



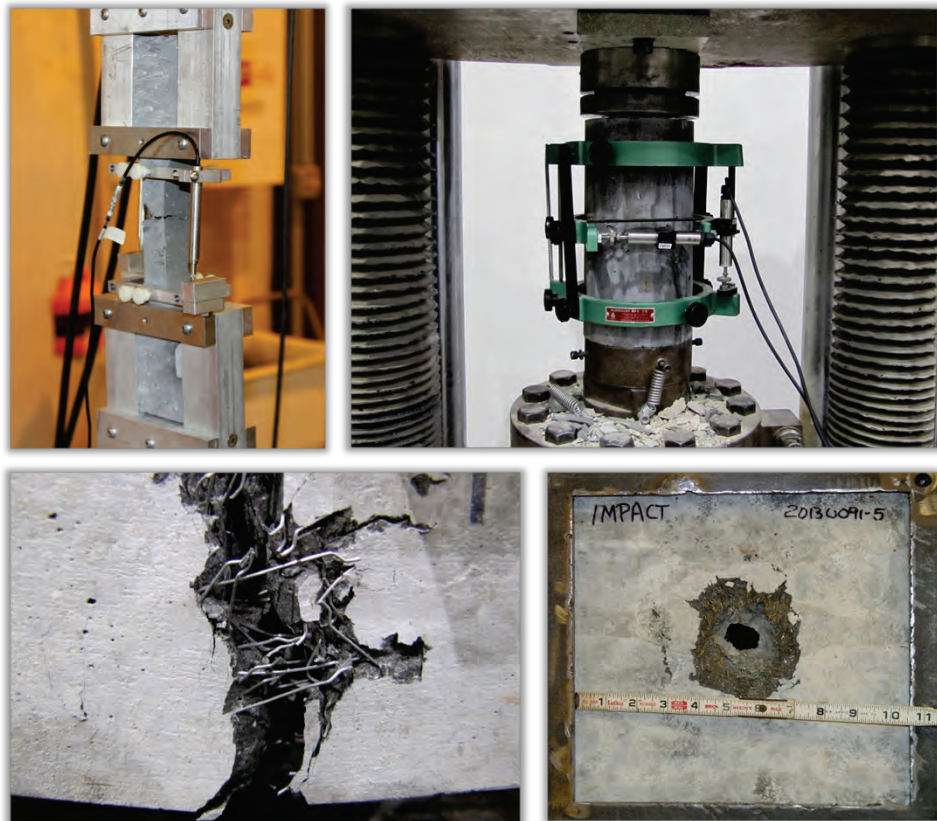
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# Impact of Steel Fiber Size and Shape on the Mechanical Properties of Ultra-High Performance Concrete

Dylan A. Scott, Wendy R. Long, Robert D. Moser,  
Brian H. Green, James L. O'Daniel, and Brett A. Williams

August 2015



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## Abstract

This investigation focused on identifying the impact of various steel fiber types, including size and shape, on the mechanical responses of an ultra-high performance concrete (UHPC) known as Cor-Tuf Baseline (CTB). CTB specimens were fabricated with four fiber types. The experimental program included testing for quasi-static and dynamic properties, focusing on penetration resistance. Fiber shape and size had a limited impact on quasi-static properties in compression but had a significant impact on quasi-static tensile properties and dynamic penetration resistance. The smaller fibers exhibited up to a 100 percent increase in tensile strength compared with their larger counterparts. However, the benefits offered by the smaller fibers primarily occurred prior to reaching the ultimate load carrying capacity. Once the ultimate strength was reached, larger fibers were more effective at bridging larger cracks. The smaller fibers also provided improved penetration resistance, with reduced residual projectile velocities and loss of material from cratering and/or spallation. Fiber shape and size did not have an impact on the transition between projectile perforation and nonperforation. The results provide insights into optimal fiber reinforcement and potential strategies to further improve the properties of UHPCs, make fiber-reinforced UHPCs more cost competitive by altering fiber dosage rates, and develop hybrid reinforcement schemes.

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## **Preface**

This study was conducted for the Technical Support Working Group (TSWG). The technical monitor was James L. O'Daniel.

The work was performed by the Concrete and Materials Branch (CMB) of the Engineering Systems and Materials Division (ESMD) and Structural Mechanics Branch (SMB) of Geosciences and Structures Division (GSD), U.S. Army Engineer Research and Development Center, Geotechnical and Structures Laboratory (ERDC-GSL). At the time of publication, Christopher M. Moore was Chief, CMB; Brad Steed was Acting Chief, SMB; Dr. Larry N. Lynch was Chief, ESMD; Bartley P. Durst was Chief, GSD; and Pamela Kinnebrew was the Technical Director for Military Engineering. The Acting Deputy Director of ERDC-GSL was Dr. Gordon W. McMahon, and the Acting Director was Dr. William P. Grogan.

LTC John T. Tucker III was the Acting Commander of ERDC, and Dr. Jeffery P. Holland was the Director.



## Unit Conversion Factors

Multiply	By	To Obtain
cubic feet	0.02831685	cubic meters
cubic yards	0.7645549	cubic meters
degrees Fahrenheit	(F-32)/1.8	degrees Celsius
feet	0.3048	meters
gallons (US liquid)	3.785412 E-03	cubic meters
inches	0.0254	meters
mils	0.0254	millimeters
ounces (mass)	0.02834952	kilograms
ounces (US fluid)	2.957353 E-05	cubic meters
pounds (force)	4.448222	newtons
pounds (force) per foot	14.59390	newtons per meter
pounds (force) per inch	175.1268	newtons per meter
pounds (force) per square inch	6.894757	kilopascals
pounds (mass)	0.45359237	kilograms
pounds (mass) per cubic foot	16.01846	kilograms per cubic meter
pounds (mass) per square foot	4.882428	kilograms per square meter
yards	0.9144	meters

# 1 Introduction

## 1.1 Background

Ultra-high performance concrete (UHPC) is a family of materials that typically exhibit compressive strengths in excess of 21,000 psi (150 MPa) and high durability due to negligible interconnected porosity. High toughness is achieved with the addition of fiber reinforcement. UHPC formulations generally consist of a high cementitious content incorporating oil-well or low-heat portland cement (i.e., with large mean particle size, high C<sub>2</sub>S content, and low C<sub>3</sub>A content), siliceous or aluminous fine aggregates, crushed quartz or some other micrometer-sized powder, silica fume, water, high-range water-reducing admixtures to control rheology, and other components that vary by manufacturer. With the high compressive strengths of UHPC comes brittle behavior similar to that of a ceramic material. To overcome this brittle behavior, steel fiber reinforcement is commonly used. The addition of steel fiber reinforcement aids in delocalizing micro- and macro-scale cracking and leads to improvements in tensile properties (e.g., tensile strength and flexural response) and minimized spallation during failure.

Various UHPC formulations exist, with the majority being developed by manufacturers. The matrices (i.e., the portions of the UHPC excluding the fiber reinforcement) are generally similar in composition and basic mechanical properties, such as compressive and tensile strength. The variations observed in UHPC failure morphology when subjected to extreme loading events are hypothesized to largely be related to the steel fiber reinforcement—including its strength, length and diameter-- and any deformations present that provide mechanical interlock with the matrix. Common steel fibers used in various UHPC formulations include the following:

- Straight brass-coated steel with lengths of 10 to 13 mm, diameters of approximately 0.2 mm, and tensile strengths in excess of 300,000 psi (2070 MPa).
- Hooked-end steel fibers with lengths of 25 to 30 mm, diameters of approximately 0.5 mm, and tensile strengths in excess of 150,000 psi (1030 MPa).

Many other fiber types, such as undulated/wavy steel fibers and steel fibers with a helical twist, are also viable options for use in UHPCs. In both cases, the primary goal is for the mechanical interlock between the fiber and the UHPC matrix to aid in the engagement of the fiber and the delocalization of damage.

While many UHPC/fiber combinations are commercially available or produced in-house, there has been only limited study of the impact of steel fiber characteristics (i.e., size, shape, and tensile properties) on the mechanical response of UHPC when the same matrix is used.

## **1.2 Cor-Tuf baseline UHPC**

As previously described, many UHPC formulations exist, including those available from commercial vendors and those developed in-house. Cor-Tuf Baseline (CTB) is one formulation of UHPC developed by engineers at the U.S. Army Engineer and Research Development Center (ERDC) under the name Cor-Tuf. This family of UHPCs is a result of more than three decades of work and investigation into the use of UHPCs and the creation of subsequent adaptations to better suit the material for a variety of applications for both military and civil infrastructures.

Stemming from research in the late 1980s and early 1990s, CTB is a generalized version of Cor-Tuf that utilizes knowledge gained from previously developed technologies such as Reactive Powder Concrete (RPC) 200 and Very-High-Strength Concrete (VHSC). RPC 200 was developed by Bouygues S.A. and first documented by Richard and Cheyrezy (1995). ERDC researchers Roy L. Campbell Sr. and Edward F. O'Neil investigated this UHPC in 1998 (Campbell et al. 1998). These UHPC mixtures were designed and produced with all the constituent materials (excluding fibers) being "powder-sized." VHSC was first documented and developed by Donald M. Walley and Billy D. Neeley of the then Concrete and Materials Division in and prior to 1995 (Neeley and Walley 1995). VHSC differed from the RPC mixtures by its use of fine aggregates that meet the gradation requirements for concrete sand. In the late 1990s, VHSC continued to be refined and optimized by ERDC researchers Dr. Edward O'Neil and Dr. Beverly DiPaolo and gave rise to the UHPC family of Cor-Tuf (O'Neil 2008). A more detailed history of each of these mixtures can be found in Green et al. (2013).

Green et al. detailed CTB as being "developed to serve as, and is currently considered by ERDC to be, a 'laboratory standard' UHPC mixture, that can

be reproduced for various projects and exhibit the same physical properties with minimal batch-to-batch variability” (O’Neil 2008). CTB’s constituent materials (excluding fibers) include a Type H oil-well cement, silica fume, silica sand, crushed silica sand (also known as silica flour), and a polycarboxylate type high-range water-reducing admixture (HRWRA). These are brand specified materials at specified proportions that allow for a highly reproducible mixture with little variability between mixtures. Table 1 shows the mixture proportions for CTB.

Table 1. CTB mixture proportion.

Material	Proportion by Weight
Cement	1.00
Silica Fume	0.389
Silica Flour	0.277
Silica Sand	0.967
HRWRA	0.0171
Water	0.208
Steel Fibers	0.310

CTB typically has an unconfined compressive strength (UCS) between 28 and 32 ksi (193 to 220 MPa) and a density of approximately 160 lb/ft<sup>3</sup> (2563 kg/m<sup>3</sup>). With this high strength, CTB (as well as most other UHPCs) has a high brittleness that can be counteracted by the inclusion of randomly distributed steel fibers.

Up to this point, the only steel fiber included in CTB was the ZP 305 fiber produced by the Bekaert Corporation. This fiber has recently been renamed as 3D 55/30 BG fiber in order to coincide with the Bekaert Corporation’s recently released brands of fibers. The fiber itself is 0.022 in. (0.55 mm) in diameter and 1.18 in. (30 mm) in length with hooked ends and is included into CTB mainly because of its relatively low cost and easy attainability. Mechanical testing performed at ERDC by Roth et al. (2010) compared the compressive, flexural, and splitting tensile properties of CTB with and without the inclusion of the ZP 305 fibers. The results of this testing showed that when steel fibers were included in the Cor-Tuf matrix, a slight increase occurred in compressive strength, a 162 percent increase occurred in flexural strength, and a 240 percent increase occurred in splitting tensile strength. An improvement in the mode of failure was also observed. CTB without fibers resulted in a catastrophic and brittle

failure as opposed to the CTB with fiber matrix that held together after fracture and absorbed more energy.

These results proved that a randomly distributed steel fiber-reinforced UHPC matrix is far superior to a UHPC matrix without fibers. However, little to no work has been done to provide insight into how the size and shape of steel fiber reinforcement could change the behavior of a UHPC matrix as a whole.

