

SMT.S.R.PATELENGINEERING COLLAGE DEPARTMENT OF CIVIL ENGINEERING 8th Semester - 2013

Design Of Multistoried Residential Building Using STAAD.Pro Package Analyzed For Earthquake Forces With Ductile Detailing As Per IS: 13920.

Prepared by

1. Patel Brijesh Y.	(090780106019)
2. Patel Mayank A.	(090780106029)
3. Patel Prashant N.	(090780106039)

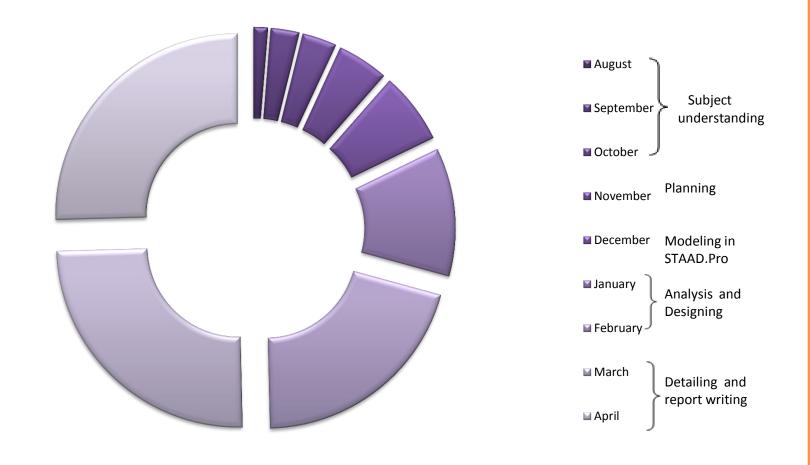
Guided by

Prof. Z.R.Chhaya Ms. Kinjal R. Patel

Flow of work

- ✤ Time line diagram
- Introduction
- Scope of work
- ✤ Aim of project
- Introduction of analysis and design
- Design of structural elements
 - Beam
 - Column
 - Slab
 - Stairs
 - Footing
 - Shear wall
- Problem definition
 - Plan of residential building
- ✤ Analysis and design of building in STAAD. Pro.

Time line diagram



Introduction

In every aspect of human civilization we needed structures to live in or to get what we need. But it is not only building structures but to build efficient structures so that it can fulfill the main purpose for what it was made for. Here comes the role of civil engineering and more precisely the role of analysis of structure.

There are many classical methods to solve design problem, and with time new software's also coming into play. Here in this project work based on software named staad pro has been used.

Few standard problems also have been solved to show how staad pro can be used in different cases. These typical problems have been solved using basic concept of loading, analysis, condition as per IS code. These basic techniques may be found useful for further analysis of problems.

Scope of work

Following points will be covered in project work

- Study of design of various elements of building
- Planning of various components of a building with column positioning
- Introduction of STAAD.Pro
- Modeling of the building in the STAAD.Pro giving all boundary conditions (supports, loading etc...)
- ✤ Analysis and Design of various structural components of the modal building
- Study of analysis Data of the software
- Detailing of beams, columns, slab with section proportioning and reinforcement.

AIM OF PROJECT

This project aims for relearning of concept of structural design with the help of computer aids. Briefly we have gone through following points through out of the project work.

- > Understanding of design and detailing concept.
- ➤ Main objective i.e. learning of STAAD.Pro software package.

➤ Learning of analysis and design methodology which can be very useful in the field.

- > Understanding of earthquake resistance design concept.
- > Approach for professional practice in the field of structural engineering

Introduction of Analysis and design

- Analysis : Analysis of the structure means to determination of the internal forces like axial compression bending moment, shear force etc. in the component member for which the member are to be designed under the action of given external load.
- Design : The design is process of section percussion from the analysis results by using suitable analysis method.

The aim of design is to achievement of an acceptable probability that structures being designed will perform satisfactorily during their intended life.

Design of Structural Elements

The design of any structure is categorized into the following two main types:

- Functional design
- Structural design

Stages in structural design :

> The process of structural design involves the following stages:

- Structural planning
- Action of forces and computation of loads
- Method of analysis
- ✤ Member design
- Detailing, drawing and preparation of schedules

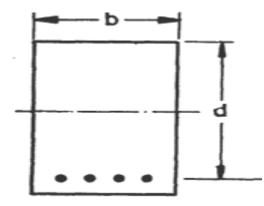
BEAM

There are three types of reinforced concrete beams

- ✤ Single reinforced beams
- Double reinforced beams
- ✤ Flanged beams

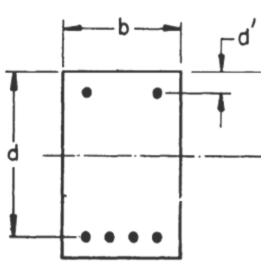
Single reinforced beams :

In singly reinforced simply supported beams steel bars are placed near the bottom of the beam where they are effective in resisting in the tensile bending stress.



Double reinforced beams:

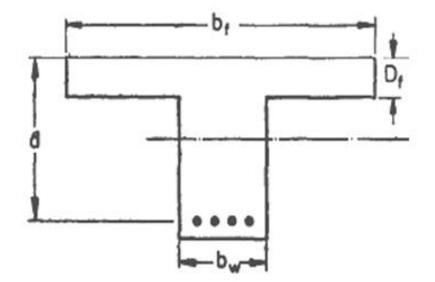
It is reinforced under compression tension regions. The necessities of steel of compression region arise due to two reasons. When depth of beam is restricted. The strength availability singly reinforced beam is in adequate.



Flanged beams:

There are two types of flanged beam

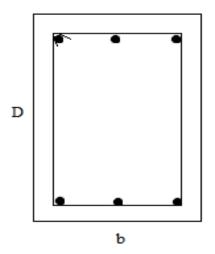
- 1. T beam
- 2. L beam



COLUMN

A column may be defined as an element used primary to support axial compressive loads and with a height of a least three times its lateral dimension.

The strength of column depends upon the strength of materials, shape and size of cross section, length and degree of proportional and dedicational restrains at its ends.



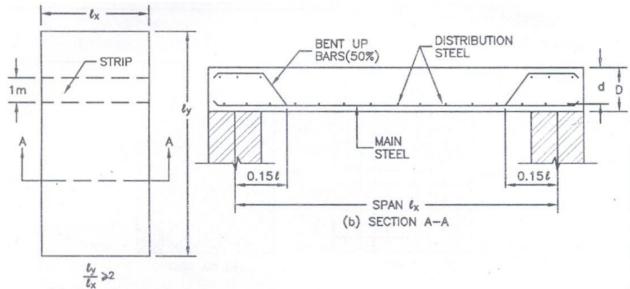
SLAB

Slabs are most widely used structural elements forming floor and roof of building. Slab support mainly transverse load and transfer them to supports by bending actions more or one directions.

On the basis of spanning direction: It is two type one way slabs and two way slab.

One way slab: When the slab is supported on two opposite side parallel edges, it spans only in the directions perpendicular to the supporting edges. It bends in one directions and main steel is provided in the directions of the span. Such a slab is known as one- way slab.

One way simply supported slab



(a) PLAN OF SLAB

Design steps:

a) Effective depth (d):

As per IS: 456-2000, P.37, Cl.23.2.1

l/d=20* M.F.

b) Effective span: (IS 456-2000, P 34)

c) Reinforcement requirements

Minimum reinforcement : (As per IS: 456-2000, P.48, Cl.26.5.2.1)

For Fe-250 pt = 0.15% of total c/s area (b*D)

For Fe-415 pt = 0.12% of total c/s area

- d) Check for cracking : (as per IS:456-2000,P.46)
- e) Check for deflection:

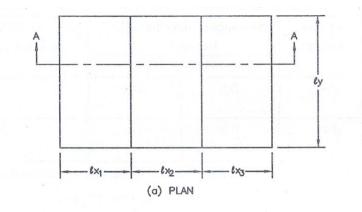
Allowable 1/d=20* M.F. Find Actual 1/d

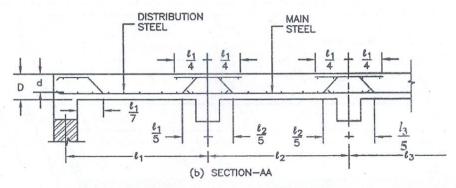
f) Checking for development length (Ld) : (as per IS:456-2000, P.44, Cl.26.2.3.3 (c))

Ld should be ≤ 1.3 M $_{1}$ /v + Lo

One way continuous slab:

In case of large halls, auditoriums, marriage halls, etc. the length is divided into equal bays by providing beams perpendicular to length. The slab provided over such area is called one way continuous slab.





Design steps:

✤ For calculation of S.F. and B.M., IS: 456-2000, P.36.

✤ As the coefficient for D.L. and L.L. are different, for D.L. and L.L. are calculated separately.

✤ In one way continuous slab, negative bending moment will be produced at the top of intermediate supports. Thus, negative reinforcement is provided over intermediate supports.

✤ For B.M. calculations, coefficient gives in IS: 456, table-12 is multiplied by wl2.

✤ For S.F. calculations, coefficient gives in IS: 456, table-13 is multiplied by wl.

