## MEEN 618: ENERGY AND VARIATIONAL METHODS

## Read: Chapter 5



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$>$ Principle of superposition
$>$ Clapeyron's Theorem
$>$ Elasticity problems, uniqueness
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## THE PRINCIPLE OF SUPERPOSITION

The principle of superposition is said to hold for a problem if the responses (i.e., displacements) under two sets of boundary conditions and loads are equal to the sum of the responses obtained by applying each set of boundary conditions and loads separately.

$$
\begin{aligned}
& \text { Set 1: } \mathbf{u}=\hat{\mathbf{u}}^{(1)} \text { on } \Gamma_{u} ; \mathbf{t}=\hat{\mathbf{t}}^{(1)} \text { on } \Gamma_{\sigma} ; \mathbf{f}=\mathbf{f}^{(1)} \text { in } \Omega \\
& \text { Set 2: } \mathbf{u}=\hat{\mathbf{u}}^{(2)} \text { on } \Gamma_{u} ; \mathbf{t}=\hat{\mathbf{t}}^{(2)} \text { on } \Gamma_{\sigma} ; \mathbf{f}=\mathbf{f}^{(2)} \text { in } \Omega
\end{aligned}
$$

Then if

$$
\mathbf{u}=\hat{\mathbf{u}}^{(1)}+\hat{\mathbf{u}}^{(2)} \text { on } \Gamma_{u} ; \mathbf{t}=\hat{\mathbf{t}}^{(1)}+\hat{\mathbf{t}}^{(2)} \text { on } \Gamma_{\sigma} ; \mathbf{f}=\mathbf{f}^{(1)}+\mathbf{f}^{(2)} \text { in } \Omega
$$

and the solution is

$$
\mathbf{u}(\mathbf{x})=\mathbf{u}^{(1)}(\mathbf{x})+\mathbf{u}^{(2)}(\mathbf{x}) \text { for all } \mathbf{x} \text { in } \Omega
$$

We say that the principle of superposition holds.

## THE PRINCIPLE OF SUPERPOSITION

## Application to Beams



The displacement field of a beam subjected to two different loads

$$
\begin{gathered}
w^{q}(x)=\frac{q_{0} L^{4}}{24 E I}\left[3-4 \frac{x}{L}+\left(\frac{x}{L}\right)^{4}\right], w^{F}(x)=\frac{F_{0} L^{3}}{6 E I}\left[2-3 \frac{x}{L}+\left(\frac{x}{L}\right)^{3}\right] \\
w(x)=w^{q}(x)+w^{F}(x)=\frac{q_{0} L^{4}}{24 E I}\left[3-4 \frac{x}{L}+\left(\frac{x}{L}\right)^{4}\right]+\frac{F_{0} L^{3}}{6 E I}\left[2-3 \frac{x}{L}+\left(\frac{x}{L}\right)^{3}\right]
\end{gathered}
$$

## THE PRINCIPLE OF VIRTUAL DISPLACEMENTS

Consider the beam shown in the figure. It can be viewed as a superposition of two different loads.

$$
F_{s}=k w_{\mathrm{A}}(0)
$$



Given Problem


## CLAYPERON'S THEOREM

Theorem: The strain energy stored in a linear elastic body is equal to one-half of the work done by external forces on the body:

$$
U=-\frac{1}{2} V_{E}
$$

Proof: We begin with the strain energy stored in a linear elastic body

$$
\begin{aligned}
U & =\frac{1}{2} \int_{\Omega} \boldsymbol{\sigma}: \varepsilon d \Omega=\frac{1}{2} \int_{\Omega} \sigma_{i j} \varepsilon_{i j} d \Omega=U^{*} \\
& =\frac{1}{4} \int_{\Omega} \sigma_{i j}\left(u_{i, j}+u_{j, i}\right) d \Omega=\frac{1}{2} \int_{\Omega} \sigma_{i j} u_{i, j} d \Omega \\
& =-\frac{1}{2} \int_{\Omega} \sigma_{i j, j} u_{i} d \Omega+\frac{1}{2} \oint_{\Gamma} n_{j} \sigma_{i j} u_{i} d \Gamma \\
& =\frac{1}{2} \int_{\Omega} f_{i} u_{i} d \Omega+\frac{1}{2} \oint_{\Gamma} t_{i} u_{i} d \Gamma=-\frac{1}{2} V_{E}
\end{aligned}
$$

