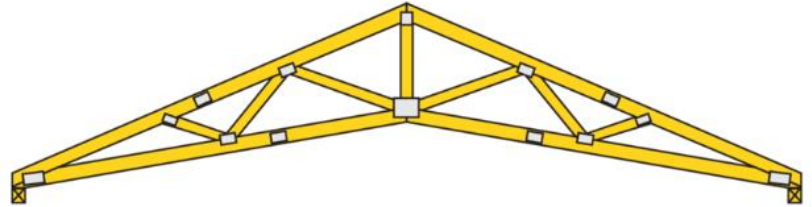


How Scissor Trusses Work:

Scissor trusses are used when a vaulted ceiling is required

Consider the 24' wide Modified Queen scissor truss with top chord at a 7/12 pitch and bottom chord at a 3/12 pitch.



The outer members are named top and bottom chords and the inner members are named the web.

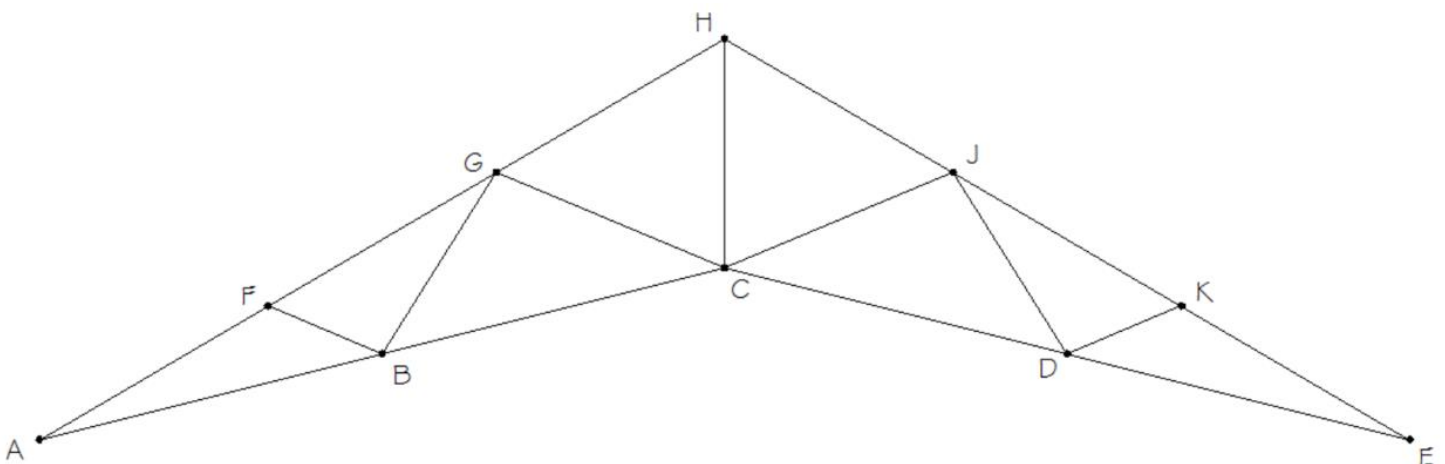
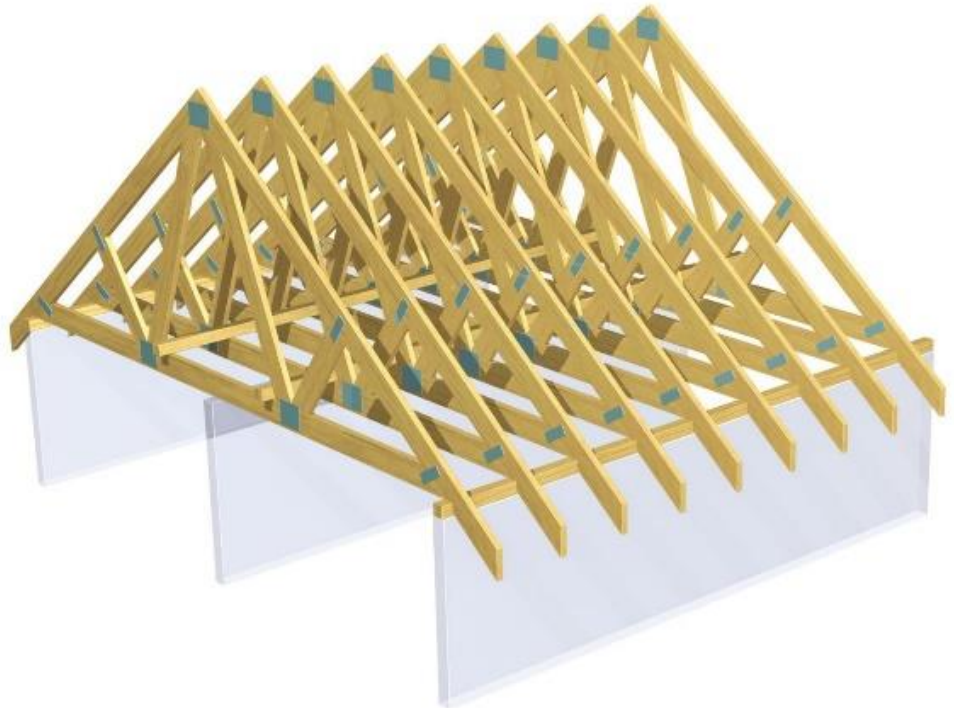
Note: *top and bottom chords are divided into equal lengths.*

