## Laboratory Manual

## Process Equipment Design-II

## List of Practical

| Expt No. | Name of Practical |
| :--- | :--- |
| $1-2$ | Drawing of sketches for various parts of equipments as per the list provided with lab <br> manual |
| 3 | P and ID and PFD |
| $4 \& 5$ | Design calculations for pressure vessel design [ Pressure vessel and Bracket support] |
| 6 | Drawing of pressure vessel in sheet/using AUTOCAD. |
| $7 \& 8$ | Design calculations for storage vessel design [ Plate thickness and Roof] |
| 9 | Drawing of storage vessel in sheet/using AUTOCAD. |
| $10 \& 11$ | Design calculations for tall vertical vessel design [ Plate thickness and Skirt support] |
| 12 | Drawing of tall vertical vessel in sheet/using AUTOCAD. |

## Drawing of various sketches:

Draw sketches and prepare tables as per the given list from book of Joshi and Mahajani, $3^{\text {rd }}$ Edition.

| Figure No. | Title of Figure |
| :--- | :--- |
|  | Basics |
| Fig. 4.22-24 | Group I |
| Table 4.6 | Group I |
|  | Chapter 5 |
| Table 5.1, 5.2 | Group II |
| Fig. 5.3-31 | Group II |
|  | Chapter 6 |
| Fig. 6.1-5 | Group III |
| Fig. 6.11-13 | Group III |
|  | Chapter 7 |
| Fig. 7.1-16 | Group III |
|  | Chapter 8 |
| Fig. 8.1-15 |  |
|  | Chapter 11 |
| Fig. 11.1 | Group IV |
| Fig. 11.9-10 | Group IV |
| Fig. 11.14-17 | Group IV |
| Fig. 11.19-24 | Group IV |
|  | Chapter 13 |
| Fig. 13.1-18 | Group V |
|  | Chapter 14 |
| Fig. 14.1-9, 14.11 | Group I |
| Table 14.1 | Group I |
| Appndix G | Codes and Standards - Group I |

## Sheet-1 Pressure Vessel Design

Design the pressure vessel with an appropriate support on the basis of the following data.

## Data:

## Shell:

Internal diameter (approx)
Internal pressure
Permissible stress at $150{ }^{\circ} \mathrm{C}$
Material - Stainless steel

## Flanges

Permissible stress (up to $250^{\circ} \mathrm{C}$ )
Gasket
Material - Carbon steel (IS-2002)
Bolts
Permissible stress (up to $50^{\circ} \mathrm{C}$ )
Permissible stress (up to $250^{\circ} \mathrm{C}$ )
Material
Nozzle - Welded to head
Internal diameter 150 mm
Thickness
Material

Use data given in table below
$130 \mathrm{~N} / \mathrm{mm}^{2}$
$0.5 \mathrm{Cr}, 18 \mathrm{Ni}, 11 \mathrm{Mo}$
$95 \mathrm{~N} / \mathrm{mm}^{2}$
Asbestos
Grade
$58.7 \mathrm{~N} / \mathrm{mm}^{2}$
$54.5 \mathrm{~N} / \mathrm{mm}^{2}$
Hot rolled carbon steel

3 mm
Same as shell

## Head

## (a) Torishperical Head (Flanged and Standard dished)

Crown radius
Knuckle radius
Total depth of head
$\mathrm{S}_{\mathrm{f}}$
Determine the thickness and blank diameter of the plate required to fabricate the head
(b) Flanged and dished head

External diameter 1200 mm
Crown radius
Knuckle radius
M.O.C.
(c) Elliptical Head

Ratio of major to minor axis
1200 mm
72 mm
Same as shell
(d) Hemispherical Head

Poisson's ratio ( $\mu$ )
Modulus of elasticity (E)
(e) Butt-welded flat Head

Stress concentration factor(C)
0.45

## Support

## Bracket or Lug Support

| Diameter of vessel | 1200 mm |
| :--- | :--- |
| Height of vessel | 2000 mm |
| Clearance of vessel bottom of foundation | 1 m |
| Weight of vessel with contents | 40 kN |
| Wind pressure | $1285 \mathrm{~N} / \mathrm{m}^{2}$ |
| Number of brackets | 4 |
| Dia. of anchor bolt circle | 1.65 m |
| Height of bracket from foundation | 22.5 m |
| Permissible stresses for structural steel |  |
| $\quad 140 \mathrm{~N} / \mathrm{mm}^{2}$ |  |
| $\quad$ Tension | $123.3 \mathrm{~N} / \mathrm{mm}^{2}$ |
| $\quad$ Compression | $157.5 \mathrm{~N} / \mathrm{mm}^{2}$ |
| $\quad$ Bending | $3.5 \mathrm{~N} / \mathrm{mm}^{2}$ |

Design Data:

| Roll No. <br> (As per sequence in <br> muster in each <br> batch) | Internal diameter <br> (mm) | Internal Max. <br> Operating Pressure, <br> Absolute, | Jacket Pressure, <br> (gauge), kg/cm ${ }^{2}$ |
| :---: | :--- | :--- | :--- |
| 1 | 1200 | 4 bar |  |
| 2 |  | 11 bar | 10 |
| 3 |  | 16 bar | 20 |
| 4 | 21 bar | 5 |  |
| 5 |  | 36 bar | 10 |
| 6 | 800 | 21 bar | No jacket |
| 7 |  | 15 bar | 20 |
| 8 |  | 9 bar | 2 |
| 9 |  | 5 bar | 10 |
| 10 |  | 10 bar | No jacket |
| 11 |  | 27 bar | 25 |
| 12 |  | 100 mm hg | No jacket |
| 13 |  | 300 mm hg | 5 |
| 14 |  | 27 bar | 10 |
| 15 |  | 0 mm hg | 5 |
| 16 |  | 5 bar | 5 |
| 17 |  | 15 bar | No jacket |
| 18 |  | 25 bar | 15 |
|  |  |  | No jacket |

Draw similar figures with proper scale according to your design calculations in A1 drawing sheets.
Fig. Nos.: Use the drawing sheet for "Reaction Vessel" available at the end of book of Joshi and Mahajani. (3rd Edition, M V Joshi and V V Mahajani)