## USES OF CLAYPERON'S THEOREM

Problem: Find $w_{a}$ using Clayperon's Theorem.
Solution: By Clayperon's Theorem,
$\frac{1}{2} F_{0} w_{a}=U^{*} \equiv \frac{1}{2} \int_{0}^{L}\left(\frac{M^{2}}{E I}+\frac{V^{2}}{G A K_{s}}\right) d x$
$M(x)=\left\{\begin{array}{ll}0, & 0 \leq x \leq a \\ F_{0}(x-a) & , a \leq x \leq L\end{array}, \quad V(x)=\left\{\begin{array}{c}0 \\ F_{0}\end{array}\right.\right.$

$\frac{1}{2} F_{0} w_{a}=\frac{1}{2} \int_{a}^{L}\left(\frac{F_{0}^{2}}{E I}(x-a)^{2}+\frac{F_{0}^{2}}{G A K_{s}}\right) d x$
$F_{0} w_{a}=\left(\frac{F_{0}^{2}}{3 E I}(x-a)^{3}+\frac{F_{0}^{2}}{G A K_{s}} x\right)_{a}^{L} \Rightarrow w_{a}=\frac{F_{0} b^{3}}{3 E I}+\frac{F_{0} b}{G A K_{s}}$

## USES OF CLAYPERON'S THEOREM

## Problem: Find $\theta_{0}$ using Clayperon's Theorem.

Solution: By Clayperon's Theorem,

$$
\begin{aligned}
& \frac{1}{2} \int_{0}^{L} q_{0} w(x) d x-\frac{1}{2} M_{0} \theta_{0}=\frac{E I}{2} \int_{0}^{L}\left(\frac{d^{2} w}{d x^{2}}\right)^{2} d x \\
& w(x)=\sum_{j=1}^{4} \Delta_{j} \varphi_{j}(x)=\Delta_{2} \varphi_{2}(x)=-x\left(1-\frac{x}{L}\right)^{2} \theta_{0} \\
& \int_{0}^{L} q_{0} w(x) d x-M_{0} \theta_{0}=E I \int_{0}^{L}\left(\frac{d^{2} w}{d x^{2}}\right)^{2} d x \\
& -\frac{q_{0}=w(0)}{12} \theta_{0}^{2}-M_{0} \theta_{0}=\frac{4 E I}{L} \theta_{0}^{2} \Rightarrow \theta_{0}=-(\frac{q_{0} L^{3}}{48 E I}+\frac{M_{0} L}{4 E L} \underbrace{}_{x=0} \Delta_{4}=-\left.\frac{d w}{d x}\right|_{x=L}
\end{aligned}
$$

## BETTI'S RECIPROCITY THEOREM

Consider a linear elastic body undergoing small strains. Let $W_{1}$ be the work produced by $\mathbf{F}_{1}$. Then, we apply force $\mathbf{F}_{2}$, which produces work $W_{1}$. When force $\mathbf{F}_{2}$ is applied, force $\mathbf{F}_{1}$ does additional work because its point of application is displaced due to the deformation caused by force $\mathbf{F}_{2}$. Let us denote this work by $W_{12}$, which is the work done by force $\mathbf{F}_{1}$ in moving through the displacement produced by force $\mathbf{F}_{2}$. Thus, the total work done is

$$
W=W_{1}+W_{2}+W_{12}
$$

When the order of application of the forces is reversed, we obtain

$$
W=W_{1}+W_{2}+W_{21}
$$

## BETTI'S \& MAXWELL'S RECIPROCITY THEOREMS

The work done in both cases should be the same because at the end the elastic body is loaded by the same pair of external forces. Thus we have

$$
W_{12}=W_{21}
$$

This is known as Betti's reciprocity theorem.
Let $\mathbf{u}_{12}$ be the displacement of point 1 produced by unit force $\mathbf{F}^{2}$ in the direction of force $\mathbf{F}^{1}$ and $\mathbf{u}_{21}$ be the displacement of point 2 produced by unit force $\mathbf{F}^{1}$ in the direction of force $\mathbf{F}^{2}$. Then by Betti's reciprocity theorem we have

$$
\mathbf{F}^{1} \cdot \mathbf{u}_{12}=\mathbf{F}^{2} \cdot \mathbf{u}_{21} \Rightarrow u_{12}=u_{21}
$$

This is the Maxwell's reciprocity theorem.

