



Post-Installed Reinforcement





- What is an Adhesive Anchoring "System"?
- Reinforced Concrete Member Design
- Anchor Theory & Code Requirements
- Alternative Methods of Design
- Design Concept of Post-Installed Reinforcing
- Applications

What is an Adhesive Anchoring "System"?

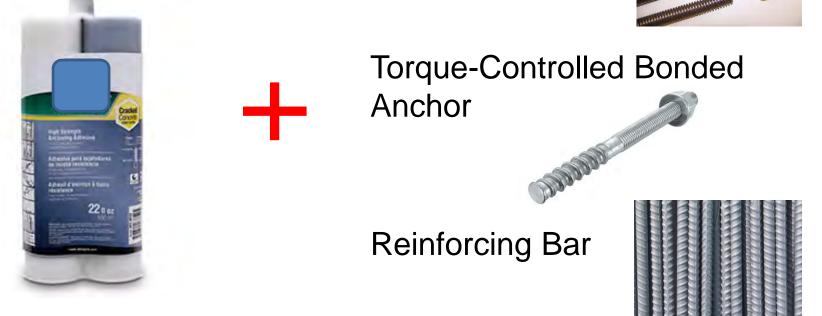
Adhesive Anchoring Systems

Adhesive "System" Has Two Components:

Adhesive

Threaded Rod



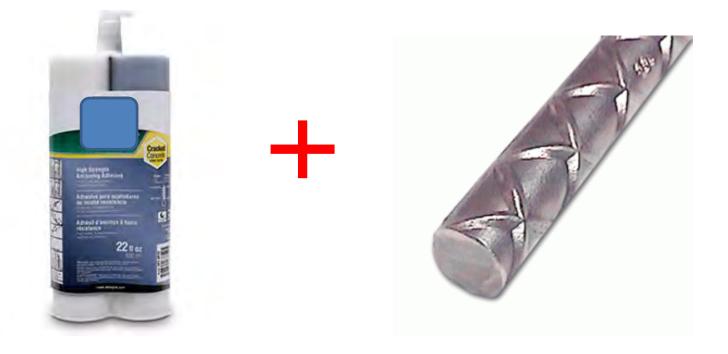


Adhesive Anchoring Systems

Adhesive "System" Has Two Components:

Adhesive

Reinforcing Bar

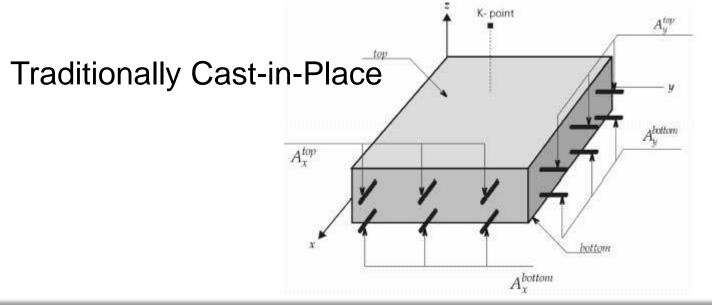


Reinforced Concrete Member Design

Reinforced Concrete Member Design

Uses Reinforcing Bar to:

- Increase bending strength
- Increase tensile strength
- Increase compressive strength (confinement)



Adhesive Anchoring Systems as Recognized by IBC

ACI 318-11 Scope

D.2.2 — This appendix applies to cast-in anchors and to post-installed expansion (torque-controlled and displacement-controlled), undercut, and adhesive anchors. Adhesive anchors shall be installed in concrete having a minimum age of 21 days at time of anchor installation. Specialty inserts, through-bolts, multiple anchors connected to a single steel plate at the embedded end of the anchors, grouted anchors, and direct anchors such as powder or pneumatic actuated nails or bolts are not included in the provisions of Appendix D. Reinforcement used as part of the embedment shall be designed in accordance with other parts of this Code.

