## ADDENDUM NO. 3

DATE:	October 31, 2019			
Project:	Terre Haute First Baptist Church Terre Haute, Indiana			
ARCHITECT:	Dauss Architects 227 W 11 <sup>th</sup> Street Anderson, IN 46016 Tel: 765.649.2258 Fax: 765.649.2337			
Email:	info@daussarchitects.com			

This Addendum is hereby incorporated into the Bidding Documents. Bidders shall prepare their bids to reflect the modifications and/or interpretations itemized herein. Attachments are to be incorporated as noted.

#### ITEMS

#### GENERAL INFORMATION

3.1. The bid time for this project has been extended to 4:00pm, Thursday, November 7. All other requirements of the bid are as specified or per previous addendum.

#### DRAWING ITEMS

- 3.2. Drawing A205: Change WT-1 to be Daltile "Balans" or approved equal in sizes as shown.
- 3.3. Drawing A206: Revised and forwarded to the plan holders on October 23. The revision included the hardware schedule omitted from the original bid sets. A copy is attached to this addenda.
- 3.4. Drawing A308: Attached as revised to clarify wall tile nomenclature and toilet accesories.
- 3.5. Drawing A309: Added toilet accessory schedule.

#### SUBMITTED QUESTIONS

- 3.6. Can we use PVC for sanitary vent if we fire wrap it in the plenum space? *Fire wrap will not be permitted for this type of application.*
- 3.7. Will Copper pro-press be acceptable for the domestic hot and cold water? Pro-press may be bid as a voluntary alternate.

3.8. Can you clarify the location of the SIPS panels without the Tectum?

It's easier to tell you where the panels are with Tectum and everything else is without. The tectum goes over the gymnasium, the storage room attached to the gymnasium and over the youth activities room. The eastern portion of the youth activities room has a double deck – Tectum on the the top of the youth activities room and then non-tectum up at the building roof line. Drawing 2/A502 shows that area.

- 3.9. Can you clarify the casework and countertop materials for the serving room and the restrooms. All cabinetry is to be premanufactured wood, wood veneered. All toilet room supports for countertops are to be laminate. All countertops are to be solid surface with the exception of the countertop in the mezzanine booth which is to be laminate.
- 3.10. Do the showers have a fiberglass insert to attach the SH-1 shower valve detail or are they brick to attach the SH-1 shower valve to? *The walls are tile.*
- 3.11. Toilet partitions specs call for either plastic laminate or solid phenolic; where indicated on drawings-it's not. Addenda #2 approved Scranton- which is Solid plastic. *Toilet partitions may be solid phenolic or solid plastic as approved.*
- 3.12. There are no fire extinguishers/cabinets shown. *Provide three (3) cabinets to be installed where directed by the Architect.*

### TERRE HAUTE FIRST BAPTIST CHURCH FAMILY LIFE CENTER TERRE HAUTE, INDIANA

3.13. Room 103. Is there any trim on the walls on the East & West ends? *There should be vinyl trim all around the room.* 

3.14. Are there solid surface sills in Rooms 104, 105, 107, 108, 121? All window sills are to be solid surface.

- 3.15. The specification section for Resilient Flooring (09650) calls for the resilient base to be Johnsonite TightLock. The material legend calls for Johnsonite Duracove. *The Duracove product is to be used.*
- 3.16. 04220 2.01 5 calls for Ground Face CMU by Kirchner Block. Do you know where these are located as I do not see them anywhere on the wall type plan or drawings. *These are not utilized on this project.*
- 3.17. Elevation A402/2 illustrates the veneer stopping 4' off the corner. But the floor plan shows what appears to be veneer the entire length of CL5 and turning the corner a few feet on CL2 and stopping. I believe the veneer needs to be ran down CL K but want to confirm.

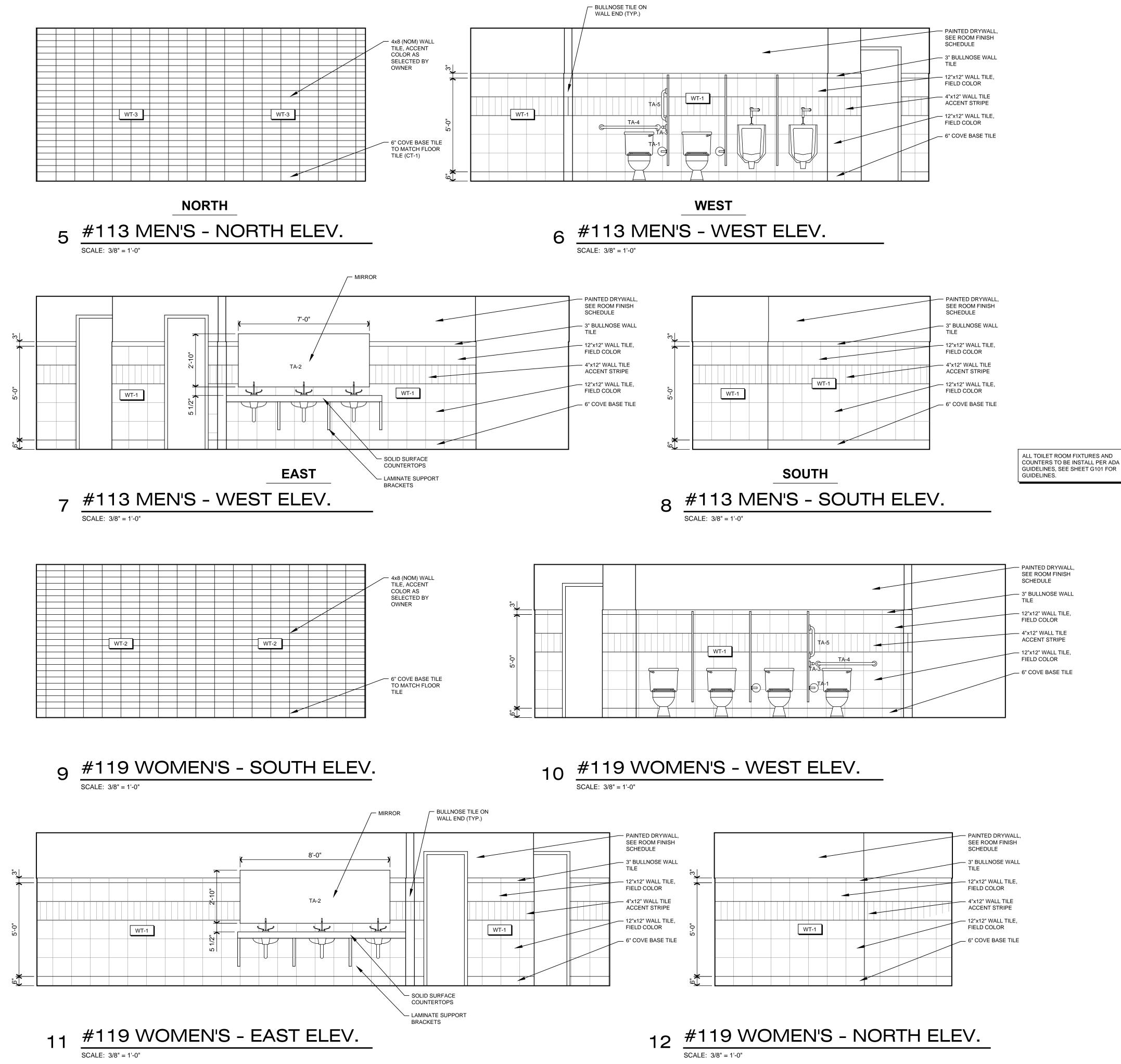
The elevation 2/A402 is correct. The veneer stops 4' past the southeast corner along the east face.

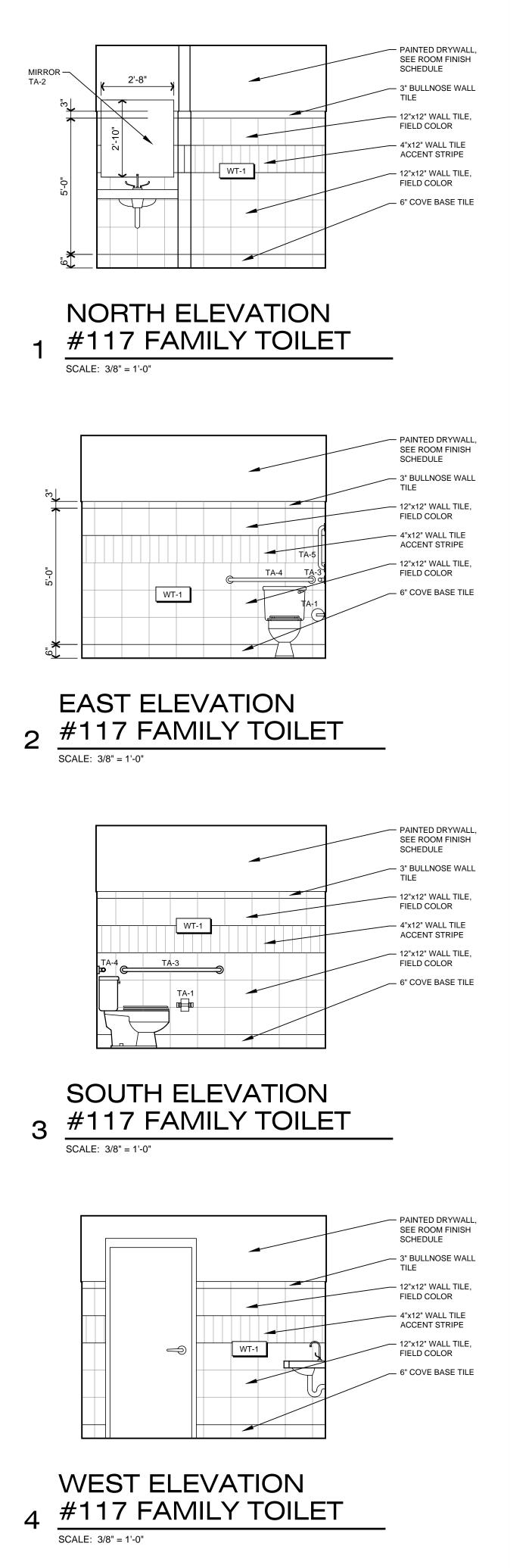
3.18. I assume we only need to carry the insulation in the cavity between the CMU and veneer. All other insulation will be supplied by the PEMB contractor. Is this correct? *that is correct regarding the insulation in the masonry wall.* All other wall insulation is by the general contractor in the stud wall cavity. The metal panels are not insulated.