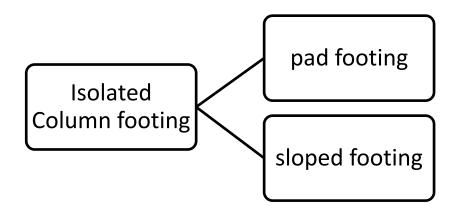
✤ Maximum shear occur at the support next to the end support. Therefore, slab must be checked for shear at this support.

The slab must be checked for deflection at the locations of maximum positive
 B.M.

The slab must be checked for development length at the end support. CIVIL ENGG. DEPT., SMT.S.R.PATEL ENGINEERING COLLAGE DABHI-UNJHA

FOOTING

Foundations are structural elements that transfer loads from the building or individual column to the earth .If these loads are to be properly transmitted, foundations must be designed to prevent excessive settlement or rotation, to minimize differential settlement and to provide adequate safety against sliding and overturning.



Problem definition

Plan of residential Building

- Analysis and design of structural components of G+5 storied building
- Analysis for Earthquake load
- Location –surat
- Masonry blocks Siporex
- S.B.C. 15 T/m^2

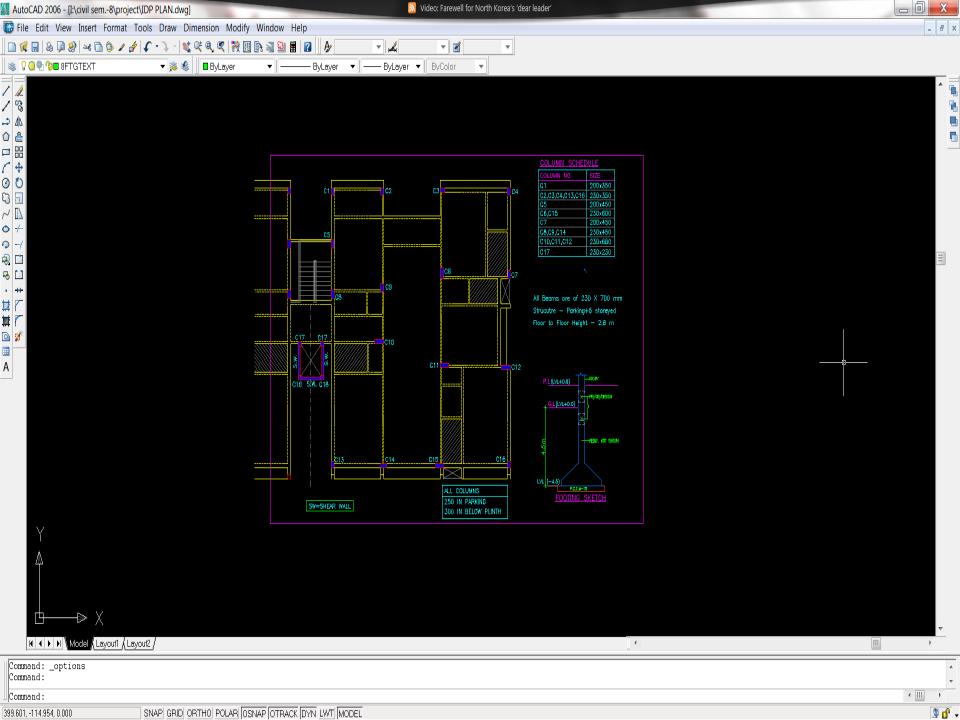
Introduction of staad. Pro.

It is one of the effective software which is used for the purpose of analysis and design of structure by the structural engineers. Our project is aimed to complete with the help of Staad.pro

Staad pro gives more precise and accurate results than manual techniques.

Features

- Analysis and design tool
- GUI based modeling
- Input file/Output file
- Results as per Indian & other standards
- Report generation



AutoCAD 2006 - [I:\civil sem8\project\line plan.dxf]	🔊 Video: Gingrich sliding in Iowa polls				
🐻 File Edit View Insert Format Tools Draw Dimension Modify Window Help			- 8 ×		
□ 🕼 🖬 💩 🖗 🌒 🖂 🗅 🌶 🖌 🗲 - 🔍 - 💐 ⊄ 🔍 ⊄ 🖏 🖼 🖬 🖓 🔛 🖟					
📚 🎗 🥘 🖓 🖬 0 🔹 🔻 💌 🗸 📕 🖬 ByLayer 🔹 🚽 —————————————————————————————————	▼				
Point Point <th></th> <th></th> <th></th>					
K K ► N Model (Layout1 (Layout2 /		[] ∢	• 		
Command: _options Command:			- -		
Command:			< III +		
664.8470, 566.9075, 0.0000 SNAP GRID ORTHO POLAR OSNAP OTRACK DYN LWT M	ODEL		S 🗗 🗸		

File Edit View Tools Select	t Geometi	etry Commands Analyze Mode Window Help	
昝 <u>N</u> ew	Ctrl+N	▋ \$\$ ▶	
<mark>≱</mark> <u>O</u> pen	Ctrl+O		
🖆 <u>C</u> lose		▲ 6 Ⅲ 入 № 17 元 6 系 19 9 4 9 4 2	
Open Archive			
Save Archive			
Close Archive		n Concrete Design RAM Connection Bridge Deck Advanced Slab Design Piping	
ProjectWise	•	ucture	(2) - Job Info 🗖 🖻 🔀
			ER DEFINED PROBLEM
Open Backup Manager		Job USE Client	
View	•		
 Job Information 		Job No. 001 Rev.	
🗟 Report Setup		Part	
Printer Setup		Ref	
Print	•	File	
A Print Preview Report		Filename	: c line plan (2).std
Export Report	•	Directory	: l:\civil sem8\project\G+5\
<u>I</u> mport			: 06-Apr-2013 03:54 PM
<u>E</u> xport			:41421 More
Save	Ctrl+S		
Save <u>A</u> s		Engine	er Checker Approved
Run External Program		Date 25-Mai	r-13
Run Technip Pipe Rack Desig	n	Comment	
Recent STAAD Files	•		
Recent STAAD Archives	•		
E <u>x</u> it			
_			
			.
			Help
Y			
X			
mont data from other program		Modeling Mr.	Input Unity KN m

Import data from other programs

File Edit View Tools Select Geometry Commands An	ialyze Mode Window Help		
) 🎽 🖨 🖬 🐇 💼 X 오 ± 오 ± 候 🗉 🚼 🎽	」 등 Q 🖮 📽 ⊑ 등 Q 🗍 🗖 ≜ ଥି 👯 🕂 🕂 ᡤ ዞ 🦻 % ? 🔟 🥂 📗 🖸 % 🧟 🖓 🖉 🖉 🗍 ᡤ ∀ 🗦 🖬 🗳 🗍 ⊑ 🚔 🗒 🛱		
] 🗗 Ø Ø Ø Ø Ø Ø Ô ↔ ↔ ♦ ♦ 0 ↔ ♦] 📰 Q €			
🔨 🕵 🏭 🕼 😺 🤯 🐻	」 🛣 あ 🗰 G 🔳 入 🛛 M 🕾 🕸 🖗 🖉 🖉 🖉 🖗 🗣 🔽 🔽 🗐 🖉 🖳 💭 🗒 🖳		
100	n RAM Connection Bridge Deck Advanced Slab Design Piping		~
k g f c line plan (2) - Whole Structure			📰 c line plan (2) - Job Info 🛛 🗖 🛛 🔀
Image: Second secon			Job USER DEFINED PROBLEM
			Client
eeemetry 1 ≉ Geometry 1 ≉			
			Job No. 001 Rev.
ቱ 🔊			Part
+ <mark>*</mark>	Import		Ref
	③ 3D DXF		File
📧 🖬 🐩 🛃 🔢 📅 🖄	© QSE ASA		Filename : c line plan (2).std
	◯ Stardyne		Directory : I:\civil sem8\project\G+5\
	© CIS/2		Date / Time : 06-Apr-2013 03:54 PM
			File size : 41421 More
	Import Cancel Help		Engineer Checker Approved
			Name
			Date 25-Mar-13
			Comment
			•
			Help
X			
For Holp, pross E1		Modelin	na Ma Input Unita UN m