Process Equipment Design-II, Lab Manual, Chemical Engineering Department, IT, NU


## Data Sheet for Pressure Vessel Design

| Customer |  | Order No. |  |
| :---: | :---: | :---: | :---: |
| Vessel name |  | . . Equipment No |  |
| Description |  |  |  |
| Drawing/sketch No |  |  |  |
| Design Code |  |  |  |
| Design pressure |  | . $\mathrm{kN} / \mathrm{m}^{2}$ Design temperature | ${ }^{\circ} \mathrm{C}$. |
| Design liquid level |  |  |  |
| Contents |  | . . . . Density | $\mathrm{kg} / \mathrm{m}^{3}$ |
| Service connections |  |  |  |
| Hydraulic test pressure |  | . $\mathrm{kN} / \mathrm{m}^{2}$ |  |
| Vessel classification |  | . . . . . . . . . . |  |
| Joint efficiencies: | Shell . | . Heads. |  |
| Materials of construction: | Shell . |  |  |
|  | Heads |  |  |
|  | Nozzles |  |  |
|  | Flanges |  |  |
| Corrosion alowances: | shell | . . . . . . mm Heads | . mm |
|  | Nozzles | . . . . . . . mm |  |

Notes/comments

Prepared by
Checked by
Date
Date

## DESIGN OF PRESSURE VESSEL

## Design of Shell:

- Thickness of shell
$\mathrm{t}_{\mathrm{h}}=(\mathrm{P} * \mathrm{Di}) /(2 * \mathrm{f} * \mathrm{~J}-\mathrm{P})$
Where, $\mathrm{P}=$ internal design pressure
$\mathrm{Di}=$ internal diameter
f = permisible stress
$\mathrm{J}=\mathrm{Joint}$ efficiency
- Check for thickness under combined loading

1) Stress in circumferential direction. Also called hoop stress,

$$
\mathrm{f}_{\mathrm{t}}=\left(\mathrm{P}^{*}(\mathrm{Di}+\mathrm{t})\right) /(2 * \mathrm{t}) \quad[\text { TENSILE }]
$$

2) Stress in the longitudinal or axial direction,

- due to internal pressure

$$
\begin{equation*}
\mathrm{f}_{1}=(\mathrm{P} * \mathrm{Di}) /\left(4^{*} \mathrm{t}\right) \tag{TENSILE}
\end{equation*}
$$

- due to weight of vessel and contents

$$
\mathrm{f}_{2}=\mathrm{W} /\left(\Pi^{*} \mathrm{t} *(\mathrm{Di}+\mathrm{t})\right) \quad[\text { COMPRESSIVE }]
$$

- Due to wind or piping in the case of vertical vessels or due to weight of vessel in case of horizontal vessel
$\mathrm{f}_{3}=\mathrm{M} /\left(\Pi^{*} \mathrm{Di}^{2}{ }^{2} \mathrm{t}\right)$
Where, $\mathrm{M}=$ Bending moment due to wind load

$$
=\mathrm{p}_{\mathrm{lw}} *(\mathrm{H} / 2) \quad(\text { If } \mathrm{H}<20 \mathrm{~m})
$$

where, $\mathrm{p}_{\mathrm{lw}}=\mathrm{k} * \mathrm{P} 1 * \mathrm{~h} 1 *$ Do
$\mathrm{H}=$ height of the vessel
$\mathrm{h} 1=$ height of vessel up to 20 m
Do = OD of the vessel
$\mathrm{k}=$ coefficient depending upon shape factor $=0.7$ (for cylindrical )
$\mathrm{P} 1=0.05 \mathrm{~V}_{\mathrm{w}}{ }^{2}=$ wind pressure
$\mathrm{V}_{\mathrm{w}}=$ velocity of wind
Total stress in the longitudinal or axial direction

$$
\mathrm{f}_{\mathrm{a}}=\mathrm{f}_{1}+\mathrm{f}_{2}+\mathrm{f}_{3}
$$

3) Stress due to piping or wind

$$
\mathrm{f}_{\mathrm{s}}=(2 * \mathrm{~T}) /\left(\Pi * \mathrm{t}^{*} \mathrm{Di}^{*}(\mathrm{Di}+\mathrm{t})\right)
$$

where , $\mathrm{T}=$ torque about the vessel axis
Combining the above stresses on the basis of shear strain energy theory criterion, the equivalent stress is

$$
\mathrm{f}_{\mathrm{R}}=\left[\left(\mathrm{f}_{\mathrm{t}}^{2}-\mathrm{f}_{\mathrm{t}}^{*} * \mathrm{f}_{\mathrm{a}}+\mathrm{f}_{\mathrm{a}}^{2}+3 \mathrm{f}_{\mathrm{s}}^{2}\right)\right]^{1 / 2}
$$

For satisfactory design
$\mathrm{f}_{\mathrm{R}}$ (tensile) $<=\mathrm{f}_{\mathrm{t}}$ (permissible),

## Design of Head

## 1. TORISPHERICAL HEAD

- Thickness subjected to internal pressure

$$
\mathrm{t}_{\mathrm{h}}=\left(\mathrm{P} * \mathrm{R}_{\mathrm{c}} * \mathrm{~W}\right) /(2 * \mathrm{f} * \mathrm{~J})
$$

where, $\mathrm{P}=$ internal design pressure
$\mathrm{R}_{\mathrm{c}}=$ crown radius
$\mathrm{W}=$ stress intensification factor
$=1 / 4\left[3+\sqrt{ }\left(\mathrm{Rc} / \mathrm{r}_{\mathrm{i}}\right)\right]$
$\mathrm{r}_{\mathrm{i}}=$ knuckle radius (internal)

- Thickness subjected to external pressure (torispherical, elliptical, hemispherical head)
$\mathrm{t}_{\mathrm{h}}=4.4 * \mathrm{Rc} * \sqrt{ } 3\left(1-\mu^{2}\right) * \sqrt{ }(\mathrm{P} / 2 \mathrm{E})$
where, $\mathrm{E}=$ modulus of elasticity
$\mu=$ Poissons ratio
$\mathrm{P}_{\mathrm{o}}=$ external pressure( internal pr. $=1.67 *$ external pr.)
- For Torispherical (standard dished ) and ellipsoidal dished head

