\begin{array}{r}
\Delta \mathrm{p}_{\mathrm{a}} \approx 0.902 \Delta \mathrm{p}=0.902(98 \times 144) \approx 12729 \mathrm{psf}=\mathrm{f}_{\mathrm{a}} \frac{\mathrm{~L}_{\mathrm{a}}}{\mathrm{~d}_{\mathrm{a}}} \frac{\rho \mathrm{~V}_{\mathrm{a}}^{2}}{2} \approx(0.027)(1000) \frac{1.94 \mathrm{~V}_{\mathrm{a}}^{2}}{2} \\
\text { Solve for } \mathrm{V}_{\mathrm{a}} \approx 22.05 \frac{\mathrm{ft}}{\mathrm{~s}} \\
\text { and } \mathrm{Q}_{\mathrm{a}}=\mathrm{A}_{\mathrm{a}} \mathrm{~V}_{\mathrm{a}} \approx 17.3 \frac{\mathrm{ft}^{3}}{\mathrm{~s}} \stackrel{?}{=} \mathrm{Q}_{\mathrm{b}}\left[1+3.77\left(\mathrm{f}_{\mathrm{b}} / \mathrm{f}_{\mathrm{c}}\right)^{1 / 2}+5.38\left(\mathrm{f}_{\mathrm{b}} / \mathrm{f}_{\mathrm{d}}\right)^{1 / 2}\right]
\end{array}
$$

The last relation is still valid just as it was in Prob. 6.188. We can iterate to find

$$
\begin{gathered}
\mathrm{f}_{\mathrm{a}} \approx 0.0270, \quad \mathrm{Q}_{\mathrm{b}} \approx 1.585 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}, \quad \mathrm{Q}_{\mathrm{c}} \approx 6.357 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}, \quad \mathrm{Q}_{\mathrm{d}} \approx 9.358 \frac{\mathrm{ft}^{2}}{\mathrm{~s}}, \quad \mathrm{f}_{\mathrm{b}} \approx 0.0307 \\
\mathrm{p}_{1}-\mathrm{p}_{2}=\mathrm{f}_{\mathrm{a}}\left(\mathrm{~L}_{\mathrm{a}} / \mathrm{d}_{\mathrm{a}}\right)\left(\rho \mathrm{V}_{\mathrm{a}}^{2} / 2\right)+\mathrm{f}_{\mathrm{b}}\left(\mathrm{~L}_{\mathrm{b}} / \mathrm{d}_{\mathrm{b}}\right)\left(\rho \mathrm{V}_{\mathrm{b}}^{2} / 2\right) \approx 12717+1382=14099 \mathrm{psf} \approx 97.9 \mathrm{psi}
\end{gathered}
$$

This is certainly close enough. We conclude the flow rate is $\mathbf{Q} \approx \mathbf{1 7 . 3} \mathbf{f t}^{\mathbf{3}} / \mathbf{s}$. Ans.
6.120 Three cast-iron pipes are laid in parallel with these dimensions:

Pipe 1: $\quad L 1=800 \mathrm{~m} \quad d 1=12 \mathrm{~cm}$
Pipe 2: $\quad L 2=600 \mathrm{~m} \quad d 2=8 \mathrm{~cm}$
Pipe 3: $\quad L 3=900 \mathrm{~m} \quad d 3=10 \mathrm{~cm}$
The total flow rate is $200 \mathrm{~m}^{3} / \mathrm{h}$ of water at $20^{\circ} \mathrm{C}$. Determine (a) the flow rate in each pipe; and (b) the pressure drop across the system.
Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For cast iron, $\varepsilon=$ 0.26 mm . Then, $\varepsilon / d_{1}=0.00217, \varepsilon / d 2=0.00325$, and $\varepsilon / d 3=0.0026$. The head losses are the same for each pipe, and the flow rates add:

$$
h_{f}=\frac{8 f_{1} L_{1} Q_{1}^{2}}{\pi^{2} g d_{1}^{5}}=\frac{8 f_{2} L_{2} Q_{2}^{2}}{\pi^{2} g d_{2}^{5}}=\frac{8 f_{3} L_{3} Q_{3}^{2}}{\pi^{2} g d_{3}^{5}} ; \text { and } Q_{1}+Q_{2}+Q_{3}=\frac{200}{3600} \frac{m^{3}}{\mathrm{~s}}
$$

Substitute and combine: $Q_{1}\left[1+0.418\left(f_{1} / f_{2}\right)^{1 / 2}+0.599\left(f_{1} / f_{3}\right)^{1 / 2}\right]=0.0556 \mathrm{~m}^{3} / \mathrm{s}$
We could either go directly to EES or begin by guessing $f 1=f 2=f 3$, which gives $Q 1=$ $0.0275 \mathrm{~m}^{3} / \mathrm{s}, Q 2=0.0115 \mathrm{~m}^{3} / \mathrm{s}$, and $Q 3=0.0165 \mathrm{~m}^{3} / \mathrm{s}$. This is very close! Further iteration gives

$$
\begin{aligned}
& \mathrm{Re}_{1}=298000, f_{1}=0.0245 ; \mathrm{Re}_{2}=177000, f_{2}=0.0275 ; \mathrm{Re}_{3}=208000, f_{3}=0.0259 \\
& Q_{1}=\mathbf{0 . 0 2 8 1} \mathbf{~ m}^{3} / \mathbf{s}, \quad Q_{2}=\mathbf{0 . 0 1 1 1} \mathbf{m}^{3} / \mathbf{s}, \quad \text { and } Q_{3}=\mathbf{0 . 0 1 6 3} \mathbf{~ m}^{3} / \mathbf{s} \quad \text { Ans. (a) } \\
& h_{\mathrm{f}}=51.4 \mathrm{~m}, \quad \Delta p=\rho g h_{\mathrm{f}}=\left(998 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)(51.4 \mathrm{~m})=\mathbf{5 0 3 , 0 0 0} \mathbf{~ P a} \text { Ans. (b) }
\end{aligned}
$$

6.121 Consider the three-reservoir system of Fig. P6.121 with the following data:

$$
\begin{array}{lll}
L 1=95 \mathrm{~m} & L 2=125 \mathrm{~m} & L 3=160 \mathrm{~m} \\
\mathrm{z} 1=25 \mathrm{~m} & \mathrm{z2}=115 \mathrm{~m} & \mathrm{z} 3=85 \mathrm{~m}
\end{array}
$$

All pipes are $28-\mathrm{cm}$-diameter unfinished concrete ( $\varepsilon=1 \mathrm{~mm}$ ). Compute the steady flow rate in all pipes for water at $20^{\circ} \mathrm{C}$.


Fig. P6.121

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. All pipes have $\varepsilon / d=1 / 280=0.00357$. Let the intersection be "a." The head loss at "a" is desired:

$$
\mathrm{z}_{1}-\mathrm{h}_{\mathrm{a}}=\mathrm{f}_{1} \frac{\mathrm{~L}_{1}}{\mathrm{~d}_{1}} \frac{\mathrm{~V}_{1}^{2}}{2 \mathrm{~g}} ; \quad \mathrm{z}_{2}-\mathrm{h}_{\mathrm{a}}=\mathrm{f}_{2} \frac{\mathrm{~L}_{2}}{\mathrm{~d}_{2}} \frac{\mathrm{~V}_{2}^{2}}{2 \mathrm{~g}} ; \quad \mathrm{z}_{3}-\mathrm{h}_{\mathrm{a}}=\mathrm{f}_{3} \frac{\mathrm{~L}_{3}}{\mathrm{~d}_{3}} \frac{\mathrm{~V}_{3}^{2}}{2 \mathrm{~g}}
$$

plus the requirement that $\mathrm{Q} 1+\mathrm{Q} 2+\mathrm{Q} 3=0 \quad$ or, for same $d, \mathrm{~V} 1+\mathrm{V} 2+\mathrm{V} 3=0$
We guess ha then iterate each friction factor to find V and Q and then check if $\Sigma \mathrm{Q}=0$.

$$
\begin{gathered}
\mathrm{h}_{\mathrm{a}}=75 \mathrm{~m}: \quad 25-75=(-) 50=\mathrm{f}_{1}\left(\frac{95}{0.28}\right) \frac{\mathrm{V}_{1}^{2}}{2(9.81)}, \text { solve } \mathrm{f}_{1} \approx 0.02754, \mathrm{~V}_{1} \approx-10.25 \frac{\mathrm{~m}}{\mathrm{~s}} \\
\text { Similarly, } 115-75=\mathrm{f}_{2}(125 / 0.28)\left[\mathrm{V}_{2}^{2} / 2(9.81)\right] \text { gives } \mathrm{f}_{2} \approx 0.02755 . \mathrm{V}_{2} \approx+7.99 \\
\text { and } 85-75=\mathrm{f}_{3}(160 / 0.28)\left[\mathrm{V}_{3}^{2} / 2(9.81)\right] \\
\text { gives } \quad \mathrm{f}_{3} \approx 0.02762, \mathrm{~V}_{3} \approx+3.53 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \sum \mathrm{~V}=+\mathbf{1 . 2 7}
\end{gathered}
$$

Repeating for $\mathrm{ha}_{\mathrm{a}}=80 \mathrm{~m}$ gives $\mathrm{V}_{1}=-10.75, \mathrm{~V}_{2}=+7.47, \mathrm{~V} 3=+2.49 \mathrm{~m} / \mathrm{s}, \sum \mathrm{V}=-0.79$. Interpolate to $\mathrm{ha} \approx 78 \mathrm{~m}$, gives $\mathrm{V} 1=-10.55 \mathrm{~m} / \mathrm{s}, \mathrm{V} 2=+7.68 \mathrm{~m} / \mathrm{s}, \mathrm{V} 3=+2.95 \mathrm{~m} / \mathrm{s}$, or:

$$
\mathrm{Q} 1=-0.65 \mathrm{~m}^{3} / \mathrm{s}, \quad \mathrm{Q} 2=+0.47 \mathrm{~m}^{3} / \mathrm{s}, \quad \mathrm{Q} 3=+0.18 \mathrm{~m}^{3} / \mathrm{s} . \quad \text { Ans. }
$$

6.122 Modify Prob. 6.121 by reducing the diameter to 15 cm , with $\varepsilon=1 \mathrm{~mm}$. Compute the flow rate in each pipe. They all reduce, compared to Prob. 6.121, by a factor of about 5.2. Can you explain this?

Solution: The roughness ratio increases to $\varepsilon / d=1 / 150=0.00667$, and all L/d's increase. Guess ha $=75 \mathrm{~m}$ : converges to $\mathrm{f} 1=0.0333, \mathrm{f} 2=0.0333, \mathrm{f} 4=0.0334$

$$
\text { and } \quad \mathrm{V} 1 \approx-6.82 \mathrm{~m} / \mathrm{s}, \quad \mathrm{~V} 2 \approx+5.32 \mathrm{~m} / \mathrm{s}, \quad \mathrm{~V} 3 \approx+2.34 \mathrm{~m} / \mathrm{s}, \quad \sum \mathrm{~V} \approx+\mathbf{0 . 8 5}
$$

We finally obtain $\quad h_{a} \approx 78.2 \mathrm{~m}$, giving $\mathrm{V} 1=-7.04 \mathrm{~m} / \mathrm{s}, \quad \mathrm{V} 2=+5.10 \mathrm{~m} / \mathrm{s}, \quad \mathrm{V} 3=+1.94$ $\mathrm{m} / \mathrm{s}$,
or: $\quad \mathrm{Q} 1=-\mathbf{0 . 1 2 4} \mathrm{m}^{\mathbf{3}} / \mathrm{s}, \quad \mathrm{Q} 2=+\mathbf{0 . 0 9 0} \mathrm{m}^{3} / \mathrm{s}, \quad \mathrm{Q} 3=+\mathbf{0 . 0 3 4} \mathrm{m}^{3} / \mathrm{s} . \quad$ Ans.
6.123 Modify Prob. 6.121 on the previous page as follows. Let $z 3$ be unknown and find its value such that the flow rate in pipe 3 is $0.2 \mathrm{~m}^{3} / \mathrm{s}$ toward the junction. (This problem is best suited for computer iteration.)

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. All pipes have $\varepsilon / d=1 / 280=0.00357$. Let the intersection be "a." The head loss at "a" is desired for each in order to check the flow rate in pipe 3.
In Prob. 6.121, with $\mathrm{z} 3=85 \mathrm{~m}$, we found Q3 to be $0.18 \mathrm{~m}^{3} / \mathrm{s}$ toward the junction, pretty close. We repeat the procedure with a few new values of z 3 , closing to $\sum \mathrm{Q}=0$ each time:

Guess $\mathrm{z}_{3}=85 \mathrm{~m}: \quad \mathrm{h}_{\mathrm{a}}=78.19 \mathrm{~m}, \quad \mathrm{Q}_{1}=-0.6508, \quad \mathrm{Q}_{2}=+0.4718, \quad \mathrm{Q}_{3}=+0.1790 \mathrm{~m}^{3} / \mathrm{s}$
$90 \mathrm{~m}: \quad 80.65 \mathrm{~m}, \quad-0.6657, \quad+0.6657, \quad+0.2099 \mathrm{~m}^{3} / \mathrm{s}$
Interpolate: $\quad h_{a} \approx 79.89, Q_{1} \approx-0.6611$,

$$
\mathrm{Q}_{2} \approx+0.4608, \quad \mathrm{Q}_{3} \approx+\mathbf{0 . 2 0 0} \mathrm{m}^{\mathbf{3}} / \mathrm{s}, \quad \mathrm{z}_{\mathbf{3}} \approx \mathbf{8 8 . 4} \mathbf{~ m} \quad \text { Ans. }
$$

6.124 The three-reservoir system in Fig. P6.124 delivers water at $20^{\circ} \mathrm{C}$. The system data are as follows:

$$
\begin{array}{lll}
D 1=8 \text { in } & D 2=6 \text { in } & D 3=9 \text { in } \\
L 1=1800 & L 2=1200 \mathrm{ft} & L 3=1600 \mathrm{ft} \\
\mathrm{ft} & &
\end{array}
$$

All pipes are galvanized iron. Compute the flow rate in all pipes.


Fig. P6.124

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft•s. For galvanized iron, take $\varepsilon=0.0005 \mathrm{ft}$. Then the roughness ratios are

$$
\varepsilon / d_{1}=0.00075 \quad \varepsilon / d_{2}=0.0010 \quad \varepsilon / d_{3}=0.00667
$$

Let the intersection be "a." The head loss at " a " is desired:

$$
\mathrm{z}_{1}-\mathrm{h}_{\mathrm{a}}=\frac{\mathrm{f}_{1} \mathrm{~L}_{1}}{\mathrm{~d}_{1}} \frac{\mathrm{~V}_{1}^{2}}{2 \mathrm{~g}} ; \quad \mathrm{z}_{2}-\mathrm{h}_{\mathrm{a}}=\frac{\mathrm{f}_{2} \mathrm{~L}_{2}}{\mathrm{~d}_{2}} \frac{\mathrm{~V}_{2}^{2}}{2 \mathrm{~g}} ; \quad \mathrm{z}_{3}-\mathrm{h}_{\mathrm{a}}=\frac{\mathrm{f}_{3} \mathrm{~L}_{3}}{\mathrm{~d}_{3}} \frac{\mathrm{~V}_{3}^{2}}{2 \mathrm{~g}} ; \quad \text { plus } \mathrm{Q}_{1}+\mathrm{Q}_{2}+\mathrm{Q}_{3}=0
$$

We guess ha then iterate each friction factor to find V and Q and then check if $\Sigma \mathrm{Q}=0$.

$$
\begin{gathered}
\text { Guess } \quad h_{\mathrm{a}}=50 \mathrm{ft}: \quad 20-50=(-) 30 \mathrm{ft}=\frac{\mathrm{f}_{1}(1800) \mathrm{V}_{1}^{2}}{(8 / 12) 2(32.2)} \text {, } \\
\\
\\
\text { solve } \mathrm{f}_{1}=0.0194, \quad \mathrm{~V}_{1}=-6.09 \frac{\mathrm{ft}}{\mathrm{~s}}
\end{gathered}
$$

Similarly, $\mathrm{f}_{2}=0.0204, \mathrm{~V}_{2} \approx+8.11 \mathrm{ft} / \mathrm{s}$ and of course $\mathrm{V}_{3}=0$. Get $\sum \mathrm{Q}=-0.54 \mathrm{ft}^{3} / \mathrm{s}$
Try again with a slightly lower ha to reduce Q1 and increase Q2 and Q3:

$$
\begin{gathered}
\mathrm{h}_{\mathrm{a}}=48 \mathrm{ft}: \text { converges to } \mathrm{Q}_{1}=-2.05 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}, \quad \mathrm{Q}_{2}=+1.62 \frac{\mathrm{ft}^{3}}{\mathrm{~s}} \\
\mathrm{Q}_{3}=+0.76 \frac{\mathrm{ft}^{3}}{\mathrm{~s}}, \quad \sum \mathrm{Q}=+0.33
\end{gathered}
$$

Interpolate to

$$
\mathrm{h}_{\mathrm{a}}=49.12 \mathrm{ft}: \quad \mathrm{Q}_{1}=-\mathbf{2} .09 \mathbf{f t}^{\mathbf{3}} / \mathrm{s}, \quad \mathrm{Q}_{2}=+\mathbf{1 . 6 1} \mathbf{f t}^{\mathbf{3}} / \mathrm{s}, \quad \mathbf{Q}_{3}=+\mathbf{0 . 4 9} \mathbf{f t}^{3} / \mathrm{s} \quad \text { Ans. }
$$

6.125 Suppose that the three cast-iron pipes in Prob. 6.120 are instead connected to meet smoothly at a point B, as shown in Fig. P6.125. The inlet pressures in each pipe are: p1 = 200 kPa ; p2 $=160 \mathrm{kPa}$; p3 = 100 kPa . The fluid is water at $20^{\circ} \mathrm{C}$. Neglect minor losses. Estimate the flow rate in each pipe and whether it is toward or away from point B.