### **1.3 Scope**

In order to address the dearth of information relating the characteristics of steel fiber reinforcement to the mechanical properties of high-strength concretes, this study investigated four commercially available off-the-shelf steel fibers in a consistent CTB matrix. These fibers consisted of two brass-coated straight fibers, a hooked-end fiber, and an undulated fiber. The experimental program investigated quasi-static properties including UCS, elastic modulus and Poisson's ratio, flexural response, splitting tensile strength, and direct tensile strength. Dynamic testing included penetration testing using fragment simulating projectiles (FSPs). The overall goal of the study was to identify the relationships between fiber characteristics and the multistrain rate response of UHPCs in order to better optimize fiber reinforcement for various loading conditions.

## 2 Experimental Program

All tested specimens utilized a consistent UHPC matrix of the CTB formulation. Keeping the UHPC matrix constant ensured that a direct comparison could be made between fiber types. Part of this procedure included a consistent curing regime and testing age of each of the four test matrices. The curing regime consisted of seven days of 72°F curing inside a fog room with 100 percent humidity followed by seven days of steam curing at a temperature of 190°F (90°C). The testing age of each CTB matrix was between 26 and 29 days. The slight variance in age was due to the large amount of testing that was conducted and the limited amount of testing equipment. This age variance should have had little effect on test results.

Testing of specimens was performed at quasi-static and dynamic strain rates using the methods described in the following sections. The overall scope of the experimental program is shown in Table 2.

Table 2. Overview of experimental program.

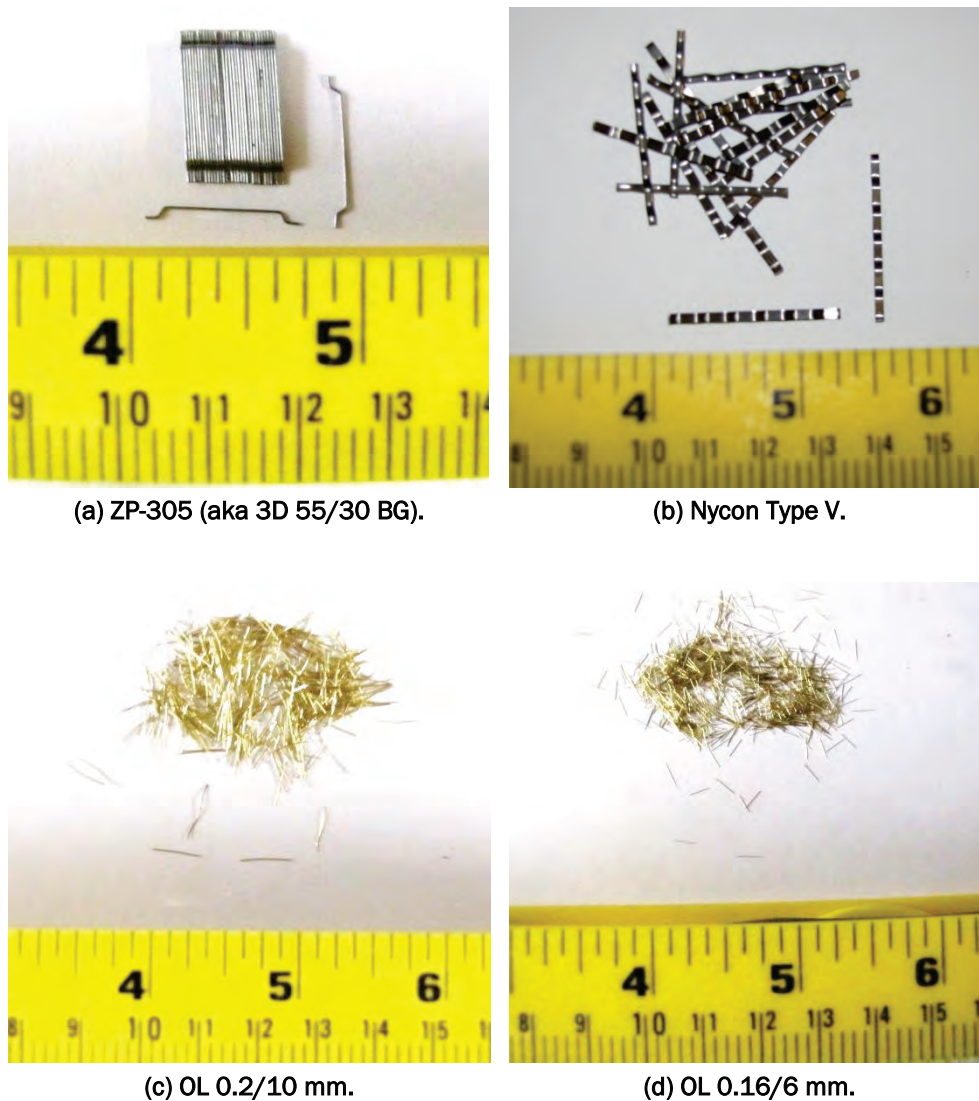
Experiments for each UHPC - Fiber combination	
Test Method	Notes
Compressive Strength	Unconfined compressive strength of 4- by 8-in. cylinders
Flexural Response	Flexural testing of 6- by 6- by 21-in. beams
Split Tensile	Splitting tensile strength of 4- by 8-in. cylinders
Direct Tensile Testing	Direct tensile testing of water-jet cut-out and cast dog bone samples
Elastic Modulus/Poisson's ratio	Elastic modulus test of 6- by 12-in. cylinders using LVDTs
FSP / Penetration	FSP testing of 2-in.-, 2.5-in.-, and 3-in.-thick panels

Note: The target for all testing was ~28-day mechanical properties.

### 2.1 Steel fiber types

This section will discuss all of the fiber types, the intent of their design, and their possible effects on the CTB matrix. Additional details including data sheets from manufacturers are in Appendix A. The fibers included in this report are the ZP 305 (Bekaert Co.), the Nycon Type V (Nycon Co.), the OL 0.2/10mm (Bekaert Co.), and the OL 0.16/6mm (Bekaert Co.). These fibers are shown in Figure 1.

Figure 1. Fiber types investigated.



The ZP 305 (3D 55/30 BG) fiber previously discussed is currently the standard fiber used in the CTB matrix. This fiber is 30 mm long and 0.5 mm in diameter. The hooked ends of these fibers are designed to “anchor” into the concrete specimen; and, as the specimen fractures, these hooks/anchors cause the fiber to stretch and eventually fail under tension in the midsection of the fiber. This failure mode sounds effective in theory (and some of the fibers do fail this way), but due to the brittleness of CTB, a large portion of the matrix appears to crumble around the fibers’ hooked ends upon fracture. This suggests that a weak Interfacial Transition Zone (ITZ) exists between steel fibers and UHPCs.

The Nycon Type V is an undulated (wavy) low-carbon steel fiber, which is 1.5 in. (38 mm) long and has a 0.046-in. (1.18-mm) filament. Instead of using the hooked ends to anchor the fiber, the waves are designed to straighten as the matrix begins to fail and the fiber begins to pull out. These waves result in higher fiber pullout strengths that allow for more slippage, thus making the entire material more ductile. This fiber encounters some of the same problems as the ZP 305. A weak ITZ between the fiber and the CTB/UHPC matrix leads to many of the fiber waves' being rendered useless due to a material crumbling around the fiber.

The next two fiber types are small, straight, brass-coated steel fibers. The Bekaert OL .2/10mm fiber is 0.008 in. (0.2 mm) in diameter and is 0.39in. (10 mm) in length. The Bekaert OL .16/6mm fiber was 0.0063 in. (0.16 mm) in diameter and 0.24 in. (6 mm) in length. The possible advantages of these fibers are their high surface area and small volume. Since fiber dosages are made on a standard percent by weight basis, replacement of larger fibers such as ZP 305s with smaller OL fibers results in a greater density of fibers dispersed in the UHPC matrix. The high-surface areas make it more likely that, upon failure, the crack propagation will encounter several fibers and possibly stop the fracture. Figure 2 shows cutouts of the flex beams that help to illustrate this point. The disadvantage of these fibers is their short length. The shorter lengths make the fiber unable to "bridge" a crack upon significant failure. These shorter OL fibers could be beneficial in the micro-cracking regime, but their inability to bridge a macro-crack renders them useless. Binary and ternary fiber blends of smaller and larger sizes could provide benefits for both micro- and macro-cracking.

Figure 2. Cross sections of UHPCs illustrating differences in fiber distribution.

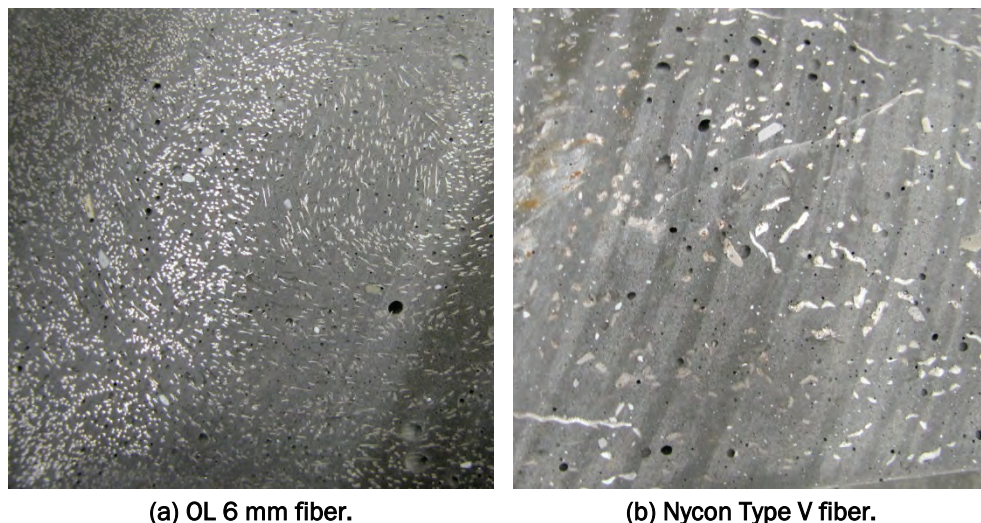




Table 3 shows each fiber's tensile strength and respective cost. The Nycon fibers are the least expensive, while the OL fibers are most expensive at about one dollar per pound more than the ZP 305. Therefore, it will be hard to fiscally justify use of the OL fibers. However, single companies provided these costs, and it is likely possible to find different sources with lower costs.

Table 3. Tensile strength and cost of fiber types investigated.

Fiber Type	Tensile Strength (ksi)	Cost (\$/lb)
Bekaert ZP 305	195	1.18
Nycon Type V	100	0.87
Bekaert OL 0.2/10mm	370	2.45
Bekaert OL 0.16/6 mm	370	2.45

## 2.2 Experimental methods

### 2.2.1 Compression: Strength, elastic modulus, and Poisson's ratio

The UCS and elastic modulus/Poisson's ratio (E&P) testing was performed to ensure that each CTB matrix met the first condition of the testing procedure. These test methods and all subsequent test methods were followed strictly and consistently to ensure that the second testing condition was met. The UCS and E&P tests were conducted according to ASTM C873 (ASTM International 2010a) and ASTM C469 (ASTM International 2010b), respectively. Photographs of the test setups are in Figures 3 and 4, respectively. Collecting quality data for the Poisson's ratios proved difficult, as the high hardness of the CTB made it difficult for the modulus testing apparatus to sit securely on the test specimen throughout the entire testing procedure, resulting in slip during the test. This will be discussed further in the results section.

### 2.2.2 Flexural response

The flexural response testing was performed according to ASTM C1609 (ASTM International 2012). This test used a beam with four-point loading. The cast beams were 6 by 6 by 21 in. (150 by 150 by 500 mm) and were cast according to ASTM C192 (ASTM International 2013). They were tested on an 18-in. (450-mm) support span. Linear variable differential transformers (LVDTs) were used to measure center-line displacement that was in turn paired with the corresponding load data to provide a plot of load versus displacement. Figure 5 shows this testing setup, both pre-test and post-test.