The external height, $h_{o}$ of a dished head ( excluding straight flange),
$\mathrm{h}_{\mathrm{o}}=\mathrm{R}_{\mathrm{co}}-\left\{\left(\mathrm{R}_{\mathrm{co}}-\mathrm{D}_{\mathrm{o}} / 2\right) *\left(\mathrm{R}_{\mathrm{co}}+\mathrm{D}_{\mathrm{o}} / 2-2 \mathrm{r}_{\mathrm{o}}\right)\right\}^{1 / 2}$
where, $\mathrm{R}_{\mathrm{co}}=$ outside crown radius
$r_{0}=$ outside knucke radius
Blank Diameter of head,
$=\mathrm{D}_{\mathrm{o}}+\mathrm{D}_{\mathrm{o}} / 42+2 * \mathrm{~S}_{\mathrm{f}}+2 / 3 * \mathrm{r}_{\mathrm{i}} \quad\left\{\right.$ where $\left.\mathrm{t}_{\mathrm{h}}<25.4 \mathrm{~mm}\right)$
OR $\quad=D_{o}+D_{o} / 24+2 * S_{f}+2 / 3 * r_{i} \quad\left\{\right.$ where $\left.t_{h}>25.4 \mathrm{~mm}\right\}$
where $\mathrm{S}_{\mathrm{f}}=$ height of straight flange

## 2. FLANGED AND SHALLOW DISHED HEAD

Thickness subjected to internal pressure
$\mathrm{t}_{\mathrm{h}}=\left(\mathrm{P} * \mathrm{Rc}^{*} \mathrm{~W}\right) /\left(2 * \mathrm{f}^{*} \mathrm{~J}\right)$

## 3. ELLIPTICAL HEAD

Thickness subjected to internal pressure
$\mathrm{t}_{\mathrm{h}}=(\mathrm{P} * \mathrm{D} * \mathrm{~W}) /\left(2 * \mathrm{f}^{*} \mathrm{~J}\right)$
where, $\mathrm{D}=$ major axis of ellipse
$\mathrm{k}=$ Major axis/minor axis [common value is 2,should not greater then 2.6]
$\mu=$ Poissons ratio
$\mathrm{W}=$ stress concentration factor $=\left(2+\mathrm{k}^{2}\right) / 6$

## 4. HEMISPHERICAL HEAD

Thickness subjected to internal pressure

$$
t_{\mathrm{h}}=(\mathrm{PD}) /(4 * \mathrm{f} * \mathrm{~J})
$$

## 5. BUTTWELDED FLAT HEAD

Thickness of head

$$
t_{h}=C * D * \sqrt{P / f}
$$

where, $\mathrm{C}=$ stress concentration factor
$\mathrm{D}=$ diameter of plate which is actually under operating pressure

## 6. CONICAL HEAD

Thickness of head

$$
\mathrm{t}_{\mathrm{h}}=\mathrm{PD} /(2 \mathrm{fJ} \cos \propto)
$$

The circumferential stress in this type of formed head, f

$$
=\mathrm{PD} /(2 \operatorname{tcos} \propto)
$$

## DESIGN OF FLANGE

Flange - the shell and top head are connected by flange joint

## 1. Gasket Design \& Selection

- $\mathrm{d}_{\mathrm{o}} / \mathrm{d}_{\mathrm{i}}=\sqrt{\left(\mathrm{y}-\mathrm{P}^{*} \mathrm{~m}\right) /\left(\mathrm{y}-\mathrm{P}^{*}(\mathrm{~m}+1)\right)}=\mathrm{X}$

Where,
$y=$ gasket seating stress
$\mathrm{m}=$ Gasket factor
$\mathrm{P}=$ internal design pressure
$\mathrm{d}_{\mathrm{i}}=\mathrm{ID}$ of gasket, $\mathrm{d}_{\mathrm{o}}=\min \mathrm{OD}$ of gasket $=\mathrm{X} \mathrm{d}_{\mathrm{i}}$
$\mathrm{d}_{\mathrm{i}}>=10 \mathrm{~mm}$ larger than $B$ (ID of flange)
[for ring and slip flange, ID of flange $=\mathrm{OD}$ of shell]
$\mathrm{d}_{\mathrm{i}}=\mathrm{Do}+5$ to 20
[For weld flange, ID of flange $=\mathrm{ID}$ of shell]

- Width of gasket

Actual gasket width in contact

$$
\mathrm{N}=\left(\mathrm{d}_{\mathrm{o}}-\mathrm{d}_{\mathrm{i}}\right) / 2
$$

So,
$\mathrm{d}_{\mathrm{o}}=\mathrm{d}_{\mathrm{i}}+(2 * \mathrm{~N})$

Basic gasket seating width $\left[b_{o}\right]$

| Type of flange facing | Basic gasket seating width, $\mathrm{b}_{\mathrm{o}}$ | Effective gasket seating <br> width, b |
| :--- | :--- | :--- |
| Plain face | $\mathrm{N} / 2$ | $\mathrm{B}=\mathrm{b}_{\mathrm{o}}$, when $\mathrm{b}_{\mathrm{o}}<=6.3$ <br> mm |
| Raised face | $\mathrm{N} / 2$ | $\mathrm{B}=2.5\left\lceil\mathrm{~b}_{\mathrm{o}}\right.$, when <br> $\mathrm{b}_{\mathrm{o}}>6.3 \mathrm{~mm}$ |
| Male and female | $\mathrm{N} / 2$ |  |
| Tongue \& groove | $(\mathrm{N}+\mathrm{W}) / 4, \mathrm{~W}-$ width of tongue |  |
| Ring type | $\mathrm{W} / 8, \mathrm{~W}-$ width of ring gasket |  |

- Diameter of gasket at location of gasket load reaction [G]

$$
\begin{array}{ll}
\mathrm{G}=\left(\mathrm{d}_{\mathrm{o}}+\mathrm{d}_{\mathrm{i}}\right) / 2 & \text { when } \mathrm{b}<=6.3 \mathrm{~mm} \\
\mathrm{G}=\mathrm{d}_{\mathrm{o}}-2 \mathrm{~b} & \text { when } \mathrm{b}>6.3 \mathrm{~mm}
\end{array}
$$

## 2. Bolt design

- Determination of bolt load under bolting up condition,

$$
\mathrm{W}_{\mathrm{m} 2}=\Pi^{*} \mathrm{~b}^{*} \mathrm{G}^{*} \mathrm{y}
$$

- Determination of bolt load under internal pressure

$$
\mathrm{W}_{\mathrm{m} 1}=\mathrm{H}+\mathrm{H}_{\mathrm{p}}
$$

Where, $\mathrm{H}=$ load due to design pressure P , acting on an area $\pi \mathrm{G}^{2} * \mathrm{P}$

$$
=\pi / 4^{*} \mathrm{G}^{2} * \mathrm{P}
$$

$\mathrm{H}_{\mathrm{p}}=$ load to achieve adequate compression of the gasket under operating condition $=\pi^{*}(2 \mathrm{~b}) * \mathrm{~m}^{*} \mathrm{G} * \mathrm{P}$