Solution: For water take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The pressure at point

(3)

Fig. P6.125 B must be a known (constant) value which makes the net flow rate equal to zero at junction $B$. The flow clearly goes from (1) to $B$, and from B to (3), but we are not sure about pipe (2). For cast iron (Table 6.1), $\varepsilon=0.26 \mathrm{~mm}$. Each pipe has a flow rate based upon its pressure drop:

$$
p_{1}-p_{B}=f_{1} \frac{L_{1}}{D_{1}} \frac{\rho V_{1}^{2}}{2} ; \quad\left|p_{2}-p_{B}\right|=f_{2} \frac{L_{2}}{D_{2}} \frac{\rho V_{2}^{2}}{2} ; \quad p_{B}-p_{3}=f_{3} \frac{L_{3}}{D_{3}} \frac{\rho V_{3}^{2}}{2}
$$

where the $f$ 's are determined from the Moody chart for each pipe's $\varepsilon / D$ and ReD. The correct value of $p \mathrm{~B}$ makes the flow rates $Q_{\mathrm{i}}=(\pi / 4) D_{\mathrm{i}}{ }^{2} V_{\mathrm{i}}$ balance at junction B . EES is excellent for this type of iteration, and the final results balance for $p \mathrm{~B}=166.7 \mathrm{kPa}$ :

$$
\begin{aligned}
& f_{1}=0.0260 ; \quad \mathrm{Re}_{1}=74300 ; \quad \varepsilon / D_{1}=0.00217 ; \quad \boldsymbol{Q}_{\mathbf{1}}=+\mathbf{0 . 0 0 7 0 1} \mathbf{~ m}^{\mathbf{3}} / \mathrm{s} \text { (toward B) } \\
& f_{2}=0.0321 ; \operatorname{Re}_{2}=18900 ; \varepsilon / D_{2}=0.00325 ; \boldsymbol{Q}_{\mathbf{2}}=\mathbf{- 0 . 0 0 1 1 9} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s} \text { (away from B) Ans. } \\
& f_{3}=0.0270 ; \quad \operatorname{Re}_{3}=74000 ; \varepsilon / D_{3}=0.00260 ; \quad \boldsymbol{Q}_{3}=\mathbf{- 0 . 0 0 5 8 2} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s} \text { (away from B) }
\end{aligned}
$$

6.126 Modify Prob. 6.124 as follows. Let all data be the same except that pipe 1 is fitted with a butterfly valve (Fig. 6.19b). Estimate the proper valve opening angle (in degrees) for the flow rate through pipe 1 to be reduced to $1.5 \mathrm{ft}^{3} / \mathrm{s}$ toward reservoir 1 . (This problem requires iteration and is best suited to a digital computer.)

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ft ${ }^{3}$ and $\mu=2.09 \mathrm{E}-4$ slug/ft•s. For galvanized iron, take $\varepsilon=0.0005 \mathrm{ft}$. Then the roughness ratios are

$$
\varepsilon / d_{1}=0.00075 \quad \varepsilon / d_{2}=0.0010 \quad \varepsilon / d_{3}=0.00667
$$

For a butterfly valve loss coefficient "K" (to be found). Let the junction be "J." The head loss at " J " is desired and then to be iterated to give the proper flow rate in pipe (1):

$$
\begin{aligned}
& \mathrm{z}_{1}-\mathrm{h}_{\mathrm{J}}=\frac{\mathrm{V}_{1}^{2}}{2 \mathrm{~g}}\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}+\mathrm{K}\right)_{1} ; \quad \mathrm{z}_{2}-\mathrm{h}_{\mathrm{J}}=\frac{\mathrm{V}_{2}^{2}}{2 \mathrm{~g}}\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}\right)_{2} ; \\
& \mathrm{z}_{3}-\mathrm{h}_{\mathrm{J}}=\frac{\mathrm{V}_{3}^{2}}{2 \mathrm{~g}}\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}\right)_{3} ; \quad \text { and } \quad \mathrm{Q}_{1}+\mathrm{Q}_{2}+\mathrm{Q}_{3}=0
\end{aligned}
$$

We know z1 $=20 \mathrm{ft}, \mathrm{z} 2=100 \mathrm{ft}$, and $\mathrm{z} 3=50 \mathrm{ft}$. From Prob. 6.124, where $\mathbf{K}=\mathbf{0}$, the flow rate was $2.09 \mathrm{ft}^{3} / \mathrm{s}$ toward reservoir 1. Now guess a finite value of K and repeat:

$$
\begin{array}{ll}
\mathrm{K}=40: & \text { converges to } \mathrm{h}_{\mathrm{J}}=50.0, \quad \mathrm{Q}_{1}=-1.59 \mathrm{ft}^{3} / \mathrm{s}, \quad \mathrm{Q}_{2}=+1.59 \mathrm{ft}^{3} / \mathrm{s} ; \quad \mathrm{Q}_{3} \approx 0 \\
\mathrm{~K}=50: & \text { converges to } \mathrm{h}_{\mathrm{J}}=50.03 \mathrm{ft}, \quad \mathrm{Q}_{1}=-1.513 \quad \mathrm{Q}_{2}=+1.591 \quad \mathrm{Q}_{3}=-0.078 \\
\mathrm{~K}=\mathbf{5 2 :} & \text { gives } \mathrm{h}_{\mathrm{J}}=50.04 \mathrm{ft}, \quad \mathbf{Q}_{\mathbf{1}}=-\mathbf{1 . 5 0 0} \mathbf{f t}^{3} / \mathbf{s} \quad \mathrm{Q}_{2}=1.591 \quad \mathrm{Q}_{3}=-0.091 \quad \text { Ans. }
\end{array}
$$

From Fig. 6.19b, a butterfly valve coefficient $\mathrm{K} \approx 52$ occurs at $\theta$ opening $\approx 3^{\circ}$. Ans.
6.127 In the five-pipe horizontal network of Fig. P6.127, assume that all pipes have a friction factor $f=0.025$. For the given inlet and exit flow rate of $2 \mathrm{ft}^{3} / \mathrm{s}$ of water at $20^{\circ} \mathrm{C}$, determine the flow rate and direction in all pipes. If $p A=120 \mathrm{lbf} / \mathrm{in}^{2}$ gage, determine the pressures at points $B, C$, and $D$.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.94 slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ $\mathrm{ft} \cdot \mathrm{s}$. Each pipe has a head loss which is known


Fig. P6.127 except for the square of the flow rate:

Pipe AC: $\quad h_{f}=\left.\frac{8 f L Q^{2}}{\pi^{2} g^{5}}\right|_{A C}=\frac{8(0.025)(3000) \mathrm{Q}_{\mathrm{AC}}^{2}}{\pi^{2}(32.2)(6 / 12)^{5}}=\mathrm{K}_{\mathrm{AC}} \mathrm{Q}_{\mathrm{AC}}^{2}$, where $\mathrm{K}_{\mathrm{AC}} \approx 60.42$
Similarly, $\quad K_{A B}=19.12, \quad K_{B C}=13.26, \quad K_{C D}=19.12, \quad K_{B D}=1933 . \quad\left(Q\right.$ in $\left.\frac{\mathrm{ft}^{3}}{\mathrm{~s}}\right)$
There are two triangular closed loops, and the total head loss must be zero for each. Using the flow directions assumed on the figure P6.127 above, we have

Loop A-B-C: $19.12 \mathrm{Q}_{\mathrm{AB}}^{2}+13.26 \mathrm{Q}_{\mathrm{BC}}^{2}-60.42 \mathrm{Q}_{\mathrm{AC}}^{2}=0$
Loop B-C-D: $13.26 \mathrm{Q}_{\mathrm{BC}}^{2}+19.12 \mathrm{Q}_{\mathrm{CD}}^{2}-1933.0 \mathrm{Q}_{\mathrm{BD}}^{2}=0$

And there are three independent junctions which have zero net flow rate:

$$
\text { Junction } \mathrm{A}: \quad \mathrm{Q}_{\mathrm{AB}}+\mathrm{Q}_{\mathrm{AC}}=2.0 ; \quad \mathrm{B}: \mathrm{Q}_{\mathrm{AB}}=\mathrm{Q}_{\mathrm{BC}}+\mathrm{Q}_{\mathrm{BD}} ; \quad \mathrm{C}: \mathrm{Q}_{\mathrm{AC}}+\mathrm{Q}_{\mathrm{BC}}=\mathrm{Q}_{\mathrm{CD}}
$$

These are five algebraic equations to be solved for the five flow rates. The answers are:

$$
\mathrm{Q}_{\mathrm{AB}}=\mathbf{1 . 1 9}, \quad \mathrm{Q}_{\mathrm{AC}}=\mathbf{0 . 8 1}, \quad \mathrm{Q}_{\mathrm{BC}}=\mathbf{0 . 9 9}, \quad \mathrm{Q}_{\mathrm{CD}}=\mathbf{1 . 8 0}, \quad \mathrm{Q}_{\mathrm{BD}}=\mathbf{0 . 2 0} \frac{\mathbf{f t}^{3}}{\mathbf{s}} \quad \text { Ans. (a) }
$$

The pressures follow by starting at $\mathrm{A}(120 \mathrm{psi})$ and subtracting off the friction losses:

$$
\begin{aligned}
& \mathrm{p}_{\mathrm{B}}=\mathrm{p}_{\mathrm{A}}-\rho \mathrm{gK}_{\mathrm{AB}} \mathrm{Q}_{\mathrm{AB}}^{2}=120 \times 144-62.4(19.12)(1.19)^{2} \\
& \mathrm{p}_{\mathrm{B}}=15590 \mathrm{psf} \div 144=\mathbf{1 0 8} \frac{\mathbf{l b f}}{\mathbf{i n}^{2}} \\
& \text { Similarly, } \quad \mathrm{p}_{\mathrm{C}} \approx \mathbf{1 0 3} \mathbf{~ p s i} \text { and } \mathrm{p}_{\mathrm{D}} \approx \mathbf{7 6} \mathbf{~ p s i} \text { Ans. (b) }
\end{aligned}
$$

6.128 Modify Prob. 6.127 above as follows: Let the inlet flow at A and the exit flow at D be unknown. Let $\mathrm{pA}-\mathrm{pB}=100 \mathrm{psi}$. Compute the flow rate in all five pipes.

Solution: Our head loss coefficients "K" from above are all the same. Head loss AB is known, plus we have two "loop" equations and two "junction" equations:

$$
\frac{\mathrm{p}_{\mathrm{A}}-\mathrm{p}_{\mathrm{B}}}{\rho \mathrm{~g}}=\frac{100 \times 144}{62.4}=231 \mathrm{ft}=\mathrm{K}_{\mathrm{AB}} \mathrm{Q}_{\mathrm{AB}}^{2}=19.12 \mathrm{Q}_{\mathrm{AB}}^{2}, \quad \text { or } \quad \mathrm{Q}_{\mathrm{AB}}=3.47 \mathrm{ft}^{3} / \mathbf{s}
$$

Two loops: $\quad 231+13.26 \mathrm{Q}_{\mathrm{BC}}^{2}-60.42 \mathrm{Q}_{\mathrm{AC}}^{2}=0$

$$
13.26 \mathrm{Q}_{\mathrm{BC}}^{2}+19.12 \mathrm{Q}_{\mathrm{CD}}^{2}-1933.0 \mathrm{Q}_{\mathrm{BD}}^{2}=0
$$

Two junctions: $\quad \mathrm{QAB}=3.47=\mathrm{QBC}+\mathrm{QBD} ; \quad \mathrm{QAC}+\mathrm{QBC}=\mathrm{QCD}$
The solutions are in exactly the same ratio as the lower flow rates in Prob. 6.127:

$$
\begin{aligned}
& \mathrm{Q}_{\mathrm{AB}}=3.47 \frac{\mathbf{f t}^{3}}{\mathbf{s}}, \quad \mathrm{Q}_{\mathrm{BC}}=2.90 \frac{\mathbf{f t}^{3}}{\mathbf{s}}, \quad \mathrm{Q}_{\mathrm{BD}}=\mathbf{0 . 5 8} \frac{\mathbf{f t}^{3}}{\mathbf{s}}, \\
& \mathrm{Q}_{\mathrm{CD}}=5.28 \frac{\mathbf{f t}^{3}}{\mathbf{s}}, \quad \mathrm{Q}_{\mathrm{AC}}=\mathbf{2 . 3 8} \frac{\mathbf{f t}^{3}}{\mathbf{s}} \quad \text { Ans. }
\end{aligned}
$$

6.129 In Fig. P6.129 all four horizontal cast-iron pipes are 45 m long and 8 cm in diameter and meet at junction $a$, delivering water at $20^{\circ} \mathrm{C}$. The pressures are known at four points as shown:

$$
\begin{array}{ll}
p 1=950 \mathrm{kPa} & p 2=350 \mathrm{kPa} \\
p 3=675 \mathrm{kPa} & p 4=100 \mathrm{kPa}
\end{array}
$$

Neglecting minor losses, determine the flow rate in each pipe.


Fig. P6.129

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. All pipes are cast iron, with $\varepsilon / d=0.26 / 80=0.00325$. All pipes have $L / d=45 / 0.08=562.5$. One solution method is to guess the junction pressure pa, iterate to calculate the friction factors and flow rates, and check to see if the net junction flow is zero:

Guess $\mathrm{p}_{\mathrm{a}}=500 \mathrm{kPa}: \quad \mathrm{h}_{\mathrm{fl}}=\frac{950000-500000}{998(9.81)}=45.96 \mathrm{~m}=\frac{8 \mathrm{f}_{1} \mathrm{~L}_{1} \mathrm{Q}_{1}^{2}}{\pi^{2} \mathrm{gd}_{1}^{5}}=1.135 \mathrm{E}_{\mathrm{f}} \mathrm{f}_{1} \mathrm{Q}_{1}^{2}$
then guess $\mathrm{f}_{1} \approx 0.02, \quad \mathrm{Q}_{1}=0.045 \mathrm{~m}^{3} / \mathrm{s}, \quad \mathrm{Re}_{1}=4 \rho \mathrm{Q}_{1} /\left(\pi \mu \mathrm{d}_{1}\right)=715000, \quad \mathrm{f}_{1 \text {-new }} \approx 0.0269$ converges to $\mathrm{f}_{1} \approx 0.0270, \quad \mathrm{Q}_{1} \approx 0.0388 \mathrm{~m}^{3} / \mathrm{s}$
Iterate also to $\mathrm{Q}_{2}=-0.0223 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}$ (away from $a$ ), $\mathrm{Q}_{3}=0.0241, \quad \mathrm{Q}_{4}=-0.0365$

$$
\sum \mathrm{Q}=+0.00403, \quad \text { so we have guessed } \mathrm{p}_{\mathrm{a}} \text { a little low. }
$$

Trying $\mathrm{pa}=530 \mathrm{kPa}$ gives $\sum \mathrm{Q}=-0.00296$, hence iterate to $\mathrm{pa} \approx 517 \mathrm{kPa}$ :

$$
\begin{aligned}
& \left.\mathrm{Q}_{1}=+\mathbf{0 . 0 3 8 0} \frac{\mathrm{m}^{3}}{\mathbf{s}} \text { (toward } \mathrm{a}\right), \quad \mathrm{Q}_{2}=-\mathbf{0 . 0 2 3 6} \frac{\mathrm{m}^{3}}{\mathrm{~s}} \\
& \mathrm{Q}_{3}=+\mathbf{0 . 0 2 2 9} \frac{\mathrm{m}^{3}}{\mathbf{s}}, \quad \mathrm{Q}_{4}=-\mathbf{0 . 0 3 7 3} \frac{\mathbf{m}^{3}}{\mathbf{s}} \text { Ans. }
\end{aligned}
$$

6.130 In Fig. P6.130 lengths $A B$ and $B D$ are 2000 and 1500 ft , respectively. The friction factor is 0.022 everywhere, and $p_{A}$ $=90 \mathrm{lbf} / \mathrm{in}^{2}$ gage. All pipes have a diameter of 6 in . For water at $20^{\circ} \mathrm{C}$, determine the flow rate in all pipes and the pressures at points $B, C$, and $D$.