1. Calculate all angles and lengths necessary to build the truss.
2. Each truss is responsible for a 2 foot section through the roof as trusses are typically placed 24" apart and thus must carry 12" on either side.
Calculate the tension and compression forces on each member given the load requirements below.

Design loads:

- a. Roof Live load = 25 lb/ft²
- b. Roof Dead load = 15 lb/ft²
- c. Ceiling Dead load = 10 lb/ft²

Note: *Live load on the roof is snow load and dead loads are roofing on the top chord and drywall on the ceiling.*



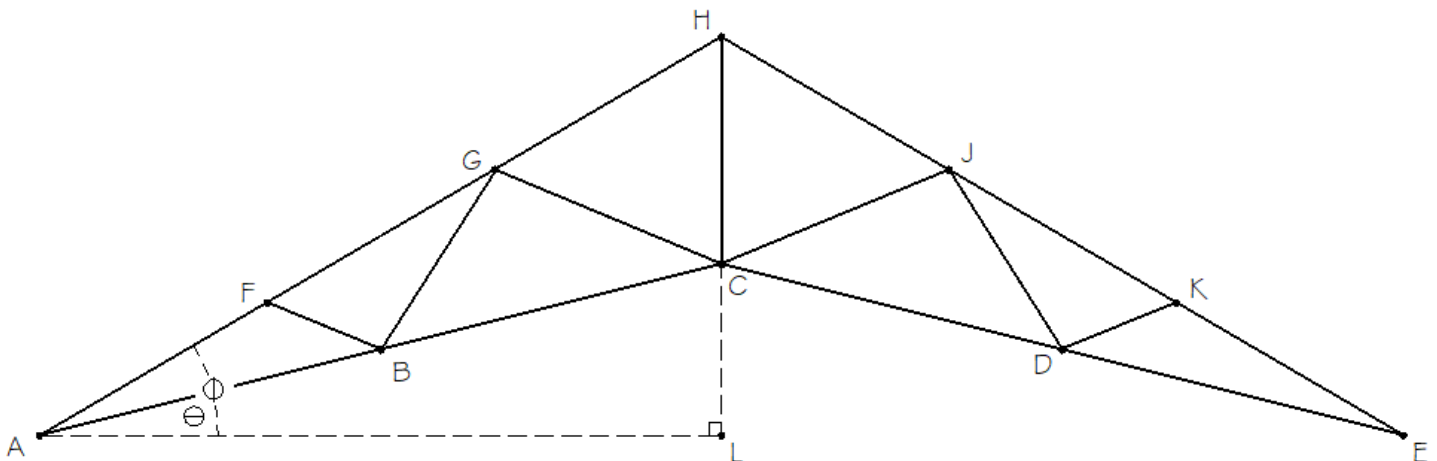
Solution for How Scissor Trusses Work:

1. Angle and Length Solutions:

Given: AE = 24 feet = 288 inches

- (1) $\theta = \tan^{-1}\left(\frac{3}{12}\right) \approx 14.04^\circ$... since the slope of the bottom chord is 3/12 and considering right $\triangle ALC$
- (2) $AC \approx 148.4''$... $\cos(14.04^\circ) = \frac{144}{AC}$
- (3) $AB = BC \approx 74.2''$... as AC is divided into equal lengths based on the design of the truss
- (4) $\Phi = \tan^{-1}\left(\frac{7}{12}\right) \approx 30.26^\circ$... since the slope of the top chord is 7/12 and considering right $\triangle ALH$
- (5) $AH \approx 166.7''$... $\cos(30.26^\circ) = \frac{144}{AH}$
- (6) $AF = FG = GH \approx 55.6''$... as AH is divided into equal lengths based on the design of the truss
- (7) $\angle FAB \approx 16.2^\circ$... $\Phi - \theta$
- (8) $FB \approx 26''$... law of cosines $\triangle AFB$
- (9) $\angle FBA \approx 36.7^\circ$... law of sines $\triangle AFB$
- (10) $\angle AFB \approx 127.1^\circ$... law of sines $\triangle AFB$
- (11) $\angle GFB \approx 52.9^\circ$... supplementary angles
- (12) $GB \approx 44.9''$... law of cosines $\triangle FGB$
- (13) $\angle FGB \approx 27.5^\circ$... law of sines $\triangle FGB$
- (14) $\angle GBC \approx 99.7^\circ$... law of sines $\triangle FGB$
- (15) $\angle GBC \approx 43.7^\circ$... supplementary angles
- (16) $GC \approx 52''$... law of cosines $\triangle GBC$
- (17) $\angle BCG \approx 36.7^\circ$... law of sines $\triangle GBC$
- (18) $\angle BGC \approx 99.7^\circ$... law of sines $\triangle GBC$
- (19) $\angle HGC \approx 52.9^\circ$... supplementary angles
- (20) $HC \approx 48''$... law of cosines $\triangle HGC$
- (21) $\angle GHC \approx 59.7^\circ$... law of sines $\triangle HGC$
- (22) $\angle GCH \approx 67.4^\circ$... law of sines $\triangle HGC$

Note: this is a gross oversimplification as the members are actually not lines. The bottom chord is a trapezoid, the top chord is a parallelogram, and the webs are a pentagon and hexagon. A real truss involves calculating all the lengths and angles in each of these polygons.



2. Compression and Tension Solutions:

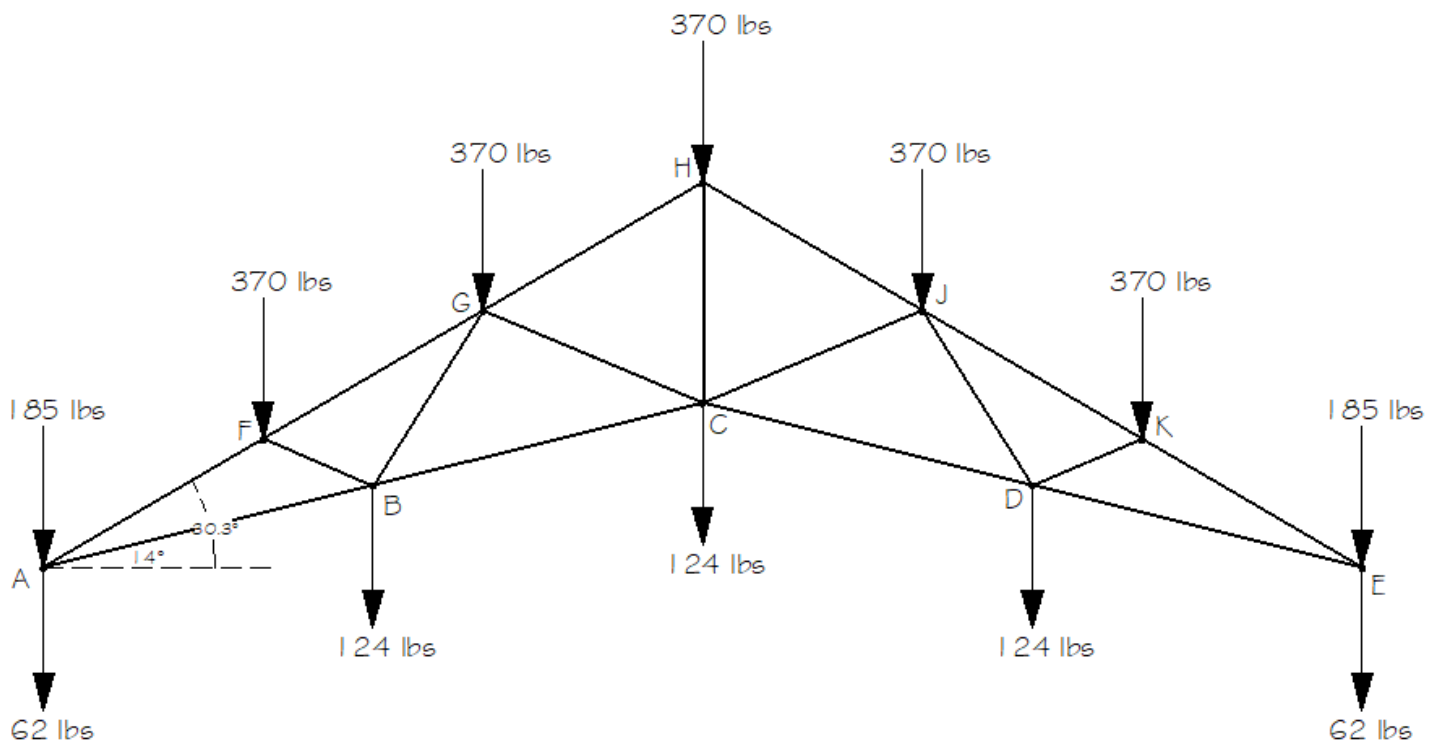
Loading the Truss:

The bottom chord AC is 12.4 feet long and carries a 2 foot wide section of the ceiling weighing 10 lbs/ft². So it must carry 248 lbs, evenly distributed along its length which results in the forces shown at joints A, B, C, D and E.

The top chord AH is 13.89 feet long and carries a 2 foot wide section of the roof weighing 40 lbs/ft². So it must carry 1111 lbs, evenly distributed along its length which results in the forces shown at joints A, F, G, H, J, K and E.

The entire truss then must carry 2718 lbs and thus the walls at A and E must each push up at 1359 lbs. This will result in A being forced up at 1112 lbs.

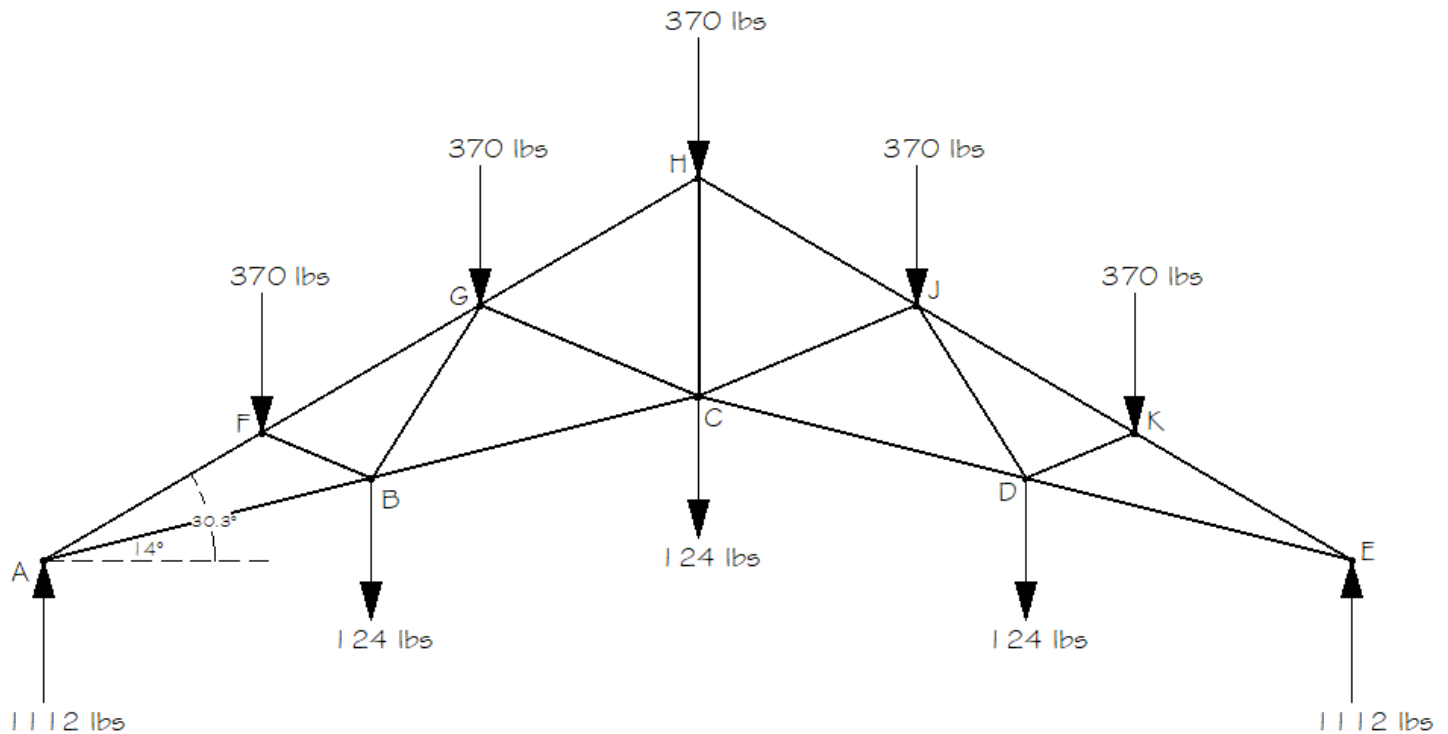
Trusses are then analyzed by looking at each joint and finding the force that each associated member must endure to ensure the joint does not move. The angular nature of the members can be resolved into horizontal and vertical components to simplify the calculations.