Theorem 4.6.2: If a linear elastic body is subjected to two different sets of forces, the work done by the first system of forces in moving through the displacements produced by the second system of forces is equal to the work done by the second system of forces in moving through the displacements produced by the first system of forces:

$$
\begin{equation*}
\int_{\Omega} \mathbf{f}^{(1)} \cdot \mathbf{u}^{(2)} d \Omega+\int_{\Gamma_{\sigma}} \mathbf{t}^{(1)} \cdot \mathbf{u}^{(2)} d \Gamma=\int_{\Omega} \mathbf{f}^{(2)} \cdot \mathbf{u}^{(1)} d \Omega+\int_{\Gamma_{\sigma}} \mathbf{t}^{(2)} \cdot \mathbf{u}^{(1)} d \Gamma, \tag{4.6.25}
\end{equation*}
$$

where $\mathbf{u}^{(1)}$ is the displacement produced by body forces $\mathbf{f}^{(1)}$ and surface forces $\mathbf{t}^{(1)}$, and $\mathbf{u}^{(2)}$ is the displacement produced by body forces $\mathbf{f}^{(2)}$ and surface forces $\mathbf{t}^{(2)}$. The left-hand side of Eq. (4.6.25), for example, denotes the work done by forces $\mathbf{f}^{(1)}$ and $\mathbf{t}^{(1)}$ in moving through the displacement $\mathbf{u}^{(2)}$ produced by forces $\mathbf{f}^{(2)}$ and $\mathbf{t}^{(2)}$.

Proof: The proof of Betti's reciprocity theorem is straightforward. Let $W_{12}$ denote the work done by forces $\left(\mathbf{f}^{(1)}, \mathbf{t}^{(1)}\right)$ acting through the displacement $\mathbf{u}^{(2)}$ produced by the forces $\left(\mathbf{f}^{(2)}, \mathbf{t}^{(2)}\right)$. Then

$$
\begin{aligned}
W_{12} & =\int_{\Omega} \mathbf{f}^{(1)} \cdot \mathbf{u}^{(2)} d \Omega+\oint_{\Gamma} \mathbf{t}^{(1)} \cdot \mathbf{u}^{(2)} d \Gamma \\
& =\int_{\Omega} f_{i}^{(1)} u_{i}^{(2)} d \Omega+\oint_{\Gamma} t_{i}^{(1)} u_{i}^{(2)} d \Gamma \\
& =\int_{\Omega} f_{i}^{(1)} u_{i}^{(2)} d \Omega+\oint_{\Gamma} n_{j} \sigma_{j i}^{(1)} u_{i}^{(2)} d \Gamma \\
& =\int_{\Omega} f_{i}^{(1)} u_{i}^{(2)} d \Omega+\int_{\Omega}\left(\sigma_{j i}^{(1)} u_{i}^{(2)}\right)_{, j} d \Omega \\
& =\int_{\Omega}\left(\sigma_{i j, j}^{(1)}+f_{i}^{(1)}\right) u_{i}^{(2)} d \Omega+\int_{\Omega} \sigma_{i j}^{(1)} u_{i, j}^{(2)} d \Omega \\
& =\int_{\Omega} \sigma_{i j}^{(1)} u_{i, j}^{(2)} d \Omega=\int_{\Omega} \sigma_{i j}^{(1)} \varepsilon_{i j}^{(2)} d \Omega .
\end{aligned}
$$