Fig. P6.130

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=1.94$ slug/ $\mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. Each pipe has a head loss which is known except for the square of the flow rate:

Pipe AC: $\quad h_{f}=\frac{8 f L Q^{2}}{\pi^{2} \mathrm{gd}^{5}}=\frac{8(0.022)(1500) \mathrm{Q}_{\mathrm{AC}}^{2}}{\pi^{2}(32.2)(6 / 12)^{5}}=\mathrm{K}_{\mathrm{AC}} \mathrm{Q}_{\mathrm{AC}}^{2}$, where $\mathrm{K}_{\mathrm{AC}} \approx \mathbf{2 6 . 5 8}$
Similarly, $\mathrm{KAB}=\mathrm{KCD}=35.44, \mathrm{KBD}=26.58$, and $\mathrm{KBC}=44.30$.
The solution is similar to Prob. 6.127, except that (1) the K's are different; and (2) junctions B and C have additional flow leaving the network. The basic flow relations are:

$$
\begin{gathered}
\text { Loop ABC: } \quad 35.44 \mathrm{Q}_{\mathrm{AB}}^{2}+44.3 \mathrm{Q}_{\mathrm{BC}}^{2}-26.58 \mathrm{Q}_{\mathrm{AC}}^{2}=0 \\
\text { Loop BCD: } \quad 44.3 \mathrm{Q}_{\mathrm{BC}}^{2}+35.44 \mathrm{Q}_{\mathrm{CD}}^{2}-26.58 \mathrm{Q}_{\mathrm{BD}}^{2}=0 \\
\text { Junctions A,B,C: } \quad \mathrm{Q}_{\mathrm{AB}}+\mathrm{Q}_{\mathrm{AC}}=2.0 ; \\
\mathrm{Q}_{\mathrm{AB}}=\mathrm{Q}_{\mathrm{BC}}+\mathrm{Q}_{\mathrm{BD}}+1.0 ; \quad \mathrm{Q}_{\mathrm{AC}}+\mathrm{Q}_{\mathrm{BC}}=\mathrm{Q}_{\mathrm{CD}}+0.5
\end{gathered}
$$

In this era of PC "equation solvers" such as MathCAD, etc., it is probably not necessary to dwell upon any solution methods. For hand work, one might guess QAB, then the other four are obtained in sequence from the above relations, plus a check on the original guess for QAB. The assumed arrows are shown above. It turns out that we have guessed the direction incorrectly on QBC above, but the others are OK. The final results are:

$$
\begin{gathered}
\left.\mathrm{Q}_{\mathrm{AB}}=\mathbf{0 . 9 4 9} \mathbf{f t}^{3} / \mathbf{s}(\text { toward } B) ; \quad \mathrm{Q}_{\mathrm{AC}}=\mathbf{1 . 0 5 1} \mathbf{f t}^{\mathbf{3}} / \mathbf{s} \text { (toward } \mathrm{C}\right) \\
\left.\mathrm{Q}_{\mathrm{BC}}=\mathbf{0 . 2 3 9} \text { (toward B); } \mathrm{Q}_{\mathrm{CD}}=\mathbf{0 . 3 1 2} \text { (toward } \mathrm{D}\right) ; \quad \mathrm{Q}_{\mathrm{BD}}=\mathbf{0 . 1 8 8} \text { (to D) Ans. (a) }
\end{gathered}
$$

The pressures start at A, from which we subtract the friction losses in each pipe:

$$
\mathrm{p}_{\mathrm{B}}=\mathrm{p}_{\mathrm{A}}-\rho \mathrm{gK}_{\mathrm{AB}} \mathrm{Q}_{\mathrm{AB}}^{2}=90 \times 144-62.4(35.44)(0.949)^{2}=10969 \mathrm{psf} \div 144=\mathbf{7 6} \mathbf{~ p s i}
$$

Similarly, we obtain $\quad \mathrm{p}_{\mathrm{C}}=11127 \mathrm{psf}=77 \mathbf{~ p s i} ; \quad \mathrm{p}_{\mathrm{D}}=10911 \mathrm{psf} \approx 76 \mathbf{~ p s i} \quad$ Ans. (b)
6.131 A water-tunnel test section has a 1-m diameter and flow properties $V=20 \mathrm{~m} / \mathrm{s}, p=$ 100 kPa , and $T=20^{\circ} \mathrm{C}$. The boundary-layer blockage at the end of the section is 9 percent. If a conical diffuser is to be added at the end of the section to achieve maximum pressure recovery, what should its angle, length, exit diameter, and exit pressure be?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The Reynolds number is very high, $\operatorname{Re}=\rho \mathrm{Vd} / \mu=(998)(20)(1) /(0.001) \approx 2.0 \mathrm{E} 7$; much higher than the diffuser data in Fig. 6.28b ( $\mathrm{Re} \approx 1.2 \mathrm{E} 5$ ). But what can we do (?) Let's use it anyway:

$$
B_{t}=0.09, \quad \text { read } C_{p, \max } \approx 0.71 \text { at } L / d \approx 25,2 \theta \approx 4^{\circ}, \mathrm{AR} \approx 8:
$$

$$
\begin{aligned}
& \text { Then } \quad \theta_{\text {cone }} \approx 2^{\circ}, \quad \mathrm{L} \approx 25 \mathrm{~d} \approx 25 \mathrm{~m}, \quad \mathrm{D}_{\text {exit }}=\mathrm{d}(8)^{1 / 2} \approx 2.8 \mathrm{~m} \quad \text { Ans. (a) } \\
& \mathrm{C}_{\mathrm{p}} \approx 0.71=\frac{\mathrm{p}_{\mathrm{e}}-\mathrm{p}_{\mathrm{t}}}{(1 / 2) \rho \mathrm{V}_{\mathrm{t}}^{2}}=\frac{\mathrm{p}_{\mathrm{e}}-100000}{(1 / 2)(998)(20)^{2}}, \quad \text { or: } \quad \mathrm{p}_{\text {exit }} \approx 242000 \text { Pa Ans. (b) }
\end{aligned}
$$

6.132 For Prob. 6.131, suppose we are limited by space to a total diffuser length of 10 meters. What should be the diffuser angle, exit diameter, and exit pressure for maximum recovery?

Solution: We are limited to $\mathrm{L} / \mathrm{D}=10.0$. From Fig. 6.28b, read $\mathrm{C}, \max \approx 0.62$ at $\mathrm{AR} \approx 4$ and $2 \theta \approx 6^{\circ}$. Ans. The exit diameter and pressure are

$$
\begin{gathered}
\mathrm{D}_{\mathrm{e}}=\mathrm{d} \sqrt{ }(\mathrm{AR})=(1.0)(4.0)^{1 / 2} \approx 2.0 \mathrm{~m} \quad \text { Ans. } \\
\left.\mathrm{C}_{\mathrm{p}, \max } \approx 0.62=\left(\mathrm{p}_{\mathrm{e}}-100000\right) /(1 / 2)(998)(20)^{2}\right], \quad \text { or: } \quad \mathrm{p}_{\text {exit }} \approx 224000 \mathrm{~Pa} \text { Ans. }
\end{gathered}
$$

6.133 A wind-tunnel test section is 3 ft square with flow properties $V=150 \mathrm{ft} / \mathrm{s}, p=15$ $\mathrm{lbf} / \mathrm{in}^{2}$ absolute, and $T=68^{\circ} \mathrm{F}$. Boundary-layer blockage at the end of the test section is 8 percent. Find the angle, length, exit height, and exit pressure of a flat-walled diffuser added onto the section to achieve maximum pressure recovery.

Solution: For air at $20^{\circ} \mathrm{C}$ and 15 psi , take $\rho=0.00238$ slug/ft ${ }^{3}$ and $\mu=3.76 \mathrm{E}-7 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. The Reynolds number is rather high, $\mathrm{Re}=\rho \mathrm{Vd} / \mu=(0.00238)(150)(3) /(3.76 \mathrm{E}-7) \approx 2.9 \mathrm{E} 6$; much higher than the diffuser data in Fig. 6.28a ( $\mathrm{Re} \approx 2.8 \mathrm{E} 5$ ). But what can we do (?) Let's use it anyway:

$$
\mathrm{B}_{\mathrm{t}}=0.08, \quad \text { read } \mathrm{C}_{\mathrm{p}, \max } \approx 0.70 \quad \text { at } \mathrm{L} / \mathrm{W}_{1} \approx 17,2 \theta \approx 9.5^{\circ}, \mathrm{AR} \approx 3.75:
$$

Then $\quad \theta_{\text {best }} \approx \mathbf{4 . 7 5 ^ { \circ }}, \quad \mathrm{L} \approx 17 \mathrm{~W}_{1} \approx \mathbf{5 1} \mathbf{f t}, \quad \mathrm{~W}_{2} \approx(\mathrm{AR}) \mathrm{W}_{1}=3.75(3) \approx \mathbf{1 1} \mathbf{f t} \quad$ Ans.

$$
\mathrm{C}_{\mathrm{p}} \approx 0.70=\frac{\mathrm{p}_{\mathrm{e}}-\mathrm{p}_{\mathrm{t}}}{(1 / 2) \rho \mathrm{V}_{1}^{2}}=\frac{\mathrm{p}_{\mathrm{e}}-15 \times 144}{(1 / 2)(0.00238)(150)^{2}}, \quad \text { or: } \quad \mathrm{p}_{\mathrm{exit}} \approx 2180 \frac{\mathbf{l b f}}{\mathbf{f t}^{2}} \text { Ans. }
$$

6.134 For Prob. 6.133 above, suppose we are limited by space to a total diffuser length of 30 ft . What should the diffuser angle, exit height, and exit pressure be for maximum recovery?

Solution: We are limited to $\mathrm{L} / \mathrm{W} 1=10.0$. From Fig. 6.28a, read $\mathrm{Cp}, \max \approx 0.645$ at $\mathrm{AR} \approx$ 2.8 and $2 \theta \approx 10^{\circ}$. Ans. The exit height and pressure are

$$
\begin{gathered}
\mathrm{W}_{\mathrm{l}, \mathrm{e}}=(\mathrm{AR}) \mathrm{W}_{1}=(2.8)(3.0) \approx \mathbf{8 . 4} \mathbf{f t} \quad \text { Ans. } \\
\mathrm{C}_{\mathrm{p}, \max } \approx 0.645=\left[\mathrm{p}_{\mathrm{e}}-(15) 144\right) \leftrightharpoons\left[(1 / 2)(0.00238)(150)^{2}\right], \quad \text { or } \quad \mathrm{p}_{\mathrm{e}}=\mathbf{2 1 8 0} \frac{\mathbf{l b f}}{\mathbf{f t}^{2}} \text { Ans. }
\end{gathered}
$$

6.135 An airplane uses a pitot-static tube as a velocimeter. The measurements, with their uncertainties, are a static temperature of $(-11 \pm 3)^{\circ} \mathrm{C}$, a static pressure of $60 \pm 2 \mathrm{kPa}$, and a pressure difference $\left(p_{o}-p_{s}\right)=3200 \pm 60 \mathrm{~Pa}$. (a) Estimate the airplane's velocity and its uncertainty. (b) Is a compressibility correction needed?

Solution: The air density is $\rho=p /(R T)=(60000 \mathrm{~Pa}) /\left[\left(287 \mathrm{~m}^{2} / \mathrm{s}^{2} \cdot \mathrm{~K}\right)(262 \mathrm{~K})\right]=0.798 \mathrm{~kg} / \mathrm{m}^{3}$.
(a) Estimate the velocity from the incompressible Pitot formula, Eq. (6.97):

$$
V=\sqrt{\frac{2 \Delta p}{\rho}}=\sqrt{\frac{2 \Delta p}{p /(R T)}}=\sqrt{\frac{2(3200 P a)}{0.798 \mathrm{~kg} / \mathrm{m}^{3}}}=90 \frac{\mathrm{~m}}{\mathrm{~s}}
$$

The overall uncertainty involves pressure difference, absolute pressure, and absolute temperature:

$$
\frac{\delta V}{V}=\left[\left(\frac{1}{2} \frac{\delta \Delta p}{\Delta p}\right)^{2}+\left(\frac{1}{2} \frac{\delta p}{p}\right)^{2}+\left(\frac{1}{2} \frac{\delta T}{T}\right)^{2}\right]^{1 / 2}=\frac{1}{2}\left[\left(\frac{60}{3200}\right)^{2}+\left(\frac{2}{60}\right)^{2}+\left(\frac{3}{262}\right)^{2}\right]^{1 / 2}=0.020
$$

The uncertainty in velocity is $2 \%$, therefore our final estimate is $V \approx \mathbf{9 0} \pm \mathbf{2 m} \mathbf{~ m}$ Ans. (a) Check the Mach number. The speed of sound is $a=(k R T)^{1 / 2}=[1.4(287)(262)]^{1 / 2}=324 \mathrm{~m} / \mathrm{s}$. Therefore
$M a=V / a=90 / 324=0.28<0.3$. No compressibility correction is needed. Ans. (b)
6.136 For the pitot-static pressure arrangement of Fig. P6.136, the manometer fluid is (colored) water at $20^{\circ} \mathrm{C}$. Estimate (a) the centerline velocity, (b) the pipe volume flow, and (c) the (smooth) wall shear stress.


Fig. P6.136

Solution: For air at $20^{\circ} \mathrm{C}$ and 1 atm , take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The manometer reads

$$
\begin{gathered}
\mathrm{p}_{\mathrm{o}}-\mathrm{p}=\left(\rho_{\text {water }}-\rho_{\text {air }}\right) \mathrm{gh}=(998-1.2)(9.81)(0.040) \approx 391 \mathrm{~Pa} \\
\text { Therefore } \quad \mathrm{V}_{\mathrm{CL}}=[2 \Delta \mathrm{p} / \rho]^{1 / 2}=[2(391) / 1.2]^{1 / 2} \approx \mathbf{2 5 . 5} \mathbf{~ m} / \mathbf{s} \quad \text { Ans. (a) }
\end{gathered}
$$

We can estimate the friction factor and then compute average velocity from Eq. (6.43):
Guess $\quad \mathrm{V}_{\mathrm{avg}} \approx 0.85 \mathrm{~V}_{\mathrm{CL}} \approx 21.7 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad$ then $\mathrm{Re}_{\mathrm{d}}=\frac{\rho \mathrm{Vd}}{\mu}=\frac{1.2(21.7)(0.08)}{1.8 \mathrm{E}-5} \approx 115,700$
Then $\quad \mathrm{f}_{\text {smooth }} \approx 0.0175, \quad \mathrm{~V}_{\text {better }}=\frac{25.5}{[1+1.33 \sqrt{ } 0.0175]} \approx 21.69 \frac{\mathrm{~m}}{\mathrm{~s}}$ (converged)
Thus the volume flow is $\mathrm{Q}=(\pi / 4)(0.08)^{2}(21.69) \approx \mathbf{0 . 1 0 9} \mathbf{m}^{\mathbf{3}} / \mathbf{s}$. Ans. (b)
Finally, $\quad \tau_{\mathrm{w}}=\frac{\mathrm{f}}{8} \rho \mathrm{~V}^{2}=\frac{0.0175}{8}(1.2)(21.69)^{2} \approx 1.23 \mathbf{P a}$ Ans. (c)
6.137 For the $20^{\circ} \mathrm{C}$ water flow of Fig. P6.137, use the pitot-static arrangement to estimate (a) the centerline velocity and (b) the volume flow in the 5 -indiameter smooth pipe. (c) What error in flow rate is caused by neglecting the $1-\mathrm{ft}$ elevation difference?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.94 slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug $/ \mathrm{ft}$.s. For the manometer reading $\mathrm{h}=2$ inches,

$$
\begin{gathered}
\mathrm{P}_{\mathrm{oB}}-\mathrm{p}_{\mathrm{A}}=\left(\mathrm{SG}_{\text {merc }}-1\right)(\rho \mathrm{g})_{\text {water }} \mathrm{h} \\
+\rho_{\text {water }} \mathrm{g}(1 \mathrm{ft}) \quad \text { but from the energy equation, } \\
\mathrm{p}_{\mathrm{A}}-\mathrm{p}_{\mathrm{B}}=\rho_{\text {water }} \mathrm{gh}_{\mathrm{f}-\mathrm{AB}}-\rho_{\text {water }} \mathrm{g}(1 \mathrm{ft}) \quad \text { Therefore } \mathrm{p}_{\mathrm{oB}}-\mathrm{p}_{\mathrm{B}}=(\mathrm{SG}-1) \rho \mathrm{gh}_{\text {mano }}+\rho \mathrm{gh}_{\mathrm{f}-\mathrm{AB}} \\
\text { where friction loss } \mathrm{h}_{\mathrm{f}-\mathrm{AB}} \approx \mathrm{f}(\Delta \mathrm{~L} / \mathrm{d})\left(\mathrm{V}^{2} / 2 \mathrm{~g}\right)
\end{gathered}
$$

Thus the pitot tube reading equals the manometer reading (of about 130 psf ) plus the friction loss between A and B (which is only about 3 psf ), so there is only a small error:

$$
(\mathrm{SG}-1) \rho \mathrm{gh}=(13.56-1)(62.4)(2 / 12) \approx 130.6 \mathrm{psf}, \quad \mathrm{~V}_{\mathrm{CL}} \approx\left[\frac{2 \Delta \mathrm{p}}{\rho}\right]^{1 / 2}=\left[\frac{2(130.6)}{1.94}\right]^{1 / 2}
$$

$$
\begin{gathered}
\text { or } \mathrm{V}_{\mathrm{CL}} \approx 11.6 \frac{\mathrm{ft}}{\mathrm{~s}}, \quad \text { so } \mathrm{V}_{\mathrm{avg}} \approx 0.85 \mathrm{~V}_{\mathrm{CL}} \approx 9.9 \frac{\mathrm{ft}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{1.94(9.9)(5 / 12)}{2.09 \mathrm{E}-5} \approx 381500, \\
\text { so } \mathrm{f}_{\text {smooth }} \approx 0.0138, \text { or } \Delta \mathrm{p}_{\text {friction }}=\mathrm{f}(\mathrm{~L} / \mathrm{d}) \rho \mathrm{V}^{2} / 2 \approx 3.2 \mathrm{lbf} / \mathrm{ft}^{2}
\end{gathered}
$$

If we now correct the pitot tube reading to $\Delta \mathrm{p}_{\text {pitot }} \approx 130.6+3.2=133.8 \mathrm{psf}$, we may iterate and converge rapidly to the final estimate:

$$
\mathrm{f} \approx 0.01375, \mathrm{~V}_{\mathrm{CL}} \approx 11.75 \frac{\mathbf{f t}}{\mathbf{s}} ; \quad \mathrm{Q} \approx 1.39 \frac{\mathbf{f t}^{3}}{\mathbf{s}} ; \quad \mathrm{V}_{\mathrm{avg}} \approx 10.17 \frac{\mathbf{f t}}{\mathbf{s}} \quad \text { Ans. }(\mathrm{a}, \mathrm{~b})
$$

The error compared to our earlier estimate $\mathrm{V} \approx 9.91 \mathrm{ft} / \mathrm{s}$ is about $\mathbf{2 . 6 \%}$ Ans. (c)
6.138 An engineer who took college fluid mechanics on a pass-fail basis has placed the static pressure hole far upstream of the stagnation probe, as in Fig. P6.138, thus contaminating the pitot measurement ridiculously with pipe friction losses. If the pipe flow is air at $20^{\circ} \mathrm{C}$ and 1 atm and the manometer fluid is Meriam red oil ( $\mathrm{SG}=$ 0.827 ), estimate the air centerline velocity for the given manometer reading of 16 cm .