ACI 318-11 Scope

D.2.3 — Design provisions are included for the following types of anchors:

(a) Headed studs and headed bolts having a geometry that has been demonstrated to result in a pullout strength in uncracked concrete equal to or exceeding $1.4N_p$, where N_p is given in Eq. (D-14);

(b) Hooked bolts having a geometry that has been demonstrated to result in a pullout strength without the benefit of friction in uncracked concrete equal to or exceeding $1.4N_p$, where N_p is given in Eq. (D-15);

(c) Post-installed expansion and undercut anchors that meet the assessment criteria of ACI 355.2; and



(d) Adhesive anchors that meet the assessment criteria of ACI 355.4.



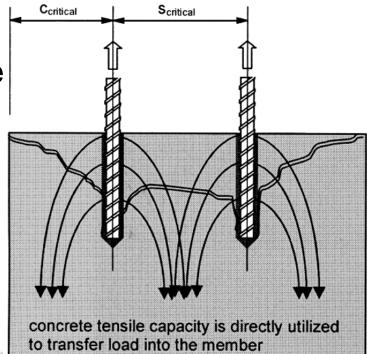


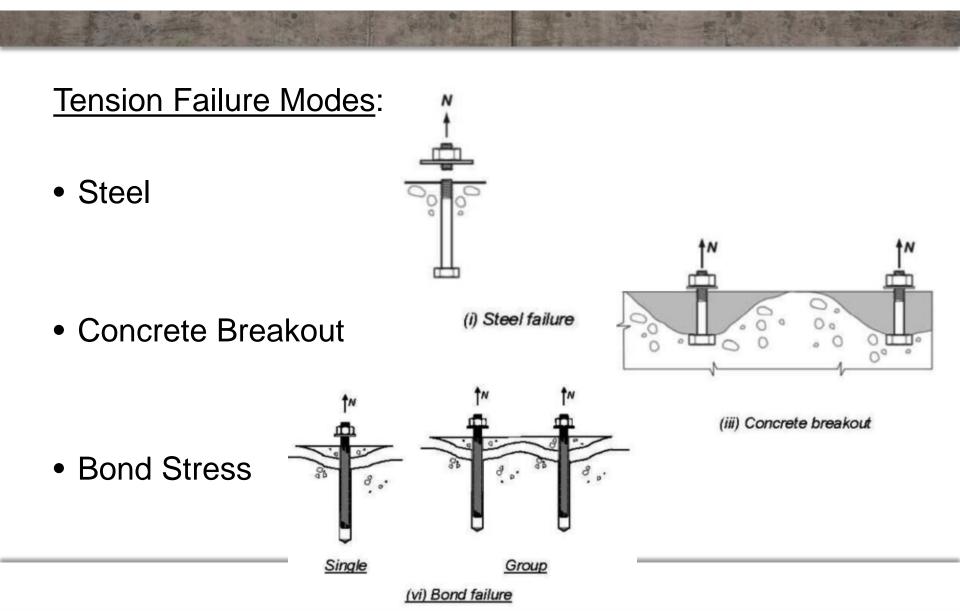
Qualification of Post-Installed Adhesive Anchors in Concrete (ACI 355.4) and Commentary

- Establishes Requirements for Code Recognition
- Addresses Conditions Using "Anchor Theory" Only
- Post-Installed Reinforcing Applying Concepts of Development & Lap Splices is Excluded

Principles of Anchor Theory

- ACI 318 Appendix D uses "Anchor Theory" as a Calculation Model
- Reinforcing Dowels Are Designed As Anchor Bolts
- Principle stresses transferred from the rods to the surrounding concrete
- Three failure modes considered for both tension and shear



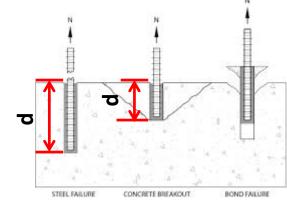


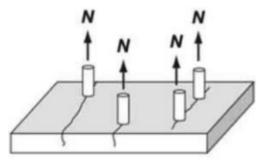
Shear Failure Modes:

 Steel 0 V ν (i) Steel failure preceede by concrete spall Concrete Breakout (iii) Concrete breakout Concrete Pryout 0 0 (ii) Concrete pryout