Using Hooke's law $\sigma_{i j}^{(1)}=C_{i j k \ell} \varepsilon_{k \ell}^{(1)}$, we obtain

$$
\begin{equation*}
W_{12}=\int_{0} C_{i j k \ell} \varepsilon_{k \ell}^{(1)} \varepsilon_{i j}^{(2)} d \Omega \tag{4.6.26}
\end{equation*}
$$

Since $C_{i j k \ell}=C_{k \ell i j}$, it follows that

$$
W_{12}=\int_{\Omega} C_{i j k \ell} \varepsilon_{k \ell}^{(1)} \varepsilon_{i j}^{(2)} d \Omega=\int_{\Omega} C_{k \ell i j} \varepsilon_{i j}^{(2)} \varepsilon_{k \ell}^{(1)} d \Omega=\int_{\Omega} \sigma_{k \ell}^{(2)} \varepsilon_{k \ell}^{(1)} d \Omega=W_{21} .
$$

One can trace back to show that $W_{21}$ is equal to the right-hand side of Eq. (4.6.25). This completes the proof.

During the proof we have also established the equality

$$
\begin{align*}
\int_{\Omega} \sigma_{i j}^{(1)} \varepsilon_{i j}^{(2)} d \Omega & =\int_{\Omega} \sigma_{i j}^{(2)} \varepsilon_{i j}^{(1)} d \Omega \\
\int_{\Omega} \boldsymbol{\sigma}^{(1)}: \boldsymbol{\varepsilon}^{(2)} d \Omega & =\int_{\Omega} \boldsymbol{\sigma}^{(2)}: \boldsymbol{\varepsilon}^{(1)} d \Omega \tag{4.6.27}
\end{align*}
$$

## Example 4.6.4

Consider a cantilever beam of length $L$ subjected to two sets of loads: a uniformly distributed load of intensity $q_{0}$ throughout the span, as shown in Fig. 4.6.6(a), and a concentrated load $F$ at the free end, as shown in Fig. 4.6.6(b). Verify Betti's reciprocity theorem, that is, the work done by the point load $F$ in moving through the displacement $w^{q}(0)$ produced by $q_{0}$ is equal to the work done by the distributed force $q_{0}$ in moving through the displacement $w^{F}(x)$ produced by the point load $F$.


Fig. 4.6.6: (a) A cantilever beam under uniformly distributed load. (b) A cantilever beam with a point load at its free end.

Solution: From Eqs. (4.6.5) and (4.6.6), the expression for deflection of the cantilever beam with uniformly distributed load $q_{0}$ alone is

$$
\begin{equation*}
w_{0}^{q}(x)=\frac{q_{0} L^{4}}{24 E I}\left[3-4\left(\frac{x}{L}\right)+\left(\frac{x}{L}\right)^{4}\right] . \tag{1}
\end{equation*}
$$

and the expression for deflection of the cantilever beam with the point load $F$ at the free end alone is

$$
\begin{equation*}
w_{0}^{F}(x)=\frac{F L^{3}}{6 E I}\left[2-3\left(\frac{x}{L}\right)+\left(\frac{x}{L}\right)^{3}\right] \tag{2}
\end{equation*}
$$

The work done by the load $F$ in moving through the displacement due to the application of the uniformly distributed load $q_{0}$ is

$$
W_{F q}=F w_{0}^{q}(0)=\frac{F q_{0} L^{4}}{8 E I}
$$

The work done by the uniformly distributed $q_{0}$ in moving through the displacement field $w^{F}(x)$ due to the application of point load $F$ is

$$
W_{q F}=\int_{0}^{L} \frac{F L^{3}}{6 E I}\left[2-3\left(\frac{x}{L}\right)+\left(\frac{x}{L}\right)^{3}\right] q_{0} d x=\frac{F q_{0} L^{4}}{8 E I}
$$

which is in agreement with $W_{F q}$.

## Example 4.6.5

Use Betti's reciprocity theorem to determine the deflection at the free end of a cantilever beam with distributed load of intensity $q_{0}$ in the span between $x=a$ and $x=L$, as shown in Fig. 4.6.7. The deflection $w^{F}(x)$ due to a point load $F$ at the free end (acting upward) is

$$
\begin{equation*}
w^{F}(x)=\frac{F L^{3}}{6 E I}\left[3\left(\frac{x}{L}\right)^{2}-\left(\frac{x}{L}\right)^{3}\right] \tag{1}
\end{equation*}
$$



Fig. 4.6.7: A cantilever beam with uniformly distributed load on a portion of the beam.