Fig. P6.138 Assume a smooth-walled tube.

Solution: For air at $20^{\circ} \mathrm{C}$ and 1 atm , take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Because of the high friction loss over 10 meters of length, the manometer actually shows poB less than pA, which is a bit weird but correct:

$$
\mathrm{p}_{\mathrm{A}}-\mathrm{p}_{\mathrm{oB}}=\left(\rho_{\text {mano }}-\rho_{\mathrm{air}}\right) \mathrm{gh}=[0.827(998)-1.2](9.81)(0.16) \approx 1294 \mathrm{~Pa}
$$

Meanwhile, $\mathrm{p}_{\mathrm{A}}-\mathrm{p}_{\mathrm{B}}=\rho \mathrm{gh}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{L}}{\mathrm{d}} \frac{\rho \mathrm{V}^{2}}{2}, \quad$ or $\quad \mathrm{P}_{\mathrm{oB}}-\mathrm{p}_{\mathrm{B}}=\frac{\mathrm{fL}}{\mathrm{d}} \frac{\rho \mathrm{V}^{2}}{2}-1294=\frac{\rho}{2} \mathrm{~V}_{\mathrm{CL}}^{2}$
Guess $\quad \mathrm{f} \approx 0.02, \quad \mathrm{~V} \approx 0.85 \mathrm{~V}_{\mathrm{CL}}, \quad$ whence $0.02\left(\frac{10}{0.06}\right)\left(\frac{1.2}{2}\right) \mathrm{V}^{2}-1294=\frac{1.2}{2}\left(\frac{\mathrm{~V}}{0.85}\right)^{2}$
Solve for $\mathrm{V} \approx 33.3 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \operatorname{Re}_{\mathrm{d}}=\frac{1.2(33.3)(0.06)}{1.8 \mathrm{E}-5} \approx 133000, \quad \mathrm{f}_{\text {better }} \approx 0.0170$,

$$
\mathrm{V} \approx \mathrm{~V}_{\mathrm{CL}}[1+1.33 \sqrt{ } \mathrm{f}] \approx 0.852 \mathrm{~V}_{\mathrm{CL}}, \quad \text { repeat to convergence }
$$

Finally converges, $\mathrm{f} \approx 0.0164, \quad \mathrm{~V} \approx 39.87 \mathrm{~m} / \mathrm{s}, \quad \mathrm{V}_{\mathrm{CL}}=\mathrm{V} / 0.8546 \approx 46.65 \mathrm{~m} / \mathrm{s}$. Ans.
6.139 Professor Walter Tunnel must measure velocity in a water tunnel. Due to budgetary restrictions, he cannot afford a pitot-static tube, so he inserts a total-head probe and a static-head probe, as shown, both in the mainstream away from the wall boundary layers. The two probes are connected to a manometer. (a) Write an expression for tunnel velocity V in terms of the parameters in the figure. (b) Is it critical that h1 be measured accurately? (c) How does part (a) differ from a pitot-static tube formula?


Fig. P6.139

Solution: Write Bernoulli from total-head inlet (1) to static-head inlet (2):

$$
p_{o}+\rho_{w} \mathrm{~g} z_{1}=p_{s}+\frac{\rho_{w}}{2} V^{2}+\rho_{w} g z_{2}, \text { Solve } V=\sqrt{\frac{2\left(p_{o}-p_{s}+\rho_{w} g h_{1}\right)}{\rho_{w}}}
$$

Combine this with hydrostatics through the manometer:

$$
\begin{gathered}
p_{s}+\rho_{w} g h_{2}+\rho_{m} g h_{3}=p_{o}+\rho_{w} g h_{1}+\rho_{w} g h_{2}+\rho_{w} g h_{3} \text {, cancel out } \rho_{w} g h_{2} \\
\text { or: } \quad p_{o}-p_{s}+\rho_{w} g h_{1}=\left(\rho_{m}-\rho_{w}\right) g h_{3}
\end{gathered}
$$

Introduce this into the expression for $V$ above, for the final result:

$$
V_{\text {tunnel }}=\sqrt{\frac{2\left(\rho_{m}-\rho_{\mathrm{w}}\right) g h_{3}}{\rho_{w}}} \quad \text { Ans. (a) }
$$

This is exactly the same as a pitot-static tube-h1 is not important. Ans. (b, c)
6.140 Kerosene at $20^{\circ} \mathrm{C}$ flows at $18 \mathrm{~m}^{3} / \mathrm{h}$ in a 5 -cm-diameter pipe. If a 2-cm-diameter thin-plate orifice with corner taps is installed, what will the measured pressure drop be, in Pa ?

Solution: For kerosene at $20^{\circ} \mathrm{C}$, take $\rho=804 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.92 \mathrm{E}-3 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The orifice beta ratio is $\beta=2 / 5=0.4$. The pipe velocity and Reynolds number are:

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{18 / 3600}{(\pi / 4)(0.05)^{2}}=2.55 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}=\frac{804(2.55)(0.05)}{1.92 \mathrm{E}-3}=53300
$$

From Eqs. (6.112) and (6.113a) [corner taps], estimate $\mathrm{Cd} \approx 0.6030$. Then the orifice
pressure-drop formula predicts

$$
\mathrm{Q}=\frac{18}{3600}=0.6030 \frac{\pi}{4}(0.02)^{2} \sqrt{\frac{2 \Delta \mathrm{p}}{804\left[1-(0.4)^{4}\right]}}, \quad \text { solve } \Delta \mathrm{p} \approx 273 \mathbf{k P a} \text { Ans. }
$$

6.141 Gasoline at $20^{\circ} \mathrm{C}$ flows at $105 \mathrm{~m}^{3} / \mathrm{h}$ in a $10-\mathrm{cm}$-diameter pipe. We wish to meter the flow with a thin-plate orifice and a differential pressure transducer which reads best at about 55 kPa . What is the proper $\beta$ ratio for the orifice?

Solution: For gasoline at $20^{\circ} \mathrm{C}$, take $\rho=680 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=2.92 \mathrm{E}-4 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. This problem is similar to Example 6.21 in the text, but we don't have to be so precise because we don't know the exact geometry: corner taps, $\mathrm{D}: \frac{1}{2} \mathrm{D}$ taps, etc. The pipe velocity is

$$
\mathrm{V}_{1}=\frac{\mathrm{Q}}{\mathrm{~A}_{1}}=\frac{105 / 3600}{(\pi / 4)(0.1)^{2}}=3.71 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \operatorname{Re}_{\mathrm{D}}=\frac{680(3.71)(0.1)}{2.92 \mathrm{E}-4} \approx 865000
$$

From Fig. 6.41, which is reasonable for all orifice geometries, read $\mathrm{Cd} \approx 0.61$. Then

$$
\begin{aligned}
\mathrm{V}_{\text {throat }}=\frac{3.71 \mathrm{~m} / \mathrm{s}}{\beta^{2}} & =\mathrm{C}_{\mathrm{d}} \sqrt{\frac{2(55000)}{680\left(1-\beta^{4}\right)}} \text {, or } \frac{\beta^{2}}{\left(1-\beta^{4}\right)^{1 / 2}} \approx 0.478 \\
& \text { Solve for } \beta \approx \mathbf{0 . 6 6} \text { Ans. }
\end{aligned}
$$

Checking back with Fig. 6.41, we see that this is about right, so no further iteration is needed for this level of accuracy.
6.142 The shower head in Fig. P6.142 delivers water at $50^{\circ} \mathrm{C}$. An orifice-type flow reducer is to be installed. The upstream pressure is constant at 400 kPa . What flow rate, in gal/min, results without the reducer? What reducer orifice diameter would decrease the flow by 40 percent?

Solution: For water at $50^{\circ} \mathrm{C}$, take $\rho=$ $988 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.548 \mathrm{E}-3 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Further assume that the shower head is a poor diffuser, so the pressure in the head is


Fig. P6.142
also about 400 kPa . Assume the outside pressure is sea-level standard, 101 kPa . From Fig. 6.41 for a 'typical' orifice, estimate $\mathrm{Cd} \approx 0.61$. Then, with $\beta \approx 0$ for the small holes, each hole delivers a flow rate of

$$
\begin{aligned}
& \quad \mathrm{Q}_{1 \text { hole }} \approx \mathrm{C}_{\mathrm{d}} \mathrm{~A}_{\text {hole }} \sqrt{\frac{2 \Delta \mathrm{p}}{\rho\left(1-\beta^{4}\right)}} \approx 0.61\left(\frac{\pi}{4}\right)(0.0015)^{2} \sqrt{\frac{2(400000-101000)}{988\left(1-0^{4}\right)}}, \\
& \text { or } \quad \mathrm{Q}_{1 \text { hole }} \approx 2.65 \mathrm{E}-5 \mathrm{~m}^{3} / \mathrm{s} \text { and } \mathrm{Q}_{\text {total }}=45 \mathrm{Q}_{1 \text { hole }} \approx 0.00119 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}\left(\approx 19 \frac{\mathrm{gal}}{\mathrm{~min}}\right)
\end{aligned}
$$

This is a large flow rate-a lot of expensive hot water. Checking back, the inlet pipe for this flow rate has $\mathrm{ReD} \approx 183000$, so $\mathrm{Cd} \approx 0.60$ would be slightly better and a repeat of the calculation would give Qno reducer $\approx 0.00117 \mathrm{~m}^{3} / \mathrm{s} \approx \mathbf{1 8 . 6} \mathbf{g a l} / \mathbf{m i n}$. Ans.
A $40 \%$ reduction would give $\mathrm{Q}=0.6(0.00117)=7.04 \mathrm{E}-4 \mathrm{~m}^{3} / \mathrm{s} \div 45=1.57 \mathrm{E}-5 \mathrm{~m}^{3} / \mathrm{s}$ for each hole, which corresponds to a pressure drop

$$
\mathrm{Q}_{1 \text { hole }}=1.57 \mathrm{E}-5=0.60\left(\frac{\pi}{4}\right)(0.0015)^{2} \sqrt{\frac{2 \Delta \mathrm{p}}{988}}, \quad \text { or } \quad \Delta \mathrm{p} \approx 108000 \mathrm{~Pa}
$$

or $\mathrm{p}_{\text {inside head }} \approx 101+108 \approx 209 \mathrm{kPa}$, the reducer must drop the inlet pressure to this.

$$
\mathrm{Q}=7.04 \mathrm{E}-4 \approx 0.61\left(\frac{\pi}{4}\right)(0.015 \beta)^{2}\left[\frac{2(400000-209000)}{988\left(1-\beta^{4}\right)}\right]^{1 / 2}, \quad \text { or } \quad \frac{\beta^{2}}{\left(1-\beta^{4}\right)^{1 / 2}} \approx 0.332
$$

Solve for $\beta \approx 0.56, \quad \mathrm{~d}_{\text {reducer }} \approx 0.56(1.5) \approx \mathbf{0 . 8 4} \mathbf{c m}$ Ans.
6.143 A 10 -cm-diameter smooth pipe contains an orifice plate with $D: \frac{1}{2} D$ taps and $\beta=$ 0.5 . The measured orifice pressure drop is 75 kPa for water flow at $20^{\circ} \mathrm{C}$. Estimate the flow rate, in $\mathrm{m}^{3} / \mathrm{h}$. What is the nonrecoverable head loss?

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. We know everything in the orifice relation, Eq. (6.104), except Cd, which we can estimate (as 0.61 ):

$$
\mathrm{Q}=\mathrm{C}_{\mathrm{d}} \mathrm{~A}_{\mathrm{t}} \sqrt{\frac{2 \Delta \mathrm{p}}{\rho\left(1-\beta^{4}\right)}}=\mathrm{C}_{\mathrm{d}} \frac{\pi}{4}(0.05)^{2} \sqrt{\frac{2(75000)}{998\left[1-(0.5)^{4}\right]}}=0.0249 \mathrm{C}_{\mathrm{d}}
$$

Guess $\quad C_{d} \approx 0.61, \quad \mathrm{Q} \approx 0.0152 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}, \quad \mathrm{Re}_{\mathrm{D}}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{D}} \approx 193000, \quad \mathrm{C}_{\mathrm{d}}($ Eq. 6.112 $) \approx 0.605$

This is converged: $\mathbf{Q}=0.0249(0.605)=0.0150 \mathrm{~m}^{3} / \mathrm{s} \approx 54 \mathbf{m}^{\mathbf{3}} / \mathbf{h}$. Ans. (a)
(b) From Fig. 6.44, the non-recoverable head loss coefficient is $\mathrm{K} \approx 1.8$, based on Vt :

$$
\begin{gathered}
\mathrm{V}_{\mathrm{t}}=\frac{\mathrm{Q}}{\mathrm{~A}_{\mathrm{t}}}=\frac{0.0150}{\pi(0.025)^{2}} \approx 7.66 \frac{\mathrm{~m}}{\mathrm{~s}} \\
\Delta \mathrm{p}_{\text {loss }}=\mathrm{K} \frac{\rho}{2} \mathrm{~V}_{\mathrm{t}}^{2}=1.8\left(\frac{998}{2}\right)(7.66)^{2} \approx 53000 \mathrm{~Pa} \text { Ans. (b) }
\end{gathered}
$$

6.144 Accurate solution of Prob. 6.143, using Fig. 6.41, requires iteration because both the ordinate and the abscissa of this figure contain the unknown flow rate $Q$. In the spirit of Example 5.8, rescale the variables and construct a new plot in which $Q$ may be read directly from the ordinate. Solve Prob. 6.143 with your new chart.

Solution: Figure 6.41 has Cd versus ReD, both of which contain Q:

$$
\mathrm{C}_{\mathrm{d}}=\frac{\mathrm{Q}}{\mathrm{~A}_{\mathrm{t}}\left[2 \Delta \mathrm{p} / \rho\left(1-\beta^{4}\right)\right]^{1 / 2}} ; \quad \operatorname{Re}_{\mathrm{D}}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{D}}, \quad \text { then } \zeta=\mathrm{C}_{\mathrm{d}}^{-1} \mathrm{Re}_{\mathrm{D}}=\frac{\beta \rho \mathrm{d}}{\mu}\left[\frac{2 \Delta \mathrm{p}}{\rho\left(1-\beta^{4}\right)}\right]^{1 / 2}
$$

The quantity $\zeta$ is independent of Q -sort of a Q -less Reynolds number. If we plot Cd versus $\zeta$, we should solve the problem of finding an unknown Q when $\Delta \mathrm{p}$ is known. The plot is shown below. For the data of Prob. 6.143, we compute

$$
\left[\frac{2 \Delta \mathrm{p}}{\rho\left(1-\beta^{4}\right)}\right]^{1 / 2}=\left[\frac{2(75000)}{998\left(1-.5^{4}\right)}\right]^{1 / 2}=12.7 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \zeta=\frac{0.5(998)(0.05)(12.7)}{0.001} \approx 316000
$$

From the figure below, read $\mathbf{C} \mathbf{d} \approx \mathbf{0 . 6 0 5}(!)$ hence $\mathrm{Q}=\mathrm{CdAt}\left[2 \Delta \mathrm{p} / \rho\left(1-\beta^{4}\right)\right]^{1 / 2}=\mathbf{5 4} \mathbf{m}^{\mathbf{3}} / \mathbf{h}$.


Fig. P6.144
6.145 The 1 -m-diameter tank in Fig. P6.145 is initially filled with gasoline at $20^{\circ} \mathrm{C}$. There is a $2-\mathrm{cm}$-diameter orifice in the bottom. If the orifice is suddenly opened, estimate the time for the fluid level $h(\mathrm{t})$ to drop from 2.0 to 1.6 meters.