For Help, press F1

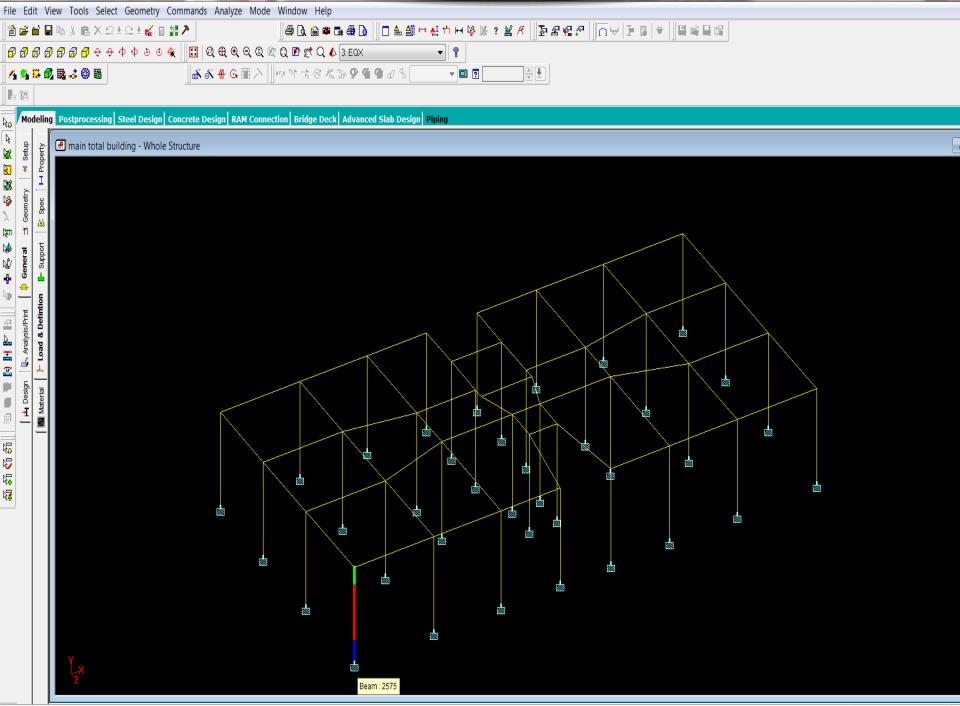
File Edit View Tools Select Geometry Commands Analy	ze Mode Window Help						
) 🏠 🖆 🖬 🐘 🐇 🛍 X Ω ± Ω ± 🔬 🖬 🗱 🏞	°aran an a						
] ि छ छ छ छ छ ि २ २ ० ० ० ० ५]⊞ Q € ९	₹ < © ₡ © № < < ↓						
✓ 呉 幕 御 殿 ☆ 穏 間	」▲ & # G Ⅲ A M M 1 1 6 4 5 9 6 4 9 4 • • •						
Nodeling Postprocessing Steel Design Concrete Design	RAM Connection Bridge Deck Advanced Slab Design Piping		~				
k g f c line plan (2) - Whole Structure			🗉 🔀 🛛 🖬 c line plan (2) - Job Info 🛛 🗖 🔯				
× · · ·			Job USER DEFINED PROBLEM				
etty			Client				
Geometry							
	🛱 Open		Job No. 001 Rev.				
eneral energy (energy)	Look in: 🔒 G+5		Part				
			Ref				
	Name	Date modified 25-03-2013 PM 0	File				
Lat Brand			Filename : c line plan (2).std				
			Directory : l:\civil sem8\project\G+5\				
			Date / Time : 06-Apr-2013 03:54 PM				
	۰. III III III III III III III III III I	•	File size : 41421 More				
	File name: line plan	Open	Engineer Checker Approved				
	Files of type: DXF files (*.dxf)		Name				
		Help	Date 25-Mar-13				
			Comment				
			^				
			v				
			Help				
			, inde				
L×.							

File Edit View Tools S	elect Geometry Commands Analyze Mode Window Help		
) 🏠 🖨 🖬 🖬 🖇 🖪 👌			
11			
∧ , ⊑ , ≝ , Ø , 2 ,	▲ ▲ G Ⅲ 入 10 17 1 1 6		
	ing Steel Design Concrete Design RAM Connection Bridge Deck Advanced Slab Design Piping		
	n (2) - Whole Structure	🖬 c line plan (2) - Job Info 🛛 🗖	
🔀 🗴 🛄		Job USER DEFINED PROBLEM	
Seometry		Client	
		Job No. 001	
		Rev.	
the state of the		Part	
	DXF Import	Ref	
	Structure Convention	File	
E - 국 13 14 같 20 14 14 14 14 14 14 14 14 14 14 14 14 14	No Change	Filename : c line plan (2).std	
THE Number		Directory : l:\civil sem8\project\G	
	© Z ∪p	Date / Time : 06-Apr-2013 03:54 P	M More
Design	OK Cancel Help	File size : 41421	viore
		Engineer Checker Ap	oproved
		Name	
		Date 25-Mar-13	
		Comment	
			*
			▼
		Help	
r z x			

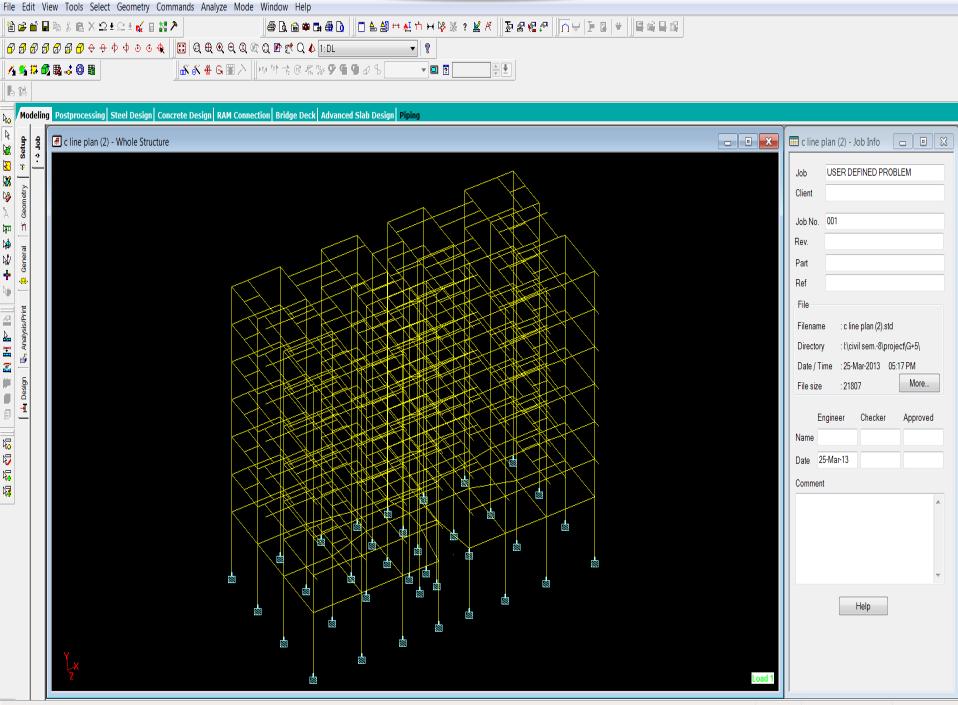
File Edit	ile Edit View Tools Select Geometry Commands Analyze Mode Window Help								
1	i 🗌 A); @ X ≏ ± ≏ ± ॡ́ [22.2] 姜 ि, ` 🛎 🛎 📑 🍕 🗋 🗍 🗖 📥	🎒 🕶 🅂 🖬 🖗	🐹 ? 🗶 🤻 🛛 🕁 #	8 6 7 <u>n</u> 4 F 6	▋▏፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟፟	
0 Ø Ø Ø	P Ø Ø	7 67 6 0 0 0 0 0	🚸 📗 🖽 🛛 🕰 🔍 🔍 🖉	R Q 10 12 Q 4	▼ ?				
1 5 5	<mark>; 6</mark>] 🛛	2 🧈 😳 🛍	🖌 💰 🚸 G 🏼 🔿	M M t C & H 9 9 9 9 9	-				
hä									
ko Mod	eling P	Postprocessing Steel Design	Concrete Design RAM Connec	ction Bridge Deck Advanced Slab De	sign Piping				
	7	Plan - Whole Structure							Image: plan - Job Info Image: State St
🐔 🛐 🐔 ही									Name ZRCHHAYA VSSHAH SII Date 21-Nov-12
									Comment
		× z							