- Determination of minimum bolt area theoretically required, $\mathrm{A}_{\mathrm{m}}$

The bolt loads either $\mathrm{W}_{\mathrm{m} 1}$ OR $\mathrm{W}_{\mathrm{m} 2}$ will create a tensile stress in the cross section of the bolt.

$$
\begin{aligned}
& \mathrm{A}_{\mathrm{m} 1}=\mathrm{W}_{\mathrm{m} 1} / \mathrm{f}_{\mathrm{a}} \\
& \mathrm{~A}_{\mathrm{m} 2}=\mathrm{W}_{\mathrm{m} 2} / \mathrm{f}_{\mathrm{b}}
\end{aligned}
$$

Where,
$\mathrm{A}_{\mathrm{m} 1}, \mathrm{~A}_{\mathrm{m} 2}=$ Cross section of the bolt under operating and bolting-up conditions respectively
$f_{a}, f_{b}=$ Permissible stress for bolting material under design \& atmospheric temp

- Number of Bolts, $n=\left[\mathrm{A}_{\mathrm{m} 1}\right.$ or $\mathrm{A}_{\mathrm{m} 2}$ (greater of two) $]$ / Root area of bolt( if table is given), otherwise
$\mathrm{n}=\mathrm{G} /\left(\mathrm{b}_{\mathrm{o}} * 2.5\right), \mathrm{n}$ should be in multiple of 4.
Depending upon the value of $n$, choose the bolt size
- Diameter of bolt $=\left\{\text { greater value of }\left(\mathrm{A}_{\mathrm{m} 1} \text { orA } \mathrm{A}_{\mathrm{m} 2}\right) * 4 /(\mathrm{n} * \pi)\right\}^{1 / 2}$
- If table is provided, then from the value of $n$, find bolt spacing $\left(B_{s}\right)$ and bolt-circlediameter(C) and root area. Bolt circle diameter can be calculated by two ways, and the larger value of $B$ should be considered.

$$
\begin{aligned}
& \mathrm{B}=\mathrm{n} \mathrm{~B}_{\mathrm{s}} / \pi \quad \text { or } \\
& \mathrm{B}=\mathrm{d}_{\mathrm{o}}+2 * \text { Dia. of bolt }+12 \mathrm{~mm}
\end{aligned}
$$

Calculation of flange outside diameter $\mathrm{A}=\mathrm{B}+$ bolt diameter

- Determination of actual bolt area, $\mathrm{A}_{\mathrm{b}}$

$$
\mathrm{A}_{\mathrm{b}}=\mathrm{n} * \text { Root area of bolt }
$$

- To prevent damage to the gasket during bolting up condition, following condition should be satisfied

$$
\mathrm{A}_{\mathrm{b}} * \mathrm{f}_{\mathrm{b}} /(\pi \mathrm{GN})<2 * \mathrm{y}
$$

## 3. Flange Thickness

```
\(\mathrm{t}_{\mathrm{f}}=\mathrm{G}^{*} \sqrt{ }(\mathrm{P} /(\mathrm{K} * \mathrm{f}))+\mathrm{C}\)
where, \(\mathrm{K}=1 /\left\{0.3+\left(1.5^{*} \mathrm{Wm}^{*} \mathrm{~h}_{\mathrm{g}}\right) /\left(\mathrm{H}^{*} \mathrm{G}\right)\right\}\)
\(\mathrm{G}=\) Diameter of gasket load reaction
\(\mathrm{P}=\) design pressure
\(\mathrm{f}=\) permissible stress
\(\mathrm{B}=\) bolt circle diameter
\(\mathrm{C}=\) corrosion allowance
\(\mathrm{Wm}=\) total bolt load (greater of Wm1\& Wm2)
\(\mathrm{h}_{\mathrm{G}}=\) radial distance from gasket load reaction to bolt circle
    \(=(\mathrm{B}-\mathrm{G}) / 2\)
\(\mathrm{H}=\) Total hydrostatic end force \(=\pi / 4 * \mathrm{G}^{2} * \mathrm{P}\)
```


## Nozzle Reinforcement Design

## - Minimum Nozzle thickness

$$
\mathrm{t}_{\mathrm{n}}=(\mathrm{P} * \mathrm{Di}) /(2 * \mathrm{f} * \mathrm{~J}-\mathrm{P})
$$

Actual thickness of the nozzle is to be used in further calculation.

## - Condition for Reinforcement

If the size of nozzle (Diameter of Nozzle) $<5 \mathrm{~cm}$, the reinforcement is not required, For diameter $>5 \mathrm{~cm}$, reinforcement is required.

## - Reinforcement for nozzle

## Area to area method of compensation

The maximum horizontal distance for compensation $A B=2 * d$
The maximum vertical distance for compensation $\mathrm{AD}=6 \mathrm{t}_{\mathrm{s}} \mathrm{OR}\left(3.5 \mathrm{t}_{\mathrm{s}}+2.5 \mathrm{t}_{\mathrm{n}}\right)$
Whichever is smaller.
Where $t_{s}$ greater value ( shell thickness or head thickness)
If the compensation is only provided by nozzle then

$$
\mathrm{H}_{1}=\mathrm{H}_{2}=2.5 \mathrm{t}_{\mathrm{s}}
$$

If the compensation is to be provided by a combination of nozzle and a compensation ring, then

$$
\mathrm{H}_{1}=2.5 \mathrm{t}_{\mathrm{n}}
$$

The area for which compensation is required is given by

$$
\mathrm{A}=\mathrm{d}^{*} \mathrm{t}_{\mathrm{s}}
$$

## Area available for compensation

a) The portion of the shell or head as excess thickness

$$
\mathrm{A}_{\mathrm{s}}=\mathrm{d}^{*}\left(\mathrm{t}_{\mathrm{s}}-\mathrm{t}_{\mathrm{s}}{ }^{\prime}-\mathrm{C}\right)
$$

b) The portion of the nozzle external to the vessel

$$
\mathrm{A}_{\mathrm{o}}=2 \mathrm{H}_{1}\left(\mathrm{t}_{\mathrm{n}}-\mathrm{t}_{\mathrm{s}}{ }^{\prime}-\mathrm{C}\right)
$$

c) The portion of the nozzle inside the vessel, if nozzle does not project inside the vessel, $\mathrm{H}_{2}=0$

$$
\mathrm{A}_{1}=2 \mathrm{H}_{2}\left(\mathrm{t}_{\mathrm{n}}-2 \mathrm{C}\right)
$$