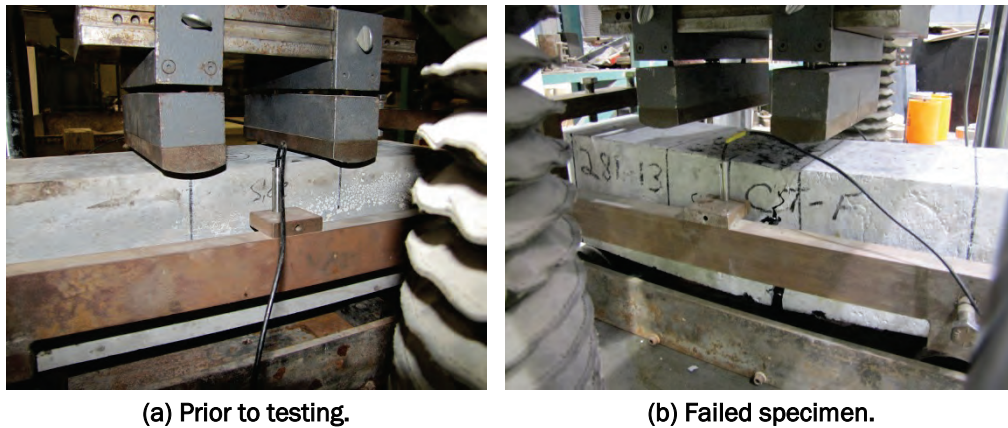
Figure 3. ASTM C873 unconfined compressive strength test setup.



Figure 4. ASTM C469 elastic modulus and Poisson's ratio test setup.



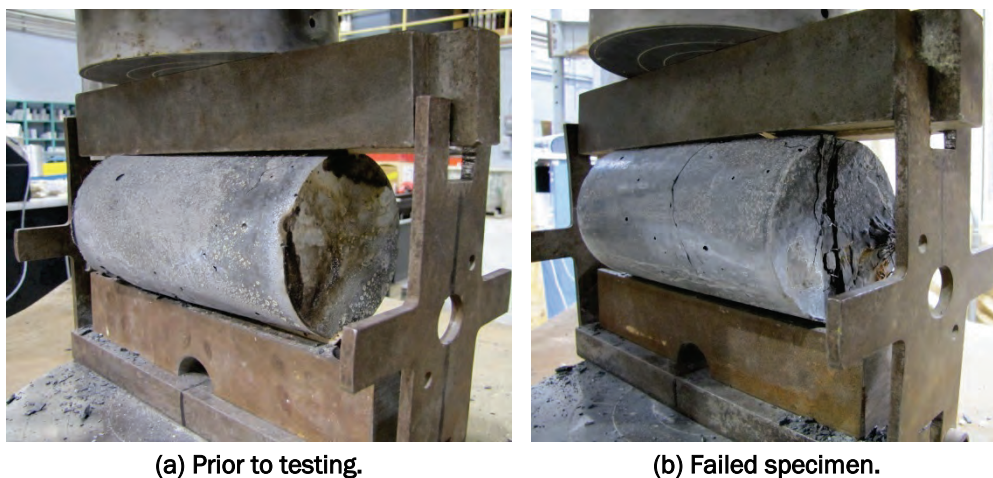
Figure 5. ASTM C1609 flexural strength test setup.



### 2.2.3 Splitting tensile strength

Splitting tensile strengths were obtained according to ASTM C496 (ASTM International 2011). Testing was performed for each material type and then subsequently compared to that material's UCS. Figure 6 shows the splitting tensile testing set-up, both pretest and posttest.

Figure 6. ASTM C496 splitting tensile strength test setup.

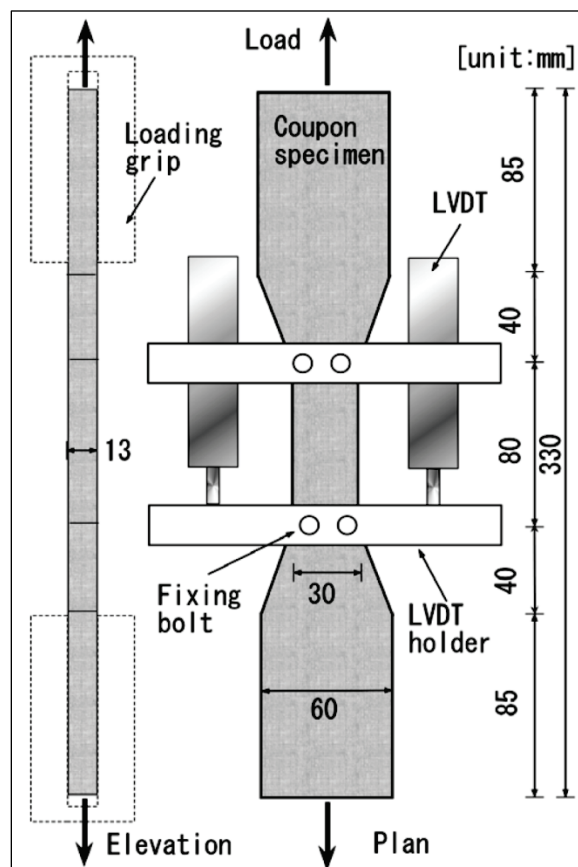


### 2.2.4 Direct tensile testing

The direct tensile testing (DTT) is an ERDC-specific test that was adapted from a 2008 Japanese Society of Civil Engineers, Concrete Engineering Series 82 (JSCE 2008). This testing procedure consists of taking a dog-bone-shaped specimen with cross-sectional dimensions of 1.18 by 1.18 in.

(30 by 30 mm) in the gage length and applying a constant specimen deformation rate of approximately 0.02 in. (0.5 mm)/min with two LVDTs on diagonally opposite corners of the gage length in order to capture the complete displacement in all directions. Each specimen was tested with a gage length of 4 in. (101.6 mm). Figure 7 shows a diagram of the dog-bone shape and test setup.

Figure 7. DTT coupon geometry and test setup (adapted from JSCE 2008).



Multiple dog-bone specimens were made in two different methods. First, specimens were cast in molds with the specified dog-bone shape. This method proved easy and effective during placement; however, fiber-alignment became a real concern particularly for fibers with lengths in excess of 1 in. (25 mm). The molds were causing fibers to align along the long axis to which the tensile stress would be applied during testing. This could result in a tendency for higher DTT strengths. The second method consisted of casting large flexural beams, cutting slabs from these beams to the specified thickness, and using a water jet to cut out the dog-bone shape from the slabs. This method avoided the problem of fiber alignment,

but due to small “waves” caused by the water jetting process, there was a possibility of stress concentrations occurring around the test section and causing a tendency for lower DTT strength results. These concerns will be further discussed in the results section.

Figures 8 and 9 show the actual test configuration, pretest and posttest, respectively. This configuration utilized a chucking mechanism suited to the shape of each specimen and was designed to allow tensile loading along the specimen's central axis. The top chuck utilized one rotational degree of freedom to allow better vertical alignment, while the bottom chuck utilized a torsional degree of freedom to prevent torsional stresses caused by possible grip misalignment.

Figure 8. DTT testing configuration.

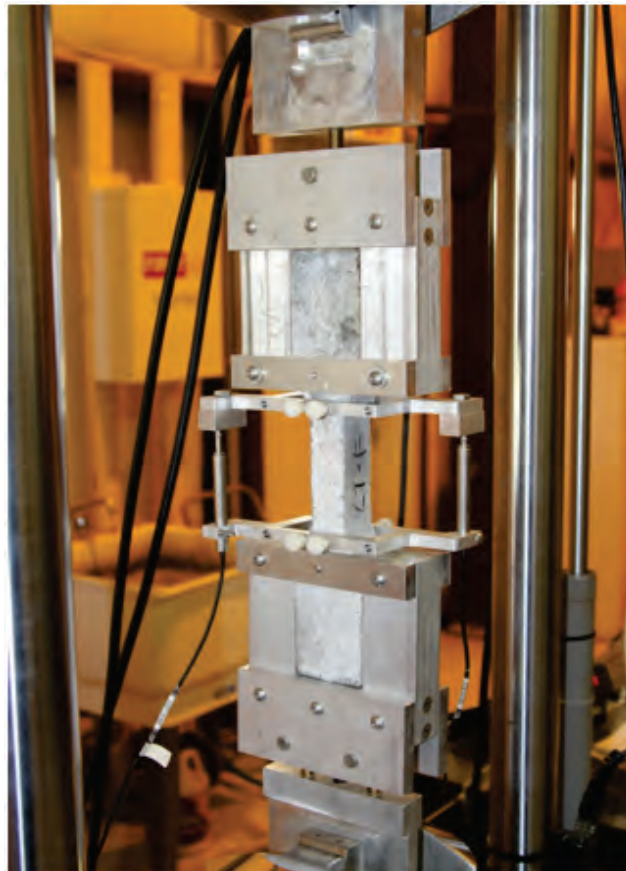
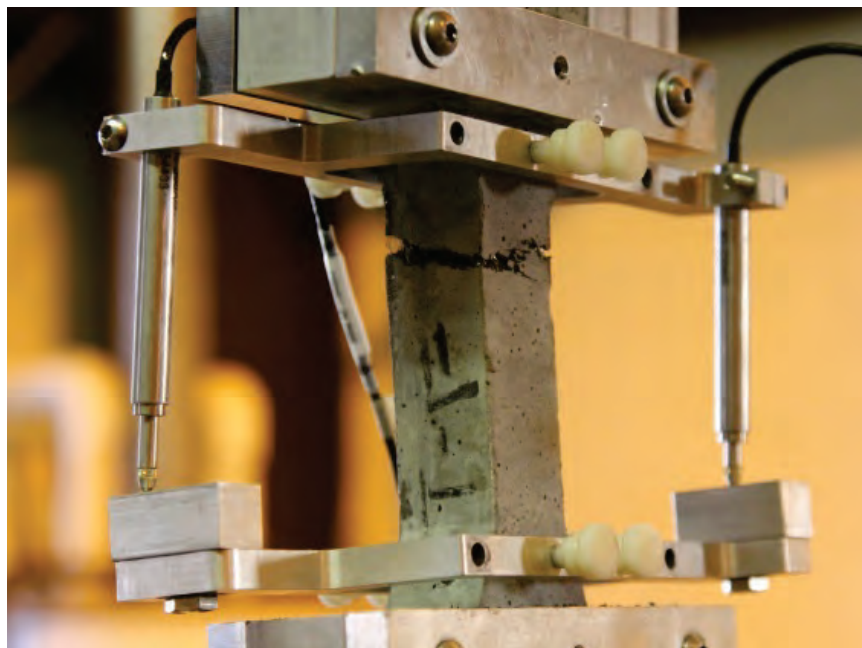




Figure 9. Typical result of DTT with tensile fracture and fiber alignment in gage length.



### 2.2.5 Dynamic penetration using fragment simulating projectiles

Direct fire experiments were conducted using a 0.50-caliber fragment simulating projectile (FSP) to examine the resistance of the UHPC panels to small projectile penetration. A single projectile was fired at each panel with approximately the same impact velocity in each test. Each direct fire experiment measured impact velocity, the residual velocity of the projectile in the event of perforation, and the final damage state of the target. Multiple posttest photographs of each panel were obtained to record the final damage state. Figure 10 shows an example of the FSP, both un-deformed pretest and deformed posttest. Samples of the UHPC panels were cast to a uniform 12-in. by 12-in. size with thicknesses of 1.0, 2.0, 2.5, and 3.0 in. A set of three panels was generated for each thickness, resulting in 12 panels for each fiber type. The desired impact velocity was approximately 3,500 ft/sec. This combination of panel thicknesses and impact velocity was chosen because it had previously generated damage states and responses that varied from complete perforation of the panel to only e cratering of the impact side of the panel (Reinhart and Thornhill 2010). All direct fire experiments were conducted in ERDC's small-arms ballistic testing facility. This included a cartridge preparation area that contained the proper equipment to hand load numerous cartridges. The range from muzzle to target in this experimental program was approximately 15 ft. A Physics Applications Inc. small-arms receiver with a sliding return-to-zero base,

shown in Figure 11, was used to fire the projectiles. The gun included a 60-in. smooth bore barrel and was cartridge fired. It could accommodate round sizes ranging from .22 caliber to 20 mm.