For Help, press F1

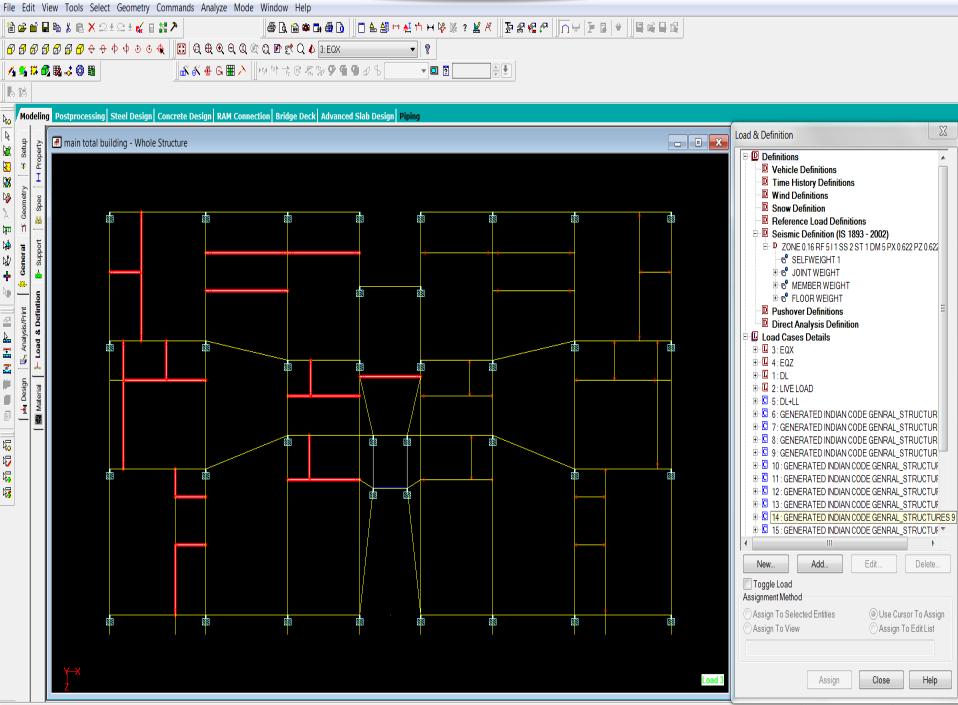
Modeling Mc



Ir

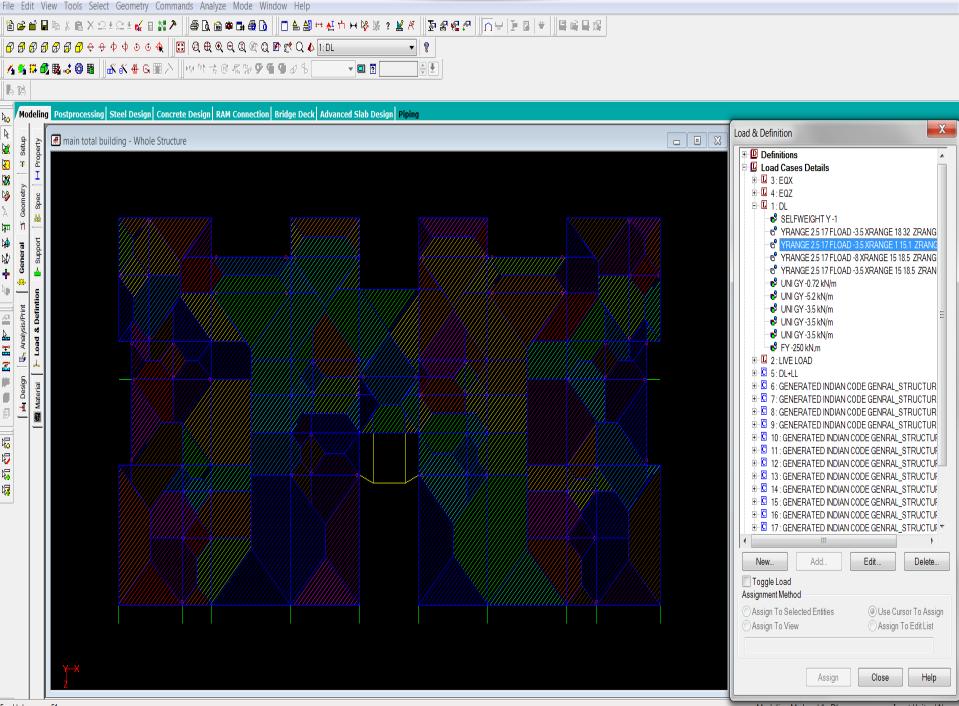


File Edit View Tools Select Geometry Commands Analyze Mode Window Help					
1212212121212121212121	▤▯◙ҩ◨◓▯▯▯◓੬ฃ∺◭ָֽֽֽ∺ױ๙๖๙๙๙ฃ๖๙๙๙๛๚๛				
6 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7 7	@ @ ₽ € Q & 1:DL ▼ ?				
🔏 😼 🛱 🐯 🤯 🏶 🖩 🛛 👘 🕷 🐇 🕀 🖩 🔿	M 5h 元 @ 標 家 9 旬 0 0 、 2				
	<u> </u>				
ko / Modeling Postprocessing Steel Design Concrete Design RAM Conn	ection Bridge Deck Advanced Slab Design Piping				
			🛄 c line plan (2) - Bea		
			Beam Node A Node		Material Beta A
	R		60 78 67	7 1 <mark>COI</mark>	NCRETE 0.
Geometry Spec	RI RURI		61 79 66 62 80 79		NCRETE 0. NCRETE 0.
	RI R		80 95 97 81 97 98		NCRETE 0. NCRETE 0.
General Support			82 94 95 83 98 94		NCRETE 0. NCRETE 0.
	R1 h2 h		91 14 6	1 COI	NCRETE 0.
			106 59 105		NCRETE 0. V
18. Print B. Print			J		
			Properties - Whole S	Structure	X
ign Us. Analy ist T- Load & 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2			Section Beta Angle		
			Ref Section	Material	
Des			1 Rect 0.70x0.23	CONCRETE	•
			2 Rect 0.35x0.20	CONCRETE	
			3 Rect 0.35x0.23 4 Rect 0.45x0.20	CONCRETE CONCRETE	
			5 Rect 0.60x0.23 6 Rect 0.45x0.20	CONCRETE CONCRETE	
			7 Rect 0.45x0.23 8 Rect 0.68x0.23	CONCRETE CONCRETE	
R10 R10			0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	COMODETE	v
			V Highlight Assigned Ge		
IN A R15				Edit	Delete
			Values	Section Database	Define
			Materials	Thickness	User Table
			Assignment Method		
	^{₩22} R17 ^{₩22}		Assign To Selected		e Cursor To Assign ign To View
	超 R15 超		Assign to Ealt List	() Ass	gir to view
Y ×					
Σ		Load 1			
C			A	ssign Close	Help
rui neip, piess ri		1			