## ATTACHEMENTS

- A206 Schedules
- A308 Toilet Room Elevations
- A309 Interior Elevations Miscellaneous
- Structural Set

### END OF ADDENDUM NO. 3

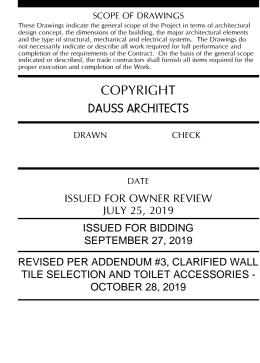






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ER S Church Street CENTI Indiana UU ≺ U + oplai Haute, Ū. apti Δ Terre Μ First B 4701 FAMIL No. 4295 STATE OF NDIANA PCHITE Carrent to SHEET TITLE INTERIOR **ELEVATIONS TOILET ROOMS** SHEET NUMBER A308

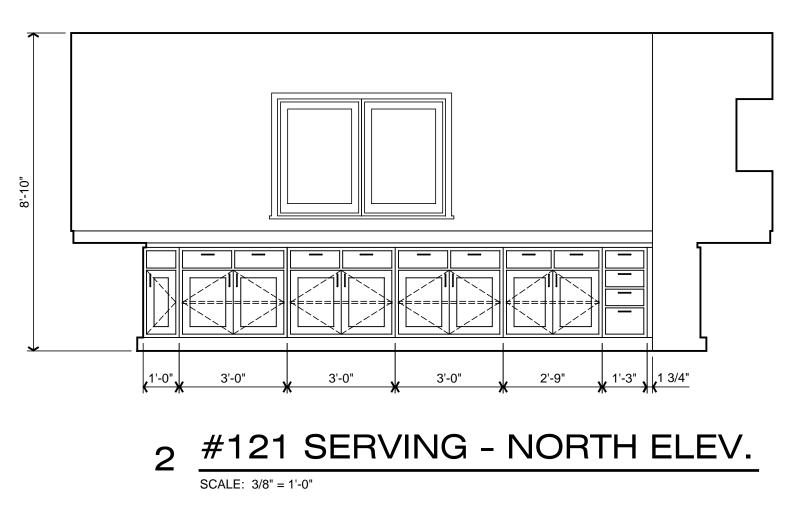
OF

SHEET

PROJECT NUMBER







MARK	
TA-1	SURFACE
TA-2	STAINLESS
TA-3	STAINLESS
TA-4	STAINLESS
TA-5	STAINLESS







TOILET ROOM ACCESSORIES							
DESCRIPTION	MFR.	MODEL NO.	REMARKS				
E MTD. TOILET PAPER HOLDER	BOBRICK	B-6867	MIN. 19" A.F.F. TO CENTERLINE				
ESS STEEL FRAMED MIRROR			40" MAX. TO REFLECTIVE SURFACE				
ESS STEEL GRAB BAR	BOBRICK	B-6806.99 x 42	34 1/2" A.F.F. TO CENTERLINE				
ESS STEEL GRAB BAR	BOBRICK	B-6806.99 x 36	34 1/2" A.F.F. TO CENTERLINE				
ESS STEEL GRAB BAR	BOBRICK	B-6806.99 x 18	40" A.F.F. TO BOTTOM				

AINLESS STEEL GRAB BAR	BOBRICK	B-6806.99 x 18	40" A.F.F. TO BOTTOM
AINLESS STEEL GRAB BAR	BOBRICK	B-6806.99 x 36	34 1/2" A.F.F. TO CENTE
AINLESS STEEL GRAB BAR	BOBRICK	B-6806.99 x 42	34 1/2" A.F.F. TO CENTE
AINLESS STEEL FRAMED MIRROR			40" MAX. TO REFLECTIV

## TOILET ROOM ACC. **GENERAL NOTES**

TOILET ROOM ACC. **REMARK NOTES** 

EXPOSED SUPPLY AND WASTE PIPES AT LAVATORIES TO RECEIVE MOLDED LAVATORY INSULATION - TRUEBRO, INC., HANDI LAV-GUARD, (COLOR - WHITE)
MANUFACTURER AND MODEL NUMBERS PROVIDED ARE BY BOBRICK, MANUFACTURERS ACCEPTED ON AN AS EQUAL BASIS.

1. MOUNTING HEIGHTS SHOWN ARE FOR A.D.A. COMPLIANT TOILET ROOMS.



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SCOPE OF DRAWINGS These Drawings indicate the general scope of the Project in terms of architectural design concept, the dimensions of the building, the major architectural elements and the type of structural, mechanical and electrical systems. The Drawings do not necessarily indicate or describe all work required for full performance and completion of the requirements of the Contract. On the basis of the general scope indicated or described, the trade contractors shall furnish all items required for the proper execution and completion of the Work.					
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	WNER REVIEW 5, 2019				
	DR BIDDING ER 27, 2019				
	NDUM #3, CLARIFIED S - OCTOBER 28, 2019				



# DOOR HARDWARE

Hardware Set # 1 (Aluminum Entrance Doors)

#### 2 ea. Continuous Hinge 2 ea. Exit Device 2 ea. Door Pulls 2 ea. Door Closer 1 ea. Cylinder 1 ea. Threshold 1 ea Weathering Kit

## Hardware Set # 2 (Aluminum Entrance Doors)

2 ea. Continuous Hinge 2 ea. Exit Device

2 ea. Door Pulls 2 ea. Door Closer 1 ea. ADA Auto Operator (right hand leaf only) 1 ea. Actuators

1 ea. Key Switch 1 ea. Cylinder 1 ea. Threshold 1 ea Weathering Kit

Hardware Set # 3 (Aluminum Entrance Lobby Doors) 2 ea. Continuous Hinge 2 ea. Door Push/Pulls

2 ea. Door Closer

## Hardware Set # 4 (Aluminum Entrance Lobby Doors)

2 ea. Continuous Hinge 2 ea. Door Push/Pulls 2 ea. Door Closer 1 ea. ADA Auto Operator (right hand leaf only) 1 ea. Actuators 1 ea. Key Switch

## Hardware Set # 5 (Wood Gym Entrance Doors)

6 ea. Heavy-duty Hinges 2 ea. Door Closer 2 ea. Exit Device w/ lever handle trim 2 ea. Kickplates interior side only 2 ea. Silencers

Note: Doors have 90 minute label.

## Hardware Set # 6 (Gym Exterior Doors)

6 ea. Heavy-duty Hinges 2 ea. Door Closer 2 ea. Exit Device w/ blank exterior trim 1 ea. Cylinders 2 ea. Kickplates interior side only 2 ea. ADA Threshold 1 ea Weathering Kit

Hardware Set # 7 (Classroo	oms)
3 ea. Hinges	

1 ea. Lockset - Entry funtion 1 ea. Wall Stop

3 ea. Silencers

Hardware Set # 8 (Youth - Exit) 3 ea. Heavy-duty Hinges

1 ea. Door Closer 1 ea. Exit Device w/ blank exterior trim

1 ea. Cylinders 1 ea. ADA Threshold 1 ea Weathering Kit

Hardware Set # 9 (Interior Aluminum Corridor

Doors) 2 ea. Continuous Hinge

- 2 ea. Door Push/Pulls 1 ea. Door Closer 1 ea. ADA Auto Operator (right hand leaf only)
- 1 ea. Actuators 1ea. Cylinder
- 1 ea. Key Switch

Hardware Set # 10 (HM Fire-Wall Door)

6 ea. Heavy-duty Hinges 2 ea. Door Closer 2 ea. Exit Device w/ lever handle trim 1ea. Cylinder 2 ea. Kickplates 2 ea. Silencers

Note: Doors have 90 minute label.

Hardware Set # 11 (Closets)

3 ea. Hinges 1 ea. Lockset - Storeroom funtion

Hardware Set # 12 (Toilet Rooms)

3 ea. Hinges 1 ea. Push Plate
1 ea. Pull Plate
1 ea. Closer
1 ea. Kickplate
1 ea. Wall Stop
3 ea. Silencers

3 ea. Silencers

Hardware Set # 13 (FR Toilet Rooms) 3 ea. Hinges 1ea. Lockset - Passage 1 ea. Closer 1 ea. Kickplate 1 ea. Wall Stop 3 ea. Silencers

Hardware Set # 14 (HM Exit)

Note: Doors have 90 minute label.

3 ea. Heavy-duty Hinges 1 ea. Door Closer 1 ea. Exit Device w/ blank exterior trim 1 ea. Kickplates interior side only 1 ea. Threshold 1ea. Peep hole 1ea. Weathering Kit 1 ea. Silencers

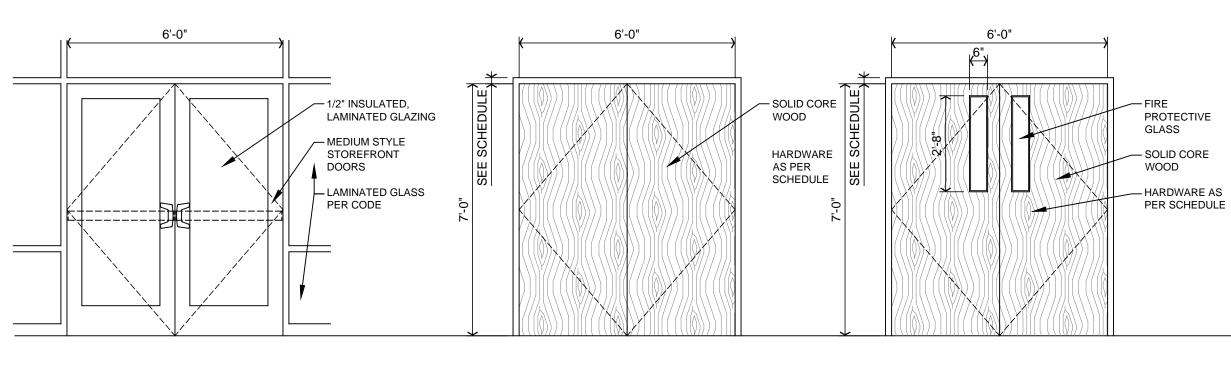
Hardware Set # 15 (Closet) 6 ea. Hinges 1ea. Lockset - Passage 1 ea. Flush bolts (top and bottom)