Figure 10. 0.50-cal FSP used in experiments shown before and after testing (i.e., undeformed and deformed).

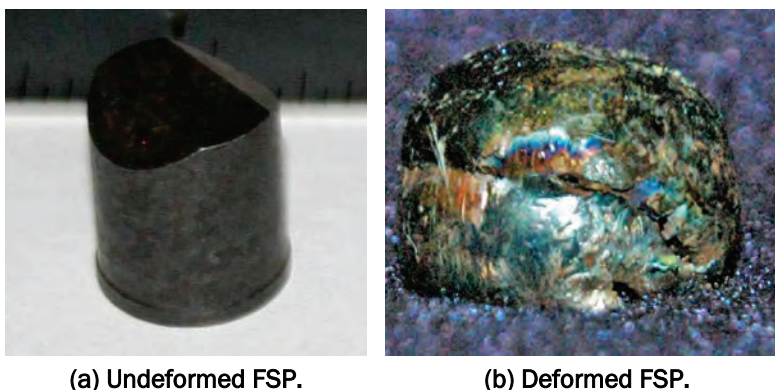


Figure 11. Physics Applications Inc. small-arms receiver used to fire the projectiles.



Projectile velocity measurements were made using a set of Oehler Research, Inc. Model 35P proof chronographs, each connected to two Oehler Model 55 light screens. The light screens attached to each chronograph were positioned 3 ft apart to capture projectile velocities. Four chronograph screens were positioned between the gun and the target to estimate the impact velocity. The velocity was measured between pairs of the screens, and an estimated impact velocity was generated for each test based on these measurements. A single pair of screens was positioned approximately 4 ft behind the test specimens, as shown in Figure 12, to measure exit (residual) velocities for each experiment. All experiments were conducted with 0-deg obliquity, or perpendicular impact.

Samples were mounted in a rigid steel fixture (Figure 13). Panels were held in position by tension in a threaded rod. Contact stresses between the panels and the fixture were reduced by rubber padding. Figure 14 shows the front view, or impact face, of a typical target.

Figure 12. Schematic of ballistic penetration test setup (1 in.=25 mm).

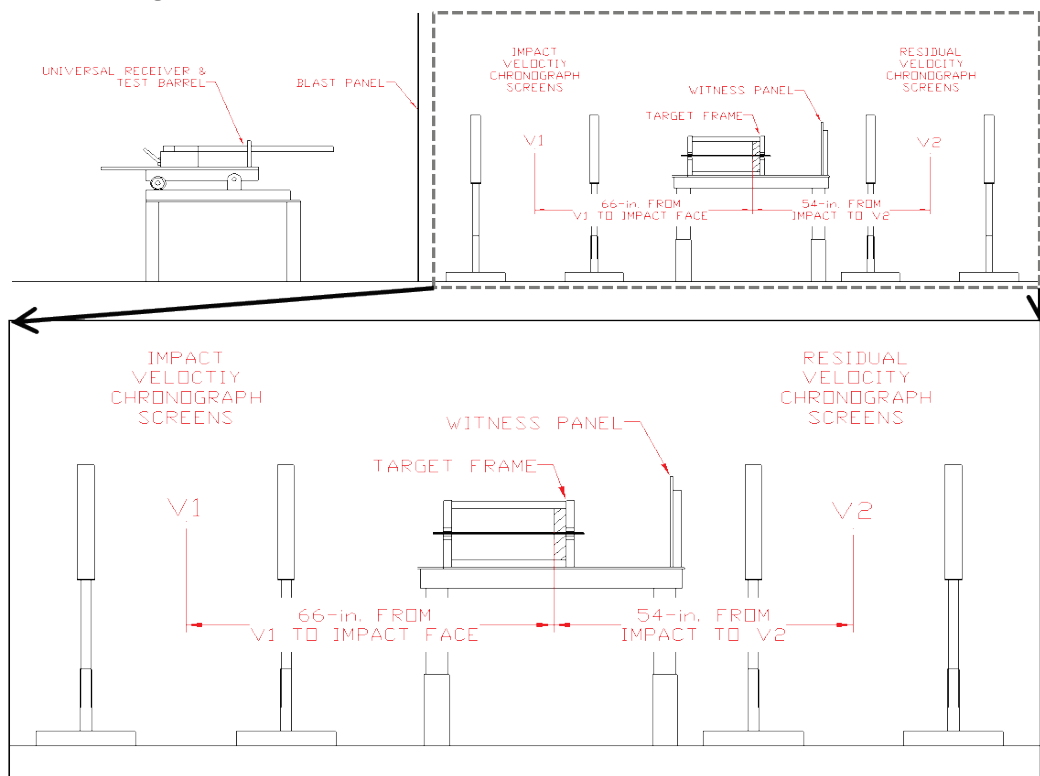


Figure 13. Side view of fixture used to hold UHPC test panels.

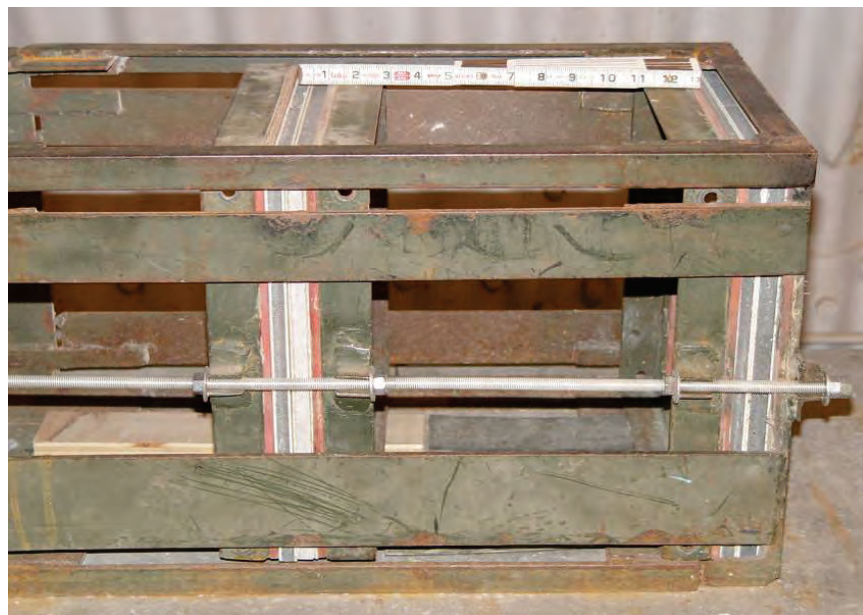
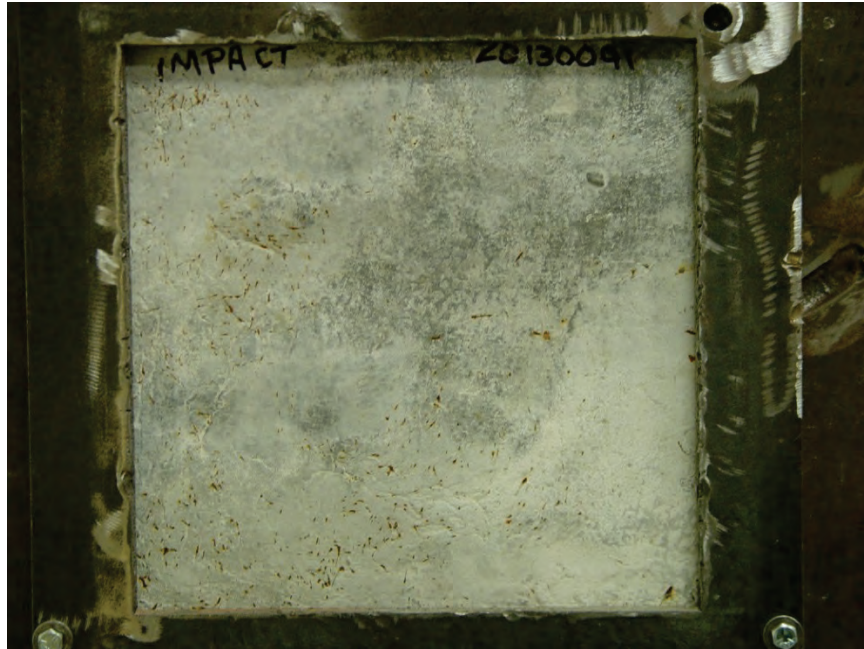




Figure 14. Front view of panel within fixture.



### 3 Quasi-Static Test Results and Discussion

#### 3.1 Compressive strength

As discussed in the testing procedures section, the UCS and E&P results were used to verify that each CTB matrix (excluding fibers) was consistent so that a direct comparison could be made between steel fiber types. Table 4 shows the results of the ASTM C873 (ASTM International 2010a) testing. It can be seen that all of the compressive strengths are similar. UHPC specimens containing the Nycon fibers resulted in lower strengths of about 26 ksi (179 MPa). There are several potential reasons for this occurrence: possible batching errors, possible cylinder casting errors, or the size of the fiber itself. The larger and relatively wider fiber has a larger ITZ that could provide a large enough failure plane during loading at high stresses that the material actually shows a lower strength. However, further testing is needed to draw that conclusion.

Table 4. ASTM C873 UCS testing results.

Fiber Type	Unconfined Compressive Strength (psi)			
	ZP305	NYCON	OL 6 mm	OL 10 mm
Sample 1	29910	26900	29570	31580
Sample 2	28860	24240	30890	30770
Sample 3	29320	26490	30500	31580
Avg.	29363	25877	30320	31310

Upon failure by compression, a majority of the cylinders containing the shorter OL fibers failed catastrophically on a shear plane. While all the cylinders that contained longer fibers held together, the cylinders with the shorter fibers failed diagonally. Figure 15 shows these failures.

#### 3.2 Elastic modulus and Poisson's ratio

The E&P testing performed according to ASTM C469 (ASTM International 2011) provided similar elastic modulus results for all fiber types. The UHPC formulation using the Nycon fibers with its lower strength also had the lowest modulus-- approximately  $7.1 \times 10^6$  psi. Each subsequent modulus rose in proportion to the higher UCSs. This possibly disproves that the NYCON's lower strengths were caused by the fiber's larger size and its interfacial interaction with the UHPC matrix which would likely influence

only the strength but not the elastic modulus. As a result, it is hypothesized that the UHPC matrix with Nycon fibers actually exhibited lower strength than the other three fiber types tested. Table 5 shows elastic modulus test results from 6- by 12-in. (152- by 304-mm) cylinders. The Poisson's ratios are not shown in this table due to the high variance in the results. The hardness/stiffness of the CTB cylinders made it difficult for the modulus testing apparatus to firmly grip the cylinder throughout testing. This resulted in some slip in the horizontal measurement for the Poisson's ratio. The results of each test specimen were plotted and, if any slip was seen, that specimen's Poisson's ratio was discarded. The remaining specimens, which showed no slip, gave Poisson's ratios ranging between 0.2 and 0.26.

Figure 15. Typical failure observed following compressive strength testing.