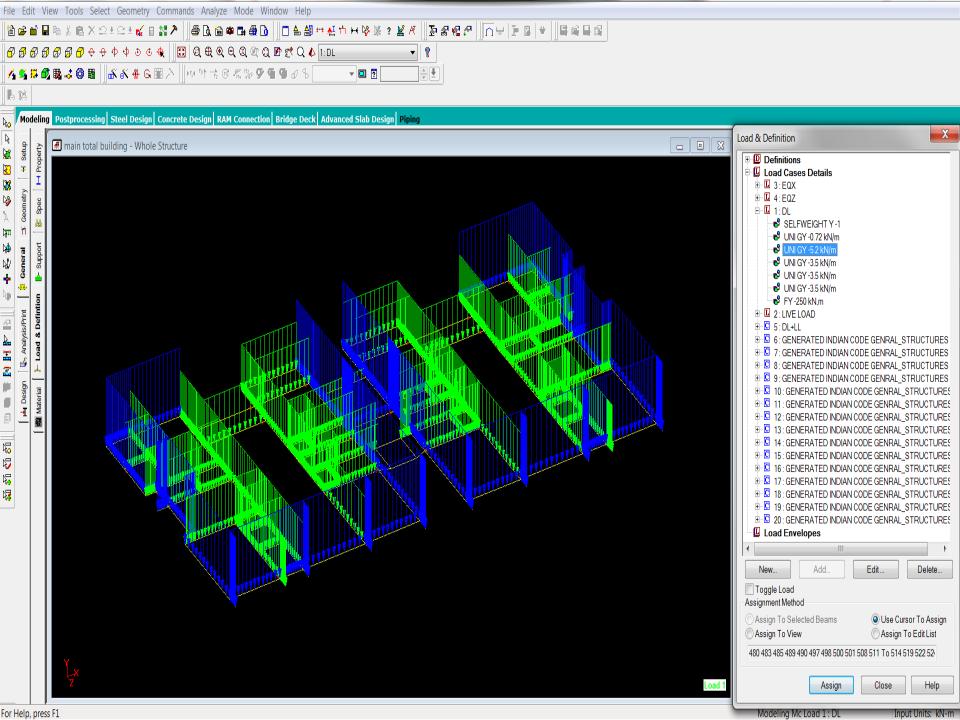


For Help, press F1

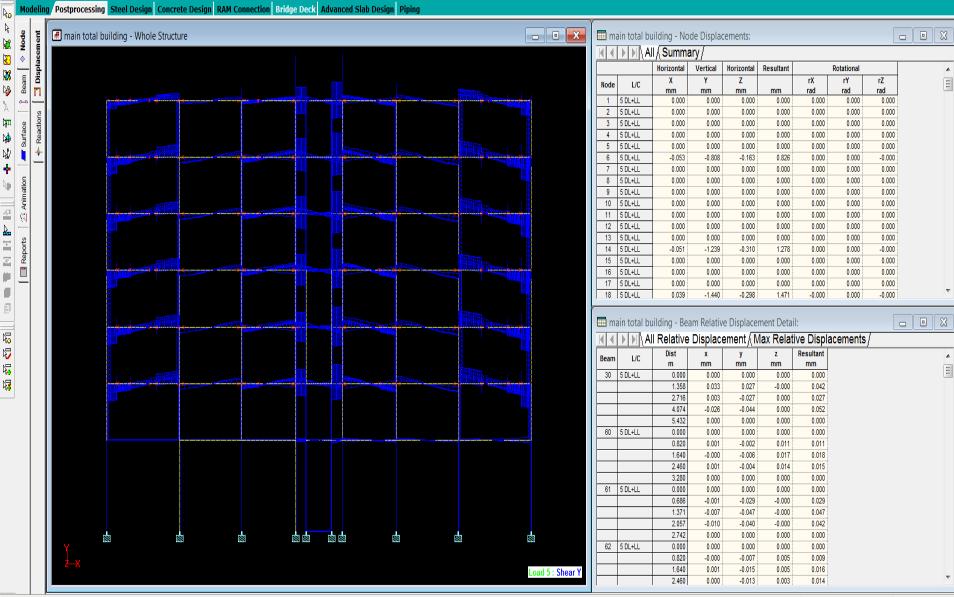
Modeling Mc Load 3 : EOX



For Help, press F1

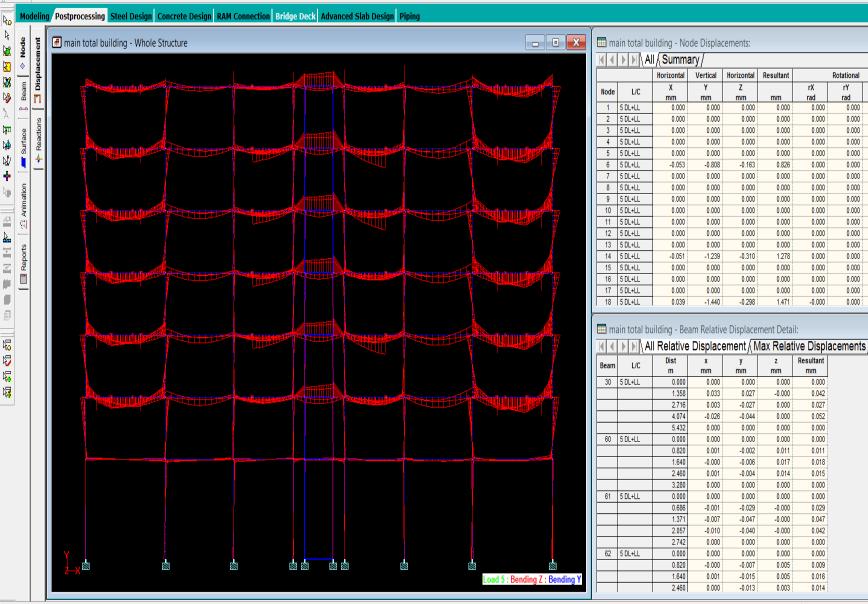


File Edit View Tools Select Results Report	Mode Window Help
ề⊯≓ 🖬 🖩 🐘 🖁 🛍 X 오±오±🛃 🖬 材	》
🗗 🗗 🗗 🗗 🛱 🗗 🗗 🕂 🕂 🗘 🕹 🕹 🍕	□ Q Q Q Q Q Q D 2
🐴 💃 🇱 💕 🗱 🤳 🌐	」≝ & # G Ⅲ > ↓₩♥ ★ @ 編 ๖ 9 ⋐ ⋐ ⊘ \$



File Edit View Tools Select Results Report Mode Window Help	
🋍 🖨 🖬 🖥 🐁 💼 🗙 으로으로 🏑 🖬 🏭 🗡	중 Q, 📾 🕸 ⊑ 중 Q, □ ≜ ᆁ ∺ 赨 ή ↦ 咚 烁 ? 봤 ↗ Ӯ ℛ ☜ 癶 ∩ \\ [ႃ ⊠ ♥ ⊑ ╡ ⊑ 露
₲₱₱₱₱₱₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽₽	Q IP 💇 Q 📣 5: DL+LL 🔹 👻
∕₄ ⊈ ♯ @ 職 尋 ֎ ■	x 🕈 📸 🕼 🦃 🍘 🖉 🖇 💿 💌 🗑 💭

6 18



For Help, press F1

Input Units: kN-m

- • X

rΖ

rad

0.000

0.000

0.000

0.000

0.000

-0.000

0.000

0.000

0.000

0.000

0.000

0.000

0.000

-0.000

0.000

0.000

0.000

-0.000

- • X

.

111

.

Ξ

File E	dit V	/iew Tools Select Geometry Commands Analyze Mode	Window Help		
11	i ni l	■■※■×♀≠♀≠≈≠₩≯	🎒 🖪 🛍 🛎 🖬 🍓 🗋 📋 🗄 🕌 🕶	·쇤市н哆泳?盥옷│Ӯぷ唸?│□↓┣▫▫♥│⊑▫≈⊑▫	
) 6 6	፻፸፼፼ኇቀቀቀቀ୬৫ቈ ∷ ፼ቒቒቒፙ፼	44 44		
11			м № t @ <i>a</i> % 9 6 9 0 8		
	odelin	g Postprocessing Steel Design Concrete Design RAM Connect	tion Bridge Deck Advanced Slab Design	Piping	
中代 後 哲 文 後 酸 Par Marine A	Steel	🖪 main total building - Whole Structure			Steel Design - Whole Structure
N 12					🗉 🗀 Current Code: AASHTO (ASD) 💌
	-				 LOAD COMB 7 GENERATED INDIAN CODE GE LOAD COMB 8 GENERATED INDIAN CODE GE
Set 1		5 STAAD Analysis and Design			E LOAD COMB 9 GENERATED INDIAN CODE GE
Geomet 🥂		++ Finished Reading Member Properties	0 sec		LOAD COMB 10 GENERATED INDIAN CODE G LOAD COMB 11 GENERATED INDIAN CODE G
ा मह	1	++ Surface Element Property ++ Begin Surface 21 of 21	15:47:18		ECAD COMB 12 GENERATED INDIAN CODE G
	5	++ Surface Element Constants	15:47:18		LOAD COMB 13 GENERATED INDIAN CODE G LOAD COMB 14 GENERATED INDIAN CODE G
	3	++ Processing Support Condition. ++ Read/Check Data in Load Cases	15:47:19 15:47:19		E LOAD COMB 14 GENERATED INDIAN CODE G
	<u>e</u>	==> Use In-Core Advanced Solver ++ Processing and setting up Load Vector.	15:47:21		🕀 🛄 LOAD COMB 16 GENERATED INDIAN CODE G
		++ Advanced Solver Factorizing Matrix	15:47:24 15:47:24		LOAD COMB 17 GENERATED INDIAN CODE G LOAD COMB 18 GENERATED INDIAN CODE G
		Factor at equation= 17000 of = 17208 Factorization done, # of non zeros= 2097021	15:47:26 15:47:26		E LOAD COMB 19 GENERATED INDIAN CODE G
1 B 1		++ Advanced Solver Saving displacement ++ Calculating Member Forces.	15:47:26 15:47:27		□ □ LOAD COMB 20 GENERATED INDIAN CODE G □ √ PERFORM ANALYSIS PRINT ALL
÷. 4		++ Analysis Successfully Completed ++			↓ **Design of beams and columns**
		++ Processing Element Forces. ++ Processing Element Corner Forces.	15:47:27 15:47:27		
i 🔳 ä		++ Processing Element Stresses. ++ Performing Concrete Design	15:47:29 15:47:35		
81		++ Calculating Section Forces ++ Creating Displacement File (DSP)	15:47:51		
		++ Creating Reaction File (REA) ++ Calculating Section Forces	15:47:51		
k≩o N⊒		++ Creating Section Force File (BMD)	15:47:53		
12 1 <u>2</u> 12 12		++ SECT DISP member 2605 1230 of 1234 ++ Creating Section Displace File (SCN)	15:47:54		
		++ Creating Element Stress File (EST) ++ Creating Element JT Stress File (EJT)	15:47:54 15:47:55		✓ Highlight Assigned Geometry
		++ Creating Element JT Force File (ECF) ++ Creating Design information File (DGN)	15:47:56		Toggle Assign
		++ Creating Surface Element Force File (SLE) SURFACE 11 OF 21	15:47:58		Select Define Commands
		€ III	•		Parameters Parameters Commands
		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	15:48:12		O Assign To Selected Beams
		0 Error(s), 608 Warning(s)	Abort		O Assign To View
					Use Cursor To Assign Select Group/Deck
					Assign To Edit List
		X			
				Load 5	Assign Close Help
For Hel	p, pres	ss F1			Modeling Mc Load 5 : DL+LL Input Units: kN-m