3 ea. Silencers

3 ea. Silencers

Hardware Set # 16 (2nd Floor Stair) 3 ea. Hinges 1ea. Lockset - Entry 1 ea. Closer 1 ea. Kickplate 1 ea. Wall Stop

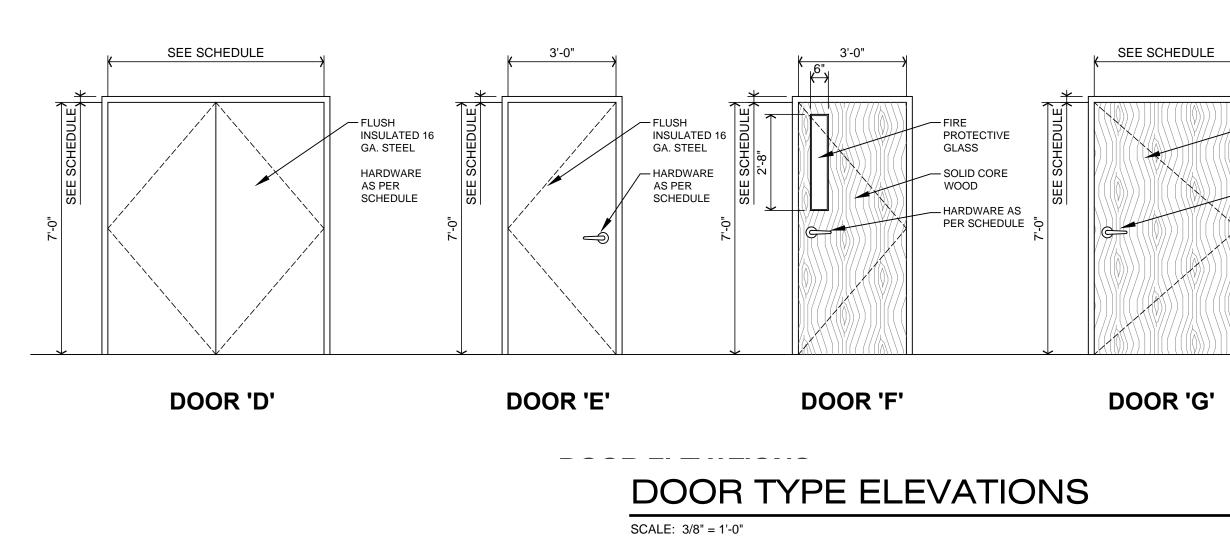
DOOR SCHEDULE											
						FRAME					
NO.	ELEV.	SIZE	MAT.	FIN.	MATR.	FIN.	JAMB DEPTH	HDWR SET	LABEL	REMARKS	NO.
101a	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	1		MEDIUM STYLE	101a
101b	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	2		MEDIUM STYLE	101b
101c	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	3		MEDIUM STYLE	101c
101d	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	4		MEDIUM STYLE	101d
103a	С	PR 3/0x7/0	S.C. WOOD	STAIN	H.M.	PAINT	8 3/4"	5	90 MIN.	4" HEAD	103a
103b	С	PR 3/0x7/0	S.C. WOOD	STAIN	H.M.	PAINT	8 3/4"	5	90 MIN.	4" HEAD	103b
103c	D	PR 3/0x7/0	INSUL. STL.	PAINT	H.M.	PAINT	8"	6		4" HEAD, 16 GA. WITH POLYEURATHANE INSULATION	103c
103d	D	PR 3/0x7/0	INSUL. STL.	PAINT	H.M.	PAINT	8"	6		4" HEAD, 16 GA. WITH POLYEURATHANE INSULATION	103d
104	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	7	45 MIN.		104
105	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	7 1/4"	7	45 MIN.		105
106a	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	7	45 MIN.		106a
106b	E	3/0x7/0	INSUL. STL.	PAINT	H.M.	PAINT	6 3/4"	8		16 GA. WITH POLYEURATHANE INSULATION	106b
107	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	7 1/4"	7	45 MIN.		107
108	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	7 1/4"	7	45 MIN.		108
109a	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	1		MEDIUM STYLE	109a
109b	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	3		MEDIUM STYLE	109b
110	A	PR 3/0x7/0	ALUM.	MILL	ALUM.	MILL	4 1/2"	9			110
111	С	PR 3/0x7/0	S.C. WOOD	STAIN	H.M.	PAINT	6 1/8"	10	90 MIN.		111
112	G	4/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	11			112
113a	G	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	12	45 MIN.		113a
113b	G	3/0x7/0	S.C. WOOD	STAIN	H.M.	PAINT	8 3/4"	13	90 MIN.	4" HEAD	113b
115	В	PR 2/8x7/0	S.C. WOOD	STAIN	H.M.	PAINT	8 3/4"	11			115
116	G	2/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	11			116
117	G	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	14	45 MIN.		117
118	G	2/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	7 1/4"	11			118
119a	G	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	12	45 MIN.		119a
119b	G	3/0x7/0	S.C. WOOD	STAIN	H.M.	PAINT	8 3/4"	13	90 MIN.	4" HEAD	119b
120	G	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	11	45 MIN.		120
121a	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	7	45 MIN.		121a
121b	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	7 1/4"	7	45 MIN.		121b
122	E	3/0x7/0	INSUL. STL.	PAINT	H.M.	PAINT	6 1/2"	14			122
123a	E	4/0x7/0	H.M.	PAINT	K.D.	PAINT	7 1/4"	7		18 GA.	123a
123b	н	10/0x10/0	ALUM.	MILL						LOCK, ELECTRIC OPENER	123b
124a	В	PR 2/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	15			124a
124b	В	PR 2/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	15			124b
125	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	16			125
201	F	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	7 5/8"	7	90 MIN.		201
202	G	3/0x7/0	S.C. WOOD	STAIN	K.D.	PAINT	6 1/8"	7	45 MIN.		202





DOOR 'B'

DOOR 'C'

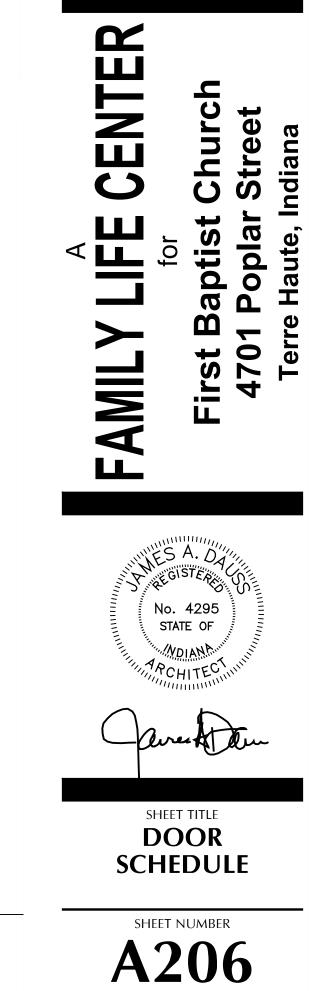




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SCOPE OF DRAWINGS These Drawings indicate the general scope of the Project in terms of architectural design concept, the dimensions of the building, the major architectural elements and the type of structural, mechanical and electrical systems. The Drawings do not necessarily indicate or describe all work required for full performance and completion of the requirements of the Contract. On the basis of the general scope indicated or described, the medae contractors shall furnish all items required for the proper execution and completion of the Work.			
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	FOR OWNER REVIEW IULY 25, 2019		
	JED FOR BIDDING TEMBER 27, 2019		
	DOOR HARDWARE		



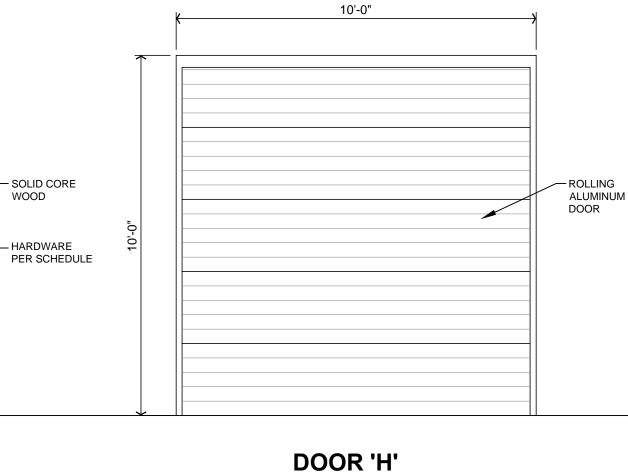
SHEETS

OF

PROJECT NUMBER

F005-1802





## DATUM ELEVATION

1. See civil drawings for USGS elevation of first floor.

GENERAL

- 1. Contractor shall include seven calendars days for the structural engineer to review each shop drawing submittal. The seven days starts upon the engineer's receipt of the shop drawings and shall conclude when the engineer returns the drawings to the entity that sent them the drawings. Contractor shall include time in their schedule above and beyond the seven days to allow for transmitting drawings from fabricator to the contractor to the architect to the engineer and back. Contractor may not make claims for delays if the engineer completes their review within the above noted time.
- 2. Shop drawings shall be submitted to the Engineer of Record, for review, for the following structural elements
- a. Cast in place concrete reinforcement placement and mix designs b. Structural steel detail and erection
- c. Steel joist design and placement
- 3. Concrete contractor is responsible for stability of excavations required to complete the work of their contract. Systems used to stabilize excavations shall be adequate to resist the force of the soil, any surcharge due to construction activities and the forces imposed on the excavation during construction activities due to wind or seismic forces.