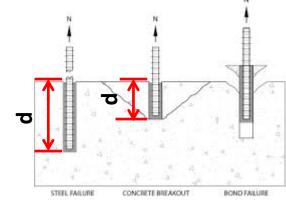
for anchors far from a free edge

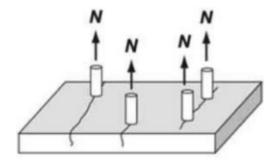
- Embedment Depth: 4d to 20d
 #8 Rebar Max. Embedment Depth = 20"
- Concrete Splitting Failure Precluded By Increasing Edge Distance
- Steel Strength Defined by Ultimate Tensile Strength, futa





- Does Not Explicitly Consider the Influence of Reinforcement in the Member
- "Supplementary Reinforcement"
 Used to Control Splitting or Increase
- "Anchor Reinforcement" Increase Concrete Breakout Capacity





ACI 355.4

R1.2.6 This standard is intended to provide parameters for the design of adhesive anchors in conjunction with the provisions of ACI 318, Appendix D. Those provisions are derived from the principles of anchor theory, whereby anchor forces are transferred to the concrete in a manner that generally precludes splitting of the concrete and where spacing, edge distance, and member thickness are explicitly considered in the evaluation of the concrete breakout capacity (Fig. R1.1(a)). It is not intended to address the assessment or design of post-installed reinforcing bars proportioned according to the concepts of development and splicing of reinforcement (Fig. R1.1(b)). While the provisions of Chapter 12 of ACI 318 may be used to establish embedment lengths for post-installed reinforcing bars in such cases, the ability of an adhesive anchor system to transfer loads to adjacent embedded bars, particularly where longer splice lengths are required, should be verified by appropriate testing. Testing for the splice length is outside the scope of this standard.

Alternative Methods of Design

Alternative Methods of Design

[A] 104.11 Alternative materials, design and methods of construction and equipment. The provisions of this code are not intended to prevent the installation of any material or to prohibit any design or method of construction not specifically prescribed by this code, provided that any such alternative has been approved. An alternative material, design or method of construction shall be *approved* where the *building* official finds that the proposed design is satisfactory and complies with the intent of the provisions of this code, and that the material, method or work offered is, for the purpose intended, not less than the equivalent of that prescribed in this code in quality, strength, effectiveness, fire resistance, durability and safety. Where the alternative material, design or method of construction is not approved, the building official shall respond in writing, stating the reasons why the alternative was not approved.

[A] 104.11.1 Research reports. Supporting data, where necessary to assist in the approval of materials or assemblies not specifically provided for in this code, shall consist of valid research reports from *approved* sources.



ACCEPTANCE CRITERIA FOR POST-INSTALLED ADHESIVE ANCHORS IN CONCRETE ELEMENTS

AC308

Approved May 2014

Compliance date January 15, 2015

- Addresses Post-Installed Reinforcing Using Development & Lap Splices
- New Design Tool
- Significant Benefits To This Design Approach

AC308 Test Program

Total of 17 Tests:

- 4 Bond strength
- 1 Bond/Splitting
- 6 Reliability
- 2 Installation Procedure
- 3 Durability
- 1 Seismic