Now calculate, $\mathrm{As}+\mathrm{Ao}+\mathrm{A}_{1}$

So area of compensation required is equal to,

$$
\mathrm{A}=\left(\mathrm{As}+\mathrm{Ao}+\mathrm{A}_{1}\right)
$$

Where,
$\mathrm{d}=$ inner diameter of nozzle
$\mathrm{t}_{\mathrm{s}}=$ actual thickness of shell or head
$\mathrm{t}_{\mathrm{s}}$ ' $=$ theoretical minimum thickness of shell or head
$\mathrm{t}_{\mathrm{n}}=$ actual thickness of nozzle
$\mathrm{t}_{\mathrm{n}}$ ' $=$ theoretical minimum thickness of nozzle
$\mathrm{C}=$ Corrosion allowance

## Design of Support

## Bracket or Lug support

For vessels of diameter Do Brackets used are

If $\mathrm{Do}>0.6 \mathrm{~m}$
$0.6<\mathrm{Do}<=3 \mathrm{~m}$
$3<\mathrm{Do}<=5 \mathrm{~m}$
Do $>5 \mathrm{~m}$

2 Brackets
4 Brackets
6 Brackets
8 Brackets

Maximum compressive load act on the bracket support
$\mathrm{P}=\{4 * \mathrm{pw}[\mathrm{H}-\mathrm{F}]\} / \mathrm{n} * \mathrm{Db}+\sum \mathrm{W} / \mathrm{n}$
Where,
$\mathrm{H}=$ height of vessel above foundation
$\mathrm{pw}=$ total force due to wind load acting on vessel
$=\mathrm{k}^{*} \mathrm{p}_{1} * \mathrm{~h} 1 *$ Do

## Sheet-2 Design of Storage Tank

Design a storage tank having volume of the tank equivalent to last three digit of roll number multiply with 100 , i.e. Roll No. 08BCH001 is having volume of 100 m 3 , and Roll No. $08 B C H 156$ is having total volume of $15600 \mathrm{m3}$. Choose proper roof and design it.

## Data:

## Shell Design :

Plate size used $=2.16 \mathrm{~m}$ width X 7.32 m length
Std. Plate thickness available $=5,6,8,10,12,14,16,18,20,24,26,28$
Density of fluid $=900 \mathrm{~kg} / \mathrm{m}^{3}$
Permissible stress for the plate $=1260 \mathrm{~kg} / \mathrm{cm}^{2}$
Density of plate material $=7700 \mathrm{~kg} / \mathrm{m}^{3}$
Use Butt welded joints.
Joint efficiency $=0.85$

## Bottom Design :

Plate size used $=2.5 \mathrm{~m}$ width X 5 m length
Use bottom plate thickness, for inside of the tank $=6 \mathrm{~mm}$
Use bottom plate thickness, near shell plate and bottom plate joint $=8 \mathrm{~mm}$
Use Lap welded joints.
Joint efficiency $=0.85$

## Roof Design :

Plate size used $=1.37 \mathrm{~m}$ width X (as per your requirement, i.e. spacing between two polygon or polygon and shell) $m$ length
Std. Plate thickness available $=5,6,8$
Permissible stress for the plate $=1260 \mathrm{~kg} / \mathrm{cm}^{2}$
Density of plate material $=7700 \mathrm{~kg} / \mathrm{m}^{3}$
Draw similar figures with proper scale according to your design calculations in A1 drawing sheets.

Fig. Nos.: 3.13, 3.15, 3.21, 4.4, 4.5, 4.12, 4.13 (From book of Brownell and Young)

## Design of Storage Tank

## Shell Design :

Calculation of shell plate thickness,

$$
\text { th }=[(\mathrm{pD}) /(2 \mathrm{fj})]+\mathrm{C}
$$

Where th $=$ Thickness of the shell plate, mm
$\mathrm{p}=$ Hydrostatic Pressure on the plate, $\mathrm{N} / \mathrm{mm}^{2}$
$\mathrm{p}=\rho(\mathrm{H}-0.30) \mathrm{g} \times 10^{-6}$
Where $\rho=$ Density of fluid filled in the tank, $\mathrm{kg} / \mathrm{m}^{3}$
$\mathrm{H}=$ Height of the tank, m
$\mathrm{g}=$ gravitational constant, $\mathrm{m} / \mathrm{sec}^{2}$
$\mathrm{D}=$ Diameter of the tank, mm
$\mathrm{f}=$ Maximum permissible stress for the shell plates, $\mathrm{N} / \mathrm{mm}^{2}$
$j=$ Welding joint efficiency
$\mathrm{C}=$ Corrosion allowance, mm
Wind girder, $\mathrm{Z}=0.059 \mathrm{D}^{2} \mathrm{H}$
Where, $\mathrm{Z}=$ Section Modulus, $\mathrm{cm}^{3}$
$\mathrm{D}=$ Diameter of tank, $\mathrm{m}^{3}$
$\mathrm{H}=$ Height of Tank, $m$
Select the proper section based on the above section modulus from Book by Brownell and Young, Appendix-

## Bottom Design :

Plate size used $=5.0 \mathrm{~m}$ width X 2.5 m length
If diameter of the tank is greater than 12 meter use annular ring plates at bottom of the tank.
Annular ring plate should extend beyond the shell outside diameter by 65 mm on both the sides.
Annular ring plate size used $=5.0 \mathrm{~m}$ width X 2.5 m length
Use Lap welded joints.
Over lap between two bottom plates inside the tank $=5 \mathrm{X}$ thickness of the bottom plate
Over lap between sketch plate and annular plate $=65 \mathrm{~mm}$
Joint efficiency $=0.85$

## Roof Design :

## Roof Curb Angle,

Area of roof curb angle, $\mathrm{Ac}=\mathrm{A}-\mathrm{As}-\mathrm{Ar}$

$$
\text { Where } \begin{aligned}
& \mathrm{Ac}=\text { Area of roof curb angle }, \mathrm{mm}^{2} \\
& \mathrm{As}=\text { Area of shell plates effective }=1.5 \mathrm{ts}(\mathrm{Rts})^{1 / 2} \\
& \mathrm{Ar}=\text { Area of roof plates effective }=0.75 \mathrm{tr}(\mathrm{R} 1 \mathrm{tr})^{1 / 2} \\
& \mathrm{tr}=\text { Thickness of roof plate }, \mathrm{mm} \\
& \mathrm{ts}=\text { Thickness of shell plate, } \mathrm{mm} \\
& \\
& \mathrm{R}=\text { Radius of tank, mm } \\
& \mathrm{R}_{1}=\text { Radius of curvature of roof, } \mathrm{mm}
\end{aligned}
$$