Solution: For gasoline at $20^{\circ} \mathrm{C}$, take $\rho=$ $680 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=2.92 \mathrm{E}-4 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The

Fig. P6.145
 orifice simulates "corner taps" with $\beta \approx 0$, so, from Eq. (6.112), $\mathrm{Cd} \approx 0.596$. From the energy equation, the pressure drop across the orifice is $\Delta \mathrm{p}=\rho \mathrm{gh}(\mathrm{t})$, or

$$
\begin{gathered}
\mathrm{Q}=\mathrm{C}_{\mathrm{d}} \mathrm{~A}_{\mathrm{t}} \sqrt{\frac{2 \rho \mathrm{gh}}{\rho\left(1-\beta^{4}\right)}} \approx 0.596\left(\frac{\pi}{4}\right)(0.02)^{2} \sqrt{2(9.81) \mathrm{h}} \approx 0.000829 \sqrt{\mathrm{~h}} \\
\text { But also } \quad \mathrm{Q}=-\frac{\mathrm{d}}{\mathrm{dt}}\left(v_{\text {tank }}\right)=-\mathrm{A}_{\text {tank }} \frac{\mathrm{dh}}{\mathrm{dt}}=-\frac{\pi}{4}(1.0 \mathrm{~m})^{2} \frac{\mathrm{dh}}{\mathrm{dt}}
\end{gathered}
$$

Set the Q's equal, separate the variables, and integrate to find the draining time:

$$
-\int_{2.0}^{1.6} \frac{\mathrm{dh}}{\sqrt{ } \mathrm{~h}}=0.001056 \int_{0}^{\mathrm{t}_{\text {final }}} \mathrm{dt}, \quad \text { or } \quad \mathrm{t}_{\text {final }}=\frac{2[\sqrt{2}-\sqrt{1.6}]}{0.001056}=283 \mathrm{~s} \approx 4.7 \mathrm{~min} \quad \text { Ans. }
$$

6.146 A pipe connecting two reservoirs, as in Fig. P6.146, contains a thin-plate orifice. For water flow at $20^{\circ} \mathrm{C}$, estimate (a) the volume flow through the pipe and (b) the pressure drop across the orifice plate.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ $998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The energy equation should include the orifice


Fig. P6.146 head loss and the entrance and exit losses:

$$
\Delta \mathrm{z}=20 \mathrm{~m}=\frac{\mathrm{V}^{2}}{2 \mathrm{~g}}\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}+\sum \mathrm{K}\right), \text { where } \mathrm{K}_{\mathrm{entr}} \approx 0.5, \mathrm{~K}_{\mathrm{exit}} \approx 1.0, \mathrm{~K}_{\text {orifice }}^{\beta=0.6} \approx 1.5 \text { (Fig. 6.44) }
$$

$$
\begin{gathered}
\mathrm{V}^{2}=\frac{2(9.81)(20)}{[\mathrm{f}(100 / 0.05)+0.5+1.0+1.5]}=\frac{392.4}{2000 \mathrm{f}+3.0} ; \text { guess } \mathrm{f} \approx 0.02, \mathrm{~V} \approx 3.02 \mathrm{~m} / \mathrm{s} \\
\text { Iterate to } \mathrm{f}_{\text {smooth }} \approx 0.0162, \quad \mathrm{~V} \approx 3.33 \mathrm{~m} / \mathrm{s}
\end{gathered}
$$

The final $\operatorname{Re}=\rho \mathrm{VD} / \mu \approx 166000$, and $\mathrm{Q}=(\pi / 4)(0.05)^{2}(3.33) \approx \mathbf{0 . 0 0 6 5 3} \mathbf{~ m}^{\mathbf{3}} / \mathbf{s}$ Ans. (a)
(b) The pressure drop across the orifice is given by the orifice formula:

$$
\begin{gathered}
\operatorname{ReD}=166000, \quad \beta=0.6, \quad \mathrm{Cd} \approx 0.609 \text { (Fig. 6.41): } \\
\mathrm{Q}=0.00653=\mathrm{C}_{\mathrm{d}} \mathrm{~A}_{\mathrm{t}}\left[\frac{2 \Delta \mathrm{p}}{\rho\left(1-\beta^{4}\right)}\right]^{1 / 2}=0.609\left(\frac{\pi}{4}\right)(0.03)^{2}\left[\frac{2 \Delta \mathrm{p}}{998\left(1-0.6^{4}\right)}\right]^{1 / 2}, \\
\Delta \mathrm{p}=\mathbf{1 0 0} \mathbf{~ k P a} \text { Ans. }
\end{gathered}
$$

6.147 Air flows through a 6-cm-diameter smooth pipe which has a 2 m -long perforated section containing 500 holes (diameter 1 mm ), as in Fig. P6.147. Pressure outside the pipe is sea-level standard air. If $p 1=105 \mathrm{kPa}$ and $Q 1=110 \mathrm{~m}^{3} / \mathrm{h}$, estimate p2 and Q2, assuming that the holes are approximated by thin-plate orifices. Hint: A


Fig. P6.147 momentum control volume may be very useful.

Solution: For air at $20^{\circ} \mathrm{C}$ and 105 kPa , take $\rho=1.25 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.8 \mathrm{E}-5 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Use the entrance flow rate to estimate the wall shear stress from the Moody chart:

$$
\begin{gathered}
\mathrm{V}_{1}=\frac{\mathrm{Q}_{1}}{\mathrm{~A}}=\frac{110 / 3600}{(\pi / 4)(0.06)^{2}}=10.8 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \operatorname{Re}_{1}=\frac{1.25(10.8)(0.06)}{1.8 \mathrm{E}-5} \approx 45000, \quad \mathrm{f}_{\text {smooth }} \approx 0.0214 \\
\text { then } \tau_{\text {wall }}=\frac{\mathrm{f}}{8} \rho \mathrm{~V}^{2}=\frac{0.0214}{8}(1.25)(10.8)^{2} \approx 0.390 \mathrm{~Pa}
\end{gathered}
$$

Further assume that the pressure does not change too much, so $\Delta$ porifice $\approx 105000-101350 \approx$ 3650 Pa. Then the flow rate from the orifices is, approximately,

$$
\beta \approx 0, \mathrm{C}_{\mathrm{d}} \approx 0.61: \quad \mathrm{Q} \approx 500 \mathrm{C}_{\mathrm{d}} \mathrm{~A}_{\mathrm{t}}(2 \Delta \mathrm{p} / \rho)^{1 / 2}=500(0.61)\left(\frac{\pi}{4}\right)(0.001)^{2}\left[\frac{2(3650)}{1.25}\right]^{1 / 2}
$$

$$
\text { or: } \mathrm{Q} \approx \mathbf{0 . 0 1 8 3} \mathrm{~m}^{3} / \mathrm{s}, \quad \text { so } \mathrm{Q}_{2}=\frac{110}{3600}-0.0183 \approx 0.01225 \mathrm{~m}^{3} / \mathrm{s}
$$

Then $\mathrm{V} 2=\mathrm{Q} 2 / \mathrm{A} 2=0.01225 /\left[(\pi / 4)(0.06)^{2}\right] \approx 4.33 \mathrm{~m} / \mathrm{s}$. A control volume enclosing the pipe walls and sections (1) and (2) yields the $x$-momentum equation:

$$
\begin{aligned}
\sum \mathrm{F}_{\mathrm{x}} & =\mathrm{p}_{1} \mathrm{~A}-\mathrm{p}_{2} \mathrm{~A}-\tau_{\mathrm{w}} \pi \mathrm{DL}=\dot{\mathrm{m}}_{2} \mathrm{~V}_{2}-\dot{\mathrm{m}}_{1} \mathrm{~V}_{1}=\rho \mathrm{AV}_{2}^{2}-\rho \mathrm{AV}_{1}^{2}, \quad \text { divide by } \mathrm{A}: \\
\mathrm{p}_{1}-\mathrm{p}_{2} & =0.390\left[\frac{\pi(0.06)(2.0)}{(\pi / 4)(0.06)^{2}}\right]+1.25(4.33)^{2}-1.25(10.8)^{2}=52+23-146 \approx-71 \mathrm{~Pa}
\end{aligned}
$$

Thus p2 $=105000+71 \approx \mathbf{1 0 5} \mathbf{~ k P a}$ also and above is correct: $\mathrm{Q} 2=\mathbf{0 . 0 1 2 3} \mathbf{m}^{\mathbf{3}} / \mathbf{s}$. Ans.
6.148 A smooth pipe containing ethanol at $20^{\circ} \mathrm{C}$ flows at $7 \mathrm{~m}^{3} / \mathrm{h}$ through a Bernoulli obstruction, as in Fig. P6.148. Three piezometer tubes are installed, as shown. If the obstruction is a thin-plate orifice, estimate the piezometer levels (a) h2 and (b) h3.


Fig. P6.148
Solution: For ethanol at $20^{\circ} \mathrm{C}$, take $\rho=789 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.0012 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. With the flow rate known, we can compute Reynolds number and friction factor, etc.:

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{7 / 3600}{(\pi / 4)(0.05)^{2}}=0.99 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \operatorname{Re}_{\mathrm{D}}=\frac{789(0.99)(0.05)}{0.0012}=32600, \quad \mathrm{f}_{\text {smooth }} \approx 0.0230
$$

From Fig. 6.44 , at $\beta=0.6, \mathrm{~K} \approx 1.5$. Then the head loss across the orifice is

$$
\Delta \mathrm{h}=\mathrm{h}_{2}-\mathrm{h}_{1}=\mathrm{K} \frac{\mathrm{~V}_{\mathrm{t}}^{2}}{2 \mathrm{~g}}=(1.5)\left[\frac{\left\{0.99 /(0.6)^{2}\right\}^{2}}{2(9.81)}\right] \approx 0.58 \mathrm{~m} \text {, hence } \mathbf{h}_{2} \approx 1.58 \mathbf{m} \quad \text { Ans. (a) }
$$

Then the piezometer change between (2) and (3) is due to Moody friction loss:

$$
\begin{gathered}
\mathrm{h}_{3}-\mathrm{h}_{2}=\mathrm{h}_{\mathrm{f}}=\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}} \frac{\mathrm{~V}^{2}}{2 \mathrm{~g}}=(0.023)\left(\frac{5}{0.05}\right) \frac{(0.99)^{2}}{2(9.81)}=0.12 \mathrm{~m}, \\
\\
\text { or } \mathrm{h}_{3}=1.58+0.12 \approx \mathbf{1 . 7} \mathbf{~ m} \text { Ans. (b) }
\end{gathered}
$$

6.149 In a laboratory experiment, air at
$20^{\circ} \mathrm{C}$ flows from a large tank through a $2-$ cm-diameter smooth pipe into a sea-level atmosphere, as in Fig. P6.149. The flow is metered by a long-radius nozzle of $1-\mathrm{cm}$ diameter, using a manometer with Meriam red oil ( $\mathrm{SG}=0.827$ ). The pipe is 8 m long. The measurements of tank pressure and manometer height are as follows:


Fig. P6.149

| ptank, | Pa | 60 | 320 | 1200 | 2050 | 2470 | 3500 | 4900 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

(gage):

| $h m a n o, ~ m m: ~$ | 6 | 38 | 160 | 295 | 380 | 575 | 820 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |

Use this data to calculate the flow rates $Q$ and Reynolds numbers $\operatorname{Re} D$ and make a plot of measured flow rate versus tank pressure. Is the flow laminar or turbulent? Compare the data with theoretical results obtained from the Moody chart, including minor losses. Discuss.

Solution: For air take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.000015 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. With no elevation change and negligible tank velocity, the energy equation would yield

$$
p_{\mathrm{tank}}-p_{\text {atm }}=\frac{\rho V^{2}}{2}\left(1+f \frac{L}{D}+K_{\text {entrance }}+K_{\text {nozzle }}\right), \quad K_{\text {ent }} \approx 0.5 \text { and } K_{n o z} \approx 0.7
$$

Since $\Delta p$ is given, we can use this expression plus the Moody chart to predict $V$ and $Q=$ $A V$ and compare with the flow-nozzle measurements. The flow nozzle formula is:

$$
V_{\text {throat }}=C_{d} \sqrt{\frac{2 \Delta p_{\text {mano }}}{\rho\left(1-\beta^{4}\right)}} \quad \text { where } \Delta p=\left(\rho_{\text {oil }}-\rho_{\text {air }}\right) g h, C_{d} \text { from Fig. } 6.42 \text { and } \beta=0.5
$$

The friction factor is given by the smooth-pipe Moody formula, Eq. (6.48) for $\varepsilon=0$. The results may be tabulated as follows, and the plot on the next page shows excellent (too good?) agreement with theory.

| ptank, Pa: | 60 | 320 | 1200 | 2050 | 2470 | 3500 | 4900 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| V, m/s (nozzle data): | 2.32 | 5.82 | 11.9 | 16.1 | 18.2 | 22.3 | 26.4 |
| Q, $\mathrm{m}^{3} / \mathrm{h}$ (nozzle data): | 2.39 | 6.22 | 12.9 | 17.6 | 19.9 | 24.5 | 29.1 |
| Q, $\mathrm{m}^{3} / \mathrm{h}$ (theory): | 2.31 | 6.25 | 13.3 | 18.0 | 20.0 | 24.2 | 28.9 |

fMoody: $\quad 0.0444 \quad 0.0331 \quad 0.0271 \quad 0.0252 \quad 0.0245 \quad 0.0234 \quad 0.0225$

6.150 Gasoline at $20^{\circ} \mathrm{C}$ flows at $0.06 \mathrm{~m}^{3} / \mathrm{s}$ through a $15-\mathrm{cm}$ pipe and is metered by a 9 -cm-diameter long-radius flow nozzle (Fig. 6.40a). What is the expected pressure drop across the nozzle?

Solution: For gasoline at $20^{\circ} \mathrm{C}$, take $\rho=680 \mathrm{~kg} / \mathrm{m}$ and $\mu=2.92 \mathrm{E}-4 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Calculate the pipe velocity and Reynolds number:

$$
\mathrm{V}=\frac{\mathrm{Q}}{\mathrm{~A}}=\frac{0.06}{(\pi / 4)(0.15)^{2}}=3.40 \frac{\mathrm{~m}}{\mathrm{~s}}, \quad \mathrm{Re}_{\mathrm{D}}=\frac{680(3.40)(0.15)}{2.92 \mathrm{E}-4} \approx 1.19 \mathrm{E} 6
$$

The ISO correlation for discharge (Eq. 6.114) is used to estimate the pressure drop:

$$
\begin{gathered}
\mathrm{C}_{\mathrm{d}} \approx 0.9965-0.00653\left(\frac{10^{6} \beta}{\operatorname{Re}_{\mathrm{D}}}\right)^{1 / 2}=0.9965-0.00653\left[\frac{10^{6}(0.6)}{1.19 \mathrm{E} 6}\right]^{1 / 2} \approx 0.9919 \\
\text { Then } \mathrm{Q}=0.06=(0.9919)\left(\frac{\pi}{4}\right)(0.09)^{2} \sqrt{\frac{2 \Delta \mathrm{p}}{680\left(1-0.6^{4}\right)}}, \\
\text { Solve } \Delta \mathbf{p} \approx 27000 \text { Pa Ans. }
\end{gathered}
$$

P6.151 An engineer needs to monitor a flow of $20^{\circ} \mathrm{C}$ gasoline at about $250 \pm 25 \mathrm{gal} / \mathrm{min}$ through a 4-in-diameter smooth pipe. She can use an orifice plate, a long-radius flow nozzle, or a venturi nozzle, all with 2-in-diameter throats. The only differential pressure gage available is accurate in the range 6 to $10 \mathrm{lbf} / \mathrm{in}^{2}$. Disregarding flow losses, which device is best?

Solution: For gasoline at $20^{\circ} \mathrm{C}$, take $\rho=680 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=2.92 \mathrm{E}-4 \mathrm{~kg} / \mathrm{m}-\mathrm{s}$. We are given $\beta=2 / 4=0.5$. The flow rate is in the range $0.0142<Q<0.0174 \mathrm{~m}^{3} / \mathrm{s}$. The pipe Reynolds number is in the range $\operatorname{Re}_{D}=460,000 \pm 10 \%$. In SI units, the throat diameter is 0.0508 m , and its area is $(\pi / 4)(0.0508 \mathrm{~m})^{2}=0.00203 \mathrm{~m}^{2}$. Our basic "obstruction" formula is Eq. (6.104):

$$
Q=C_{d} A_{t} \sqrt{\frac{2 \Delta p}{\rho\left(1-\beta^{4}\right)}}=C_{d}\left(0.00203 m^{2}\right) \sqrt{\frac{2 \Delta p}{\left(680 \mathrm{~kg} / \mathrm{m}^{3}\right)\left\{1-(0.5)^{4}\right\}}}=0.0158 \pm 0.0016 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}
$$

It remains only to determine $\mathrm{C}_{\mathrm{d}}$ for the three devices and then calculate $\Delta p$. The results are:

Orifice plate, $\mathrm{D}: 1 / 2 \mathrm{D}$ taps: $\mathrm{C}_{\mathrm{d}} \approx 0.605, \quad \Delta p=6.2$ to $9.3 \mathrm{lbf} / \mathrm{in}^{2}$ Ans.

Long-radius flow nozzle: $\quad \mathrm{C}_{\mathrm{d}} \approx 0.99, \quad \Delta p=2.3$ to $3.5 \mathrm{lbf} / \mathrm{in}^{2}$

Venturi nozzle: $\quad \mathrm{C}_{\mathrm{d}} \approx 0.977, \quad \Delta p=2.4$ to $3.6 \mathrm{lbf} / \mathrm{in}^{2}$

Only the orifice plate, with its high losses, is compatible with the available pressure gage.
6.152 Kerosene at $20^{\circ} \mathrm{C}$ flows at $20 \mathrm{~m}^{3} / \mathrm{h}$ in an 8 -cm-diameter pipe. The flow is to be metered by an ISA 1932 flow nozzle so that the pressure drop is 7 kPa . What is the proper nozzle diameter?