		Testing	Bar size	Assessment			Bar embed	Minimum	
Test no.			Test parameters	US/M ^{≜1}	ang	Load & displ.	f."	ment lb	sample size N _{min}
		April 1995 Automation	Service condition tests		Aur I	-	-		
la	9.4.3.1	Bond resistance	Tension, confined, single reinforcing bar [†]	#4/12	5	10.25.2 10.25.3	low	7 <i>d</i> ,	Five
16	9.4.3.1	Bond resistance	Tension, confined, single reinforcing bar [†]	#8/25	4	10.25.2 10.25.3	low	7d,	Five
lc	9.4.3.1	Bond resistance	Tension, confined, single reinforcing bar ¹	d _{k max}	1	10.25.2 10.25.3	low	7d,	Five
1d	9.4.3.1	Bond resistance ⁸⁵	Tension, confined, single reinforcing bar ¹	d _{b,max}	9	10.25.2 10.25.3	high	7d,	Five
2	9.4.3.2	Bond/splitting behavior	Tension, confined, reinforcing bars in corner condition	#8/25		10.25.6	low	35d _b	Six
			Reliability tests						
3	9.4.4.1	Sensitivity to hole cleaning, dry substrate	Tension, confined, single reinforcing bar [†]	d _{b,max}	≥ 0. 8	10.25.7	low	7d,	Five
4	9.4.4.2	Sensitivity to installation in saturated concrete	Tension, confined, single reinforcing bar [†]	dima	≥ 0. 8	10.25.7	low	7d,	Five
5	9.4.4.3	Sensitivity to freezing/thawing conditions	Tension, confined, single reinforcing bar [†]	#4/12	≥0.9	10.25.7	high	7d,	Five
6	9.4.4.4	Sensitivity to sustained load at elevated temperature*	Tension, confined, single reinforcing bar [*]	#4/12	≥0.9	10.25.7	low	7d,	Five
7	9.4.4.5	Decreased installation temperature*	Tension, confined, single reinforcing bar	#4/12	≥0.9	10.25.7	low	7 <i>d</i> _h	Five
8	9.4.4.6	Sensitivity to installation direction*	Tension, confined, single reinforcing bar [®]	dime	≥0.9	10.25.7	low	7d,	Five
-			Installation procedure verific	ation					
9	9.4.5.1	Installation at deep embedment	Bar installation in injected hole, horizontal	d _{himut}	Ū.	10.25.8	7	60 <i>d</i> _b	Three
10	9.4.5.2	Injection verification	Injection in clear tube	dimer	-	10.25.8	-	60d _b	Three
			Durability	~	÷.,				
lla	9.4.6.1.1	Resistance to alkalinity*	Slice test	#4/12		10.25.10	low	~	Ten
116	9.4.6.1.2	Resistance to sulfur*	Slice test	#4/12	100	10.25.10	low	\rightarrow	Ten
12	9.4.7	Corrosion resistance	Current and potential test	#4/12	1	10.25.9	low	$2^3/_{4^*}$	Three
			Special conditions					-	
13	9.4.8	Seismic qualification for reinforcing bar connections**	Cyclic tension, confined, single reinforcing bar	d _{a mar}	\mathcal{L}	10.25.11	low	7d,	Five

Qualified Adhesive Anchor Systems

Evaluation Report

Qualified Adhesive Anchoring System:

2.0 USES

The Hilti HIT-RE 500-SD Adhesive Anchoring System and Post-Installed Reinforcing Bar System are used to resist static, wind and earthquake (Seismic Design Categories A through F) tension and shear loads in cracked and uncracked normal-weight concrete having a specified compressive strength, f'c, of 2,500 psi to 8,500 psi (17.2 MPa to 58.6 MPa).

The anchor system complies with anchors as described in Section 1909 of the 2012 IBC and is an alternative to cast-in-place and post-installed anchors described in Section 1908 of the 2012 IBC, and Sections 1911 and 1912 of the 2009 and 2006 IBC. The anchor systems may also be used where an engineered design is submitted in accordance with Section R301.1.3 of the IRC.

The post-installed reinforcing bar system is an alternative to cast-in-place reinforcing bars governed by ACI 318 and IBC Chapter 19.

Evaluation Report

Qualified Adhesive Anchoring System:

3.2.5 Steel Reinforcing Bars for Use in Post-Installed Reinforcing Bar Connections: Steel reinforcing bars used in post-installed reinforcing bar connections are deformed bars (rebar). Tables 35, 36, 37, and Figure 8 summarize reinforcing bar size ranges. The embedded portions of reinforcing bars must be straight, and free of mill scale, rust and other coatings that may impair the bond with the adhesive. Reinforcing bars must not be bent after installation, except as set forth in Section 7.3.2 of ACI 318 with the additional condition that the bars must be bent cold, and heating of reinforcing bars to facilitate field bending is not permitted.