Solution: The work done by the point load $F$ in moving through the displacement due to the application of the uniformly distributed load $q_{0}$ is

$$
\begin{equation*}
W_{F q}=F w_{0}^{q}(L) \tag{2}
\end{equation*}
$$

The work done by the uniformly distributed load $q_{0}$ in moving through the displacement field $w^{F}(x)$ due to the application of point load $F$ is

$$
\begin{equation*}
W_{q F}=\int_{a}^{L} \frac{F L^{3}}{6 E I}\left[3\left(\frac{x}{L}\right)^{2}-\left(\frac{x}{L}\right)^{3}\right] q_{0} d x=\frac{F q_{0}}{24 E I}\left(3 L^{4}-4 L a^{3}+a^{4}\right) \tag{3}
\end{equation*}
$$

By Betti's reciprocity theorem, we have $W_{q F}=W_{F q}$. Hence, the deflection at the free end of the beam due to the distributed load is

$$
\begin{equation*}
w_{0}^{q}(L)=\frac{q_{0}}{24 E I}\left(3 L^{4}-4 L a^{3}+a^{4}\right) . \tag{4}
\end{equation*}
$$

## Example 4.6 .6

Consider a cantilever beam of length $L$ and constant $E I$ and subjected to a point load $F_{0}$ at the free end [see Fig. 4.6.9(a)]. Use Maxwell's theorem to determine the deflection at $x=a$ from the free end. Use the following data: $E=24 \times 10^{6} \mathrm{psi}, I=120 \mathrm{in}^{4}, F_{0}=1,000 \mathrm{lb}$, $a=36 \mathrm{in}$, and $b=108 \mathrm{in}$.
Solution: By Maxwell's theorem, the displacement $w_{\text {BA }}$ at point B $(x=a)$ produced by unit load at point $\mathrm{A}(x=0)$ is equal to the deflection $w_{\mathrm{AB}}$ at point A produced by unit load at point B . We are required to find $w(0)=w_{\mathrm{BA}} F_{0}$. Thus, we must determine $w_{\mathrm{AB}}$ (which,


Fig. 4.6.9: A cantilever beam with a point load at the free end.
presumably, is easier to compute by some way than to compute $w(0)$ directly). Let $w_{\mathrm{B}}$ and $\theta_{\mathrm{B}}$ denote the deflection and slope, respectively, at point B owing to a load $F=1$ applied at point B. Then the deflection at point A due to load $F=1$ is [see Fig. 4.6.9(b)]

$$
\begin{equation*}
w_{\mathrm{AB}}=w_{\mathrm{B}}+\theta_{\mathrm{B}} a \tag{1}
\end{equation*}
$$

and the required solution is

$$
\begin{equation*}
w(0)=w_{\mathrm{BA}} F_{0}=w_{\mathrm{AB}} F_{0}=F_{0}\left(w_{\mathrm{B}}+a \theta_{\mathrm{B}}\right) . \tag{2}
\end{equation*}
$$

The values of $w_{\mathrm{B}}$ and $\theta_{\mathrm{B}}$ can be computed using Eq. (4.6.6) as

$$
\begin{align*}
w_{\mathrm{B}} & =\frac{b^{3}}{6 E I}\left[2-3 \frac{\bar{x}}{b}+\left(\frac{\bar{x}}{b}\right)^{3}\right]_{\bar{x}=0}=\frac{b^{3}}{3 E I},  \tag{3}\\
\theta_{\mathrm{B}} & =-\left.\frac{d w}{d x}\right|_{\bar{x}=0}=\frac{b^{2}}{2 E I}\left[1-\left(\frac{\bar{x}}{b}\right)^{2}\right]_{\bar{x}=0}=\frac{b^{2}}{2 E I} . \tag{4}
\end{align*}
$$