Solution: For kerosene at $20^{\circ} \mathrm{C}$, take $\rho=804 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=1.92 \mathrm{E}-3 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. We cannot calculate the discharge coefficient exactly because we don't know $\beta$, so just estimate Cd:

$$
\begin{gathered}
\text { Guess } \mathrm{C}_{\mathrm{d}} \approx 0.99 \text {, then } \mathrm{Q} \approx 0.99\left(\frac{\pi}{4}\right)(0.08 \beta)^{2} \sqrt{\frac{2(7000)}{804\left(1-\beta^{4}\right)}}=\frac{20}{3600} \frac{\mathrm{~m}^{3}}{\mathrm{~s}} \\
\text { or: } \frac{\beta^{2}}{\left(1-\beta^{4}\right)^{1 / 2}} \approx 0.268, \text { solve } \beta \approx 0.508, \\
\operatorname{Re}_{\mathrm{D}}=\frac{4(804)(20 / 3600)}{\pi(1.92 \mathrm{E}-3)(0.08)} \approx 37000
\end{gathered}
$$

Now compute a better Cd from the ISA nozzle correlation, Eq. (6.115):

$$
\mathrm{C}_{\mathrm{d}} \approx 0.99-0.2262 \beta^{4.1}+\left(0.000215-0.001125 \beta+0.00249 \beta^{4.7}\right)\left(\frac{10^{6}}{\mathrm{Re}_{\mathrm{D}}}\right)^{1.15} \approx 0.9647
$$

Iterate once to obtain a better $\beta \approx 0.515, \mathrm{~d}=0.515(8 \mathrm{~cm}) \approx 4.12 \mathrm{~cm}$ Ans.
6.153 Two water tanks, each with base area of $1 \mathrm{ft}^{2}$, are connected by a 0.5 -indiameter long-radius nozzle as in Fig. P6.153. If $\mathrm{h}=1 \mathrm{ft}$ as shown for $t=0$, estimate the time for $\mathrm{h}(\mathrm{t})$ to drop to 0.25 ft .

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.94 slug $/ \mathrm{ft}^{3}$ and $\mu=2.09 \mathrm{E}-5$ slug/ft.s. For a long-radius nozzle with $\beta \approx 0$, guess $\mathrm{Cd} \approx$ 0.98 and Kloss $\approx 0.9$ from Fig. 6.44. The elevation difference $h$ must balance the head losses in the nozzle and submerged exit:


Fig. P6.153

$$
\begin{aligned}
& \Delta \mathrm{z}= \sum \mathrm{h}_{\text {loss }}=\frac{\mathrm{V}_{\mathrm{t}}^{2}}{2 \mathrm{~g}} \sum \mathrm{~K}=\frac{\mathrm{V}_{\mathrm{t}}^{2}}{2(32.2)}\left(0.9_{\text {nozzle }}+1.0_{\text {exit }}\right)=\mathrm{h}, \text { solve } \mathrm{V}_{\mathrm{t}}=5.82 \sqrt{\mathrm{~h}} \\
& \text { hence } \mathrm{Q}=\mathrm{V}_{\mathrm{t}}\left(\frac{\pi}{4}\right)\left(\frac{1 / 2}{12}\right)^{2} \approx 0.00794 \sqrt{\mathrm{~h}}=-\frac{1}{2} \mathrm{~A}_{\text {tank }} \frac{\mathrm{dh}}{\mathrm{dt}}=-0.5 \frac{\mathrm{dh}}{\mathrm{dt}}
\end{aligned}
$$

The boldface factor $\mathbf{1} / \mathbf{2}$ accounts for the fact that, as the left tank falls by dh , the right tank rises by the same amount, hence $\mathrm{dh} / \mathrm{dt}$ changes twice as fast as for one tank alone. We can separate and integrate and find the time for $h$ to drop from 1 ft to 0.25 ft :

$$
\int_{0.25}^{1.0} \frac{\mathrm{dh}}{\sqrt{\mathrm{~h}}}=0.0159 \int_{0}^{\mathrm{t}_{\text {final }}} \mathrm{dt}, \quad \text { or: } \quad \mathrm{t}_{\text {final }}=\frac{2(\sqrt{1}-\sqrt{0.25})}{0.0159} \approx 63 \mathrm{~s} \quad \text { Ans. }
$$

6.154 Water at $20^{\circ} \mathrm{C}$ flows through the orifice in the figure, which is monitored by a mercury manometer. If $d=3 \mathrm{~cm}$, (a) what is $h$ when the flow is $20 \mathrm{~m}^{3} / \mathrm{h}$; and (b) what is $Q$ when $h=58 \mathrm{~cm}$ ?

Solution: (a) Evaluate $\mathrm{V}=\mathrm{Q} / \mathrm{A}=2.83 \mathrm{~m} / \mathrm{s}$ and $\operatorname{ReD}=\rho \mathrm{VD} / \mu=141,000, \beta=0.6$, thus $\mathrm{Cd} \approx 0.613$.


Fig. P6.154

$$
Q=\frac{20}{3600}=C_{d} \frac{\pi}{4} d^{2} \sqrt{\frac{2 \Delta p}{\rho\left(1-\beta^{4}\right)}}=(0.613) \frac{\pi}{4}(0.03)^{2} \sqrt{\frac{2(13550-998)(9.81) h}{998\left(1-0.6^{4}\right)}}
$$

where we have introduced the manometer formula $\Delta \mathrm{p}=\left(\rho_{\text {mercury }}-\rho_{\text {water }}\right) \mathrm{gh}$.

$$
\text { Solve for: } \quad \boldsymbol{h} \approx 0.58 \mathrm{~m}=58 \mathbf{~ c m} \quad \text { Ans. (a) }
$$

Solve this problem when $h=58 \mathrm{~cm}$ is known and $Q$ is the unknown. Well, we can see that the numbers are the same as part (a), and the solution is

$$
\text { Solve for: } \boldsymbol{Q} \approx 0.00556 \mathrm{~m}^{3} / \mathrm{s}=\mathbf{2 0} \mathbf{~ m}^{\mathbf{3}} / \mathbf{h} \text { Ans. (b) }
$$

6.155 It is desired to meter a flow of $20^{\circ} \mathrm{C}$ gasoline in a $12-\mathrm{cm}$-diameter pipe, using a modern venturi nozzle. In order for international standards to be valid (Fig. 6.40), what is the permissible range of (a) flow rates, (b) nozzle diameters, and (c) pressure drops? (d) For the highest pressure-drop condition, would compressibility be a problem?

Solution: For gasoline at $20^{\circ} \mathrm{C}$, take $\rho=680 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=2.92 \mathrm{E}-4 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Examine the possible range of Reynolds number and beta ratio:

$$
\begin{aligned}
& 1.5 \mathrm{E} 5<\mathrm{Re}_{\mathrm{D}}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{D}}=\frac{4(680) \mathrm{Q}}{\pi(2.92 \mathrm{E}-4)(0.12)}<2.0 \mathrm{E} 5, \\
& \text { or } \mathbf{0 . 0 0 6 1}<\mathbf{Q}<\mathbf{0 . 0 0 8 1} \frac{\mathbf{m}^{3}}{\mathbf{s}} \quad \text { Ans. (a) } \\
& 0.316<\beta=\mathrm{d} / \mathrm{D}<0.775, \quad \text { or: } \mathbf{3 . 8}<\mathbf{d}<\mathbf{9 . 3} \mathbf{~ c m} \text { Ans. (b) }
\end{aligned}
$$

For estimating pressure drop, first compute $\mathrm{Cd}(\beta)$ from Eq. (6.116): $0.924<\mathrm{Cd}<0.985$ :

$$
\mathrm{Q}=\mathrm{C}_{\mathrm{d}} \frac{\pi}{4}(0.12 \beta)^{2} \sqrt{\frac{2 \Delta \mathrm{p}}{680\left(1-\beta^{4}\right)}}, \quad \text { or: } \quad \Delta \mathrm{p}=2.66 \mathrm{E} 6\left(1-\beta^{4}\right)\left[\frac{\mathrm{Q}}{\mathrm{C}_{\mathrm{d}} \beta^{2}}\right]^{2}
$$

put in large Q , small $\beta$, etc. to obtain the range $\mathbf{2 0 0}<\Delta \mathrm{p}<\mathbf{1 8 0 0 0}$ Pa Ans. (c)
6.156 Ethanol at $20^{\circ} \mathrm{C}$ flows down through a modern venturi nozzle as in Fig. P6.156. If the mercury manometer reading is 4 in, as shown, estimate the flow rate, in gal/min.

Solution: For ethanol at $20^{\circ} \mathrm{C}$, take $\rho=$ 1.53 slug $/ \mathrm{ft}^{3}$ and $\mu=2.51 \mathrm{E}-5$ slug/ft•s. Given $\beta=0.5$, the discharge coefficient is

$$
\mathrm{Cd}=0.9858-0.196(0.5)^{4.5} \approx 0.9771
$$



Fig. P6.156

The 9-inch displacement of manometer taps does not affect the pressure drop reading, because both legs are filled with ethanol. Therefore we proceed directly to $\Delta \mathrm{p}$ and Q :

$$
\Delta \mathrm{p}_{\text {nozzle }}=\left(\rho_{\text {merc }}-\rho_{\text {eth }}\right) \mathrm{gh}=(26.3-1.53)(32.2)(4 / 12) \approx 266 \mathrm{lbf} / \mathrm{ft}^{2}
$$

Hence $\quad \mathrm{Q}=\mathrm{C}_{\mathrm{d}} \mathrm{A}_{\mathrm{t}}\left[\frac{2 \Delta \mathrm{p}}{\rho\left(1-\beta^{4}\right)}\right]^{1 / 2}=0.9771\left(\frac{\pi}{4}\right)\left(\frac{3}{12}\right)^{2} \sqrt{\frac{2(266)}{1.53\left(1-0.5^{4}\right)}} \approx 0.924 \frac{\mathrm{ft}^{3}}{\mathrm{~s}} \quad$ Ans.
6.157 Modify Prob. 6.156 if the fluid is air at $20^{\circ} \mathrm{C}$, entering the venturi at a pressure of 18 psia. Should a compressibility correction be used?

Solution: For air at $20^{\circ} \mathrm{C}$ and 18 psi , take $\rho=0.00286$ slug/ft ${ }^{3}$ and $\mu=3.76 \mathrm{E}-7 \mathrm{slug} / \mathrm{ft} \cdot \mathrm{s}$. With $\beta$ still equal to $0.5, \mathrm{Cd}$ still equals 0.9771 as previous page. The manometer reading is

$$
\begin{gathered}
\Delta \mathrm{p}_{\text {nozzle }}=(26.3-0.00286)(32.2)(4 / 12) \approx 282 \mathrm{lbf} / \mathrm{ft}^{2}, \\
\text { whence } \quad \mathrm{Q}=0.9771\left(\frac{\pi}{4}\right)\left(\frac{3}{12}\right)^{2} \sqrt{\frac{2(282)}{0.00286\left(1-0.5^{4}\right)}} \approx \mathbf{2 2 . 0} \frac{\mathbf{f t}^{3}}{\mathbf{s}} \quad \text { Ans. }
\end{gathered}
$$

From this result, the throat velocity $\mathrm{Vt}=\mathrm{Q} / \mathrm{At} \approx 448 \mathrm{ft} / \mathrm{s}$, quite high, the Mach number in the throat is approximately $\mathrm{Ma}=0.4$, a (slight) compressibility correction might be expected. [Making a one-dimensional subsonic-flow correction, using the methods of Chap. 9, results in a throat volume flow estimate of $\mathrm{Q} \approx 22.8 \mathrm{ft}^{3} / \mathrm{s}$, about $4 \%$ higher.]
6.158 Water at $20^{\circ} \mathrm{C}$ flows in a long horizontal commercial-steel 6-cm-diameter pipe which contains a classical Herschel venturi with a $4-\mathrm{cm}$ throat. The venturi is connected to a mercury manometer whose reading is $h=40 \mathrm{~cm}$. Estimate (a) the flow rate, in $\mathrm{m}^{3} / \mathrm{h}$, and (b) the total pressure difference between points 50 cm upstream and 50 cm downstream of the venturi.

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. For commercial steel, $\varepsilon \approx 0.046 \mathrm{~mm}$, hence $\varepsilon / d=0.046 / 60=0.000767$. First estimate the flow rate:

$$
\Delta \mathrm{p}=\left(\rho_{\mathrm{m}}-\rho_{\mathrm{w}}\right) \mathrm{gh}=(13560-998)(9.81)(0.40) \approx 49293 \mathrm{~Pa}
$$

Guess $\quad \mathrm{C}_{\mathrm{d}} \approx 0.985, \quad \mathrm{Q}=(0.985)\left(\frac{\pi}{4}\right)(0.04)^{2} \sqrt{\frac{2(49293)}{998\left[1-(4 / 6)^{4}\right]}} \approx 0.0137 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}$
Check $\operatorname{Re}_{\mathrm{D}}=\frac{4 \rho \mathrm{Q}}{\pi \mu \mathrm{D}} \approx 291000$

At this Reynolds number, we see from Fig. 6.42 that Cd does indeed $\approx 0.985$ for the Herschel venturi. Therefore, indeed, $\mathrm{Q}=0.0137 \mathrm{~m}^{3} / \mathrm{s} \approx 49 \mathrm{~m}^{3} / \mathrm{h}$. Ans. (a)
(b) 50 cm upstream and 50 cm downstream are far enough that the pressure recovers from its throat value, and the total $\Delta \mathrm{p}$ is the sum of Moody pipe loss and venturi head loss. First work out the pipe velocity, $\mathrm{V}=\mathrm{Q} / \mathrm{A}=(0.0137) /\left[(\pi / 4)(0.06)^{2}\right] \approx 4.85 \mathrm{~m} / \mathrm{s}$. Then

$$
\begin{aligned}
& \operatorname{Re}_{\mathrm{D}}=291000, \quad \frac{\varepsilon}{\mathrm{~d}}=0.000767 \text {, then } \mathrm{f}_{\text {Moody }} \approx 0.0196 ; \text { Fig. 6.44: } \mathrm{K}_{\text {venturi }} \approx 0.2 \\
& \text { Then } \quad \Delta \mathrm{p}=\Delta \mathrm{p}_{\text {Moody }}+\Delta \mathrm{p}_{\text {venturi }}=\frac{\rho \mathrm{V}^{2}}{2}\left(\mathrm{f} \frac{\mathrm{~L}}{\mathrm{~d}}+\mathrm{K}\right) \\
&=\frac{998(4.85)^{2}}{2}\left[0.0196\left(\frac{1.0}{0.06}\right)+0.2\right] \approx \mathbf{6 2 0 0} \text { Pa Ans. (b) }
\end{aligned}
$$

6.159 A modern venturi nozzle is tested in a laboratory flow with water at $20^{\circ} \mathrm{C}$. The pipe diameter is 5.5 cm , and the venturi throat diameter is 3.5 cm . The flow rate is measured by a weigh tank and the pressure drop by a water-mercury manometer. The mass flow rate and manometer readings are as follows:

| $\dot{m}, \mathrm{~kg} / \mathrm{s}:$ | 0.95 | 1.98 | 2.99 | 5.06 | 8.15 |
| :--- | :--- | :--- | :--- | :--- | :--- |
| $h, \mathrm{~mm}:$ | 3.7 | 15.9 | 36.2 | 102.4 | 264.4 |

Use these data to plot a calibration curve of venturi discharge coefficient versus Reynolds number. Compare with the accepted correlation, Eq. (6.116).

Solution: For water at $20^{\circ} \mathrm{C}$, take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The given data of mass flow and manometer height can readily be converted to discharge coefficient and Reynolds number:

$$
\begin{gathered}
\mathrm{Q}=\frac{\dot{\mathrm{m}}}{998}=\mathrm{C}_{\mathrm{d}}\left(\frac{\pi}{4}\right)(0.035)^{2} \sqrt{\frac{2(13.56-1) \rho_{\mathrm{w}}(9.81) \mathrm{h}}{\rho_{\mathrm{w}}\left[1-(3.5 / 5.5)^{4}\right]}}, \quad \text { or: } \quad \mathrm{C}_{\mathrm{d}} \approx \frac{\dot{\mathrm{~m}}(\mathrm{~kg} / \mathrm{s})}{16.485 \sqrt{ } \mathrm{~h}_{\text {meters }}} \\
\operatorname{Re}_{\mathrm{D}}=\frac{4 \dot{\mathrm{~m}}}{\pi \mu \mathrm{D}}=\frac{4 \dot{\mathrm{~m}}}{\pi(0.001)(0.055)} \approx 23150 \dot{\mathrm{~m}}(\mathrm{~kg} / \mathrm{s})
\end{gathered}
$$

The data can then be converted and tabulated as follows:

| h, m: | 0.037 | 0.0159 | 0.0362 | 0.1024 | 0.2644 |
| :--- | :--- | :--- | :--- | :--- | :--- |
| Cd: | 0.947 | 0.953 | 0.953 | 0.959 | 0.962 |
| ReD: | 22000 | 46000 | 69000 | 117000 | 189000 |

These data are plotted in the graph below, similar to Fig. 6.42 of the text:


They closely resemble the "classical Herschel venturi," but this data is actually for a modern venturi, for which we only know the value of Cd for $1.5 \mathrm{E} 5<\operatorname{ReD} \leq 2 \mathrm{E} 5$ :

Eq. (6.116) $\quad C_{d} \approx 0.9858-0.196\left(\frac{3.5}{5.5}\right)^{4.5} \approx \mathbf{0 . 9 6 0}$
The two data points near this Reynolds number range are quite close to $0.960 \pm 0.002$.
6.160 The butterfly-valve losses in Fig. $6.19 b$ may be viewed as a type of Bernoulli obstruction device, as in Fig. 6.39. The "throat area" $A_{t}$ in Eq. (6.104) can be interpreted as the two slivers of opening around the butterfly disk when viewed from upstream. First fit the average loss Kmean versus the opening angle in Fig. $6.19 b$ to an exponential curve. Then use your curve fit to compute the "discharge coefficient" of a butterfly valve as a function of the opening angle. Plot the


Fig. P6.160 results and compare them to those for a typical flow-meter.