OR

Use std. Minimum roof curb angle data given in the book, i.e for $\mathrm{D}>36$ meter, Size of roof curb angle $=100 \mathrm{~mm} \times 100 \mathrm{~mm} \times 10 \mathrm{~mm}$

## Structured Supported Roof :

Design of steps :

1. Choose the min. thickness of the roof plates.
2. Assume the slope of the roof if it is not provided.
3. Determine the no. of polygons required for construction of roof considering the maximum length of rafter is in the range of 6.0 m to 8.0 m and that of the girder is in the range of 7.1 m to 9.1 m .
4. Determine the no. of girders required per polygon based on the choosen length of the girder from the following eq.

$$
\begin{aligned}
& \mathrm{L}=2 \mathrm{RSin}(360 / 2 \mathrm{~N}) \\
& \text { Where, L = Length of girder, m } \\
& \mathrm{R}=\text { Radius of the tank, } \mathrm{m} \\
& \mathrm{~N}=\text { No. of sides of polygon }
\end{aligned}
$$

Based on this N value find the actual length of girder.
5. Calculate the maximum rafter spacing

$$
1_{\max }=\mathrm{t}(2 \mathrm{f} / \mathrm{P})^{1 / 2}
$$

Where, $1_{\text {max }}=$ Maximum rafter spacing, $m$ $\mathrm{f}=$ Permissible stress, $\mathrm{N} / \mathrm{mm}^{2}$
$\mathrm{P}=$ Total load on the roof, $\mathrm{N} / \mathrm{mm}^{2}$
Maximum rafter spacing on roof curb angle $=1.91 \mathrm{~m}$
6. Minimum no. of rafters required between the outermost polygon and shell,

$$
\mathrm{n}_{\min }=(2 \pi \mathrm{R}) / \mathrm{l}
$$

Where, $\mathrm{R}=$ radius of tank, m
Actual no. of rafters should be the multiple of the no. of the sides of the polygon. Based on the actual no. of rafters recalculate the actual rafter spacing on the girders of the referred polygon,

$$
1=(\mathrm{NL}) / \mathrm{n}
$$

Where, $n=$ Actual no. of rafters on the girders of respective polygon
7. Minimum no. of rafters required between the outermost polygon and the inner polygon,

$$
\mathrm{n}_{\min }=(\mathrm{NL}) / \mathrm{l}_{\max }
$$

Repeat the same procedure to find the actual no. of rafters and rafter spacing.
8. Repeat for the step 7 to find the no. of rafter and rafter spacing on the inner polygon and center column.
9. Selection of rafter is based on eq.

$$
\begin{aligned}
& \begin{array}{l}
\begin{array}{l}
\mathrm{Z}=\mathrm{M}_{\max } / \mathrm{f} \\
\text { Where, } \mathrm{M}_{\max }=
\end{array} \\
\begin{array}{r}
\text { Maximum bending movement based on the total load on } \\
\text { the rafter, N.mm } \\
\mathrm{M}_{\max }=\left(\mathrm{WY}^{2}\right) / 8
\end{array} \\
\text { Where, } \mathrm{W}=\text { Total load on the rafter, } \mathrm{kg} / \mathrm{mm} \\
\mathrm{Y}=\text { Distance between the shell plate and outermost } \\
\text { polygon or distance between the two polygons, } \mathrm{mm} .
\end{array} \\
& \mathrm{f}=\text { Permissible stress, } \mathrm{N} / \mathrm{mm}^{2} \\
& \mathrm{Z}=\text { Section modulus, } \mathrm{mm}^{3}
\end{aligned}
$$

Initially neglect the weight of rafter,

Base on this section modulus find the std. Section available to meet the required value from Appendix G, item 1 of Book by Brownell and Young. In the selection of the rafter initially load due to weight of the rafter is unknown so first calculate Z only based on the roof load and after selecting proper section for rafter repeat the calculation for Z and check. If calculated Z value is small than that of the std. Value for the given section then selected rafter is correct otherwise repeat the calculation.
10. Repeat the calculation for other spacing inside the tank, i.e. between two polygon or between innermost polygon and central column.
11. Selection of girder is based on equation,

$$
\begin{gathered}
\begin{array}{c}
\mathrm{Z}=\mathrm{M}_{\max } / \mathrm{f} \\
\text { Where, } \mathrm{M}_{\max }=
\end{array} \begin{array}{r}
\text { Maximum bending movement based on the total load on } \\
\text { the girder, N.mm }
\end{array} \\
\mathrm{M}_{\max }=\left(\mathrm{WL}^{2}\right) / 8 \\
\text { Where, } \mathrm{W}=\text { Total load on the girder, } \mathrm{kg} / \mathrm{mm} \\
\mathrm{~L}=\text { Length of girder, mm. } \\
\mathrm{f}=\text { Permissible stress, } \mathrm{N} / \mathrm{mm}^{2} \\
\mathrm{Z}
\end{gathered}
$$

Initially neglect the weight of girder,
Total Girder load $=($ Total load on one rafter $) \times($ Total no. of rafters per one girder $), \mathrm{N} . \mathrm{mm}$
Base on this section modulus find the std. Section available to meet the required value from Appendix G, item 1 of Book by Brownell and Young. In the selection of the girder initially load due to weight of the rafter is unknown so first calculate Z only based on total rafter load and after selecting proper section for rafter repeat the calculation for $Z$ and check. If calculated $Z$ value is small than that of the std. Value for the given section then selected girder is correct otherwise repeat the calculation.
12. Repeat the calculation for other girder of other polygon.
13. Selection of column size,

$$
\begin{aligned}
& \left(\mathrm{L}^{\prime} / \mathrm{r}\right)<=180, \\
& \begin{aligned}
\text { where, } \mathrm{L}^{\prime} & =\text { Length of the column, } \mathrm{m} \\
= & \text { Height of tank }+(\text { slop of the roof })([\mathrm{D} / 2]-\mathrm{R}) \\
& \text { where, } D=\text { Diameter of the tank, } m \\
& \mathrm{R}=\text { Radius of circle which circumscribe the polygon, } m \\
& \mathrm{r}=\text { Radius of gyration, } m
\end{aligned}
\end{aligned}
$$

take $(\mathrm{L} / \mathrm{r})=180$,
Find the value of $r$ based on this value select such std. column which has next higher radius of gyration from the Appendix G, item No. 9 of Book by Brownell and Young.