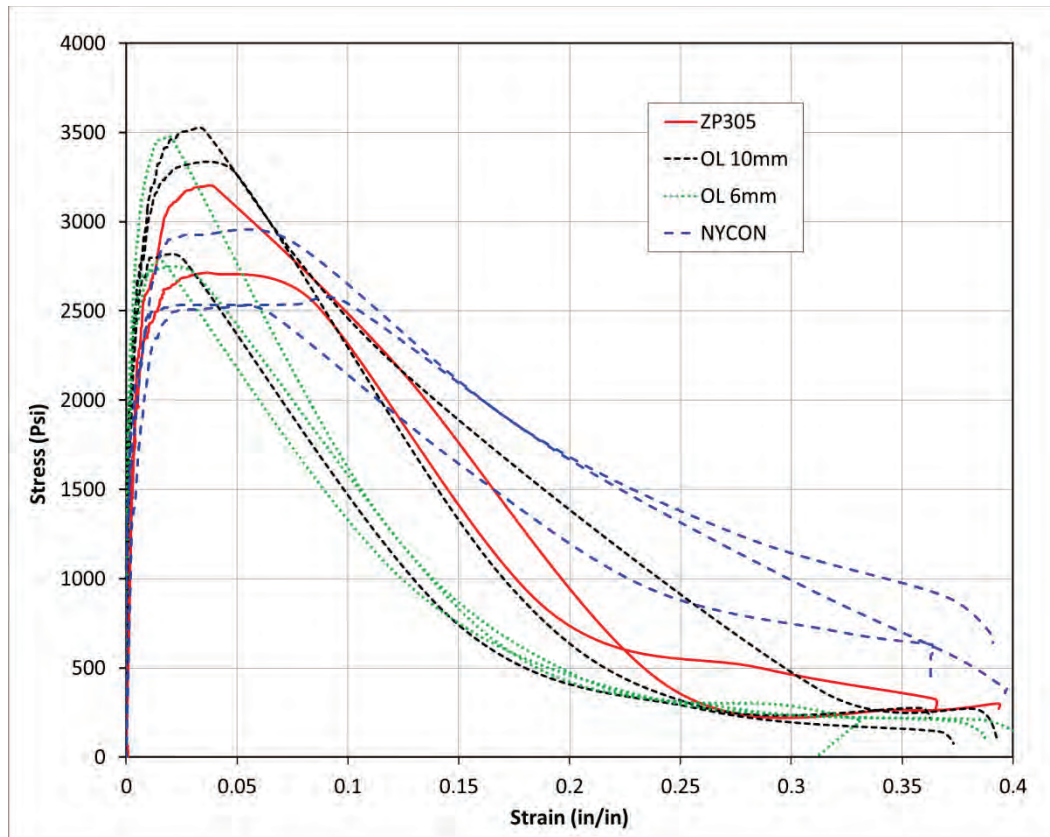
Table 5. ASTM C469 elastic modulus testing results.

Fiber Type	Elastic Modulus (psi x 106)			
	ZP305	NYCON	OL 6mm	OL 10mm
Sample 1	7.25	6.90	7.25	7.35
Sample 2	7.30	7.25	7.40	7.35
Sample 3	7.10	7.15	7.40	7.45
Avg.	7.25	7.10	7.35	7.40

### 3.3 Flexural response

The flexural response was obtained according to ASTM C1609 (ASTM International 2012). The ultimate flexural strengths for each fiber type were compared to that matrix's corresponding UCS and were expressed as a percentage of UCS. Each fiber type's ultimate flexural strength ranged between 10 and 11 percent. Figure 16 shows these results as plots of applied load versus center-line displacement.

Figure 16. Flexural response stress vs. strain on tensile face.



The difference between these fiber types can be clearly seen. Toughness is the ability of a material to absorb energy and is defined by ASTM C1609 (ASTM International 2012) as being the area under the flexural strength curve up to  $L/150$ , where  $L$  is the support span length. Therefore,  $L/150 = 0.12$  in. The OL 6mm fibers mixture failed in a brittle manner. The sharp drop upon failure means this material has a low toughness, and it is likely failing this way because the short length of the fiber is unable to “bridge” the crack and hold the matrix together. In the case of the OL 10-mm fibers with similar shape and aspect ratio but larger overall size, the flexural response exhibited higher nonlinearity near the maximum strength. Results from the ZP-305 and Nycon fibers suggest that, as fiber sizes become longer, post yield load carrying capacity and toughness actually increase. However, this is only in the quasi-static condition. The benefits in flexure offered by longer fibers should be considered along with the benefits offered by smaller fibers for more localized loading conditions.

### 3.4 Splitting tensile strength

Splitting tensile properties were obtained for each fiber type according to ASTM C496 (ASTM International 2011). The results were then expressed as a percentage of UCS. Each matrix had a splitting tensile strength that ranged between 11 percent and 12 percent of its corresponding UCS. These results are shown in Table 6.

Table 6. ASTM C496 splitting tensile strength test results.

Fiber Type	Splitting Tensile Strength (psi)			
	ZP305	NYCON	OL 6mm	OL 10mm
Sample 1	3591	2718	3575	3520
Sample 2	3176	2942	3293	3040
Sample 3	3658	3386	3584	3555
Average	3475	3015	3484	3372

### 3.5 Direct tensile testing

Figure 17 shows the stress-strain responses observed in DTTs for samples that were cast into dog-bone shaped molds. Figure 18 provides the stress-strain responses observed in DTTs for samples that were cast as flexural beams and then cut into the dog-bone shape using a water jet. Summaries of the results are provided in Table 7 and 8. In Table 7, the results from 10mm Sample #1 were excluded because of an unhydrated cement agglomerate in the cross section. Multiple exclusions were made in Table 8 to address sample defects: 10mm sample #5 for uneven distribution of fiber in the vicinity of failure, Nycon samples #1 and #2 for no fiber bridging the crack, ZP 305 sample #3 for minimal fibers bridging the crack and an entrapped air void present, and ZP 305 samples #1 and #5 for large air voids near the edge of the gage length. Figure 19 provides a comparison of all the data. Appendix B provides images of the cut dog-bones, both pretest and posttest, with their corresponding loads that show examples of typical failure morphologies that were observed.

In both the cast and the cut specimens, there was a clear trend in the relationship between fiber size and tensile behavior. The smaller 6mm OL fibers exhibited the highest strength and stress vs. strain linearity. As fiber size increased, tensile strength, as well as toughness, reduced significantly. The two largest fibers, Nycon Type V and ZP-305, exhibited the lowest strengths and extremely low toughness with rapid reductions in load carrying capacity following the ultimate stress peak.

Figure 17. Stress vs. strain DTT results for mold-cast specimens that exhibited longitudinal fiber orientation.

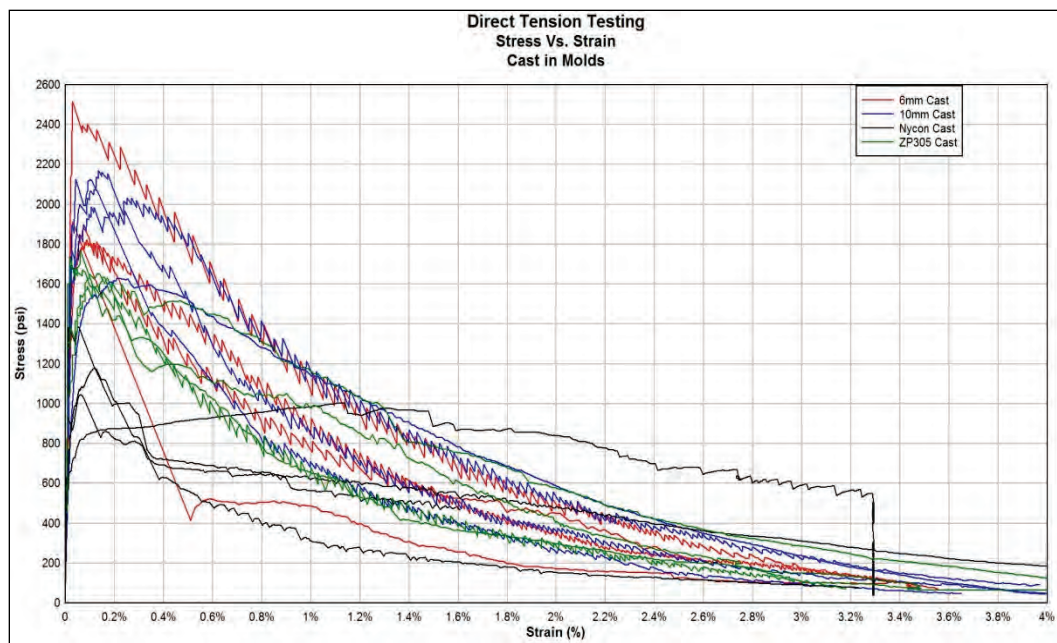


Table 7. DTT results from specimens cast in molds.

Specimen	Tensile Strength (lbf)			
	6 mm	10 mm	Nycon	ZP 305
1	2665	2274	1639	2465
2	2622	2773	1403	2307
3	2545	3026	1457	2364
4	3507	2893	1936	2283
Average	2835	2742	1609	2355
Average excluding Imperfections	2835	2897	1609	2355



Figure 18. Stress vs. strain DTT results for specimens that were cut from larger flexural beams to minimize preferential fiber orientation.

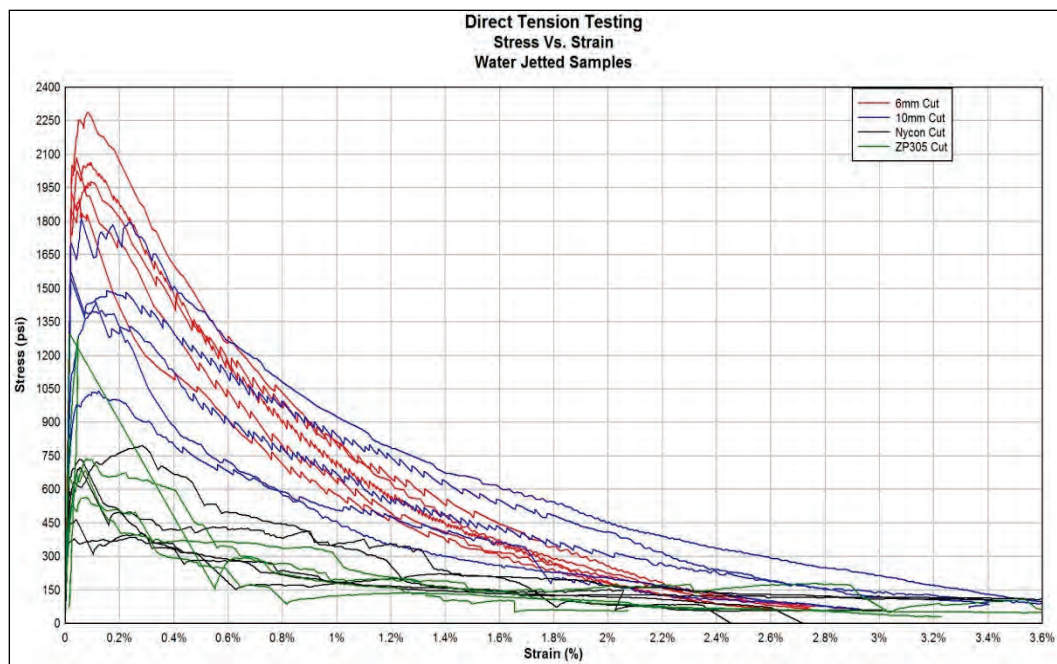
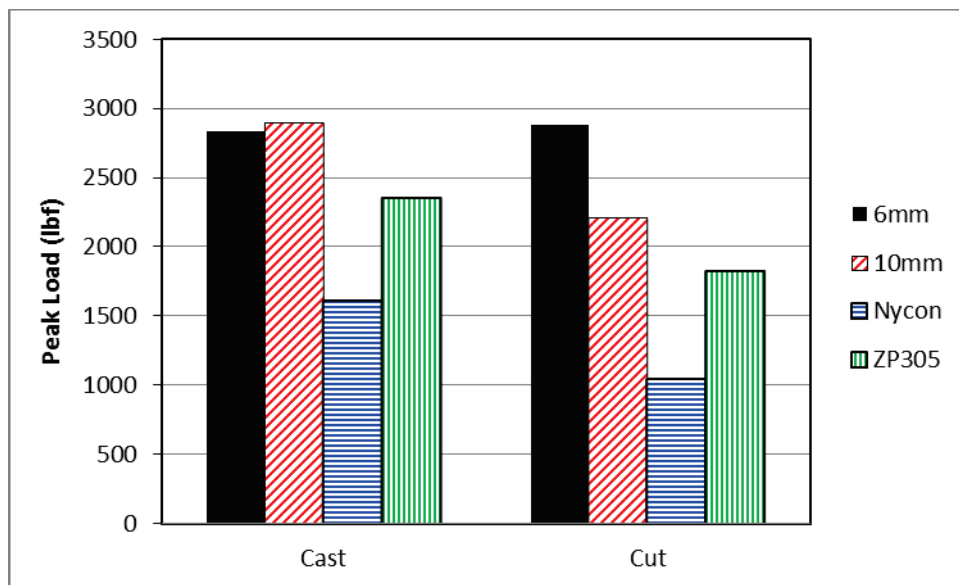


Table 8. DTT results from specimens cut using a waterjet from larger flexural beams.