Out put file of beam

Out put file of column

File Edit View Help		File Edit View Help		
) 📽 🖶 🚑 🚑 17 🔳 K?		j⊯ 🖻 🎒 A II II II 102		
X	792. 2050 TO 2121 2138 TO 2210 2227 TO 2299 2316 TO 2388 2405 TO 2477 2494 TO 2566 -			
WARNING	793. 2583 TO 2609	WARNING DXF IMPORT OF LINE PLAN.DXF PAGE NO. 2134		
**WARNING- JOINT NO. 1(🔺	PAGE 771 Ends Here >	**WARNING- JOINT NO. 1(
**WARNING- THIS STRUCTL =	DXF IMPORT OF LINE PLAN.DXF PAGE NO. 772	**WARNING-THIS STRUCTU		
WARNING about Floor/("WARNING" about FIOD/		
*WARNING- NO MEMBERS		*WARNING-NO MEMBERS MAIN REINFORCEMENT : Provide 4 - 16 dia. (0.92%, 804.25 Sg.mm.)		
**WARNING: IF THIS UBC/IE	BEAM NO. 30 DESIGN RESULTS	**WARNING: IF THIS UBC/IE (Equally distributed)		
**WARNING - THERE WERE		**WARNING - THERE WERE TIE REINFORCEMENT : Provide 8 mm dia. rectangular ties @ 250 mm c/c		
***WARNING - INSTABILITY	M25 Fe415 (Main) Fe415 (Sec.)	***WARNING - INSTABILITY		
***WARNING - INSTABILITY		***WARNING - INSTABILITY SECTION CAPACITY BASED ON REINFORCEMENT REQUIRED (KNS-MET)		
***WARNING - INSTABILITY	LENGTH: 5432.5 mm SIZE: 230.0 mm X 700.0 mm COVER: 40.0 mm	***WARNING - INSTABILITY		
***WARNING - INSTABILITY		***WARNING - INSTABILITY Puz : 1194.38 Muz1 : 31.71 Muy1 : 21.56		
***WARNING - INSTABILITY	SUMMARY OF REINF. AREA (Sg.mm)	***WARNING - INSTABILIT ***WARNING - INSTABILIT INTERACTION RATIO: 0.37 (as per Cl. 39.6, IS456:2000)		
***WARNING - INSTABILITY ***WARNING - INSTABILITY		***WARNING - INSTABILITY INTERACTION RATIO: 0.37 (as per Cl. 39.6, 18456:2000)		
***WARNING - INSTABILITY	SECTION 0.0 mm 1358.1 mm 2716.3 mm 4074.4 mm 5432.5 mm	***WARNING - INSTABILITY SECTION CAPACITY BASED ON REINFORCEMENT PROVIDED (KNS-MET)		
***WARNING - INSTABILITY		***WARNING - INSTABILITY		
***WARNING - INSTABILITY	TOP 308.56 308.56 308.56 308.56 308.56	WORST LOAD CASE: 16		
***WARNING - INSTABILITY	REINF. (Sq. mm) (Sq. mm) (Sq. mm) (Sq. mm) (Sq. mm)	***WARNING - INSTABILITY END JOINT: 224 Puz : 1225.65 Muz : 52.58 Muy : 35.18 IR: 0.67		
***WARNING - INSTABILITY	BOTTOM 308.56 308.56 308.56 308.56 308.56	***WARNING - INSTABILITY		
***WARNING - INSTABILITY	BOTTOM 308.56 308.56 308.56 308.56 308.56 REINF. (Sq. mm) (Sq. mm) (Sq. mm) (Sq. mm)	***WARNING - INSTABILITY		
***WARNING - INSTABILITY	(54. mm) (54. mm) (54. mm) (54. mm)	***WARNING - INSTABILITY ====================================		
***WARNING - INSTABILITY		***WARNING - INSTABILITY		
***WARNING - INSTABILITY		***WARNING - INSTABILITY COLUMN NO. 388 DESIGN RESULTS		
***WARNING - INSTABILITY	SUMMARY OF PROVIDED REINF. AREA	***WARNING - INSTABILITY		
***WARNING - INSTABILITY		***WARNING - INSTABILITY M25 Fe415 (Main) Fe415 (Sec.)		
***WARNING - INSTABILITY	SECTION 0.0 mm 1358.1 mm 2716.3 mm 4074.4 mm 5432.5 mm	***WARNING - INSTABILITY		
***WARNING - INSTABILITY	TOP 4-101 4-101 4-101 4-101	***WARNING - INSTABILITY LENGTH: 2800.0 mm CROSS SECTION: 250.0 mm X 350.0 mm COVER: 40.0 mm		
***WARNING - INSTABILITY	TOP 4-101 4-101 4-101 4-101 4-101 REINF. 1 layer(s) 1 layer(s) 1 layer(s) 1 layer(s) 1 layer(s)	***WARNING - INSTABILITY ** GUIDING LOAD CASE: 4 END JOINT: 42 SHORT COLUMN		
***WARNING - INSTABILITY	Norme - relation (a) - relation (a) - relation (a)	***WARNING - INSTABILITY		
***WARNING - INSTABILITY	BOTTOM 4-101 4-101 4-101 4-101	***WARNING - INSTABILITY		
***WARNING - INSTABILITY	REINF. 1 layer(s) 1 layer(s) 1 layer(s) 1 layer(s) 1 layer(s)	***WARNING - INSTABILITY REQD. STEEL AREA : 700.00 Sq.mm.		
***WARNING - INSTABILITY		***WARNING - INSTABILITY REQD. CONCRETE AREA: 86800.00 Sq.mm.		
***WARNING - INSTABILITY	SHEAR 2 legged 8í	***WARNING - INSTABILITY MAIN REINFORCEMENT : Provide 4 - 16 dia. (0.92%, 804.25 Sq.mm.)		
***WARNING - INSTABILITY	REINF. @ 140 mm c/c	***WARNING - INSTABILITY (Equally distributed)		
***WARNING - INSTABILIT\		***WARNING - INSTABILITY TIE REINFORCEMENT : Provide 8 mm dia. rectangular ties @ 250 mm c/c		
***WARNING - INSTABILITY		***WARNING - INSTABILITY SECTION CAPACITY BASED ON REINFORCEMENT REQUIRED (KNS-MET)		
***WARNING - INSTABILITY	DXF IMPORT OF LINE PLAN.DXF PAGE NO. 773	***WARNING - INSTABILITY		
***WARNING - INSTABILITY		***WARNING - INSTABILITY Puz : 1194.38 Muz1 : 32.07 Muy1 : 21.80		
***WARNING - INSTABILIT) 🔻		***WARNING - INSTABILITY T		
<		IIII INTERACTION RATIO: 0.38 (as per Cl. 39.6, IS456:2000)		
RESULTS	OUTPUT FOR BAR COMBINATION	RESULTS		