Allowable compressive stress for the column may be calculated,

$$
\begin{aligned}
& \mathrm{f}^{\mathrm{f}}=\mathrm{f} /\left[1+\left(\left\{\mathrm{L}^{\prime}\right\}^{2} / 18000 \mathrm{r}^{2}\right)\right] \\
& \text { where, } \mathrm{f}^{\prime}=\text { Allowable compressive stress for the column, } \mathrm{N} / \mathrm{mm}^{2} \\
& \mathrm{f}=\text { Permissible stress for the given material, } \mathrm{N} / \mathrm{mm}^{2}
\end{aligned}
$$

Actual induced stress for the column $=\mathrm{P} / \mathrm{a}$
Where, $\mathrm{P}=$ Total Load on the column, N
$=[($ Load on the girder $)($ Length of the girder $)+[($ Load due $t$ to weight of the of column)(Length of the column)]
$\mathrm{a}=$ Cross section area of column, $\mathrm{mm}^{2}$
For satisfactory design Actual stress ( $\mathrm{P} / \mathrm{a}$ ) should be less than the allowable stress ( $\mathrm{f}^{\prime}$ ).

## Sheet-3 Design of Tall Vertical Vessel

Design a column with an appropriate support on the basis of the following data.

## Data:

## Shell:

Internal diameter (approx)
Working pressure
Working temperature
Base Chamber Height
Top Chamber Height
Material - Carbon Steel (Sp. Gr. 7.7)
Permissible Tensile stress
Insulation thickness
Density of Insulation

## Elliptical Head Design - Welded to shell

Ratio of major to minor axis
M.O.C.

Permissible tensile stress

## Support

Skirt Support Design
Height
M.O.C.
4.9 m

Carbon steel

## Trays

Sieve Tray Design
Number of Trays
Spacing between the trays
Hole diameter
Number of Holes

Thickness of the plate
Downcomer
Centre - Rectangular
Side - Chord type
Clearance from tray surface
Weir height
Height above tray
Effective length
Centre to side - Distributing
Overflow
Side to centre - distributing Overflow
M.O.C. - for trays, downcomers and weirs

Size $30 \times 262 \mathrm{~cm}$
Size $30 \times 170 \mathrm{~cm}$
50 cm
25 mm
25 mm
262 cm
170 cm
170 cm
262 cm
Stainless steel

Use data provided in table below according to sequence of batch muster 2.74 m
1.05 m
$95 \mathrm{~N} / \mathrm{mm}^{2}$
100 mm
$7700 \mathrm{M} / \mathrm{m}^{2}$

## Supports for Trays

Purlins - Channels are angles
Live load - (liquid+ liquid downcomer impact) $\quad 2100 \mathrm{~N} / \mathrm{m}^{2}$
M.O.C.

Carbon steel
Permissible tensile stress
Weight of Attachment, i.e Pipes, ladder, platform, etc
Weight of liquid and tray, etc.
$127.5 \mathrm{~N} / \mathrm{mm}^{2}$

Weight of Column (approx)
$1400 \mathrm{~N} / \mathrm{m}^{2}$
$920 \mathrm{~N} / \mathrm{m}^{2}$
Wind Pressure
Design Data: for Tall Vertical vessel

| $\begin{array}{c}\text { Roll No. } \\ \text { (As per sequence in } \\ \text { muster in each } \\ \text { batch) }\end{array}$ | $\begin{array}{c}\text { Internal diameter } \\ (\mathbf{m m})\end{array}$ | $\begin{array}{c}\text { Internal Max. } \\ \text { Operating Pressure, } \\ \text { gauge (or Absolute), }\end{array}$ | $\begin{array}{c}\text { Internal Max. } \\ \text { Temperature, }{ }^{\mathbf{o}} \mathbf{C}\end{array}$ |
| :---: | :--- | :--- | :--- |
| 1 |  | $\mathrm{~N} / \mathrm{mm}^{2}$ |  |$]$|  |
| :---: |
| 2 |

Draw similar figures with proper scale according to your design calculations in A1 drawing sheets.

Fig. Nos.: Fig. 11.1,5,11.10(a),11.11(a),11.15,11.28,11.29, Fig. 13.7(a), 13.11, 13.12, 13.13 (3rd Edition, M V Joshi and V V Mahajani)

## DESIGN OF TALL VERTICAL VESSEL

$>$ Thickness of the top of the shell end (determined on the basis of circumferential stress)
$\mathrm{t}=\left[(\mathrm{p} * \mathrm{Di}) /\left(2 * \mathrm{f}^{*} \mathrm{~J}-\mathrm{p}\right)\right]+\mathrm{c}$
where $\mathrm{p}=$ internal design stress
$\mathrm{J}=$ joint efficiency
$\mathrm{Di}=$ internal diameter
$\mathrm{f}=$ circumferential stress
$\mathrm{c}=$ corrosion allowance
$>$ This thickness may be satisfactory up to certain distance from the top of the shell.
Let $\mathrm{X}=$ distance from the top up to which we can keep thickness $=\mathrm{t}$

## $>$ The individual stresses at distance $X$ in axial direction are

## 1. Axial stress due to pressure

$\mathrm{f}_{\text {ap }}=\left(\mathrm{p}^{*} \mathrm{Di}^{*}\right) /\left(4^{*}(\mathrm{t}-\mathrm{c})\right)$
this is same throughout the column height.
2. Stresses due to dead loads.
a) Compressive stress due to weight of shell up to a distance X

$$
\begin{aligned}
\mathrm{f}_{\mathrm{ds}} & =\text { Wt. of shell/cross section of shell } \\
& =\left[(\Pi / 4) *\left(\mathrm{Do}^{2}-\mathrm{Di}^{2}\right) * \rho s * \mathrm{X}\right] /\left(\Pi * \mathrm{Dm}^{*}(\mathrm{t}-\mathrm{c})\right)
\end{aligned}
$$

Where,

$$
\begin{aligned}
& \mathrm{Do}, \mathrm{Di}=\text { internal and external diameter of shell } \\
& \rho \mathrm{ps} \quad=\text { Density of shell material }
\end{aligned}
$$