FOUNDATIONS

- 1. Proofroll slab on grade areas with a medium-weight roller or other suitable equipment to check for pockets of soft material hidden beneath a thin crust of better soil. Any unsuitable materials thus exposed should be removed and replaced with compacted, engineered fill as outlined in the specifications. Proofrolling operations shall be monitored by the Testing Agency.
- 2. All engineered fill beneath slabs and over footings should be compacted to a dry density of at least 90% of the Modified Proctor maximum dry density (ASTM D-1557). All fill which shall be stressed by foundation loads shall be approved granular materials compacted to a dry density of at least 95% (ASTM D-1557). Coordinate all fill and compaction operations with the Specifications and the Subsurface Investigation.
- 3. Compaction shall be accomplished by placing fill in approx. 8" lifts and mechanically compacting each lift to at least the specified minimum dry density. For large areas of fill, field density tests shall be performed for each 3000 square feet of building area for each lift as necessary to insure adequate compaction is being achieved.
- 4. Exterior footings shall bear a minimum of 3'-0'' below finish grade.
- 5. For information regarding subsurface conditions, refer to the Report of Geotechnical Engineering Investigation prepared by Patriot Engineering and Environmental, Inc., Project No. 19-0502-0265, Dated May 7, 2019. Footings have been designed for the following net allowable soil bearing pressures:
  - A. Column Footings 1,500 psf
- B. Continuous Wall Footings 1,200 psf
- 6. It is essential that the foundations be inspected to insure that all loose, soft or otherwise undesirable material (such as organics, existing fill, etc.) is removed and that the foundation will bear on satisfactory material. The Testing Agency shall inspect the subgrade and perform any necessary tests to insure that the actual bearing capacities meet or exceed the design capacities. The Testing Agency shall verify the bearing capacity at each spread column footing and every 10 feet on center for strip footings prior to placement of concrete.
- 7. Place footings the same day the excavation is performed. If this is not possible, the footings shall be adequately protected against any detrimental change in condition, such as from disturbance, rain and freezing.
- 8. It is the responsibility of the Contractor and each Sub-Contractor to verify the location of all utilities and services shown, or not shown, and establish safe working conditions before commencing work.
- 9. The Contractor shall layout the entire building and field verify all dimensions prior to excavation.

## **CONCRETE**

- 1. Reinforced concrete has been designed in accordance with the "Building Code Requirements for Structural Concrete" (ACI 318), latest edition.
- 2. Mixing, transporting, and placing of concrete shall conform to the latest edition of the "Specifications for Structural Concrete for Buildings" (ACI 301).
- 3. Concrete in the following areas shall consist of natural sand fine aggregates and normal weight coarse aggregates conforming to ASTM C33, Type I Portland cement conforming to ASTM C150, and shall have the following compressive strength (F'c) at 28 days:
- 3,000 psi: Concrete footings (wall and column pad)
- 4,000 psi: Isolated Concrete piers
- 4,000 psi: Concrete slabs on grade (see ''Concrete Slabs On Grade'' general notes)
- 4,000 psi with 6% +/- 1-1/2% entrained air by volume: Retaining walls, knee walls and foundation walls. Curbs, sidewalks and other Concrete exposed to de-icers or freeze/thaw conditions
- 4. Fly ash may be used as a pozzolan to replace a portion of the cement in a concrete mix, subject to the approval of the Structural Engineer. Fly ash, when used, shall conform to ASTM C618. Concrete mixes using fly ash shall be proportioned to account for the properties of the specific fly ash used and to account for the fly ash effects on the properties of the concrete. The ratio of the amount of the fly ash to the amount portland cement in the mix shall not exceed 20 percent.
- 5. Adjustment of slump by adding water to the mix at the job site shall occur as follows. Adjustment shall be made one time only with a maximum of 2 gallons of water per cubic yard such that the specified slumps and water/cement ratios are not exceeded. The concrete shall be mixed after the addition of water one minute per cubic yard to a maximum of 5 minutes.
- 6. Protect the concrete surface between finishing operations on hot, dry days or any time plastic shrinkage cracks could develop by using wet burlap, plastic membranes or fogging. Protect concrete surfaces at all times from rain, hail or other iniurious effects.
- 7. Horizontal joints will not be permitted in concrete construction except as shown on the contract documents. Vertical joints shall occur at locations approved by the Structural Engineer.

CONCRETE SLABS ON GRADE

- 1. Slabs on grade shall be constructed in accordance with the latest edition of the "Guide for Concrete Floor and Slab Construction" (ACI 302.1R)
- 2. In addition to the specifications noted elsewhere, concrete for interior flatwork shall conform to the following (FLY ASH IS NOT PERMITTED IN INTERIOR FLOOR SLABS):

564 lbs. of cement per cubic yard of concrete, minimum - Fly ash IS NOT permitted 0.45 maximum water cement ratio 3 inch maximum slump prior to addition of plasticizers

- 5 inch maximum slump if plasticizers are not used
- Aaareaate size: 3/4" minimum, 1 1/2" maximum Midrange or highrange water reducing admixture required
- 3. In addition to the specifications noted elsewhere, concrete for exterior flatwork curbs and walls exposed to freeze/thaw shall conform to the following:

564 lbs. of cement per cubic yard, minimum 0.45 maximum water cement ratio by weight 6% + / - 1 - 1 / 2% entrained air by volume 3 inch maximum slump prior to addition of plasticizers 5 inch maximum slump if plasticizers are not used Aggregate size: 3/4" minimum, 1 1/2" maximum Midrange or highrange water reducing admixture required CONCRETE SLABS ON GRADE (CONT.)

- 4. Place concrete in a manner so as to prevent segregation of the mix. Delay floating and troweling operations until the concrete has lost surface water sheen or all free water. Do not sprinkle free cement on the slab surface.
- 5. Provide curing of concrete slabs immediately after finishing using a sprayed on liquid curing compound. Other methods may be used with approval by structural engineer. 6. Slabs on grade shall receive a smooth trowel finish, and be placed to achieve the
- following minimum tolerances: Overall values: FF = 35, FL = 25

Local values: FF = 25, FL = 15

CONCRETE REINFORCING

- 1. Reinforcement, other than cold drawn wire for spirals and welded wire fabric, shall have deformed surfaces in accordance with A.S.T.M. A615.

- 3. Welded wire fabric shall conform to ASTM A185, unless noted 4. Where hooks are indicated, provide standard hooks per A.C.I, and C.R.S.I. for all bars
- unless other hook dimensions are shown on the plans or details.
- the Concrete Reinforcing Steel Institute's "Manual of Standard Practice".
- 6. Details of reinforcing steel fabrication and placement shall conform to ACI 315 -"Details and Detailing of Concrete Reinforcement" and ACI 315R - "Manual of Engineering and Placing Drawings for Reinforced Concrete Structures", unless otherwise indicated.
- 7. Provide individual high chairs with support bars, as required for the support of top reinforcement for supported slabs. Do not provide standees.

8. Field bending of reinforcing steel is prohibited. 9. Minimum concrete cover over main reinforcing steel shall be as indicated in sections. STRUCTURAL STEEL NOTES

- 1. Steel construction shall conform to the American Institute of Steel Construction,
- ANSI/AISC 360, latest edition.
- 2. All plates, channels, bars, and angles shall be ASTM A36, unless noted otherwise.
- 3. All anchor rods shall be ASTM F1554, Grade 55 (Fy=55 ksi) unless noted otherwise. 4. All pipe shapes shall be ASTM A53, Type E, Grade B.
- 5. All square and rectangular shapes shall be ASTM A500, Grade B.
- 6. All structural wide flange shapes shall be ASTM A-992, Fy=50 ksi.
- 7. Details for design, fabrication and erection of all structural steel shall be in accordance with the latest A.I.S.C. Standards unless otherwise noted or specified.
- 8. Provide temporary erection guying and bracing as required. 9. Unless otherwise shown or noted on the Plans, provide 8" bearing each end for all
- loose lintels and beams.
- 10. For loose lintels, masonry shelf angles and other such items generally not shown on the Structural Plans, see the Architectural Plans,
- 11. Steel columns below grade shall be encased in a minimum of 4" of concrete or 1/4" thick Dissco 540 Mastic (or approved equal). A heavy bodied, pliable, bituminous, waterproofing compound of trowel consistency engineered for underground service under extreme conditions.
- 12. Fabricate simple span beams not specifically noted to receive camber so that after erection, any minor camber due to rolling or shop assembly, shall be upward.
- 13. Structural steel shall be shop-painted with a rust inhibiting primer. See specifications for color and compatibility with finish paint. 14. The Erector shall shim between parallel roof beams and joists with differential mill and
- induced cambers for level deck bearing.
- 15. Whether or not explicitly shown on the Structural Drawings, all angles, bent plates, clips, closures, etc., attached to structural steel members to form slab edges, to provide lateral support for cladding, etc., shall be field-installed unless otherwise approved by the Architect/Engineer.
- 16. Provide cap plates/end plates to close off exposed, open ends of all tubular members, unless noted otherwise

STEEL CONNECTION NOTES

- 1. Typical beam-to-beam and beam-to-column connections shall be bearing type using A325-N bolts, unless noted otherwise.
- 2. Shop connections unless otherwise shown, may be either bolted or welded. All field connections shall be bolted unless otherwise shown on the Structural Drawinas.
- 3. Connections shall be designed by the Steel Fabricator to support the reactions shown on the framing plan. Connections not shown shall be designed by the Steel Fabricator in accordance with the AISC "Manual of Steel Construction". Non-composite simple span beam connections shall be designed for one-half the load capacity as given in the AISC "Uniform Load Constants for Beams Laterally Supported" tables. Composite beams shall be designed for 3/4 of this load.
- 4. All beam-to-beam connections shall be double angle, unless shown or noted otherwise. 5. All beam-to-column connections shall be at the column centerline, unless noted
- otherwise. Shear tab connections to tubes are permitted.
- 6. Typical beam-to-beam, and beam-to-column field-bolted connections may be tightened to the snug-tight condition, unless otherwise shown or noted. Bolted connections in moment frames, bracing connections, and those designated F.T. (fully-tensioned) on the Plans shall be fully-tensioned connections utilizing load-indicating washers, or twist-off bolts
- 7. All welding shall be in conformance with AWS D1.1, using E70XX electrodes, unless shown or noted otherwise.
- 8. Holes in steel shall be drilled or punched. All slotted holes shall be provided with smooth edges. Burning of holes in structural steel shall not be allowed without approval of the Architect/Engineer.
- 9. Minimum thickness of all connection material to be 5/16" unless noted.
- 10. Grout beneath base plates and under preset bearing plates shall be a prepackaged, non-metallic and non-aaseous. It shall be non-shrink when tested in accordance with ASTM—C1107 Grade B or C at a fluid consistency (flow cone) of 20 to 30 seconds. Thirty-minute-old grout shall flow through the flow cone after slight agitation, in temperatures of 40 degrees F to 90 degrees F. Grout shall be bleed free and attain 5,000-psi compressive strength in 28 days at fluid consistency. Provide double nuts for adjustment of column base plates, unless noted otherwise.