Out put file of shear wall

UNARYING	le Edit View Help		
WANKG C FAGE 4804 Ends Here >	ê 🖻 🎒 🗛 📭 🗏 🕅		
WG: THE LOAD COM wG: THE FUNCTION UNE THEORY OF LINE FLAN. DXF FAGE NO. 4805 WG: THE STOREY OF MG: THE STOREY O	X		
NNG: THE LOAD COM NNG: THE STORPY OF SHEANWALL NO. : 1 DESIGN CODE : IS 456-2000 SHEANWALL NO. : 1 DESIGN CODE : IS 456-2000 SHEANWALL NO. : 1 DESIGN CODE : IS 456-2000 SHEANWALL NO. : 1 DESIGN CODE : IS 456-2000 NIG: THE LOAD COM NNG: THE LOAD COM NGC THE LOAD COM NGC	WARNING -	PAGE 4804 Ends Here >	
ING: HE STORY OF	IING : THE LOAD CON 🔺	DXF IMPORT OF LINE PLAN.DXF PAGE NO. 4805	
G:THE STORY DF SHEANMAIL NO. : 1 DESIGN CODE : IS 456-2000 G:THE LOAD COM NIDEN : 1.60 M PC : 30.00 MPA G:THE LOAD COM HEIGHT : 2.80 M PT : 415.00 MPA G:THE LOAD COM HEIGHT : 2.80 M PT : 415.00 MPA G:THE LOAD COM HEIGHT : 2.80 M PT : 415.00 MPA G:THE LOAD COM EOGE ELEMENTS : MIN. REINFORCING RATIO : G:THE LOAD COM MIDTH : 360.00 MH HORIZONTAL : 0.00200 G:THE LOAD COM THICKNESS : 150.00 MH VERTICAL : 0.00200 G:THE LOAD COM THICKNESS : 150.00 MH VERTICAL : 0.00200 G:THE LOAD COM THICKNESS : 150.00 MH VERTICAL : 0.00200 G:THE LOAD COM THICKNESS : 150.00 MH VERTICAL : 0.00200 G:THE LOAD COM ELEVEL GOV.LOAD HORIZONTAL/ VERTICAL : 0.00200 ELEVEL G:THE LOAD COM INFO.FOR (RATIO)/ (RATIO)/ (AREA/PATIO) EDGE/ G:THE LOAD COM UNR . LINK HOR. LINK VER. LINK EDGE/ G:THE LOAD COM INFO.FOR (RATIO)/ (DA J 3 3.75/ 3 NOS. & DIA/ G:THE LOAD COM INFO. REQUIRED NOT REQUIRED	IG : THE LOAD CON		
NG: THE LOAD COM SHEARWALL NO. : 1 1 DESIGN CODE : IS 455-2000 NG: THE LOAD COM NIDTH : 1.80 M PC : 30.00 MPA NG: THE LOAD COM HEIGHT : 2.80 M PY : 415.00 MPA NG: THE LOAD COM EDGE ELEMENTS : MIN. REINFORCING RATIO : NG: THE LOAD COM EDGE ELEMENTS : MIN. REINFORCING RATIO : NG: THE LOAD COM HIDTH : 350.00 MM CORC. COVER : 25.000 MM NG: THE LOAD COM HIDTH : 350.00 MM VERTICAL : 0.00200 THIGNNESS : 150.00 MM VERTICAL : 0.00120 NG: THE LOAD COM HIDTH : 350.00 MM VERTICAL : 0.00120 NG: THE LOAD COM NG: THE LOAD COM REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: 194/149*2) ING: THE LOAD COM NG: THE LOAD COM REET / SEGE// EDGE/ NG: THE LOAD COM NO. FOR (RATIO) / (RATIO) / (RATIO) / (RAEA/FARTIO) NG: THE LOAD COM NG: THE LOAD COM NO. FOR (RATIO) / (COLDING SUMMAR / VERTICAL / EDGE/ EDGE/ NG: THE LOAD COM NO. FOR REATION / URL SUMMAR / VERTICAL / EDGE/ EDGE/ NG: THE LOAD COM NO. FREQUIRED NO. FREQUIRED -0.84 3/ & EDGA /	NG : THE STOREY DF		
NAS THE LOAD COM NAS THE LOAD		SHEARWALL NO. : 1 DESIGN CODE : IS 456-2000	
G:THELOAD COM G:THELOAD COM G:THEL			
G: THE LOAD CON THICKNESS : 150.00 MM CONC. COVER : 25.000 MM G: THE LOAD CON EDGE ELEMENTS : NIN. REINFORCING RATIO : G: THE LOAD CON WIDTH : 360.00 MM VERTICAL : 0.00200 G: THE LOAD CON WIDTH : 360.00 MM VERTICAL : 0.00120 G: THE LOAD CON WIDTH : 360.00 MM VERTICAL : 0.00120 G: THE LOAD CON G: THE LOAD CON REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: NM/NM*2) EDGE/ G: THE LOAD CON G: THE LOAD CON REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: NM/NM*2) EDGE/ G: THE LOAD CON G: THE LOAD CON HOR: JCONTAL / VERTICAL / EDGE/ G: THE LOAD CON HOR: JCONTAL / VERTICAL / EDGE/ G: THE LOAD CON HOR: JCONTAL / VERTICAL / EDGE/ G: THE LOAD CON HOR: JLNR VERTICAL / EDGE/ G: THE LOAD CON HOR: JLNR HOR: LLNR VERTICAL / EDGE/ G: THE LOAD CON G: THE LOAD CON HOR: LLNR VERTICAL / EDGE/ LLNR HOR: STORY DF EDGE/ G: THE LOAD CON G: THE LOAD CON J/ <		WIDTH : 1.80 M FC : 30.00 MPA	
THE LOAD CON FUDER ELEMENTS : MIN. REINFORCING RATIO : THE LOAD CON WIDT : 36.00 MM HORIZONTAL : 0.00200 THE LOAD CON THICKNESS : 150.00 MM VERTICAL : 0.00120 THE LOAD CON THICKNESS : 150.00 MM VERTICAL : 0.00120 THE LOAD CON THE CAD CON REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: MM/MAY2) EDGE/ THE LOAD CON LEVEL GOV.LOAD HORIZONTAL/ VERTICAL/ EDGE/ THE LOAD CON LEVEL GOV.LOAD HORIZONTAL/ VERTICAL/ EDGE/ THE LOAD CON HEY MO. FOR (RATIO)/ (RATIO)/ (AREA/RATIO) THE LOAD CON HEY MO. FOR (RATIO)/ (BATIO)/ (AREA/RATIO) THE LOAD CON HEX HOR. LINK VER. LINK THE LOAD CON THE LOAD CON LINK HOR. LINK VER. LINK THE LOAD CON THE LOAD CON 14/ (0.00201) (0.01601) (150.855/ 0.00070) THE LOAD CON 14/ (0.00201) (0.01862) (150.855/ 0.00070) THE LOAD CON 14/		HEIGHT : 2.80 M FY : 415.00 MPA	
THE LOAD CON EDGE ELEMENTS : MIN. REINFORCING RATIO : THE LOAD CON WIDTH : 360.00 MM HORIZONTAL : 0.00200 THE LOAD CON THICKNESS : 150.00 MM VERTICAL : 0.00120 THE LOAD CON THICKNESS : 150.00 MM VERTICAL : 0.00120 THE LOAD CON THICKNESS : 150.00 MM VERTICAL : 0.00120 THE LOAD CON REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: MM/MC*2) THE THE LOAD CON LEVEL GOV.LOAD HORIZONTAL/ VERTICAL/ EDGE/ EDGE/ THE LOAD CON LEVEL GOV.LOAD HORIZONTAL/ VERTICAL EDGE/ EDGE/ THE LOAD CON HOR. HOR.LINK VER. LINK EDGE/ THE LOAD CON HOR.LINK HOR. LINK VER. LINK EDGE/ THE LOAD CON -0.84 3/ & DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ THE LOAD CON -0.56 3/ & DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ THE LOAD CON -0.56 3/ & DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ THE LOAD CON -0.28 3/ & DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ THE LOAD CON -0.28 3/ & DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ THE LOAD CON -0.28 3/ & DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/		THICKNESS : 150.00 MM CONC. COVER : 25.000 MM	
G. THELOAD CON WIDTH: 360.00 MM HORLZONTAL: 0.00200 G. THELOAD CON THILGANESS: 150.00 MM VERTICAL: 0.00120 G. THELOAD CON G. THELOAD CON REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: MM/MG*2) G. THELOAD CON			
THICKNESS : 150.00 MM VERTICAL : 0.00120 THICKNESS : 150.00 MM VERTICAL			
Int CLAR COM ITHE LOAD COM ITHE STOREY DF EDGE/ ITHE LOAD COM ITHE LO			
G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THELOAD CON G:THE STOREY DF G:THE STOREY DF S:THE STO		THICKNESS : 150.00 MM VERTICAL : 0.00120	
MG: THE LOAD COM REINFORCING SUMMARY (REEAR SPACING/AREA UNITS: MM/MM*2) MG: THE LOAD COM			
MG: THE LOAD CON MG: THE LOAD CON MG: THE LOAD CON LEVEL GOV. LOAD HORIZONTAL/ VERTICAL/ EDGE/ MG: THE LOAD CON MO NO. FOR (RATIO) / (RATIO) / (RAEA/RATIO) MG: THE LOAD CON MO NO. FOR (RATIO) / (RATIO) / (RAEA/RATIO) MG: THE LOAD CON MOR. / VERT. / EDGE/ EDGE/ MG: THE LOAD CON UTINK VERT. / EDGE/ MG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.84 MG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.56 MG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.56 MG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.56 MG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.28 MG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.28 MG: THE LOAD CON NOT REQUIRED	NG : THE LOAD CON		
MG: THE LOAD COW		REINFORCING SUMMARY (REBAR SPACING/AREA UNITS: MM/MM^2)	
MG: THE LOAD COW LEVEL GOV.LOAD HORIZONTAL/ VERTICAL/ EDGE/ MG: THE LOAD COW (M) NO. FOR (RATIO)/ (RATIO)/ (AREA/RATIO) MG: THE LOAD COW HOR./ VERT./ EDGE/ MG: THE LOAD COW VOETT./ EDGE/ MG: THE LOAD COW UNINK HOR. LINK VER. LINK NG: THE LOAD COW -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ NG: THE LOAD COW -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ NG: THE LOAD COW -0.964 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ NG: THE LOAD COW NOT REQUIRED NOT REQUIRED -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ NG: THE LOAD COW -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 25.12/ 3 NOS. & DIA/ NG: THE LOAD COW NOT REQUIRED NOT REQUIRED -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ NG: THE LOAD COW NG: THE LOAD COW NOT REQUIRED NOT REQUIRED -0.28 3/ <td></td> <td></td> <td></td>			
Image: Intel Load Colv Image: Ima	IG : THE LOAD CON		
IG: THE LOAD COW HOR. / IG: THE STOREY DF EDGE/ IG: THE LOAD COW IINK VER. LINK IG: THE LOAD COW -0.84 3/8 BIA @ 167.00/10 10 DIA @ 33.75/3 NOS. & DIA/ IG: THE LOAD COW -0.84 3/8 BIA @ 167.00/10 IDIA @ 33.75/3 NOS. & DIA/ IG: THE LOAD COW -0.84 3/8 BIA @ 167.00/10 IDIA @ 33.75/3 NOS. & DIA/ IG: THE LOAD COW -0.84 3/8 BIA @ 167.00/10 IDIA @ 33.75/3 NOS. & DIA/ IG: THE LOAD COW -0.56 3/8 BIA @ 167.00/16 IDIA @ 33.75/3 NOS. & DIA/ IG: THE LOAD COW -0.56 3/8 BIA @ 167.00/16 IDIA @ 77.14/3 NOS. & DIA/ IG: THE LOAD COW -0.28 3/8 BIA @ 167.00/16 IDIA @ 25.12/3 NOS. & DIA/ IG: THE LOAD COW -0.28 3/8 BIA @ 167.00/10 IDIA @ 25.12/3 NOS. & DIA/ IG: THE LOAD COW -0.28 3/8 BIA @ 167.00/10 IDIA @ 25.12/3 NOS. & DIA/ IG: THE LOAD COW -0.28 3/8 BIA @ 167.00/10 IDIA @ 25.12/3 NOS. & DIA/	IG : THE LOAD CON	LEVEL GOV.LOAD HORIZONTAL/ VERTICAL/ EDGE/	
3: THE STOREY DF VERT. / 5: THE STOREY DF LINK HOR. LINK VER. LINK 5: THE LOAD CON -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ 5: THE LOAD CON -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ 5: THE LOAD CON -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ 5: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 10 DIA @ 77.14/ 3 NOS. & DIA/ 5: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ 5: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ 5: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ 5: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ 5: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ 6: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ 6: THE LOAD CON	G : THE LOAD CON	(M) NO. FOR (RATIO) / (RATIO) / (AREA/RATIO)	
ETHE STOREY DF EDGE/ S: THE STOREY DF LINK HOR. LINK S: THE LOAD CON -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ S: THE LOAD CON -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ S: THE LOAD CON -0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. & DIA/ S: THE LOAD CON 14/ (0.00201) (0.01601) (150.855/0.00070) S: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ S: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. & DIA/ S: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. & DIA/ S: THE LOAD CON 3/ NOT REQUIRED NOT REQUIRED	G : THE LOAD CON		
1. THE LOAD COW 5: THE LOAD COW 5: THE LOAD COW 6: THE LOAD COW 5: THE LOAD COW 6: THE LOAD COW	G : THE STOREY DF		
3: THE LOAD COM 0: THE LOAD COM	G : THE STOREY DF		
-0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. 8 DIA/ 14/ (0.00201) (0.01601) (150.855/0.00070) 3/ THE LOAD CON THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. 8 DIA/ -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. 8 DIA/ THE LOAD CON THE CON THE LOAD CON TH	: THE LOAD CON	LINK HOR. LINK VER. LINK	
3: THE LOAD CON 14/ (0.00201) (0.01601) (150.855/0.00070) 3/ NOT REQUIRED 3: THE LOAD CON NOT REQUIRED 5: THE LOAD CON		-0.84 3/ 8 DIA @ 167.00/ 10 DIA @ 33.75/ 3 NOS. 8 DIA/	
3/ 3/ 3/ NOT REQUIRED NOT REQUIRED NOT REQUIRED 3/ NOT REQUIRED	G : THE LOAD CON		
IG: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. 8 DIA/ IG: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. 8 DIA/ IG: THE LOAD CON 14/ (0.00201) (0.01862) (150.855/0.00070) IG: THE LOAD CON 3/ IG: THE LOAD CON NOT REQUIRED IG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ IG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ IG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ IG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ IG: THE LOAD CON -0.28 3/ 8 DIA @ NOT REQUIRED -0.00070) IG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.00070) IG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.00070) IG: THE LOAD CON NOT REQUIRED NOT REQUIRED -0.00070) IG: THE LOAD CON NOTE : ALL HEIGHTS ARE IN LOCAL CO			
NG: THE LOAD CON -0.56 3/ 8 DIA @ 167.00/ 16 DIA @ 77.14/ 3 NOS. 8 DIA/ NG: THE LOAD CON 14/ (0.00201) (0.01862) (150.855/0.00070) NG: THE LOAD CON 3/ NOT REQUIRED -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ NG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ NG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ NG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ NG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ NG: THE LOAD CON		NOT REQUIRED NOT REQUIRED	
3: THE LOAD CON 14/ (0.00201) (0.01862) (150.855/0.00070) 3/ NOT REQUIRED 3/ NOT REQUIRED 1: THE LOAD CON 0.028 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ 1: THE LOAD CON 14/ (0.00201) (0.02134) (150.855/0.00070) 1: THE LOAD CON 14/ (0.00201) (0.02134) (150.855/0.00070) 3/ NOT REQUIRED -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ 1: THE LOAD CON 14/ (0.00201) (0.02134) (150.855/0.00070) 3/ NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED			
G:THE LOAD CON G:THE LOAD CON MOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIR			
NG: THE LOAD CON NOT REQUIRED NG: THE LOAD CON -0.28 3/ 8 DIA @ 167.00/ 10 DIA @ 25.12/ 3 NOS. 8 DIA/ NG: THE LOAD CON 14/ (0.00201) (0.02134) (150.855/ 0.00070) NG: THE LOAD CON 3/ NG: THE LOAD CON NOT REQUIRED NOT REQUIRED NG: THE LOAD CON NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT THE LOAD CON NOT REQUIRED NOT REQUIRED NOT THE LOAD CON NOT REQUIRED NOT REQUIRED NOT THE LOAD CON NOT REQUIRED NOT REQUIRED NOTE : ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	IG : THE LOAD CON		
NG :THE LOAD COW NG : THE LOAD COW NG : THE LOAD COW NG : THE LOAD COW NOT REQUIRED NOT REQUIRED NOT REQUIRED NOT E : ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	NG : THE LOAD CON		
G:THE LOAD CON G:THE LOAD CON HOT REQUIRED NOT REQUIRED NOT REQUIRED NOT E: ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	G : THE LOAD CON	NOT REQUIRED NOT REQUIRED	
NG: THE LOAD COW 14/ (0.00201) (0.02134) (150.855/0.00070) NG: THE LOAD COM 3/ NG: THE LOAD COM NOT REQUIRED NG: THE LOAD CON NOT REQUIRED NG: THE LOAD CON NOT REQUIRED NOTE : ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	NG : THE LOAD CON		
ING:THE LOAD CON ING:THE LOAD CON ING:THE LOAD CON ING:THE LOAD CON ING:THE LOAD CON NOTE : ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	IING : THE LOAD CON		
IING:THE LOAD CON NOT REQUIRED NOT REQUIRED IING:THE LOAD CON NOTE : NOTE : IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII	IING : THE LOAD CON		
NING: THE LOAD CON VING: THE SURFACE.	NING : THE LOAD CON		
ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	ING : THE LOAD CON	· · ·	
		NOTE :	
RESULTS NUMBER OF REINFORCING LAYERS IN EACH DIRECTION : 1		ALL HEIGHTS ARE IN LOCAL COORDINATE SYSTEM OF THE SURFACE.	
	RESULTS	NUMBER OF REINFORCING LAYERS IN EACH DIRECTION : 1	