Solution: The two "slivers" referred to are shown. The total sliver area equals total area reduced by a cosine factor:

$$
\mathrm{A}_{\text {sliver }}=\mathrm{A}_{\text {total }}(1-\cos \theta), \quad \text { where } \mathrm{A}_{\text {total }}=\pi \mathrm{R}^{2}, \mathrm{R}=\text { valve and pipe radius }
$$

The "effective" velocity passing through the slivers may be computed from continuity:

$$
\mathrm{V}_{\text {eff }}=\frac{\mathrm{Q}}{\mathrm{~A}_{\text {slivers }}}=\frac{\mathrm{Q}}{\mathrm{~A}_{\text {total }}} \frac{\mathrm{A}_{\text {total }}}{\mathrm{A}_{\text {slivers }}}=\frac{\mathrm{V}_{\text {pipe }}}{\mathrm{A}_{\mathrm{s}} / \mathrm{A}_{\mathrm{t}}}=\frac{\mathrm{V}_{\text {pipe }}}{(1-\cos \theta)}
$$

Then the problem suggests that the loss coefficients might correlate better (and not vary so much or be so large as in Fig. 6.19b) if the loss is based on effective velocity:

$$
\mathrm{K}_{\text {better }}=\frac{\mathrm{h}_{\text {loss }}}{\mathrm{V}_{\text {eff }}^{2} / 2 \mathrm{~g}}=\frac{\mathrm{h}_{\text {loss }}}{\mathrm{V}_{\text {pipe }}^{2} / 2 \mathrm{~g}}\left(\frac{\mathrm{~V}_{\text {pipe }}}{\mathrm{V}_{\text {eff }}}\right)^{2}=\mathrm{K}_{\text {Fig.6.19 }}(1-\cos \theta)^{2}
$$

So we take the data for traditional " $K$ " in Fig. 6.19b, multiply it by $(1-\cos \theta)^{2}$, and replot it below. Actually, we have taken three exponential curve-fits, one for each manufacturer's data shown in Fig. 6.19, to give an idea of the data uncertainty:

$$
\# 1: \mathrm{K}_{1} \approx 3500 \mathrm{e}^{-0.109 \theta} ; \quad 2: \mathrm{K}_{2} \approx 930 \mathrm{e}^{-0.091 \theta} ; \quad \# 3: \mathrm{K}_{3} \approx 2800 \mathrm{e}^{-0.112 \theta}, \quad \theta \text { in degrees }
$$

The calculations are made and are shown plotted below. This idea works fairly well, but the K's still vary a bit over the range of $\theta$. However, all K's are now of order unity, which is a better correlation than the huge variations shown in Fig. 6.19b.

6.161 Air flows at high speed through a Herschel venturi monitored by a mercury manometer, as shown in Fig. P6.161. The upstream conditions are 150 kPa and $80^{\circ} \mathrm{C}$. If $h=37 \mathrm{~cm}$, estimate the mass flow in $\mathrm{kg} / \mathrm{s}$. [HINT: The flow is compressible.]

Solution: The upstream density is $\rho 1=$ $p 1 /(R T)=(150000) /[287(273+80)]=1.48$ $\mathrm{kg} / \mathrm{m}^{3}$. The clue "high speed" means that we had better use the compressible venturi


Fig. P6.161 formula, Eq. (6.117):

$$
\dot{m}=C_{d} Y A_{t} \sqrt{\frac{2 \rho_{1}\left(p_{1}-p_{2}\right)}{1-\beta^{4}}} \text { where } \beta=4 / 6 \text { for this nozzle. }
$$

The pressure difference is measured by the mercury manometer:

$$
p_{1}-p_{2}=\left(\rho_{\text {merc }}-\rho_{\text {air }}\right) g h=\left(13550-1.48 \mathrm{~kg} / \mathrm{m}^{3}\right)\left(9.81 \mathrm{~m} / \mathrm{s}^{2}\right)(0.37 \mathrm{~m})=49200 \mathrm{~Pa}
$$

The pressure ratio is thus $(150-49.2) / 150=0.67$ and, for $\beta=2 / 3$, we read $Y \approx 0.76$ from Fig. 6.45. From Fig. 6.43 estimate $C d \approx 0.985$. The (compressible) venturi formula thus predicts:

$$
\dot{m}=0.985(0.76)\left[\frac{\pi}{4}(0.04 m)^{2}\right] \sqrt{\frac{2(1.48)(49200)}{1-(2 / 3)^{4}}}=\mathbf{0 . 4 0} \frac{\mathbf{k g}}{\mathbf{s}} \quad \text { Ans. }
$$

6.162 Modify Prob. 6.161 as follows. Find the manometer reading $h$ for which the mass flow through the venturi is approximately $0.4 \mathrm{~kg} / \mathrm{s}$. [HINT: The flow is compressible.]

Solution: This is, in fact, the answer to Prob. 6.161, but who knew? The present problem is intended as an iteration exercise, preferably with EES. We know the upstream pressure and density and the discharge coefficient, but we must iterate for $Y$ and $p 2$ in the basic formula:

$$
\dot{m}=C_{d} Y A_{t} \sqrt{\frac{2 \rho_{1}\left(p_{1}-p_{2}\right)}{1-\beta^{4}}}=0.40 \mathrm{~kg} / \mathrm{s}
$$

The answer should be $h=0.37 \mathrm{~m}$, as in Prob. 6.161, but the problem is extremely sensitive to the value of $h$. A $10 \%$ change in $h$ causes only a $2 \%$ change in mass flow.

The actual answer to Prob. 6.161 was a mass flow of $0.402 \mathrm{~kg} / \mathrm{s}$. EES reports that, for mass flow exactly equal to $0.400 \mathrm{~kg} / \mathrm{s}$, the required manometer height is $\boldsymbol{h}=\mathbf{0 . 3 6 1} \mathbf{~ m}$. Ans.

## FUNDAMENTALS OF ENGINEERING EXAM PROBLEMS: Answers

FE 6.1 In flow through a straight, smooth pipe, the diameter Reynolds number for transition to turbulence is generally taken to be
(a) 1500
(b) $\underline{2300}$
(c) 4000
(d) 250,000
(e) 500,000

FE 6.2 For flow of water at $20^{\circ} \mathrm{C}$ through a straight, smooth pipe at $0.06 \mathrm{~m}^{3} / \mathrm{h}$, the pipe diameter for which transition to turbulence occurs is approximately
(a) 1.0 cm
(b) 1.5 cm
(c) 2.0 cm
(d) 2.5 cm
(e) 3.0 cm

FE 6.3 For flow of oil $(\mu=0.1 \mathrm{~kg} /(\mathrm{m} \cdot \mathrm{s})$, $\mathrm{SG}=0.9$ ) through a long, straight, smooth 5-cmdiameter pipe at $14 \mathrm{~m}^{3} / \mathrm{h}$, the pressure drop per meter is approximately
(a) 2200 Pa
(b) 2500 Pa
(c) $10,000 \mathrm{~Pa}$
(d) 160 Pa
(e) 2800 Pa

FE 6.4 For flow of water at a Reynolds number of 1.03E6 through a 5-cm-diameter pipe of roughness height 0.5 mm , the approximate Moody friction factor is
(a) 0.012
(b) 0.018
(c) $\underline{0.038}$
(d) 0.049
(e) 0.102

FE 6.5 Minor losses through valves, fittings, bends, contractions etc. are commonly modeled as proportional to
(a) total head
(b) static head
(c) velocity head
(d) pressure drop
(e) velocity

FE 6.6 A smooth 8-cm-diameter pipe, 200 m long, connects two reservoirs, containing water at $20^{\circ} \mathrm{C}$, one of which has a surface elevation of 700 m and the other with its surface elevation at 560 m . If minor losses are neglected, the expected flow rate through the pipe is
(a) $0.048 \mathrm{~m}^{3} / \mathrm{h}$
(b) $2.87 \mathrm{~m}^{3} / \mathrm{h}$
(c) $134 \mathrm{~m}^{3} / \mathrm{h}$
(d) $172 \mathrm{~m}^{3} / \underline{h}$
(e) $385 \mathrm{~m}^{3} / \mathrm{h}$

FE 6.7 If, in Prob. FE 6.6 the pipe is rough and the actual flow rate is $90 \mathrm{~m}^{3} / \mathrm{hr}$, then the expected average roughness height of the pipe is approximately
(a) 1.0 mm
(b) 1.25 mm
(c) 1.5 mm
(d) 1.75 mm
(e) 2.0 mm

FE 6.8 Suppose in Prob. FE 6.6 the two reservoirs are connected, not by a pipe, but by a sharp-edged orifice of diameter 8 cm . Then the expected flow rate is approximately
(a) $90 \mathrm{~m}^{3} / \mathrm{h}$
(b) $579 \mathrm{~m}^{\underline{3}} / \underline{h}$
(c) $748 \mathrm{~m}^{3} / \mathrm{h}$
(d) $949 \mathrm{~m}^{3} / \mathrm{h}$
(e) $1048 \mathrm{~m}^{3} / \mathrm{h}$

FE 6.9 Oil $(\mu=0.1 \mathrm{~kg} /(\mathrm{m} \cdot \mathrm{s})$, $\mathrm{SG}=0.9$ ) flows through a $50-\mathrm{m}$-long smooth 8 -cmdiameter pipe. The maximum pressure drop for which laminar flow is expected is approximately
(a) 30 kPa
(b) 40 kPa
(c) 50 kPa
(d) 60 kPa
(e) 70 kPa

FE 6.10 Air at $20^{\circ} \mathrm{C}$ and approximately 1 atm flows through a smooth 30 -cm-square duct at 1500 cubic feet per minute. The expected pressure drop per meter of duct length is
(a) 1.0 Pa
(b) $\underline{2.0 \mathrm{~Pa}}$
(c) 3.0 Pa
(d) 4.0 Pa
(e) 5.0 Pa

FE 6.11 Water at $20^{\circ} \mathrm{C}$ flows at 3 cubic meters per hour through a sharp-edged $3-\mathrm{cm}-$ diameter orifice in a 6-cm-diameter pipe. Estimate the expected pressure drop across the orifice.
(a) 440 Pa
(b) 680 Pa
(c) 875 Pa
(d) 1750 Pa
(e) 1870 Pa

FE 6.12 Water flows through a straight $10-\mathrm{cm}$-diameter pipe at a diameter Reynolds number of 250,000 . If the pipe roughness is 0.06 mm , what is the approximate Moody friction factor?
(a) 0.015
(b) 0.017
(c) $\underline{0.019}$
(d) 0.026
(e) 0.032

FE 6.13 What is the hydraulic diameter of a rectangular air-ventilation duct whose cross-section is 1 meter by 25 cm ?
(a) 25 cm
(b) 40 cm
(c) 50 cm
(d) 75 cm
(e) 100 cm

FE 6.14 Water at $20^{\circ} \mathrm{C}$ flows through a pipe at $300 \mathrm{gal} / \mathrm{min}$ with a friction head loss of 45 ft . What is the power required to drive this flow?
(a) 0.16 kW
(b) 1.88 kW
(c) 2.54 kW
(d) 3.41 kW
(e) 4.24 kW

FE 6.15 Water at $20^{\circ} \mathrm{C}$ flows at $200 \mathrm{gal} / \mathrm{min}$ through a pipe 150 m long and 8 cm in diameter. If the friction head loss is 12 m , what is the Moody friction factor?
(a) 0.010
(b) 0.015
(c) $\underline{0.020}$
(d) 0.025
(e) 0.030

## COMPREHENSIVE PROBLEMS

C6.1 A pitot-static probe will be used to measure the velocity distribution in a water tunnel at $20^{\circ} \mathrm{C}$. The two pressure lines from the probe will be connected to a U-tube manometer which uses a liquid of specific gravity 1.7. The maximum velocity expected in the water tunnel is $2.3 \mathrm{~m} / \mathrm{s}$. Your job is to select an appropriate U-tube from a manufacturer which supplies manometers of heights $8,12,16,24$ and 36 inches. The cost increases significantly with manometer height. Which of these should you purchase?

Solution: The pitot-static tube formula relates velocity to the difference between stagnation pressure po and static pressure ps in the water flow:

$$
p_{o}-p_{s}=\frac{1}{2} \rho_{w} V^{2}, \quad \text { where } \rho_{w}=998 \frac{\mathrm{~kg}}{\mathrm{~m}^{3}} \quad \text { and } \quad V_{\max }=2.3 \frac{\mathrm{~m}}{\mathrm{~s}}
$$

Meanwhile, the manometer reading $h$ relates this pressure difference to the two fluids:

$$
\begin{gathered}
p_{o}-p_{s}=\left(\rho_{\operatorname{mano}}-\rho_{w}\right) g h=\rho_{w}\left(S G_{\operatorname{mano}}-1\right) g h \\
\text { Solve for } h_{\max }=\frac{V_{\max }^{2}}{2 g\left(S G_{\operatorname{mano}}-1\right)}=\frac{(2.3)^{2}}{2(9.81)(1.7-1)}=0.385 \mathrm{~m}=15.2 \mathrm{in}
\end{gathered}
$$

It would therefore be most economical to buy the 16-inch manometer. But be careful when you use it: a bit of overpressure will pop the manometer fluid out of the tube!

C6.2 A pump delivers a steady flow of water ( $\rho, \mu$ ) from a large tank to two other higher-elevation tanks, as shown. The same pipe of diameter $d$ and roughness $\varepsilon$ is used throughout. All minor losses except through the valve are neglected, and the partiallyclosed valve has a loss coefficient Kvalve. Turbulent flow may be assumed with all kinetic energy flux correction coefficients equal to 1.06. The pump net head $H$ is a known function of QA and hence also of VA = QA/Apipe, for example, $\mathrm{H}=\mathrm{a}-\mathrm{bV}_{\mathrm{A}}^{2}$, where $a$ and $b$ are constants. Subscript J refers to the junction point at the tee where branch A splits into B and C. Pipe length LC is much longer than LB. It is desired to predict the pressure at J , the three pipe velocities and friction factors, and the pump head. Thus there are 8 variables: H, VA, VB, VC, fA, fB, fC, pJ. Write down the eight equations needed to resolve this problem, but do not solve, since an elaborate iteration procedure, or an equation solver such as EES, would be required.

Solution: First, equation (1) is clearly the pump performance:

$$
\begin{equation*}
\mathrm{H}=\mathrm{a}-\mathrm{bV} \mathrm{~A}_{\mathrm{A}}^{2} \tag{1}
\end{equation*}
$$

$$
3 \text { Moody factors: } \begin{align*}
f_{A} & =f c n\left(V_{A}, \frac{\varepsilon}{d}\right)  \tag{2}\\
f_{B} & =f c n\left(V_{B}, \frac{\varepsilon}{d}\right)  \tag{3}\\
f_{C} & =f c n\left(V_{C}, \frac{\varepsilon}{d}\right) \tag{4}
\end{align*}
$$

Conservation of mass (constant area) at the junction J: VA $=\mathrm{VB}+\mathrm{VC}$
Finally, there are three independent steady-flow energy equations:

$$
\begin{align*}
& \text { (1) to (2): } z_{1}=z_{2}-H+f_{A} \frac{L_{A}}{d} \frac{V_{A}^{2}}{2 g}+f_{B} \frac{L_{B}}{d} \frac{V_{B}^{2}}{2 g}  \tag{6}\\
& \text { (1) to (3): } z_{1}=z_{3}-H+f_{A} \frac{L_{A}}{d} \frac{V_{A}^{2}}{2 g}+f_{C} \frac{L_{C}}{d} \frac{V_{C}^{2}}{2 g}+K_{\text {valve }} \frac{V_{C}^{2}}{2 g}  \tag{7}\\
& \text { (J) to (2): } \frac{p_{J}}{\rho g}+z_{J}=\frac{p_{a t m}}{\rho g}+z_{2}+f_{B} \frac{L_{B}}{d} \frac{V_{B}^{2}}{2 g}
\end{align*}
$$



Fig. PC6. 2

C6.3 The water slide in the figure is to be installed in a swimming pool. The manufacturer recommends a continuous water flow of $1.39 \mathrm{E}-3 \mathrm{~m}^{3} / \mathrm{s}$ (about $22 \mathrm{gal} / \mathrm{min}$ ) down the slide to ensure that customers do not burn their bottoms. An 80\%-efficient pump under the slide, submerged 1 m below the water surface, feeds a 5 -m-long, 4 -cmdiameter hose, of roughness 0.008 cm , to the slide. The hose discharges the water at the top of the slide, 4 m above the water surface, as a free jet. Ignore minor losses and assume $\alpha=1.06$. Find the brake horsepower needed to drive the pump.


Fig. PC6. 3
Solution: For water take $\rho=998 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.001 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. Write the steady-flow energy equation from the water surface (1) to the outlet (2) at the top of the slide:

$$
\begin{aligned}
\frac{p_{a}}{\rho g}+\frac{\alpha_{1} V_{1}^{2}}{2 g}+z_{1}= & \frac{p_{a}}{\rho g}+\frac{\alpha_{2} V_{2}^{2}}{2 g}+z_{2}+h_{f}-h_{\text {pump }}, \quad \text { where } V_{2}=\frac{1.39 E-3}{\pi(0.02)^{2}}=1.106 \frac{\mathrm{~m}}{\mathrm{~s}} \\
& \text { Solve for } h_{\text {pump }}=\left(z_{2}-z_{1}\right)+\frac{V_{2}^{2}}{2 g}\left(\alpha_{2}+f \frac{L}{d}\right)
\end{aligned}
$$

Work out Red $=\rho \mathrm{Vd} / \mu=(998)(1.106)(0.04) / 0.001=44200, \varepsilon / d=0.008 / 4=0.002$, whence $f$ Moody $=0.0268$. Use these numbers to evaluate the pump head above:

$$
h_{\text {pump }}=(5.0-1.0)+\frac{(1.106)^{2}}{2(9.81)}\left[1.06+0.0268\left(\frac{5.0}{0.04}\right)\right]=4.27 \mathrm{~m}
$$

whence $\quad \mathbf{B H P}_{\text {required }}=\frac{\rho g Q h_{\text {pump }}}{\eta}=\frac{998(9.81)(1.39 E-3)(4.27)}{0.8}=\mathbf{7 3}$ watts Ans.

