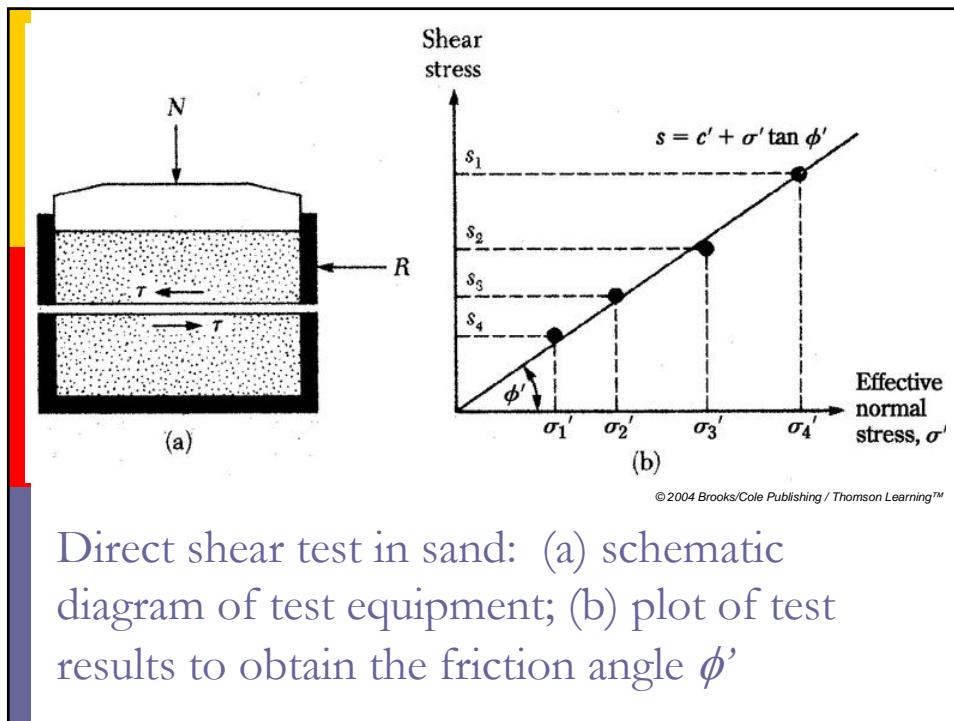
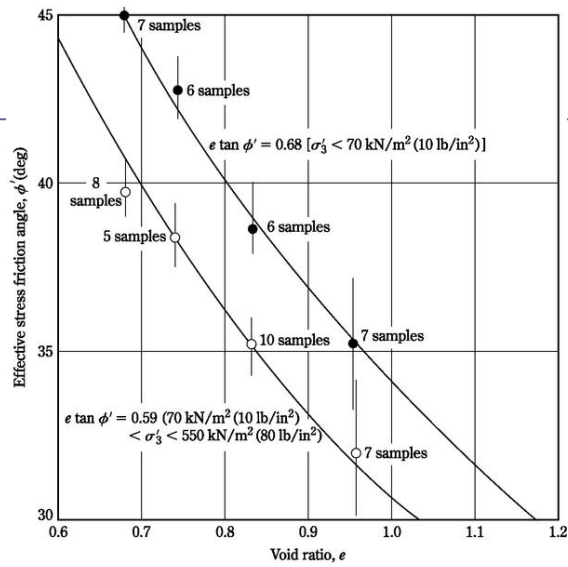


# Shear Strength of Soil

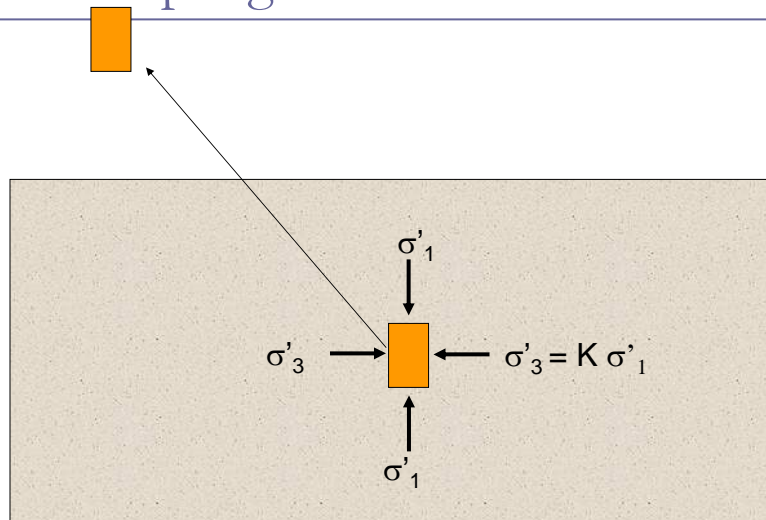


Direct shear test in sand: (a) schematic diagram of test equipment; (b) plot of test results to obtain the friction angle  $\phi'$