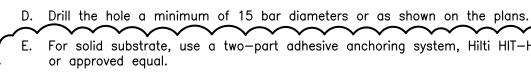
STEEL DECK NOTES

- 1. All steel deck material, fabrication and installation shall conform to the Steel Deck Institute "SDI SPECIFICATIONS AND COMMENTARY" and "CODE OF RECOMMENDED STANDARD PRACTICE", current edition, unless noted.
- 2. Provide members for deck support at all deck span changes. Provide L3x3x3/16 deck support at all columns where required.
- 3. All deck shall be provided in a minimum of 3-span lengths where possible.
- 4. All welding of steel deck shall be in conformance with AWS Specification D1.3. Provide welding washers for all floor decks less than 22 gauge in thickness.

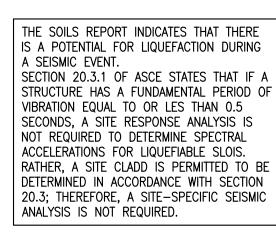
- 2. Reinforcing steel shall conform to ASTM A615. Grade 60, unless noted.
- 5. Reinforcement shall be supported and secured against displacement in accordance with

STEEL DECK NOTES (CONT.)

- 5. Mechanical fasteners may be used in lieu of welding, providing fasteners meet, or exceed the strength of specified welds. Submit fastener design data to the Engineer of Record for review.
- 6. Where white bottom deck is indicated, primer shall be compatible with finish paint. See Specifications.
- 7. Substitution of fiber secondary reinforcement for welded wire fabric on supported slabs is prohibited
- 8. Do not suspend any items, such as ductwork, mechanical or electrical fixtures, ceilings, etc. from steel roof deck.
- 9. Roof deck sidelaps shall be attached at ends of cantilevers and at a maximum spacing of 12" o.c. from cantilevered roof deck ends. The roof deck must be completely fastened to the supports and at the sidelaps before any load is applied to the cantilever.
- 10. Submit shop drawings for review of general conformance to design concept in accordance with Specifications in the Project Manual. Erection drawings shall show type of deck, shop finish, accessories, method of attachment, edge details, deck openings and reinforcement, and sequence of installation.
- 11. Installation holes shall be sealed with a closure plate 2 gauges thicker than deck and mechanically fastened to deck. Steel deck holes visible from below will be rejected. Deck units that are bent, warped, or damaged in any way which would impair the strength and appearance of the deck shall be removed from the site.
- 12. Where gauge metal pourstops are indicated, supply pourstops designed to meet, or exceed the gauges listed in the SDI Pourstop Selection Table (min. 18 ga.) as required for slab depth, concrete weight, and cantilever distance, unless noted otherwise.
- 13. The Erector shall shim between parallel roof beams and joists with differential mill and induced cambers for level deck bearing.
- 14. Design joists for uplift per the "Wind Uplift Pressure Table" on this sheet and "Roof Deck Attachment Patterns" on the drawings for additional information.
- EXPANSION and SLEEVE ANCHORS
- 1. Expansion and sleeve anchors shall be carbon steel anchors as manufactured by Hilti Fastening Systems or approved equal. Install anchors in conformance with the manufacturer's recommendations and the contract documents.
- 2. Masonry cores receiving 3/4" diameter anchors shall be filled with coarse grout conforming to ASTM C476 with a minimum compressive strenath of 3.000 psi at 28 days. Do not install anchors in mortar ioint.
- 3. Do not install expansion anchors in concrete until concrete has attained the specified 28-day compressive strength.
- 4. Test expansion, sleeve and epoxy anchors prior to connecting steel to anchors.
- DRILLED-IN DOWELS & ANCHOR BOLTS/RODS
- 1. All reinforcing steel and threaded rod anchors to be installed in 2-part chemical anchoring system shall be treated as follows:
  - A. Drill holes larger than bar or rod to be embedded. Coordinate hole diameter with Manufacturer's recommendations.
  - B. Holes must be cleaned and prepared in accordance with Manufacturer's recommendations.
  - C. When reinforcing steel is encountered during drilling for installation of anchors, stop drilling, use a sensor to locate the reinforcing in the surrounding area and install anchor(s) as close as possible to the original location. Contact the Engineer of Record for direction when the revised location is more than 2" from the original location, or when the original function of the anchorage is significantly altered. When in doubt, contact the Engineer of Record for direction.



- For solid substrate, use a two-part adhesive anchoring system, Hilti HIT-HY 100 F. For anchorage into hollow substrate, use a two-part adhesive anchoring system,
- Hilti HIT-HY 100 injectable mortar and mesh sleeves, or approved equal. G. Reinforcing steel dowels shall be ASTM A615, Grade 60, unless noted.
- H. Anchor rods shall be ISO 898 5.8 (Hilti HAS-E), unless noted. Provide finish as noted on the Drawings. If not noted, provide hot-dip galvanized finish for interior applications. Provide stainless steel finish for all exterior applications, unless
- 2. All threaded rod anchors shall be load tested in the field to verify the pull-out and shear values associated with the type of anchor or anchoring system specified on the drawinas.





ARCHITECTURE PLANNING

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PO BOX 1006

PRE-ENGINEERED STRUCTURES APPLICABLE CODES AND DESIGN DATA THE BUILDING STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE INDIANA BUILDING CODE, 2014 EDITION (IBC 2012) ROOF DESIGN LOADS: LIVE LOAD .... ...20 PSF DEAD LOAD .. COLLATERAL LOAD ..... .... 3 PSF ROOF SNOW LOADS: GROUND SNOW LOAD (Pg) .....20 PSF FLAT ROOF SNOW LOAD (Pf) ......15 PSF SNOW EXPOSURE FACTOR (Ce) .....1 SNOW LOAD IMPORTANCE FACTOR (I)...1 THERMAL FACTOR (Ct) .....1 WIND LOAD DESIGN DATA ULTIMATE DESIGN WIND SPEED (3 SECOND GUST) ..... 115 MPH WIND EXPOSURE ..... OCCUPANCY CATEGORY .. INTERNAL PRESSURE COEFFICIENT (GCpi) .... COMPONENTS AND CLADDING DESIGN PRESSURE ..... \* SEISMIC DESIGN DATA SEISMIC IMPORTANCE FACTOR (I).....1 SEISMIC DESIGN CATEGORY ..... SPECTRAL RESPONSE COEFFICIENTS & ACCELERATIONS Sds ...... \* Ss ...... \* Sd1 ...... \* S1 ...... \* SITE CLASS .... .. SEE SOILS REPORT DESIGN BASE SHEAR ... SEISMIC RESPONSE COEFFICIENT (Cs) ..... \* RESPONSE MODIFICATION FACTOR (R) ...... \* BASIC SEISMIC RESISTING SYSTEM ......\* ANALYSIS PROCEDURE UTILIZED ......\*

\* - SEE DRAWINGS PROVIDED BY PRE-ENGINEERED BUILDING MANUFACTURER

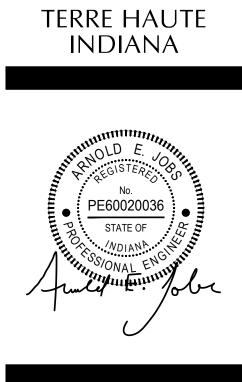
CONNECTOR STRUCTURE
APPLICABLE CODES AND DESIGN DATA THE BUILDING STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH THE INDIANA BUILDING CODE, 2014 EDITION (IBC 2012)
MEZZANINE DESIGN LOADS: LIVE LOAD
ROOF SNOW LOADS:GROUND SNOW LOAD (Pg)FLAT ROOF SNOW LOAD (Pf)SNOW EXPOSURE FACTOR (Ce)SNOW LOAD IMPORTANCE FACTOR (I) 1THERMAL FACTOR (Ct)
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SEISMIC DESIGN DATA     SEISMIC IMPORTANCE FACTOR (I)
DESIGN BASE SHEAR

## info@daussarchitects.com SCOPE OF DRAWINGS ese Drawings indicate the general scope of the Project in terms of architectur-sign concept, the dimensions of the building, the najor architectural element of the type of structural, mechanical and electrical systems. The Drawings do to necessarily indicate or describe all work required for full performance and mpletion of the requirements of the Contract. On the basis of the general scc dicated or described, the trade contractors shall furnish all items required for oper execution and completion of the Work. COPYRIGHT DAUSS ARCHITECTS CHECK DRAWN FOX AEJ DATE ISSUED FOR STATE REVIEW SEPTEMBER 10, 2019 1\ 10-29-19 REVISED MEZZANINE