Calculating the Forces on the Truss:

Note: (T) = tension force ... a force that would stretch a member, thus a member in tension will pull away from a joint, resisting the stretch it is experiencing.

(C) = compression force ... a force that would compress a member, thus a member in compression will push into a joint, resisting the compression it is experiencing.



The wall at A exerts a force of 1112 lbs vertically, and a force of 0 lbs horizontally at A, which members AF and AB must work together to counter. Let's assume members AF and AB are pulling away from A in tension.

Vertically:

$$AF \cdot \sin(30.3^\circ) + AB \cdot \sin(14^\circ) + 1112 = 0$$

Horizontally:

$$AF \cdot \cos(30.3^\circ) + AB \cdot \cos(14^\circ) = 0$$

Note: $AF \approx -3844$ & $AB \approx 3421$... solving the system of horizontal and vertical linear equations (the negative indicate my assumption of tension in AF was incorrect)

- (1) **AF \approx 3844 lbs (C)**
- (2) **AB \approx 3421 lbs (T)**

AF exerts a force of 1939 lbs vertically, and a force of 3319 lbs horizontally at F, which members FG and FB must work together to counter. Let's assume members FG and FB are pulling away from A in tension.

Vertically:

$$FG \cdot \sin(30.3^\circ) - FB \cdot \sin(22.6^\circ) + 1939 - 370 = 0 \quad \dots \quad 22.6^\circ = 52.9^\circ - 30.3^\circ$$

Horizontally:

$$FG \cdot \cos(30.3^\circ) + FB \cdot \cos(22.6^\circ) + 3319 = 0$$

Note: $FG \approx -3415$ & $FB \approx -401$... solving the system of horizontal and vertical linear equations (the negatives indicate my assumption of tension in AF and FB was incorrect)

(3) **$FG \approx 3415$ lbs (C)**

(4) **$FB \approx 401$ lbs (C)**

FB exerts a force of -154 lbs vertically, and a force of 370 lbs horizontally at B. AB exerts a force of -828 lbs vertically, and a force of -3319 lbs horizontally at B. Members BG and BC must work together to counter these forces. Let's assume members BG and BC are pulling away from B in tension.

Vertically:

$$BG \cdot \sin(57.7^\circ) + BC \cdot \sin(14^\circ) - 154 - 828 - 124 = 0 \quad \dots \quad 57.7^\circ = 43.7^\circ + 14^\circ$$

Horizontally:

$$BG \cdot \cos(57.7^\circ) + BC \cdot \cos(14^\circ) + 370 - 3319 = 0$$

Note: $BG \approx 520$ & $BC \approx 2753$... solving the system of horizontal and vertical linear equations

(5) **$BG \approx 520$ lbs (T)**

(6) **$BC \approx 2753$ lbs (T)**

FG exerts a force of 1723 lbs vertically, and a force of 2948 lbs horizontally at G. BG exerts a force of -440 lbs vertically, and a force of -278 lbs horizontally at G. Members GH and GC must work together to counter these forces. Let's assume members GH and GC are pulling away from G in tension.

Vertically:

$$GH \cdot \sin(30.3^\circ) - GC \cdot \sin(22.6^\circ) + 1723 - 440 - 370 = 0$$

Horizontally:

$$GH \cdot \cos(30.3^\circ) + GC \cdot \cos(22.6^\circ) + 2948 - 278 = 0$$

Note: $GH \approx -2343$ & $GC \approx -701$... solving the system of horizontal and vertical linear equations (the negatives indicate my assumption of tension in AF and FB was incorrect)

(7) **$GH \approx 2343$ lbs (C)**

(8) **$GC \approx 701$ lbs (C)**

GH (and thus JH as well based on symmetry) exert forces of 1182 lbs vertically at H. Member HC must counter these forces. Let's assume members HC is pulling away from H in tension.

Vertically:

$$2 * 1182 - HC - 370 = 0$$

(9) **HC \approx 1994 lbs (T)**

Note: symmetry of the truss makes the right side unnecessary to calculate

Note: good as a check to ensure the vertical force at C is also zero: $1994 - 2 * 701 * \sin(22.6^\circ) - 2 * 2753 * \sin(14^\circ) - 124 \approx 0$