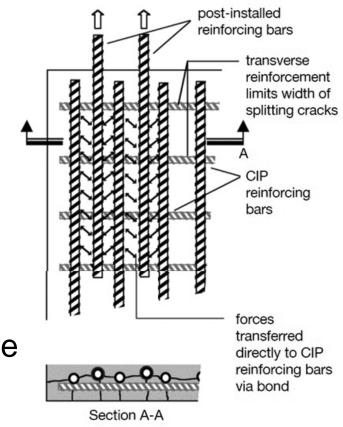
Evaluation Report

	Symbol	Criteria Section of Reference Standard		Bar size							
DESIGN INFORMATION			Units	#3	#4	#5	#6	#7	#8	#9	#10
Nominal reinforcing	db	ASTM A615/A706	in.	0.375	0.500	0.625	0.750	0.875	1.000	1.125	1.250
bar diameter	uь		(mm)	(9.5)	(12.7)	(15.9)	(19.1)	(22.2)	(25.4)	(28.6)	(31.8)
Nominal bar area	A _b	ASTM A615/A706	in ² (mm ²)	0.11 (71.3)	0.20 (126.7)	0.31 (197.9)	0.44 (285.0)	0.60 (387.9)	0.79 (506.7)	1.00 (644.7)	1.27 (817.3)
Development length for $f_y = 60$ ksi and f_c	Id	ACI 318 12.2.3	in.	12.0	14.4	18.0	21.6	31.5	36.0	40.5	45.0
= 2,500 psi (normal weight concrete) ⁴	, and the second s		(mm)	(304.8)	(365.8)	(457.2)	(548.6)	(800.1)	(914.4)	(1028.7)	(1143)
Development length for $f_y = 60$ ksi and f_c	la	ACI 318 12.2.3	in.	12.0	12.0	14.2	17.1	24.9	28.5	32.0	35.6
= 4,000 psi (normal weight concrete) ⁴	·a		(mm)	(304.8)	(304.8)	(361.4)	(433.7)	(632.5)	(722.9)	(812.8)	(904.2)

Development & Lap Splices for Post-Installed Reinforcing

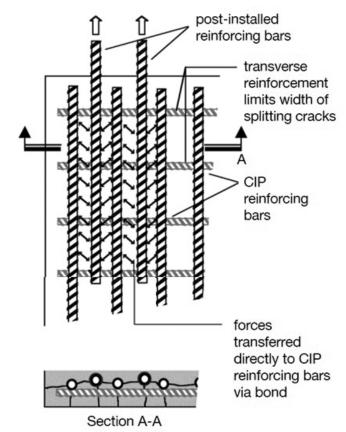
Concept of Development & Lap Splices

- Max. Embedment Depth: 60d #8 Rebar = 60"
- Assumes:
 - 1. Reinforcing Bar Reach Minimum Yield Strength
 - 2. Reinforcing Bar is Controlled by Yield Strength
 - 3. Embedment is Sufficient to Preclude Splitting



Concept of Development & Lap Splices

- Post-Installed Reinforcing is Limited to Straight Reinforcing Bars
- ACI 318 **Chapter 12** is Used to Determine Development Length
- ACI 318 **Chapter 7** is Used to Determine Spacing and Cover Requirements
- ACI 318 Chapter 21 is Used for Earthquake-Resistant Structures Steel Steel Detailing



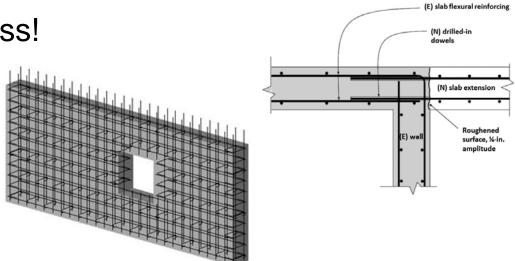
Powerful Design Tool

- Bypasses "Anchor Theory" Design Considerations:
 - 1. Concrete breakout
 - 2. Splitting reduction factor
 - 3. Ultimate tensile strength
 - 4. Embedment Depth Limit = 20d
- "Post-Installed Reinforcement" Concept:
 - 1. Develops bar sufficiently to preclude splitting
 - 2. Develops bar sufficiently to yield reinforcing
 - 3. Embedment Depth Limit = 60d

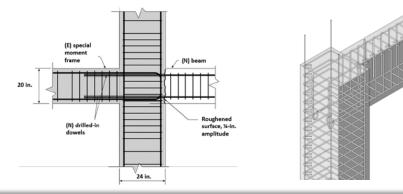


The Possibilities Are Endless!

- Slab Extension
- Stacked Wall Panels



- Moment Frame Extension
- Column Extensions









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