Specimen	Tensile Strength (lbf)			
	6 mm	10 mm	Nycon	ZP 305
1	2659	2534	649	1029
2	2917	2198	538	1827
3	2949	2154	1114	798
4	2814	1957	977	1824
5	3076	1452	1027	976
Average	2883	2059	861	1291
Average excluding imperfections	2883	2211	1039	1826

Figure 19. Summary of DTT results with comparison between specimens that were cast vs. those that were cut from larger flexural beams.



Issues associated with fiber alignment are also easily seen in these results, which provide a direct comparison between the two methods of preparation. Higher tensile strengths occurred from the cast specimens versus the water-jetted samples in all fibers except for the Bekaert 6 mm. The strengths for the Bekaert 6 mm cast and cut specimens were similar and were the least affected by the small specimen size.

In addition, the effect of fiber loading by weight or volume percentage vs. by number of fibers is also clearly a factor, as many of the defects observed during testing were attributed to a lack of fibers crossing the plane of failure. For example, when examined following testing, many of the Nycon Type V and ZP-305 specimens had no fibers bridging the fracture plane. The smaller OL fibers, since they are far greater in number, more easily bridged the fracture plane.

While the results do generally suggest that smaller fibers help to improve tensile strengths in UHPC, the issues associated with fiber size vs. sample size are significant. Tests on much larger specimens would be required to better predict tensile properties and minimize the effects of preferential fiber orientation during sample fabrication.



## 4 Dynamic Test Results and Discussion

Each panel was impacted with an FSP and resulted in damage. In each case, fibers were exposed with some only on the impact side of the panel and others on both sides due to cratering on the front face of the panel, spalling of concrete on the back face of the panel, and/or perforation of the panel by the penetrator. Details for each test, including impact velocity, residual velocity, FSP/round characteristics, and panel characteristics are provided in Appendix C. All but one of the experiments had an impact velocity within 2.5 percent of the desired 3,500 ft/sec, and that one test (Test #12) was approximately 4.0 percent low at 3,343 ft/sec. Several of the tests (11, 27, and 43) had the residual screens tripped by material spalling off the backside of the panel and, therefore, did not record a residual FSP velocity.

The damage states, or responses, of the panels were consistent across the different fiber types. The FSP perforated through each of the panels that had thicknesses of 1.0 and 2.0 in. (25 and 50 mm) (Figures 20 and 21). An impact crater and significant backside spall was created for each of the 2.5-in. (63-mm)-thick panels (Figure 22). For each of the 3.0-in. (76-mm-) thick panels, a smaller impact side crater was generated, and minimal, if any, backside damage was visible (Figure 23).

Figure 20. Typical damage to 1.0-in. (25-mm-) thick panel, showing impact and exit faces with full penetration by the FSP used in testing.

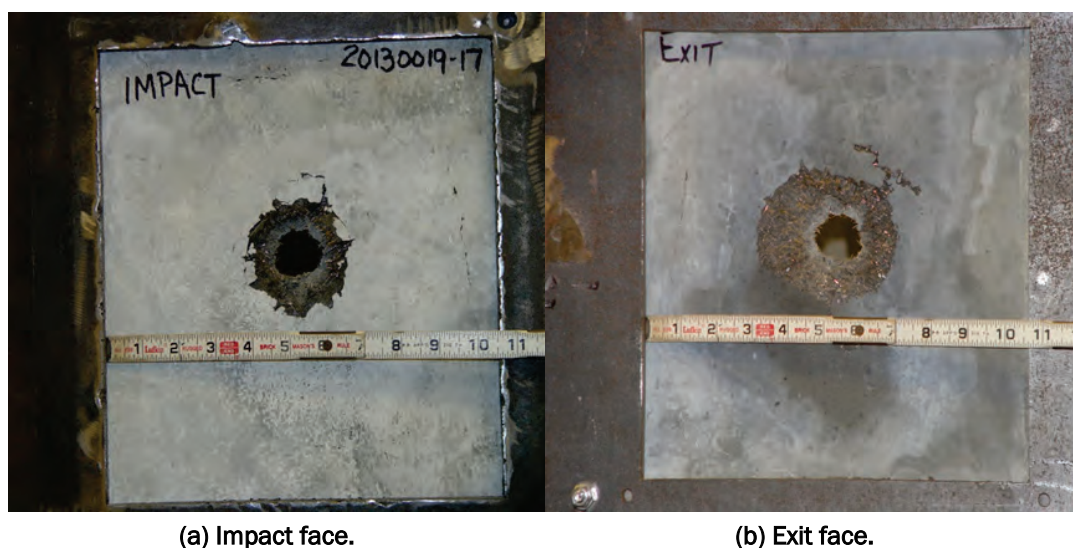


Figure 21. Typical damage to 2.0-in. (50-mm-) thick panel, showing impact and exit faces with full penetration by the FSP used in testing.



(a) Impact face.

(b) Exit face.

Figure 22. Typical damage to 2.5-in. (63-mm-) thick panel showing impact and exit faces with no penetration by the FSP used in testing. Spallation of material was observed on the back/exit faces of the panels.



(a) Impact face.

(b) Exit face.

Figure 23. Typical damage to 3.0-in. (76-mm-) thick panel, showing impact and exit faces with no penetration by the FSP used in testing. No spallation was observed on the back/exit faces of panels.



(a) Impact face.

(b) Exit face.

Figures 24 and 25 contain the averaged residual velocities for the 1.0- and 2.0-in.- (25- and 50-mm-) thick panels, respectively, with the different fiber types. While the differences were not large, the panels made with 6-mm fibers generally produced a lower residual velocity for both panel thicknesses. As the FSP did not perforate through any of the 2.5- or 3.0-in. (63- or 76-mm-) thick panels, there were no residual velocities for those cases.

Figure 24. Average residual velocities (ft/s) for 1.0-in. (25-mm-) thick panels.

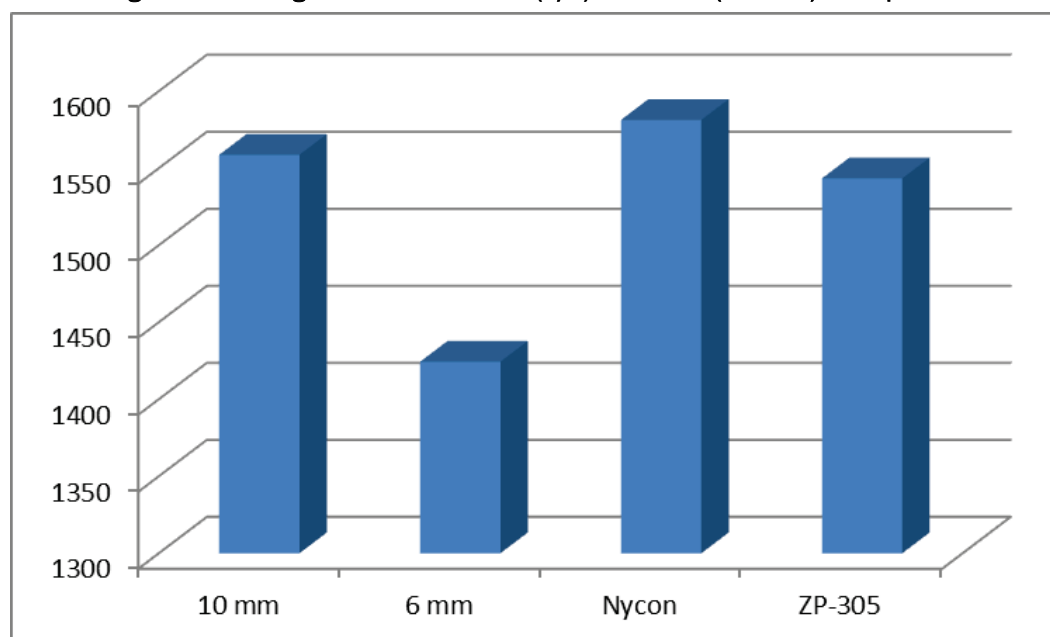
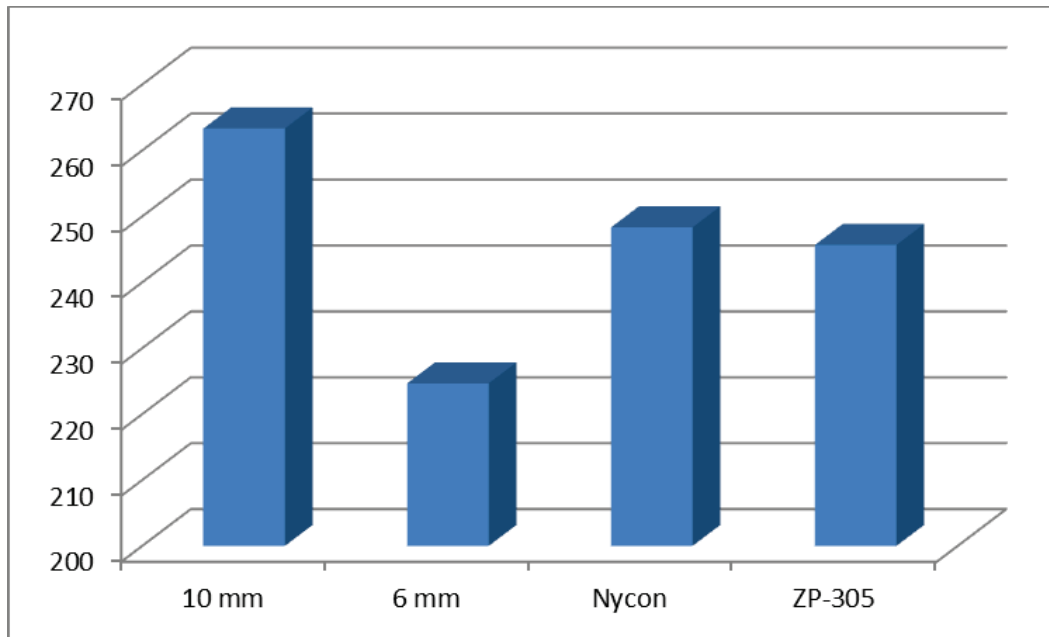


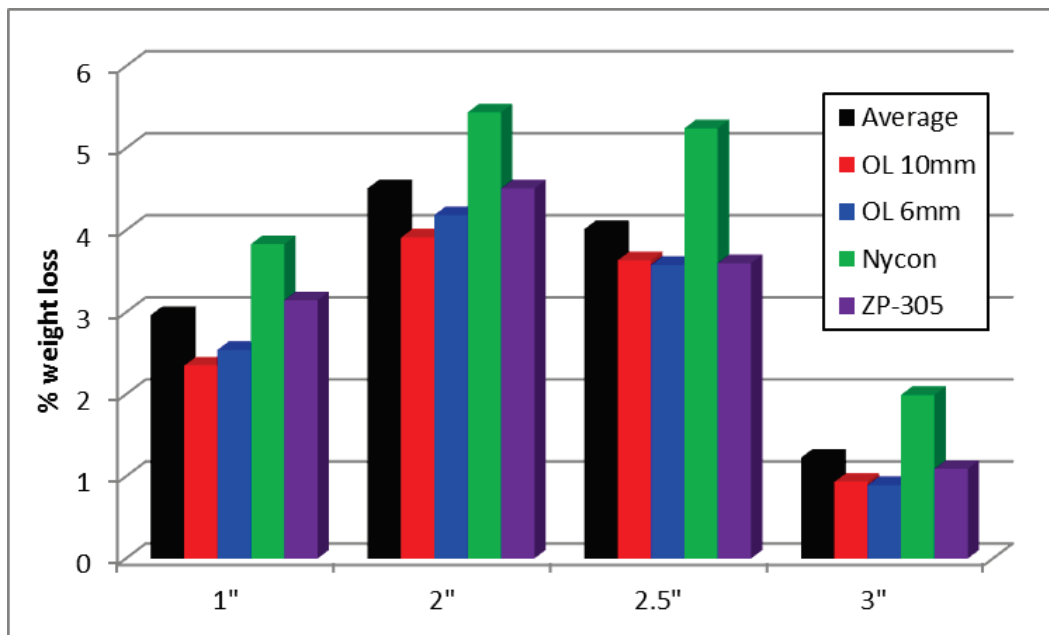
Figure 25. Average residual velocities (ft/s) for the 2.0-in. (50-mm-) thick panels.