C6.4 Suppose you build a house out in the 'boonies,' where you need to run a pipe to the nearest water supply, which fortunately is about 1 km above the elevation of your house. The gage pressure at the water supply is 1 MPa . You require a minimum of $3 \mathrm{gal} / \mathrm{min}$ when your end of the pipe is open to the atmosphere. To minimize cost, you want to buy the smallest possible diameter pipe with an extremely smooth surface.
(a) Find the total head loss from pipe inlet to exit, neglecting minor losses.
(b) Which is more important to this problem, the head loss due to elevation difference, or the head loss due to pressure drop in the pipe?
(c) Find the minimum required pipe diameter.

Solution: Convert $3.0 \mathrm{gal} / \mathrm{min}$ to $1.89 \mathrm{E}-4 \mathrm{~m}^{3} / \mathrm{s}$. Let 1 be the inlet and 2 be the outlet and write the steady-flow energy equation:


Fig. $\mathbf{C 6 . 4}$

$$
\frac{p_{1 \text { gage }}}{\rho g}+\frac{\alpha_{1} V_{y}^{2}}{2 g}+z_{1}=\frac{p_{2 g q e}}{\rho g}+\frac{\alpha_{2} V_{2}^{2}}{2 g}+z_{2}+h_{f}
$$

$$
\text { or: } \quad h_{f}=z_{1}-z_{2}+\frac{p_{1 \text { gage }}}{\rho g}=1000 \mathrm{~m}+\frac{1 E 6 \mathrm{kPa}}{998(9.81)}=1000+102=1102 \mathrm{~m} \quad \text { Ans. (a) }
$$

(b) Thus, elevation drop of 1000 m is more important to head loss than $\Delta \mathrm{p} / \rho \mathrm{g}=102 \mathrm{~m}$.
(c) To find the minimum diameter, iterate between flow rate and the Moody chart:

$$
\begin{gathered}
h_{f}=f \frac{L}{d} \frac{V^{2}}{2 g}, \quad L=6000 \mathrm{~m}, \quad \frac{1}{\sqrt{f}}=-2 \log \left(\frac{2.51}{\operatorname{Re} \sqrt{f}}\right), \quad V=\frac{Q}{\pi d^{2} / 4} \\
Q=1.89 \mathrm{E}-4 \frac{\mathrm{~m}^{3}}{\mathrm{~s}}, \quad \operatorname{Re}=\frac{V d}{v}
\end{gathered}
$$

We are given $\mathrm{hf}=1102 \mathrm{~m}$ and $\nu_{\text {water }}=1.005 \mathrm{E}-6 \mathrm{~m}^{2} / \mathrm{s}$. We can iterate, if necessary, or use EES, which can swiftly arrive at the final result:

$$
\mathrm{f}_{\text {smooth }}=0.0266 ; \quad \mathrm{Re}=17924 ; \quad \mathrm{V}=1.346 \mathrm{~m} / \mathrm{s} ; \quad \mathbf{d}_{\min }=\mathbf{0 . 0 1 3 4} \mathbf{~ m} \quad \text { Ans. (c) }
$$

C6.5 Water at $20^{\circ} \mathrm{C}$ flows, at the same flow rate $Q=9.4 \mathrm{E}-4 \mathrm{~m}^{3} / \mathrm{s}$, through two ducts, one a round pipe, and one an annulus, as shown. The cross-section area $A$ of each duct is identical, and each has walls of commercial steel. Both are the same length. In the crosssections shown, $R=15 \mathrm{~mm}$ and $a=25 \mathrm{~mm}$.
(a) Calculate the correct radius $b$ for the annulus.
(b) Compare head loss per unit length for the two ducts, first using the hydraulic diameter and second using the 'effective diameter' concept.
(c) If the losses are different, why? Which duct is more 'efficient'? Why?


Fig. C6.5

Solution: (a) Set the areas equal:

$$
A=\pi R^{2}=\pi\left(a^{2}-b^{2}\right), \quad \text { or: } \quad b=\sqrt{a^{2}-R^{2}}=\sqrt{(25)^{2}-(15)^{2}}=20 \mathrm{~mm} \quad \text { Ans. (a) }
$$

(b) Find the round-pipe head loss, assuming $v=1.005 \mathrm{E}-6 \mathrm{~m}^{2} / \mathrm{s}$ :

$$
\begin{gathered}
V=\frac{Q}{A}=\frac{9.4 E-4 \mathrm{~m}^{3} / \mathrm{s}}{\pi(0.015 \mathrm{~m})^{2}}=1.33 \frac{\mathrm{~m}}{\mathrm{~s}} ; \quad \operatorname{Re}=\frac{(1.33)(0.030)}{1.005 E-6}=39700 ; \\
\frac{\varepsilon}{d}=0.00153, f_{\text {Moody }}=0.0261
\end{gathered}
$$

Thus hf/L $=(\mathrm{f} / \mathrm{d})\left(\mathrm{V}^{2} / 2 \mathrm{~g}\right)=(0.0261 / 0.03)\left(1.33^{2}\right) / 2 / 9.81=\mathbf{0 . 0 7 8 5} \quad$ (round) Ans. (b)
Annulus: $\mathrm{Dh}=4 \mathrm{~A} / \mathrm{P}=2(\mathrm{a}-\mathrm{b})=20 \mathrm{~mm}$, same $\mathrm{V}=1.33 \mathrm{~m} / \mathrm{s}$ :

$$
\begin{gathered}
\operatorname{Re}_{D h}=\frac{V D_{h}}{v}=26500, \frac{\varepsilon}{D_{h}}=0.0023, f_{\text {Moody }}=0.0291, \\
h_{f} / L \approx\left(\frac{f}{D_{h}} \frac{V^{2}}{2 g}\right) \approx \mathbf{0 . 1 3 1} \quad \text { (annulus) Ans. (b) }
\end{gathered}
$$

Effective-diameter concept: $\mathrm{b} / \mathrm{a}=0.8$, Table 6.3: $\mathrm{Deff}=0.667 \mathrm{Dh}=13.3 \mathrm{~mm}$. Then

$$
\begin{aligned}
& \operatorname{Re}_{\text {Deff }}=17700, \frac{\varepsilon}{D_{\text {eff }}}=0.00345, f_{\text {Moody }}=0.0327, \\
& \frac{h_{f}}{L}=\frac{f}{D_{h}} \frac{V^{2}}{2 g}=\mathbf{0 . 1 4 7} \quad \text { (annulus- } D_{\text {eff }} \text { ) Ans. (b) }
\end{aligned}
$$

NOTE: Everything here uses Deff except hf, which by definition uses Dh!
We see that the annulus has about $85 \%$ more head loss than the round pipe, for the same area and flow rate! This is because the annulus has more wall area, thus more friction. Ans. (c)

C6.6 John Laufer (NACA Tech. Rep. 1174, 1954) gave velocity data for $20^{\circ} \mathrm{C}$ airflow in a smooth 24.7 -cm-diameter pipe at $\mathrm{Re} \approx 5 \mathrm{E} 5$ :

| u/uCL: | 1.0 | 0.997 | 0.988 | 0.959 | 0.908 | 0.847 | 0.818 | 0.771 | 0.690 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $r / R:$ | 0.0 | 0.102 | 0.206 | 0.412 | 0.617 | 0.784 | 0.846 | 0.907 | 0.963 |

The centerline velocity $u C L$ was $30.5 \mathrm{~m} / \mathrm{s}$. Determine (a) the average velocity by numerical integration and (b) the wall shear stress from the log-law approximation. Compare with the Moody chart and with Eq. (6.43).

Solution: For air at $20^{\circ} \mathrm{C}$, take $\rho=1.2 \mathrm{~kg} / \mathrm{m}^{3}$ and $\mu=0.00018 \mathrm{~kg} / \mathrm{m} \cdot \mathrm{s}$. The average velocity is defined by the (dimensionless) integral

$$
\mathrm{V}=\frac{1}{\pi \mathrm{R}^{2}} \int_{0}^{\mathrm{R}} \mathrm{u}(2 \pi \mathrm{r}) \mathrm{dr}, \quad \text { or: } \quad \frac{\mathrm{V}}{\mathrm{u}_{\mathrm{CL}}}=\int_{0}^{1} \frac{\mathrm{u}}{\mathrm{u}_{\mathrm{CL}}} 2 \eta \mathrm{~d} \eta \text {, where } \eta=\frac{\mathrm{r}}{\mathrm{R}}
$$

Prepare a spreadsheet with the data and carry out the integration by the trapezoidal rule:

$$
\int_{0}^{1} \frac{\mathrm{u}}{\mathrm{u}_{\mathrm{c}}} 2 \eta \mathrm{~d} \eta \approx\left[\left(\mathrm{u} / \mathrm{u}_{\mathrm{c}}\right)_{2} \eta_{2}+\left(\mathrm{u} / \mathrm{u}_{\mathrm{c}}\right)_{1} \eta_{1}\right]\left(\eta_{2}-\eta_{1}\right)+\left[\left(\mathrm{u} / \mathrm{u}_{\mathrm{c}}\right)_{3} \eta_{3}+\left(\mathrm{u} / \mathrm{u}_{\mathrm{c}}\right)_{2} \eta_{2}\right]\left(\eta_{3}-\eta_{2}\right)+\cdots
$$

The integral is evaluated on the spreadsheet below. The result is V/uCL $\approx 0.8356$,

$$
\text { or } \quad \mathrm{V} \approx(0.8356)(30.5) \approx 25.5 \mathrm{~m} / \mathrm{s} \text {. Ans. (a) }
$$

The wall shear stress is estimated by fitting the log-law (6.28) to each data point:

$$
\text { For each }(\mathrm{u}, \mathrm{y}), \frac{\mathrm{u}}{\mathrm{u}^{*}} \approx \frac{1}{\kappa} \ln \left(\frac{\mathrm{yu}^{*}}{v}\right)+\mathrm{B}, \quad \kappa \approx 0.41 \text { and } \mathrm{B} \approx 5.0
$$

We know $v$ for air and are given $u$ and $y$ from the data, hence we can solve for $u^{*}$. The spreadsheet gives $\mathrm{u}^{*} \approx 1.1 \mathrm{~m} / \mathrm{s} \pm 1 \%$, or $\tau \mathrm{w}=\rho \mathrm{u}^{*}=(1.2)(1.1)^{2} \approx \mathbf{1 . 4 5}$ Pa. Ans. (b)

| $\mathbf{y} / \mathbf{R}$ | $\mathbf{r} / \mathbf{R}$ | $\mathbf{u} / \mathbf{u C L}$ | $\int_{\mathbf{u} / \mathbf{u C L} \mathbf{2} \boldsymbol{\pi} \mathbf{r} / \mathbf{R ~ d r} / \mathbf{R}}$ | $\mathbf{u}^{*}$ |
| :---: | :---: | :---: | :---: | :---: |
| 1.000 | 0.000 | 1.000 | .0000 | - |
| 0.898 | 0.102 | 0.997 | .0104 | 1.126 |
| 0.794 | 0.206 | 0.988 | .0421 | 1.128 |
| 0.588 | 0.412 | 0.959 | .1654 | 1.126 |
| 0.383 | 0.617 | 0.908 | .3613 | 1.112 |
| 0.216 | 0.784 | 0.847 | .5657 | 1.099 |
| 0.154 | 0.846 | 0.818 | .6498 | 1.101 |
| 0.093 | 0.907 | 0.771 | .7347 | 1.098 |
| 0.037 | 0.963 | 0.690 | .8111 | 1.097 |
| 0.000 | 1.000 | 0.000 | $\mathbf{. 8 3 5 6}$ | - |

We make similar estimates from the Moody chart by evaluating Re and $f$ and iterating:

$$
\begin{aligned}
& \text { Guess } \mathrm{V} \approx 25 \mathrm{~m} / \mathrm{s}, \quad \text { then } \operatorname{Re}=\frac{1.2(25)(0.247)}{0.00018} \approx 412000, \mathrm{f}_{\text {smooth }} \approx 0.0136 \\
& \mathrm{~V}_{\text {better }}=\mathrm{u}_{\mathrm{CL}} /[1+1.3 \sqrt{ } \mathrm{f}] \approx 26.5, \quad \text { whence } \mathrm{Re} \approx 436000, \mathrm{f}_{\text {better }} \approx 0.0135
\end{aligned}
$$

This converges to $\quad \mathbf{V} \approx 26.5 \mathrm{~m} / \mathrm{s}$ Ans. and $\tau_{\mathbf{w}}=(\mathrm{f} / 8) \rho \mathrm{V}^{2} \approx \mathbf{1 . 4 2 ~ P a . ~ A n s . ~}$

C6.7 Consider energy exchange in fully-developed laminar flow between parallel plates, as in Eq. (6.63). Let the pressure drop over a length $L$ be $\Delta p$. Calculate the rate of work done by this pressure drop on the fluid in the region ( $0<x<L$, $-h<y<+h$ ) and compare with the integrated energy dissipated due to the viscous function $\Phi$ from Eq. (4.50) over this same region. The two should be equal. Explain why this is so. Can you relate the viscous drag force and the wall shear stress to this energy result?

Solution: From Eq. (6.63), the velocity profile between the plates is parabolic:

$$
u=\frac{3}{2} V\left(1-\frac{y^{2}}{h^{2}}\right) \quad \text { where } V=\frac{h^{2}}{3 \mu} \frac{\Delta p}{L} \text { is the average velocity }
$$

Let the width of the flow be denoted by $b$. The work done by pressure drop $\Delta p$ is:

$$
\dot{W}_{\text {pressure }}=\Delta p V A=\left(\frac{3 \mu L V}{h^{2}}\right)(V)(2 h b)=\frac{6 \mu L b V^{2}}{h}
$$

Meanwhile, from Eq. (4.50), the viscous dissipation function for this fully-developed flow is:

$$
\Phi=\mu\left(\frac{\partial u}{\partial y}\right)^{2}=\mu\left(\frac{3 V y}{h^{2}}\right)^{2}=\frac{9 \mu V^{2} y^{2}}{h^{4}}
$$

Integrate this to get the total dissipated energy over the entire flow region of dimensions $L$ by $b$ by $2 h$ :

$$
\dot{E}_{\text {dissipated }}=L b \int_{-h}^{+h}\left(\frac{9 \mu V^{2} y^{2}}{h^{4}}\right) d y=\frac{6 \mu L b V^{2}}{h}=\dot{W}_{\text {pressure }}!\text { Ans. }
$$

The two energy terms are equal. There is no work done by the wall shear stresses (where $u=0$ ), so the pressure work is entirely absorbed by viscous dissipation within the flow field. Ans.

C6.8 This text has presented the traditional correlations for turbulent smooth-wall friction factor, Eq. (6.38), and the law-of-the-wall, Eq. (6.28). Recently, groups at Princeton and Oregon [56] have made new friction measurements and suggest the following smooth-wall friction law:

$$
\frac{1}{\sqrt{f}}=1.930 \log _{10}\left(\operatorname{Re}_{D} \sqrt{f}\right)-0.537
$$

In earlier work, they also report that better values for the constants $\kappa$ and $B$ in the log-law, Eq. (6.28), are $\kappa \approx 0.421 \pm 0.002$ and $B \approx 5.62 \pm 0.08$. (a) Calculate a few values of $f$ in the range $1 \mathrm{E} 4 \leq \mathrm{Re}_{D} \leq 1 \mathrm{E} 8$ and see how the two formulas differ. (b) Read Ref. 56 and briefly check the five papers in its bibliography. Report to the class on the general results of this work.

Solution: The two formulas are practically identical except as the Reynolds number is very high or very low. The new formula was fit to new, and extensive, friction data in Ref. 56 and can thus be said to be slightly more accurate. Here is a table of calculations.

| $\mathrm{Re}_{D}$ | $f_{\text {Prandtl }}$ | $f$ Ref.56 | Difference |
| :--- | :---: | :--- | :---: |
| 3000 | 0.04353 | 0.04251 | $-2.41 \%$ |
| 10000 | 0.03089 | 0.0305 | $-1.10 \%$ |
| 30000 | 0.02349 | 0.02344 | $-0.18 \%$ |
| 100000 | 0.01799 | 0.01811 | $0.62 \%$ |
| 300000 | 0.01447 | 0.01464 | $1.22 \%$ |
| 1000000 | 0.01165 | 0.01186 | $1.76 \%$ |
| $3.0 \mathrm{E}+06$ | 0.009722 | 0.009938 | $2.17 \%$ |
| $1.0 \mathrm{E}+07$ | 0.008104 | 0.008316 | $2.56 \%$ |
| $3.0 \mathrm{E}+07$ | 0.006949 | 0.007153 | $2.86 \%$ |
| $1.0 \mathrm{E}+08$ | 0.005941 | 0.006134 | $3.15 \%$ |

They differ by no more than $3 \%$.