Therefore, we have

$$
\begin{align*}
w(0) & =F_{0}\left(w_{\mathrm{B}}+a \theta_{\mathrm{B}}\right)=F_{0}\left(\frac{b^{3}}{3 E I}+\frac{b^{2} a}{2 E I}\right)=\frac{F_{0} b^{2}}{6 E I}(3 a+2 b) \\
& =\frac{1,000 \times(108)^{2}}{6 \times 24 \times 10^{6} \times 120}(3 \times 36+2 \times 108)=0.2187 \mathrm{in} . \tag{5}
\end{align*}
$$

Principle of virtual displacements and the associated Euler equations [see Eqs. (4.2.2), (4.2.5), and (4.2.6)]

$$
\begin{gather*}
\delta W=\delta W_{I}+\delta W_{E}=0 .  \tag{4.7.1}\\
\boldsymbol{\nabla} \cdot \boldsymbol{\sigma}+\mathbf{f}=\mathbf{0} \text { in } \Omega ; \quad \hat{\mathbf{n}} \cdot \boldsymbol{\sigma}-\hat{\mathbf{t}}=\mathbf{0} \text { on } \Gamma_{\sigma} . \tag{4.7.2}
\end{gather*}
$$

Unit dummy-displacement method [see Eq. (4.2.7)]

$$
\begin{equation*}
\mathbf{F}_{0} \cdot \delta \mathbf{u}_{0}=\int_{\Omega} \boldsymbol{\sigma}: \delta \varepsilon^{0} d \Omega \tag{4.7.3}
\end{equation*}
$$

The principle of minimum total potential energy and the associated Euler equations [see Eqs. (4.3.5) and (4.3.18)]

$$
\begin{equation*}
\delta \Pi \equiv \delta\left(U+V_{E}\right)=0 \tag{4.7.4}
\end{equation*}
$$

$$
\begin{equation*}
\mu \nabla^{2} \mathbf{u}+(\lambda+\mu) \boldsymbol{\nabla}(\boldsymbol{\nabla} \cdot \mathbf{u})+\mathbf{f}=\mathbf{0} \text { in } \Omega, \quad \text { and } \quad \mathbf{t}-\hat{\mathbf{t}}=\mathbf{0} \text { on } \Gamma_{\sigma} . \tag{4.7.5}
\end{equation*}
$$

Castigliano's Theorem I [see Eq. (4.3.22)]

$$
\begin{equation*}
\frac{\partial U}{\partial \mathbf{u}_{i}}=\mathbf{F}_{i} . \tag{4.7.6}
\end{equation*}
$$

Principle of virtual forces and the associated Euler equations [see Eqs. (4.4.5) and (4.4.10)]

$$
\begin{gather*}
\delta W^{*}=\delta W_{I}^{*}+\delta W_{E}^{*}=0 .  \tag{4.7.7}\\
\varepsilon-\frac{1}{2}\left[\nabla \mathbf{u}+(\nabla \mathbf{u})^{\mathrm{T}}\right]=\mathbf{0} \text { in } \Omega ; \quad \mathbf{u}-\hat{\mathbf{u}}=\mathbf{0} \quad \text { on } \Gamma_{u} . \tag{4.7.8}
\end{gather*}
$$

Unit dummy-load method [see Eq. (4.4.11)]

$$
\begin{equation*}
\delta \mathbf{F}_{0} \cdot \mathbf{u}_{0}=\int_{\Omega} \boldsymbol{\varepsilon}: \delta \boldsymbol{\sigma}^{0} d \Omega \tag{4.7.9}
\end{equation*}
$$

The principle of minimum total complementary energy and the associated Euler equations [see Eqs. (4.5.2) and (4.5.5)]

$$
\begin{equation*}
\delta \Pi^{*} \equiv \delta\left(U^{*}+V_{E}^{*}\right)=0 . \tag{4.7.10}
\end{equation*}
$$

$$
\begin{equation*}
\mathbf{C}^{*}: \boldsymbol{\sigma}-\frac{1}{2}\left[\nabla \mathbf{u}+(\boldsymbol{\nabla} \mathbf{u})^{\mathrm{T}}\right]=\mathbf{0} \text { in } \Omega ; \quad \mathbf{u}-\hat{\mathbf{u}}=\mathbf{0} \text { on } \Gamma_{u} . \tag{4.7.11}
\end{equation*}
$$