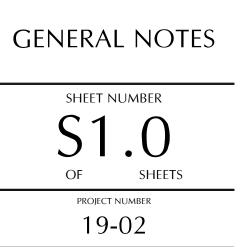


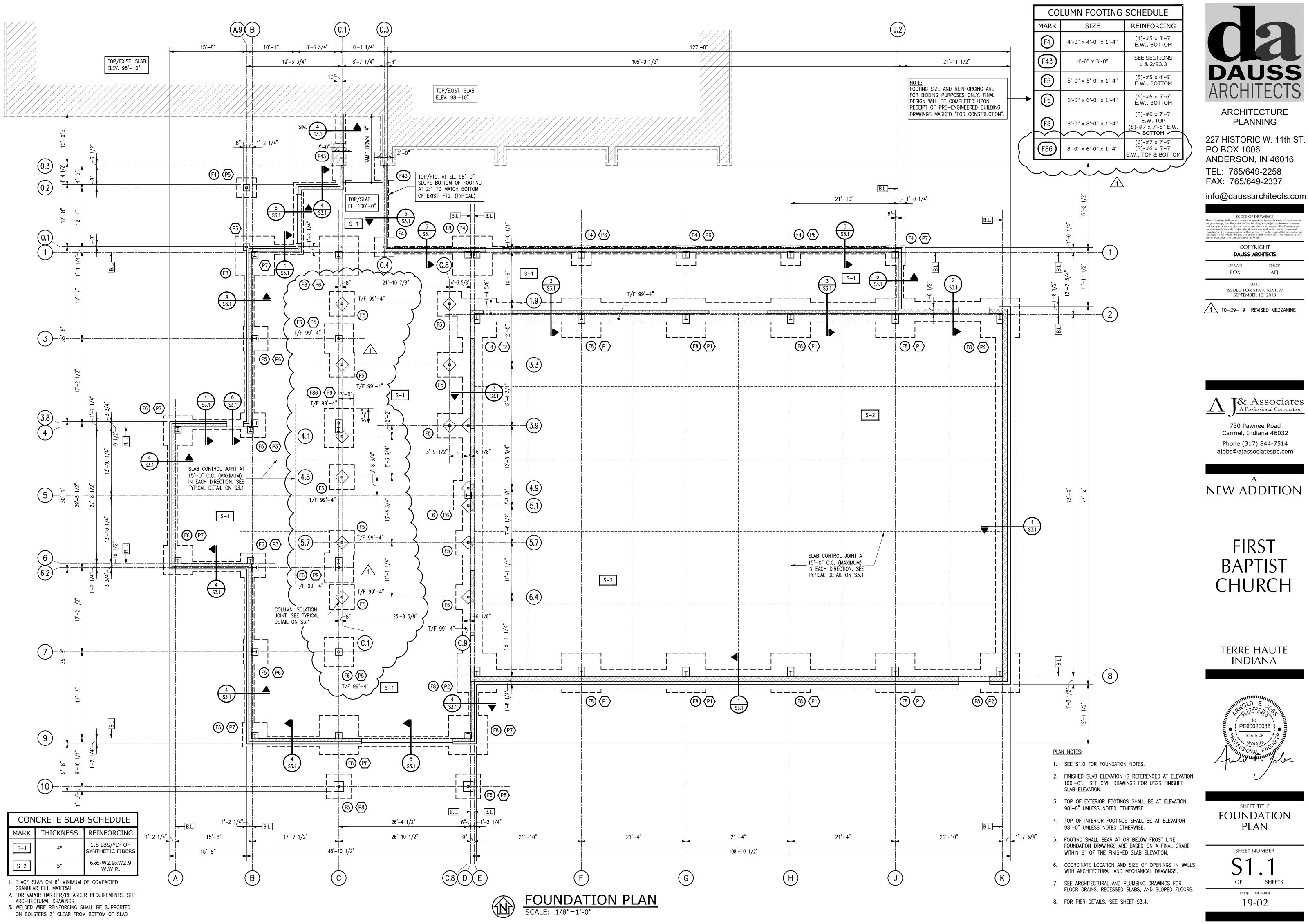
# NEW ADDITION





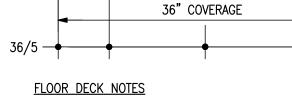
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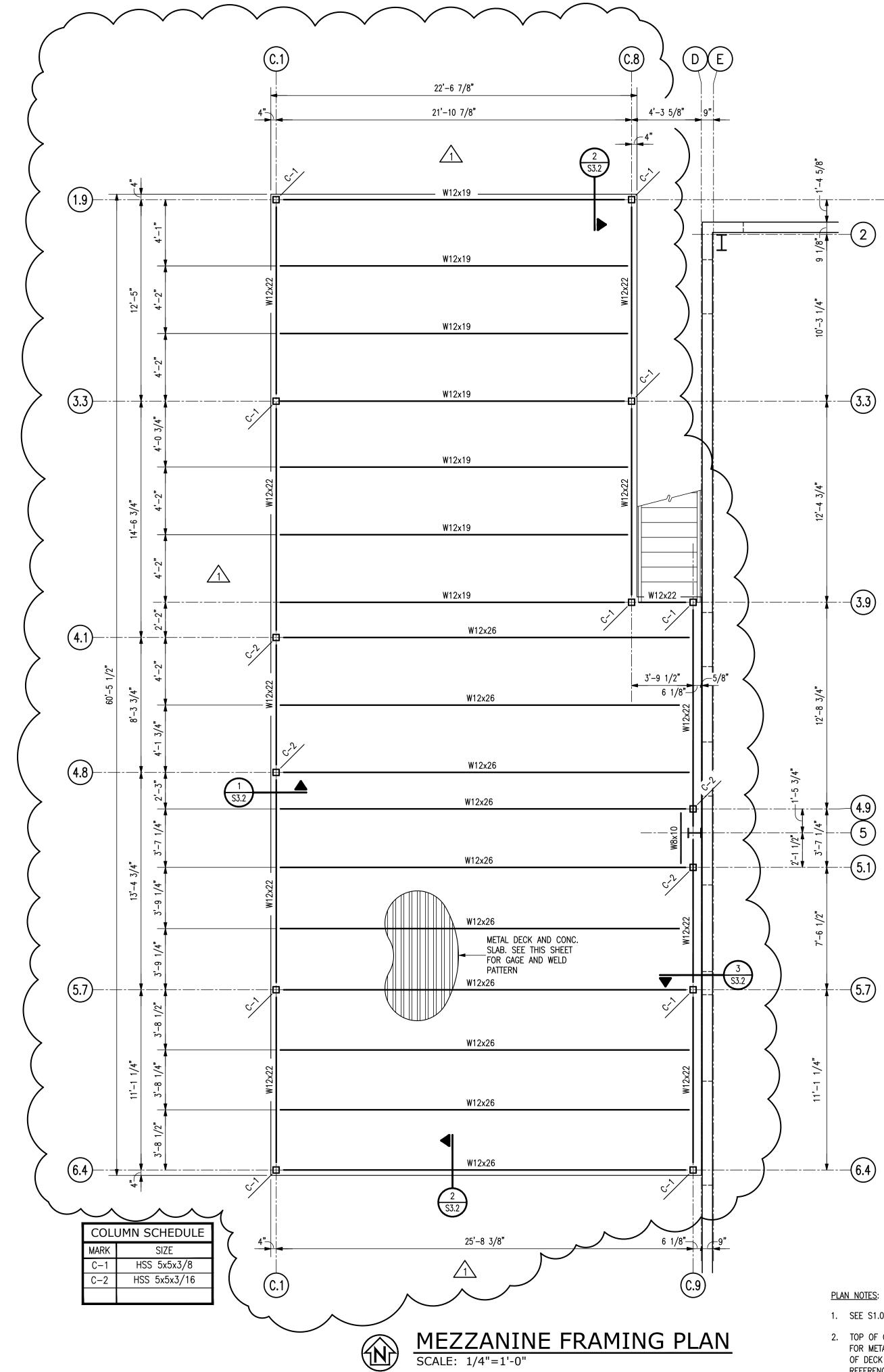


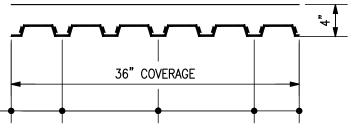


## SLAB + FLOOR DECK ATTACHMENT

- 4. FASTEN DECK WITH (3)-#10 TEK SCREWS AT SIDELAPS BETWEEN SUPPORTS.
- 2. FASTEN DECK TO ALL SUPPORTS WITH 5/8" PUDDLE WELDS IN PATTERN SHOWN ABOVE.
- PROVIDE 1.5VL, 20 GAGE, GALVANIZED, COMPOSITE DECK WITH 2 1/2" CONCRETE TOPPING SLAB (4" TOTAL DEPTH). REINFORCE SLAB WITH 1.5 LBS/FT<sup>3</sup> OF SYNTHETIC FIBERS.







DAUSS ARCHITECTS ARCHITECTURE

PLANNING

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SCOPE OF DRAWINGS These Drawings indicate the general scope of the Project in terms of architectural design concept, the dimensions of the building, the major architectural elements and the type of structural, mechanical and electrical systems. The Drawings do not necessarily indicate or describe all work required for full performance and completion of the requirements of the Contract. On the basis of the general scope indicated or described all works shall furnish all items required for the proper execution and completion of the Work.				
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## A NEW ADDITION