Dm = mean diameter of shell
b) Compressive stress due to wt of insulation at height ' X '
$\mathrm{f}_{\mathrm{d}(\mathrm{ins})}=\left(\Pi^{*}\right.$ Dins $^{*}$ tins $\left.* ~ \rho i n s\right) /\left(\Pi * \mathrm{Dm}^{*}(\mathrm{t}-\mathrm{c})\right)$
Where,
Dins,tins, $\rho$ ins $=$ diameter, thickness, $\&$ density of insulation
c) Compressive load due to liquid in column and trays up to a height X
$\mathrm{f}_{\mathrm{d}(\text { liq }+ \text { tray })}=\sum(\mathrm{wt}$ of (liq.+tray) per unit height $(\mathrm{X})) /\left(\Pi^{*} \mathrm{Dm} *(\mathrm{t}-\mathrm{c})\right)$
$\sum($ wt of (liq.+tray) per unit height $(\mathrm{X})=($ no of tray up to ht X$) *($ Wt of one tray
+liquid on that tray $) *\left((\Pi / 4) * \mathrm{Di}^{2}\right)$
No of trays up to height $\mathrm{X}=[(\mathrm{X}-$ top disengaging space $) /$ tray spacing $]+1$
d) Compressive stress due to attachment such as internals, top head, platforms and ladders up to a height of X
$\mathrm{f}_{\mathrm{d}(\mathrm{att})}=\left(\sum \mathrm{wt}\right.$ of attachment per unit height $\left.(\mathrm{X})\right) /\left(\Pi^{*} \mathrm{Dm}^{*}(\mathrm{t}-\mathrm{c})\right)$

## TOTAL COMPRESSIVE DEAD WEIGHT STRESS $\mathbf{f d x}=\mathbf{f d s}+\mathbf{f d}(\mathbf{i n s})+\mathbf{f d}(\mathbf{l i q}+$ tray $)+\mathbf{f d}($ att $)$

3. Stresses due to wind load at distance X

Wind pressure
Up to 20 m height: $40-100 \mathrm{kgf} / \mathrm{cm}^{2}$
$>20 \mathrm{~m} \quad: 100-200 \mathrm{kgf} / \mathrm{cm}^{2}$
wind load $=0.7 * \mathrm{pw} *$ Do * X
where,
$\mathrm{pw}=$ wind pressure
Bending Moment created by wind force at X from top
Mwx $=($ wind load $*$ distance $) / 2$
$=\left(0.7 * \mathrm{pw}^{*} \mathrm{Do}^{*} \mathrm{X}^{2}\right) / 2$
stresses induced by wind load
fwx $=$ Mwx $/ Z$
$=\left(\left(0.7 * \mathrm{pw}^{*} \mathrm{Do}^{*} \mathrm{X}^{2}\right) / 2\right) /\left((\Pi / 4)^{*} \mathrm{Do}^{2} *(\mathrm{t}-\mathrm{c})\right)$
where, $\mathrm{Z}=$ modulus of section
$=(\Pi / 4) \mathrm{Do}^{2}(\mathrm{t}-\mathrm{c})$
The stresses will be compressive on downwind side and tensile on the upwind side
4. Stresses due to eccentricity of loads ( tensile or compressive according to the position of load )
$\mathrm{fe}=\mathrm{w}_{\mathrm{e}} * \mathrm{e} /\left((\Pi / 4) * \mathrm{Do}^{2 *}(\mathrm{t}-\mathrm{c})\right)$
where,
$\mathrm{w}_{\mathrm{e}}=$ summation of eccentric loads
$\mathrm{e}=$ eccentricity
5. Stresses due to seismic loads

$$
\mathrm{fsx}=\mathrm{Msx} /\left((\Pi / 4) * \mathrm{Do}^{2 *}(\mathrm{t}-\mathrm{c})\right)
$$

where,
$\mathrm{Msx}=\left(\mathrm{CWX}^{2} / 3\right)^{*}\left[(3 \mathrm{H}-\mathrm{X}) / \mathrm{H}^{2}\right]$
Where
C = seismic coefficient
$\mathrm{W}=$ total wt of column
$\mathrm{H}=$ Height of column

## A. DETERMINATION OF HEIGHT X

Maximum axial tensile stresses
ftmax $=$ fap $-\mathrm{fdx}+\mathrm{fwx}+\mathrm{fex}+\mathrm{fsx}($ For internal pressure $)$
$\mathrm{ftmax}=\mathrm{fwx}+\mathrm{fex}+\mathrm{fsx}-\mathrm{fdx}-\mathrm{fap}($ For external pressure $)$
Now ftmax <= J*ft(allow)
Where, $\mathrm{J}=$ joint efficiency
$\mathrm{fwx}-\mathrm{fdx}+(-) \mathrm{fap}+\mathrm{fex}+\mathrm{fsx}=\mathrm{J} * \mathrm{ft}($ allow $)$
so,

$$
\begin{aligned}
\mathrm{J} * \mathrm{f}(\text { allow })=\left[1.4 * \mathrm{pw}^{*} \mathrm{X}^{2}\right] & /\left[\prod^{*} \mathrm{Do} *(\mathrm{t}-\mathrm{c})\right]+(-)\left(\mathrm{p} * \mathrm{Di}^{*}\right) /(4 *(\mathrm{ts}-\mathrm{c})) \\
& -\mathrm{fdx}+\mathrm{fex}+\mathrm{fsx}
\end{aligned}
$$

This is of the form
$a X^{2}+b X+c=0$
so its solution is $\mathrm{X}=\left(-\mathrm{b}+-\sqrt{ } \mathrm{b}^{2}-4 \mathrm{ac}\right) / 2 \mathrm{a}$
Maximum actual compressive stress
$\mathrm{fcmax}=\mathrm{fdx}-\mathrm{fap}+\mathrm{fwx}+\mathrm{fex}+\mathrm{fsx}$ ( For internal pressure)
$f c m a x=f d x+f a p+f w x+f e x+f s x($ For external pressure $)$
where, fcmax <= J*fc(allow)

$$
\mathrm{fc}(\text { allow })=(1 / 12) *\left(\mathrm{E} / \sqrt{ } 3 *\left(1-\mu^{2}\right)\right) *[(\mathrm{t}-\mathrm{c}) /(\mathrm{Do} / 2)]
$$

$\mathrm{fdx}+(-) \mathrm{fap}+\mathrm{fwx}+\mathrm{fex}+\mathrm{fs}=\mathrm{J} * \mathrm{fc}($ allow $)$
This is of the form
$a X^{2}+b X+c=0$
so its solution is $\mathrm{X}=\left(-\mathrm{b}+-\sqrt{ } \mathrm{b}^{2}-4 \mathrm{ac}\right) / 2 \mathrm{a}$
final value of X is lesser of the two