Castigliano's Theorem II [see Eq. (4.5.13)]

$$
\begin{equation*}
\frac{\partial U^{*}}{\partial \mathbf{F}_{i}}=\mathbf{u}_{i} \tag{4.7.12}
\end{equation*}
$$

Clapeyron's Theorem [see Eq. (4.6.10)]

$$
\begin{equation*}
\frac{1}{2} \int_{\Omega} \boldsymbol{\sigma}: \varepsilon d \Omega=\frac{1}{2}\left[\int_{\Omega} \mathbf{f} \cdot \mathbf{u} d \Omega+\oint_{\Gamma} \mathbf{t} \cdot \mathbf{u} d \Gamma\right] . \tag{4.7.13}
\end{equation*}
$$

Betti's Reciprocity Theorem [see Eqs. (4.6.25) and (4.6.27)]

$$
\begin{gather*}
\int_{\Omega} \mathbf{f}^{(1)} \cdot \mathbf{u}^{(2)} d \Omega+\int_{\Gamma_{\sigma}} \mathbf{t}^{(1)} \cdot \mathbf{u}^{(2)} d \Gamma=\int_{\Omega} \mathbf{f}^{(2)} \cdot \mathbf{u}^{(1)} d \Omega+\int_{\Gamma_{\sigma}} \mathbf{t}^{(2)} \cdot \mathbf{u}^{(1)} d \Gamma .  \tag{4.7.14}\\
\int_{\Omega} \boldsymbol{\sigma}^{(1)}: \boldsymbol{\varepsilon}^{(2)} d \Omega=\int_{\Omega} \boldsymbol{\sigma}^{(2)}: \boldsymbol{\varepsilon}^{(1)} d \Omega \tag{4.7.15}
\end{gather*}
$$

Maxwell's Reciprocity Theorem [see Eqs. (4.6.28)]

$$
\begin{equation*}
\mathbf{F}^{1} \cdot \mathbf{u}_{12}=\mathbf{F}^{2} \cdot \mathbf{u}_{21} \quad \text { or } \quad u_{12}=u_{21} \tag{4.7.16}
\end{equation*}
$$

The principle of the minimum total potential energy

$$
\left.\begin{array}{c}
\delta \Pi=0, \Pi=U+V_{E} \\
\mu \nabla^{2} \mathbf{u}+(\boldsymbol{\lambda}+\boldsymbol{\mu}) \nabla(\nabla \cdot \mathbf{u})+\mathbf{f}=\mathbf{0} \text { in } \boldsymbol{\Omega}, \\
\mu \hat{\mathbf{n}} \cdot \nabla \mathbf{u}+(\boldsymbol{\lambda}+\boldsymbol{\mu}) \hat{\mathbf{n}}(\nabla \cdot \mathbf{u})=\hat{\mathbf{t}} \text { on } \Gamma_{\sigma}
\end{array}\right] \begin{gathered}
\text { Castgliano's Theorem I } \\
\mathbf{F}_{i}=\frac{\partial U}{\partial \mathbf{u}_{i}}
\end{gathered}
$$

The principle of the minimum total complementary energy

$$
\begin{array}{r}
\delta \Pi^{*}=0, \Pi^{*}=U^{*}+V_{E} \\
\mathbf{C}^{*}: \sigma-\frac{1}{2}\left[\nabla \mathbf{u}+(\nabla \mathbf{u})^{\mathrm{T}}\right]=\mathbf{0} \text { in } \boldsymbol{\Omega} \\
\mathbf{u}-\hat{\mathbf{u}}=\mathbf{0} \text { on } \Gamma_{u} \\
\downarrow \\
\begin{array}{r}
\text { Castgliano's Theorem II } \\
\mathbf{u}_{i}=\frac{\partial U^{*}}{\partial \mathbf{F}_{i}}
\end{array}
\end{array}
$$

Fig. 4.7.1: A flow chart of various energy principles and methods.