Loss of weight due to the damage ranged from 0.75 percent to 5.5 percent over the series of experiments (Figure 26). Although the 1.0-in. (25-mm) panels were perforated, they did not lose as much mass as the 2.0- or 2.5-in.- (50- or 63-mm-) thick panels. The 3.0-in. (76-mm) panels exhibited the least loss of mass due to having only smaller impact side craters.

Additional detailed photographs of each shot are available upon request.

Figure 26. Percent weight loss for each fiber type and panel thickness.



## 5 Summary and Conclusions

The overall goal of this effort was to provide a better understanding of the effect of fiber size and shape on the response of UHPC at quasi-static and dynamic rates of loading. The testing utilized a laboratory standard UHPC known as Cor-Tuf Baseline (CTB) that was produced using four fiber types: Bekaert ZP-305 (in the standard CTB formulation), Nycon Type V, Bekaert OL .2/10 mm, and Bekaert OL .16/6 mm. Based on the results of the experimental investigation, the following conclusions were made:

- Fiber size and shape have a negligible impact on compressive strength, elastic modulus, and Poisson's ratio. This result was anticipated as compressive properties are more strongly correlated with the properties of the matrix than of the fiber reinforcement, which is provided to improve the tensile properties.
- The results of flexural and direct tensile testing showed a strong impact of fiber geometry. Smaller fibers, such as the 6-mm OL fibers, provided a significant increase in tensile strength, stress vs. strain linearity, and toughness. As fiber sizes increased, tensile strengths generally decreased. Even with their reduced strength, specimens made with the larger ZP-305 and Nycon Type V fiber did exhibit some post-yield load carrying capacity. The results indicated that the smaller OL fibers significantly improved properties and limited damage and nonlinearity prior to reaching the ultimate strength. Once damage was initiated, the larger fibers were more effective at bridging larger cracks and redistributing stresses.
- The dynamic response of specimens impacted with FSPs provided similar relationships between fiber size and response. The smaller OL fibers were more effective in reducing residual velocities of projectiles and minimizing mass loss due to cratering and/or spallation. Large fibers exhibited higher residual velocities and more extensive damage on the impact and exit faces of UHPC panels. Fiber size and shape did not have a significant impact on whether or not a panel was perforated during testing, as all UHPC-fiber combinations transitioned from a non-perforated to a perforated state at the 2.0- to 2.5-in. (50- to 63-mm) panel thickness transition.



The primary conclusion from this research was that fiber size and shape do have a significant impact on both quasi-static and dynamic properties of UHPCs. Given the fact that fiber dosage rates were made on a percentage basis, the number density of fibers also has a significant impact on correlations between fiber type and UHPC response. This is likely one of the primary reasons that smaller OL fibers performed significantly better in most tests than their larger counterparts. Another issue encountered was that larger fibers require larger test articles in order to uniformly distribute the fibers and minimize preferential fiber orientation. This was an issue in the direct tensile testing that required a specimen with limited cross-sectional area in the gage length.

Based on observations of specimen behavior and the influence of fiber type, it is likely that combinations of fiber types may be more advantageous and provide additional improvements in mechanical properties at a similar cost. For example, given the fact that OL fibers are more expensive but provide improved tensile properties, perhaps the dosage rate of OL fibers could be reduced to minimize the impact of their cost while maintaining properties similar to CTB produced using ZP-305 fibers. Future research on hybrid fiber systems may also yield leap-ahead results with multiple fiber types to bridge multiple damage states. Such a material may include small OL fibers to delocalize damage prior to reaching the ultimate strength combined with larger ZP-305 fibers to bridge larger length scale cracking.

## References

- ASTM International. 2010a. *Standard test method for compressive strength of concrete cylinders cast in place in cylindrical molds*. Designation: C872/873M. West Conshohocken, PA:ASTM International. doi: 10.1520/C0873\_C0873M-10A. [www.astm.org](http://www.astm.org).
- \_\_\_\_\_. 2010b. *Standard test method for static modulus of elasticity and Poisson's ratio of concrete in compression*. Designation: C469/496M. West Conshohocken, PA:ASTM International. doi: 10.1520/C0469\_C0469M-10. [www.astm.org](http://www.astm.org).
- \_\_\_\_\_. 2011. *Standard test method for splitting tensile strength of cylindrical concrete specimens*. Designation: C 496/496M. West Conshohocken, PA:ASTM International. doi: 10.1520/C0496\_C0496M-11. [www.astm.org](http://www.astm.org).
- \_\_\_\_\_. 2012. *Standard test method for flexural performance of fiber-reinforced concrete (using beam with third-point loading)*. Designation: C1609/1609M. West Conshohocken, PA: ASTM International. doi: 10.1520/C1609\_C1609M-12. [www.astm.org](http://www.astm.org).
- \_\_\_\_\_. 2013. *Standard practice for making and curing concrete test specimens in the laboratory*. Designation: C 192/192 M. West Conshohocken, PA: ASTM International. doi: 10.1520/C0873\_C0873M-10A. [www.astm.org](http://www.astm.org).
- Campbell, R.L., E.F. O'Neil, W.M. Dowd, and C.E. Dauriac. 1998. *Reactive powder concrete for producing sewer, culvert, and pressure pipes*. Construction Productivity Advancement Research (CPAR) Program, CPAR-SL-98-3. Vicksburg, MS: U.S. Army Engineer Waterways Experiment Station.
- Green, B., R. Moser, D. Scott, and W. Long. 2014. Ultra-high performance concrete history and usage by the United States Army Engineer Research and Development Center. ASTM C09 Symposium on Ultra-High Performance Concrete; ASTM Special Technical Publication 1581, 2014. *ASTM Journal of Advances in Civil Engineering Materials* [http://www.astm.org/DIGITAL\\_LIBRARY/JOURNALS/ACEM/PAGES/ACEM20140031.htm](http://www.astm.org/DIGITAL_LIBRARY/JOURNALS/ACEM/PAGES/ACEM20140031.htm)
- Japanese Society of Civil Engineers (JSCE). 2008. *Recommendations for design and construction of high performance fiber reinforced cement composites with multiple fine cracks*. Tokyo: Japan Society of Civil Engineers.
- Neeley, B. D., and D. M. Walley. 1995. Very high-strength concrete. *The Military Engineer* 87: 36-37.
- O'Neil, E. F. 2008. *On engineering the microstructure of high-performance concretes to improve strength, rheology, toughness, and frangibility*. PhD diss., Northwestern University, Evanston, IL.
- Reinhart, W. D., and T. F. Thornhill III. 2010. *Ballistic penetration test results for ductal and ultra-high performance concrete samples*. SAND2010-2222. Albuquerque, NM: Sandia National Laboratories.

- Richard, P., and M. Cheyrezy. 1995. Composition of reactive powder concretes. *Cement and Concrete Research* 25(7): 1501-1511. [http://dx.doi.org/10.1016/0008-8846\(95\)00144-2](http://dx.doi.org/10.1016/0008-8846(95)00144-2).
- Roth, Jason M., S. T. Rushing, G. O. Flores, D. Sham, and J. W. Stevens. 2010. *Laboratory investigation of the characterization of Cor-Tuf flexural and splitting tensile properties*. ERDC/GSL-TR-10-46. Vicksburg, MS: U.S. Army Engineer Research and Development Center.



## Appendix A: Fiber Data Sheets




Figure A1. Technical data sheet for ZP 305 fiber.

# Dramix®



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## DATA SHEET

## ZP 305



ZP 305

**What is Dramix®?**

Dramix® steel fibres, from industry specialist Bekaert, have set a new standard for concrete reinforcement with their unique combination of flexibility and cost-efficiency.

Dramix® is a cold drawn steel wire fibre with hooked ends for optimum anchorage. What you get from Dramix® reinforced concrete is ductility and high load bearing capacity. On top of that, you'll find a quick and easy way to an efficient and cost effective solution.

**ZP 305 offers you:**

- ✓ **Dramix® Hi Perform**  
Dramix® Hi Perform fibres are providing high performance and crack resistance. These are products of choice to create optimal ductility. Dramix® Hi Perform is used for structural, designed applications, in situ, precast or sprayed.
- ✓ **Dramix® Extreme**  
Dramix® Extreme fibres are special engineered for extreme performant crack-control. They are made of thin wire diameters, giving a very dense wire network in each m³ concrete. The standard low dosage with Dramix® Extreme makes the products very cost-effective.

**GEOMETRY AND PERFORMANCE**

length (l)



30 mm

diameter (d)




0,55 mm

16.750 Fibres/kg     Minimum dosage: 25 kg per m³ (according to CE)  
Minimum fibre network: 12,6 km per m³ (for 25 kg/m³)

**MATERIAL PROPERTIES**

Tensile strength: Rm nom: 1.345 N/mm²  
Tolerances: ± 7,5% Avg  
Young's Modulus (Emod): ± 210.000 N/mm²

**PACKAGING**




60 BAGS (20kg)  
1200 kg




BIG BAG  
1100 kg

**STORAGE**



KEEP DRY



NO STACKING

**APPROVALS**



ISO 9001



CE  
EN 14889-1



ASTM A820

- CE LABEL:** Dramix® is certified for CE mark system 1: steel fibres for structural use. For detailed info: CE info sheet available on request.
- ISO 9001 LABEL:** Dramix® is ISO 9001 certified. The same quality standards are applied in the following plants:
  - Belgium
  - Czech Republic
  - Turkey
  - Brazil
  - China
  - Indonesia
- OTHER LABELS:** Bekaert is close to the market. Bekaert seeks conformance to every necessary, national product quality standard, in case one or more requirements are not covered in the CE or ISO 9001 certifications.

Bekaert will advise on the most suitable fibre for your application.  
Ask for our recommendations on handling, dosing and mixing at:  
[www.bekaert.com/dosingdramix](http://www.bekaert.com/dosingdramix)  
[infobuilding@bekaert.com](mailto:infobuilding@bekaert.com)

Modifications reserved.  
All details describe our products in general form only. For detailed information, product specifications available on request.

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designed by aputable - 713605 - 10/2010

Figure A2. Technical data sheet for Nycon Type V fiber.

**NYCON-SF TYPE V (CONTINUOUSLY DEFORMED)**  
High Performance Steel Fiber, ASTM A820



**NYCON-SF TYPE V CONTINUOUSLY DEFORMED STEEL FIBERS** are low-carbon, drawn wire for reinforced concrete. NYCON-SF fibers distribute stresses within the concrete and provide improvement to ductility.



**NYCON-SF TYPE V Physical Properties**

Filament Diameter	0.046" (1.18 mm)
Fiber Length	1.5" (38 mm)
Specific Gravity	7.8
Tensile Strength	100 ksi (689 MPa)
Flexural Strength	29,000 ksi (203 GPa)
Melting Point	2760° F (1516° C)
Color	Gray
Water Absorption	Nil
Alkali Resistance	High
Corrosion Resistance	High

**Description**

NYCON-SF TYPE V steel fiber products are drawn, continuously deformed steel fibers for use in reinforced concrete and shotcrete.