FIRST BAPTIST CHURCH

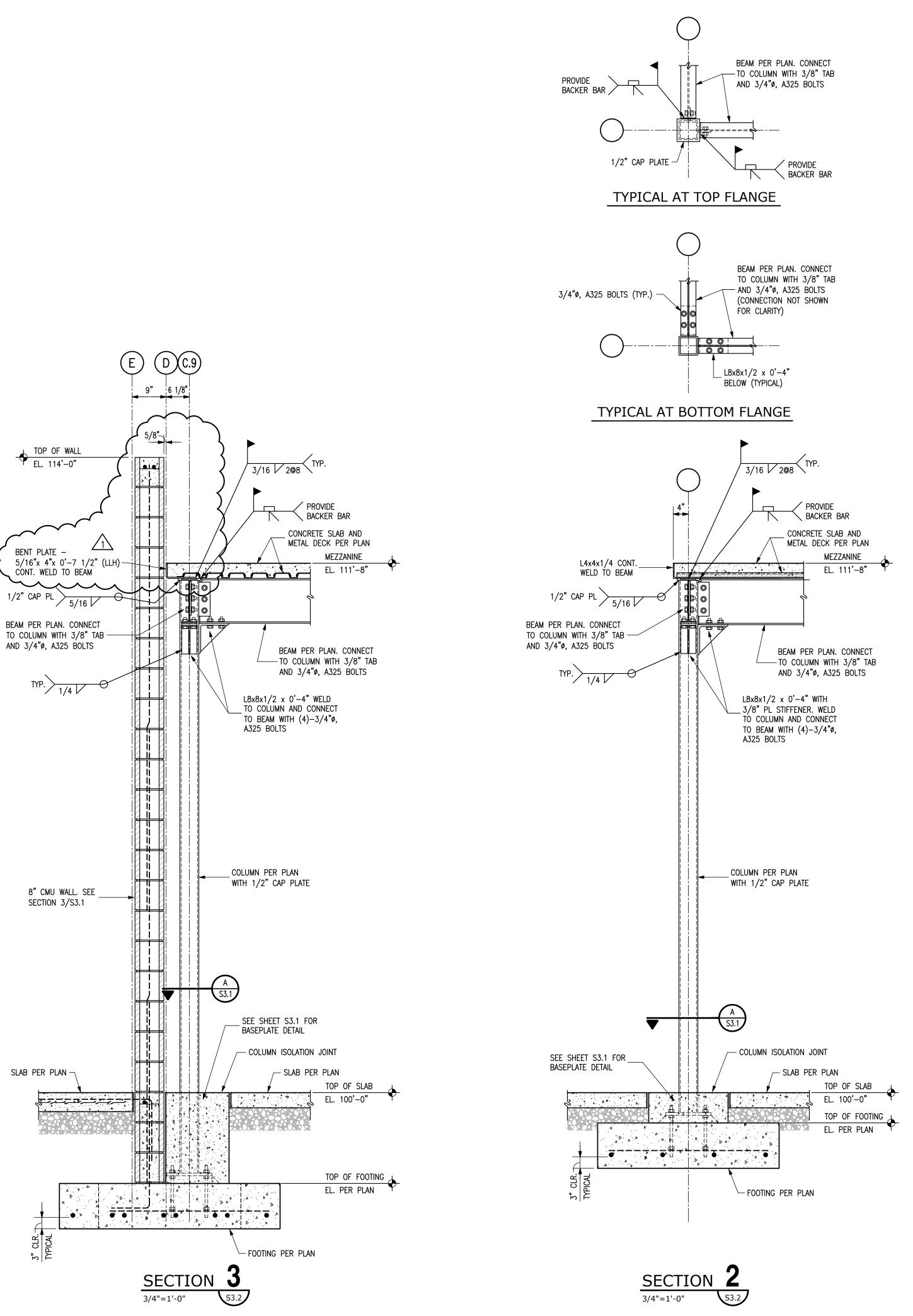
TERRE HAUTE INDIANA





1. SEE S1.0 FOR GENERAL NOTES.

2. TOP OF CONCRETE SLAB ELEVATION AT 111'-8". SEE THIS SHEET FOR METAL DECK AND SLAB INFORMATION. TOP OF STEEL/BOTTOM OF DECK ELEVATION AT 111'-4". ELEVATIONS ARE GIVEN ABOVE REFERENCE FIRST FLOOR FINISHED SLAB ELEVATION 100'-0".