Column design as per IS : 456:2000

File Edit View Help		
🖻 🕒 🎒 🗛 📭 🗐 😣		
WARNING	SECTION CAPACITY BASED ON REINFORCEMENT REQUIRED (KNS-MET)	*
RESULTS	Puz : 1850.91 Muz1 : 74.54 Muy1 : 47.65	
TOTAL APPLIED LOAD 3 TOTAL REACTION LOAD 3 TOTAL APPLIED LOAD 4	INTERACTION RATIO: 0.94 (as per Cl. 39.6, IS456:2000)	
TOTAL REACTION LOAD 4 TOTAL APPLIED LOAD 1 TOTAL REACTION LOAD 1 TOTAL APPLIED LOAD 2 TOTAL REACTION LOAD 2 CONCRETE DESIGN	DXF IMPORT OF LINE PLAN.DXF PAGE NO. 2209	
ANALYSIS RESULTS MEMBER FORCES ALL	COLUMN NO. 1978 DESIGN RESULTS	
SUPPORT REACTION ALL STORY DRIFT	M25 Fe415 (Main) Fe415 (Sec.)	
SHEARWALL DESIGN	LENGTH: 4500.0 mm CROSS SECTION: 300.0 mm X 680.0 mm COVER: 40.0 mm	
	** GUIDING LOAD CASE: 4 SHORT(Z) /BRACED LONG(Y)	H
	REQD. STEEL AREA : 1632.00 Sq.mm. REQD. CONCRETE AREA: 202368.00 Sq.mm. MAIN REINFORCEMENT : Provide 16 - 12 dia. (0.89%, 1809.56 Sq.mm.) (Equally distributed) TIE REINFORCEMENT : Provide 8 mm dia. rectangular ties @ 190 mm c/c SECTION CAPACITY BASED ON REINFORCEMENT REQUIRED (KNS-MET)	
	Puz : 2784.60 Muz1 : 165.72 Muy1 : 65.57	
	INTERACTION RATIO: 0.44 (as per Cl. 39.6, IS456:2000)	
	COLUMN NO. 1979 DESIGN RESULTS	
	M25 Fe415 (Main) Fe415 (Sec.)	
	LENGTH: 4500.0 mm CROSS SECTION: 300.0 mm X 350.0 mm COVER: 40.0 mm	
		•

Column design as per IS : 13920

File Edit View Help		
📽 🖻 🎒 🚜 📭 🗏 🕺		
10	<pre>****NOTE: SOME OF THE BEAMS CONNECTED TO THE COLUMN NO. 1978 ARE NOT DESIGNED. HENCE ONLY SHEAR FORCE FROM ANALYSIS WILL BE CONSIDERED FOR SHEAR DESIGN. C O L U M N NO. 1978 D E S I G N R E S U L T S</pre>	A
TOTAL REACTION LOAD 2 ANALYSIS RESULTS MEMBER FORCES ALL SUPPORT REACTION ALL STORY DRIFT SHEARWALL DESIGN	M25 Fe415 (Main) Fe415 (Sec.) LENGTH: 4500.0 mm CROSS SECTION: 300.0 mm X 680.0 mm COVER: 40.0 mm ** GUIDING LOAD CASE: 4 SHORT(Z) /BRACED LONG(Y)	
CONCRETE DESIGN	<pre>REQD. STEEL AREA : 1632.00 Sq.mm. REQD. CONCRETE AREA: 202368.00 Sq.mm. MAIN REINFORCEMENT : Provide 16 - 12 dia. (0.89%, 1809.56 Sq.mm.) (Equally distributed) CONFINING REINFORCEMENT : Provide 10 mm dia. rectangular ties @ 75 mm c/c over a length 750.0 mm from each joint face towards midspan as per Cl. 7.4.6 of IS-13920. 4 number overlapping hoop along with crossties are provided along Y direction. (Clause 7.3.2 of IS-13920) TIE REINFORCEMENT : Frovide 8 mm dia. rectangular ties @ 150 mm c/c</pre>	
	SECTION CAPACITY BASED ON REINFORCEMENT REQUIRED (KNS-MET)	
	COLUMN NO. 1979 DESIGN RESULTS	=

3D view