*NYCON-SF TYPE V steel fibers meet the requirements of ASTM A820.*

APPLICATIONS	BENEFITS
<b>Shrinkage Control</b> Dosage: 20-30 lbs per CY, 10-15 kg/m <sup>3</sup>	Length and count is a consideration. Impending or modifying crack growth is one of the benefits of a high fiber count. There are durability benefits provided at this dosage level.
<b>Light Traffic and/or Static Loading</b> Dosage: 30-40 lbs per CY, 15-20 kg/m <sup>3</sup>	Steel fibers provide secondary shrinkage reinforcement and durability benefits. A degree of load carrying can be assigned to the steel fibers at the upper end of the dose range.
<b>Medium Dynamic and/or Static Loading</b> Dosage: 40-60 lbs per CY, 20-30 kg/m <sup>3</sup>	Steel fibers provide secondary shrinkage reinforcement, durability benefits and for slab-on-grade, a degree of load carrying capacity. Shotcrete and precast products also benefit from steel fibers.
<b>Heavy Dynamic and/or Static Loading</b> Dosage: 60-100 lbs per CY, 30-50 kg/m <sup>3</sup>	At this dosage, steel fibers translate into enhanced engineering properties. Applications include runways, taxi-ways, tunnel liners, slope stabilization and jointless floors.
<b>Special Applications</b> Dosage: 100-160 lbs per CY, 50-80 kg/m <sup>3</sup>	These applications include ballistic blast walls and thin architectural panels.

800-456-9266

www.nycon.com

sales@nycon.com

Figure A3. Technical data sheet for OL 6-mm/0.16mm fiber.

# Dramix®



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## DATA SHEET

## OL 6/.16



OL 6/.16

**What is Dramix®?**

Dramix® steel fibres, from industry specialist Bekaert, have set a new standard for concrete reinforcement with their unique combination of flexibility and cost-efficiency.

The OL range is a cold drawn Ultra High tensile wire. The fibre is formed by cutting straight pieces of this wire.

OL fibres are typically used in high dosage for high strength concrete.

**OL 6/.16 offers you:**

✓ **Dramix® Hi Perform**

Dramix® Hi Perform fibres provide high performance and crack resistance. These are products of choice to create optimal ductility in high strength but brittle concretes from 100 N/mm² onwards.

GEOMETRY AND PERFORMANCE

length (l)



6 mm

diameter (d)



0.16 mm



Bright



High Carbon

1.060.000 Fibres/kg

MATERIAL PROPERTIES

Tensile strength:  $R_m$  min: 2.600 N/mm²

PACKAGING



36 boxes (25kg)  
900 kg



Handle with care: Verify our handling, mixing and dosing recommendations.

STORAGE



KEEP DRY



NO STACKING

APPROVALS



ISO 9001



CE

- ISO 9001 LABEL:** Dramix® OL type is ISO 9001 certified. Production plant:
  - Slovak Republic
- CE LABEL:** Dramix® is certified for CE mark system 1: steel fibres for structural use. For detailed info: CE info sheet available on request.

Bekaert will advise on the most suitable fibre for your application.  
Ask for our recommendations on handling, dosing and mixing at:  
[www.bekaert.com/dosingdramix](http://www.bekaert.com/dosingdramix)  
[infobuilding@bekaert.com](mailto:infobuilding@bekaert.com)

Modifications reserved.  
All details describe our products in general form only. For detailed information, product specifications available on request.


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design by spantab - 7.07.05 - 04/2012






Figure A4. Technical data sheet for OL 10-mm/0.2mm fiber.

# Dramix®




**BEKAERT**

better together

## DATA SHEET

## OL 10/.20



OL 10/.20


**What is Dramix®?**

Dramix® steel fibres, from industry specialist Bekaert, have set a new standard for concrete reinforcement with their unique combination of flexibility and cost-efficiency.

The OL range is a cold drawn Ultra High tensile wire. The fibre is formed by cutting straight pieces of this wire.

OL fibres are typically used in high dosage for high strength concrete.

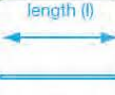
**OL 10/.20 offers you:**

 **Dramix® Hi Perform**

Dramix® Hi Perform fibres provide high performance and crack resistance. These are products of choice to create optimal ductility in high strength but brittle concretes from 100 N/mm<sup>2</sup> onwards.


**GEOMETRY AND PERFORMANCE**

length (l)




10 mm


diameter (d)



0,20 mm



Bright




High Carbon

408.000 Fibres/kg


**MATERIAL PROPERTIES**

Tensile strength: Rm min: 2.600 N/mm<sup>2</sup>


**PACKAGING**




36 boxes (25kg)  
900 kg

 **Handle with care:** Verify our handling, mixing and dosing recommendations.

**STORAGE**




**KEEP DRY**



**NO STACKING**

**APPROVALS**



ISO 9001

• **ISO 9001 LABEL:** Dramix® OL type is ISO 9001 certified. Production plant:  
• Slovak Republic

Bekaert will advise on the most suitable fibre for your application.  
Ask for our recommendations on handling, dosing and mixing at:  
[www.bekaert.com/dosingdramix](http://www.bekaert.com/dosingdramix)  
[infobuilding@bekaert.com](mailto:infobuilding@bekaert.com)

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## Appendix B: DTT Specimens Pretest/Posttest

Figure B1. DTT specimens cut using waterjet from larger flexural beams prior to testing.

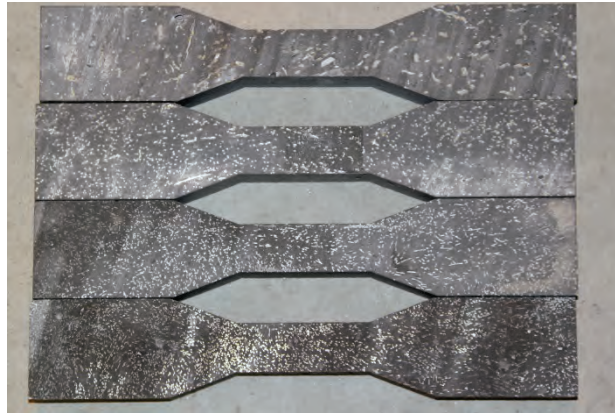


Figure B2. DTT test specimens following testing.



(a) ZP-305.

(b) Nycon Type V.



(c) OL 0.2/10 mm.



(d) OL 0.16/6 mm.

## Appendix C: FSP Test Results

Table C1. FSP experiment characteristics, velocity, and weight loss results.

Shot #	Strike Velocity (ft/s)	Residual Velocity (ft/s)	Sample Description	Sample Dimensions	% Wt. Loss
1	3526	1644	297-13 CT-F 10mm	12" x 12" x 1"	2.299
2	3542	1517	297-13 CT-F 6mm	12" x 12" x 1"	2.006
3	3475	1559	289-13 CT-F NYCON	11 15/16" x 12" x 1 1/32"	3.720
4	3549	1615	281-13 CT-F ZP-305	11 15/16" x 11 15/16" x 1"	3.046
5	3529	267	297-13 CT-F 10mm	12" x 12" x 1 31/32"	4.194
6	3533	232	297-13 CT-F 6mm	12" x 11 31/32" x 2"	4.533
7	3452	221	289-13 CT-F NYCON	12" x 11 31/32" x 2"	5.459
8	3449	255	281-13 CT-F ZP-305	12" x 11 31/32" x 2"	4.340
9	3540	0	297-13 CT-F 10mm	12" x 11 31/32" x 2 1/2"	3.252
10	3520	0	297-13 CT-F 6mm	12" x 12" x 2 1/2"	3.735
11	3546	N/R	289-13 CT-F NYCON	12" x 12" x 2 1/2"	5.542
12	3358	0	281-13 CT-F ZP-305	12 1/32" x 12 1/32" x 2 1/2"	3.352
13	3535	0	297-13 CT-F 10mm	12" x 12" x 3"	1.172
14	3517	0	297-13 CT-F 6mm	12" x 12" x 3"	0.846
15	3574	0	289-13 CT-F NYCON	12" x 12 1/32" x 3 1/16"	1.945
16	3546	0	281-13 CT-F ZP-305	12" x 12" x 3"	1.173
17	3542	1353	297-13 CT-F 10mm	11 31/32" x 11 31/32" x 1 1/16"	2.511
18	3528	1416	297-13 CT-F 6mm	11 31/32" x 11 31/32" x 1 1/16"	2.696
19	3542	1557	289-13 CT-F NYCON	12" x 11 15/16" x 1"	3.606
20	3546	1626	281-13 CT-F ZP-305	12" x 12" x 1"	2.926
21	3553	250	297-13 CT-F 10mm	12" x 12" x 2"	3.827
22	3534	210	297-13 CT-F 6mm	12" x 12" x 2"	4.454
23	3579	262	289-13 CT-F NYCON	12" x 12" x 2"	5.309
24	3487	238	281-13 CT-F ZP-305	12" x 12" x 2"	4.468
25	3523	0	297-13 CT-F 10mm	12" x 11 31/32" x 2 1/2"	3.814
26	3538	0	297-13 CT-F 6mm	12" x 12" x 2 1/2"	3.323
27	3550	N/R	289-13 CT-F NYCON	12" x 12" x 2 1/2"	5.340
28	3558	0	281-13 CT-F ZP-305	12" x 12" x 2 1/2"	3.771
29	3535	0	297-13 CT-F 10mm	12" x 12" x 3"	0.897
30	3537	0	297-13 CT-F 6mm	12" x 12" x 3"	0.828

Shot #	Strike Velocity (ft/s)	Residual Velocity (ft/s)	Sample Description	Sample Dimensions	% Wt. Loss
31	3563	0	289-13 CT-F NYCON	12" x 12" x 3"	1.799
32	3580	0	281-13 CT-F ZP-305	12" x 11 31/32" x 3"	1.100
33	3561	1679	297-13 CT-F 10mm	12" x 12" x 1"	2.246
34	3568	1340	297-13 CT-F 6mm	11 31/32" x 11 31/32" x 1"	2.925
35	3585	1628	289-13 CT-F NYCON	11 15/16" x 12" x 1 1/32"	4.171
36	3525	1389	281-13 CT-F ZP-305	12" x 11 31/32" x 1"	3.472
37	3530	273	297-13 CT-F 10mm	12" x 12" x 2 1/32"	3.717
38	3523	232	297-13 CT-F 6mm	12" x 11 31/32" x 2"	3.565
39	3526	262	289-13 CT-F NYCON	12" x 12" x 2"	5.546
40	3527	244	281-13 CT-F ZP-305	12" x 11 31/32" x 2"	4.723
41	3526	0	297-13 CT-F 10mm	12" x 12 1/32" x 2 17/32"	3.838
42	3531	0	297-13 CT-F 6mm	12" x 12" x 2 9/16"	3.679
43	3506	N/R	289-13 CT-F NYCON	12" x 12" x 2 1/2"	4.845
44	3500	0	281-13 CT-F ZP-305	12" x 12" x 2 1/2"	3.678
45	3499	0	297-13 CT-F 10mm	12" x 12" x 3"	0.734
46	3486	0	297-13 CT-F 6mm	12" x 12" x 3"	1.009
47	3510	0	289-13 CT-F NYCON	12" x 12 1/32" x 3 1/16"	2.241
48	3510	0	281-13 CT-F ZP-305	12" x 12" x 3"	1.004

Notes: Blue shading indicates 1-in. nominal thickness, red shading indicates 2-in. nominal thickness, green shading indicates 2.5-in. nominal thickness, and purple shading indicates 3-in. nominal thickness. "N/R" indicates that chronograph was erroneously triggered by spalled material rather than the intended FSP.



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14. ABSTRACT This investigation focused on identifying the impact of various steel fiber types, including size and shape, on the mechanical responses of an ultra-high performance concrete (UHPC) known as Cor-Tuf Baseline (CTB). CTB specimens were fabricated with four fiber types. The experimental program included testing for quasi-static and dynamic properties, focusing on penetration resistance. Fiber shape and size had a limited impact on quasi-static properties in compression but had a significant impact on quasi-static tensile properties and dynamic penetration resistance. The smaller fibers exhibited up to a 100 percent increase in tensile strength compared with their larger counterparts. However, the benefits offered by the smaller fibers primarily occurred prior to reaching the ultimate load carrying capacity. Once the ultimate strength was reached, larger fibers were more effective at bridging larger cracks. The smaller fibers also provided improved penetration resistance, with reduced residual projectile velocities and loss of material from cratering and/or spallation. Fiber shape and size did not have an impact on the transition between projectile perforation and nonperforation. The results provide insights into optimal fiber reinforcement and potential strategies to further improve the properties of UHPCs, make fiber-reinforced UHPCs more cost competitive by altering fiber dosage rates, and develop hybrid reinforcement schemes.					
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