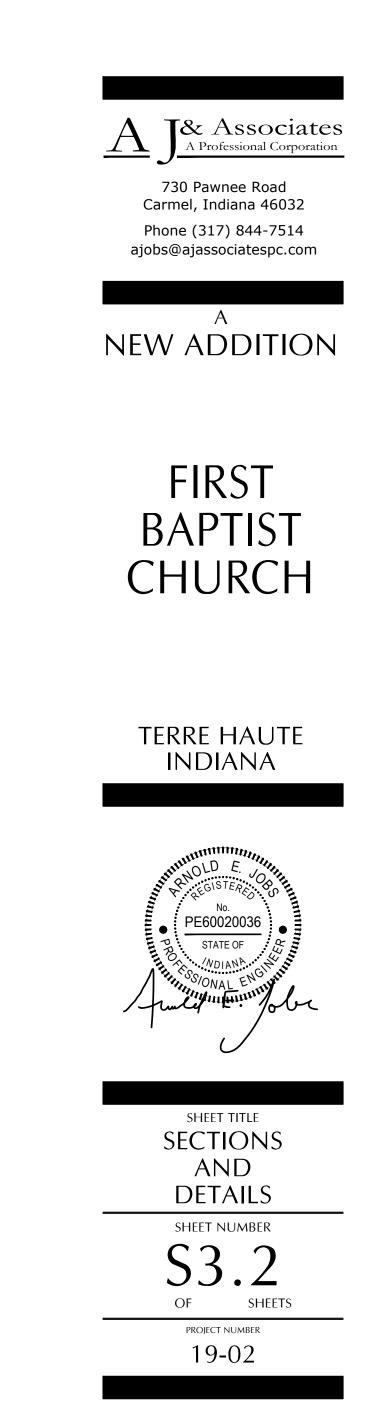


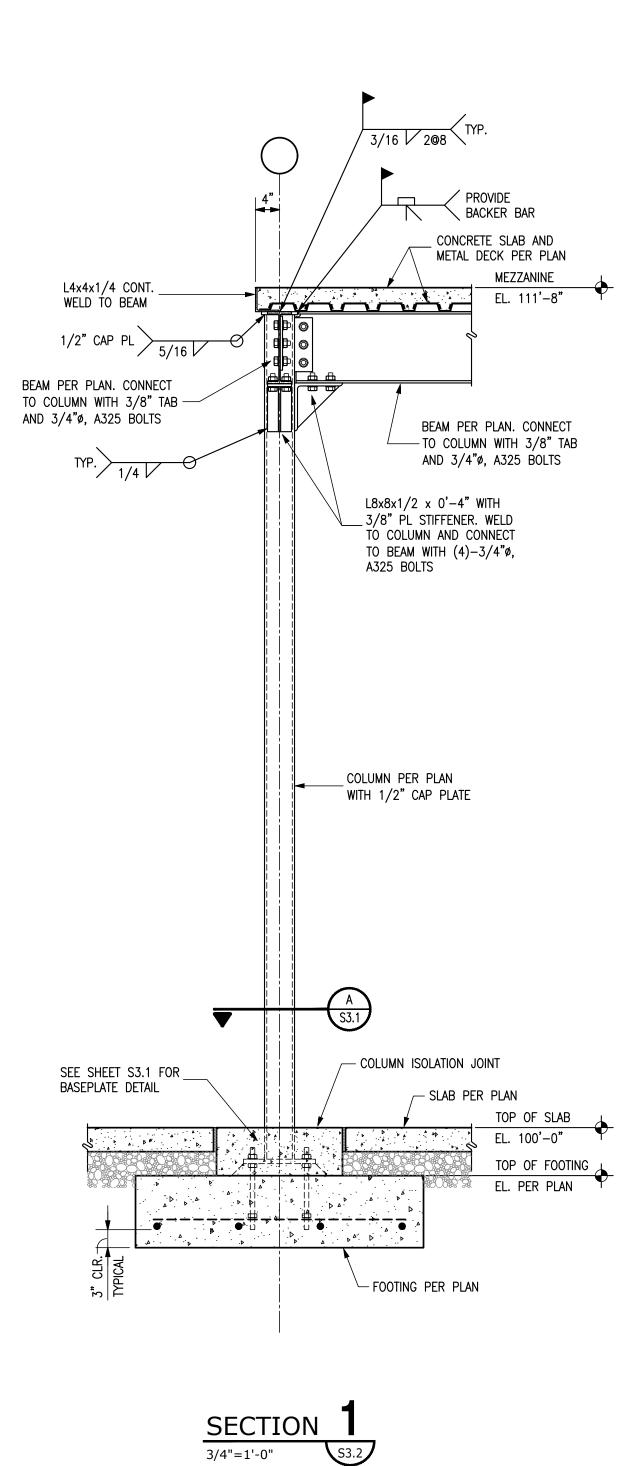
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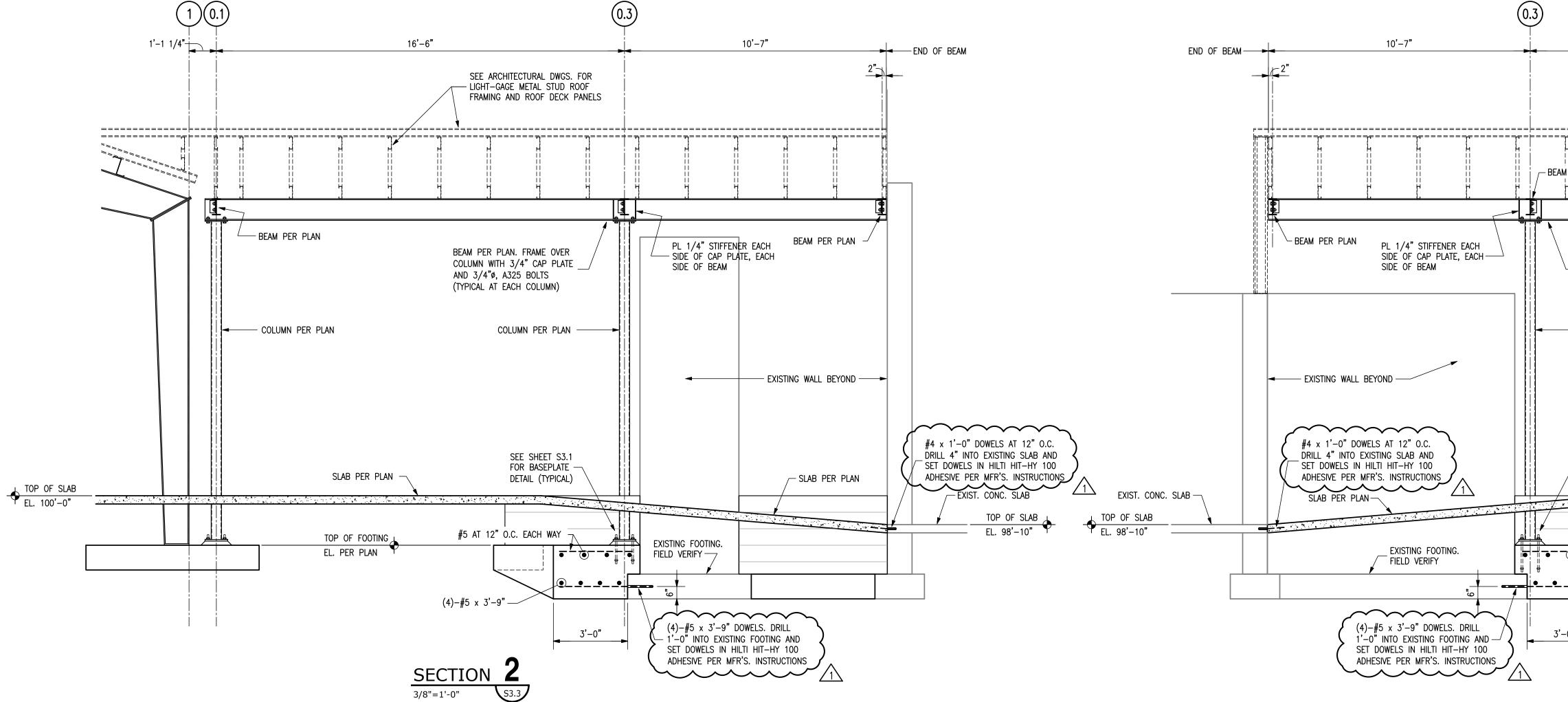
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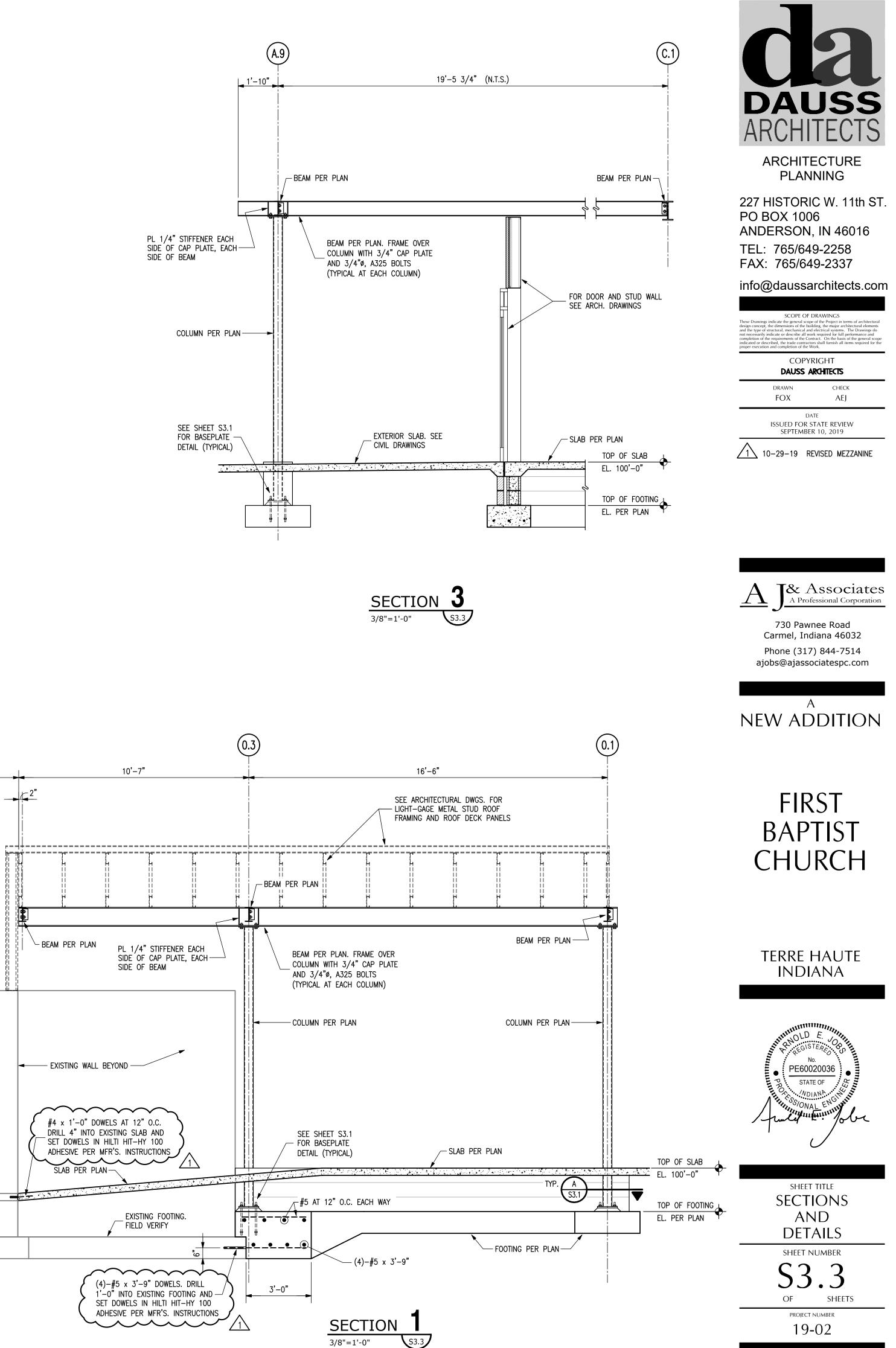
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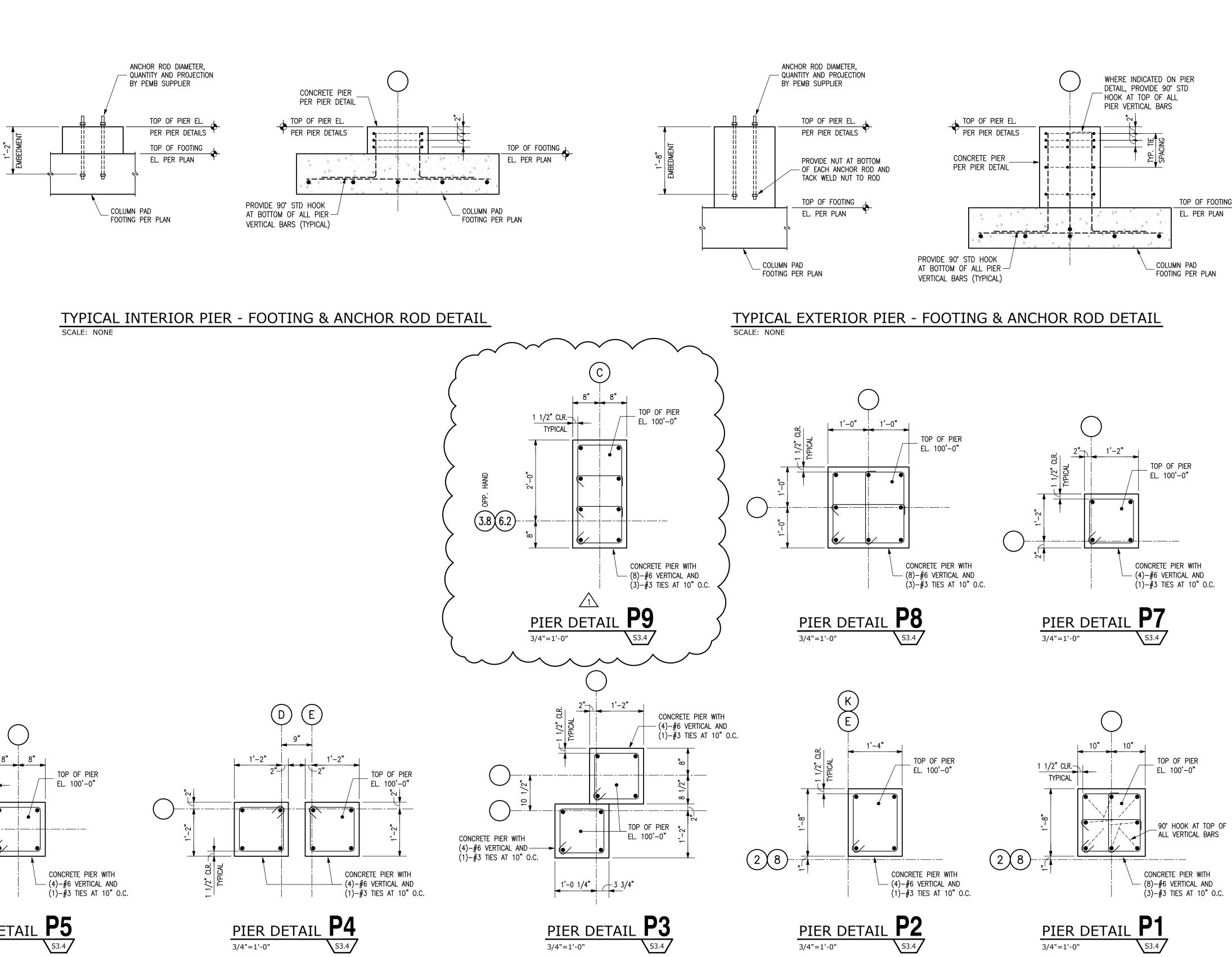
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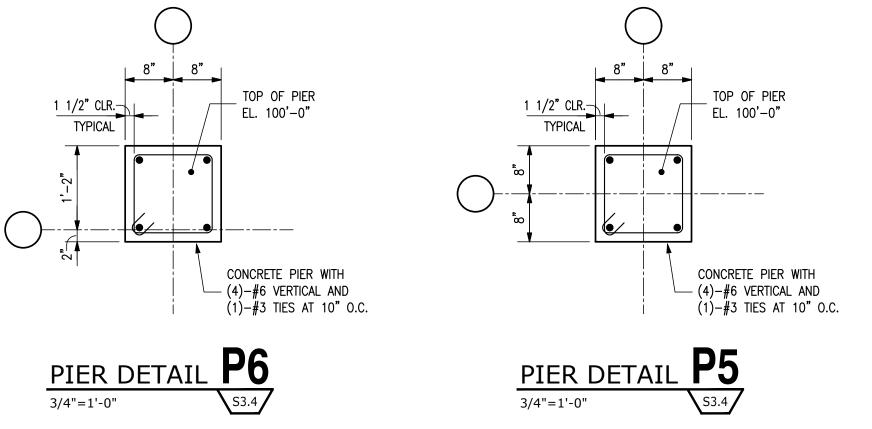












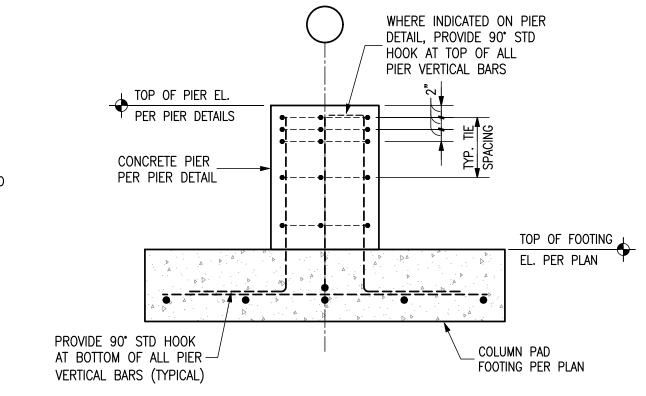


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# NEW ADDITION

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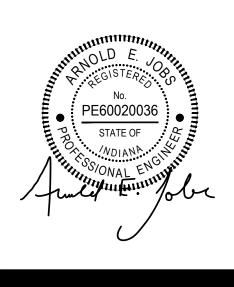
ajobs@ajassociatespc.com

**T**& Associates

A Professional Corporation

# FIRST BAPTIST CHURCH





SHEET TITLE PIER DETAILS SHEET NUMBER S3.4 OF SHEETS PROJECT NUMBER 19-02