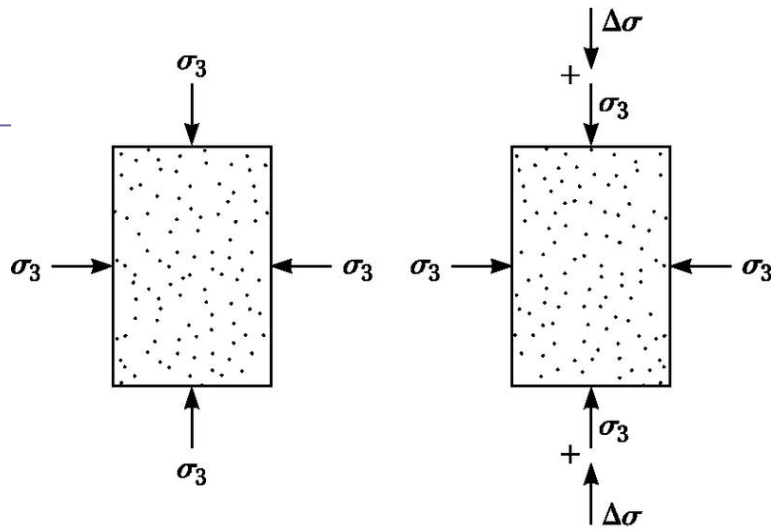
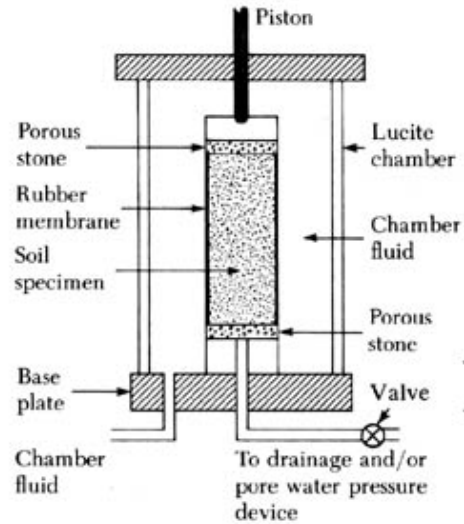


Variation of friction angle  $\phi'$  with void ratio for Chattahoochee River sand (after Vesic, 1963)

## Soil Sampling

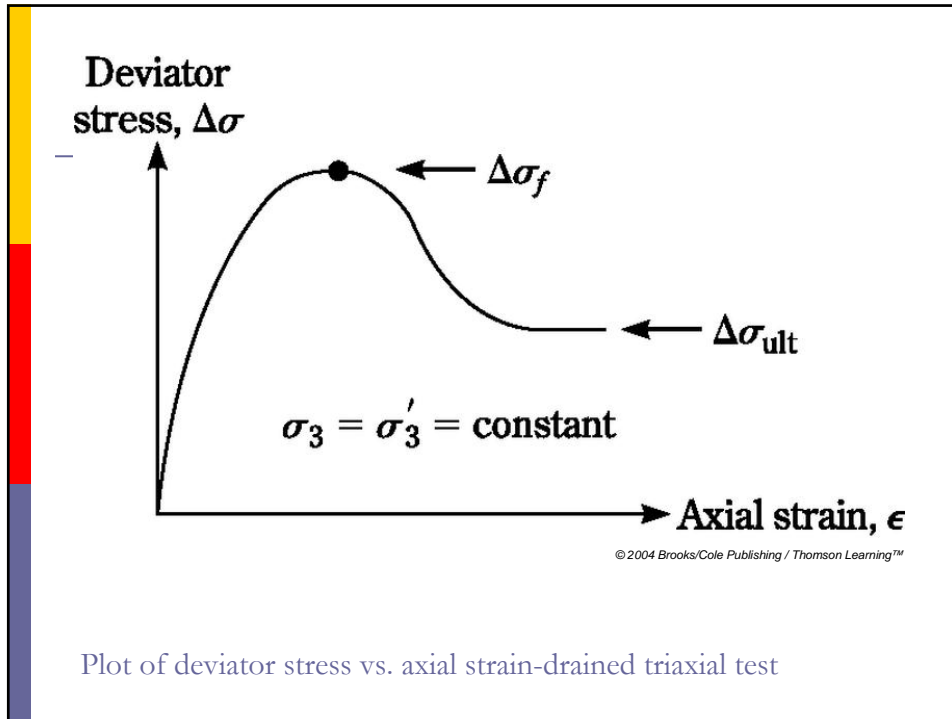


# Triaxial Test Equipment



Sequences of stress application in triaxial test

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## Triaxial Testing

- Application of cell pressure ( $\sigma_3$ ):
  - UNCONSOLIDATED (U) –
    - No drainage allowed
    - $e$  constant (fixed) if  $S_r = 1$
  - CONSOLIDATED (C) –
    - Drainage allowed when applying cell pressure
    - $e$  decreases due to sample consolidation.

## Triaxial Testing

---

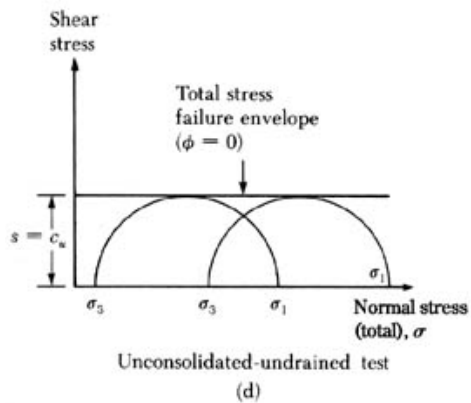
- Application of deviatoric stress ( $\sigma_1$ ):
- Deviator stress =  $\sigma_1 - \sigma_3$ 
  - Undrained (U)
    - No drainage allowed
    - $e$  constant (fixed) if  $S_r = 1$
  - Drained (D)
    - Drainage allowed when applying cell deviator stress
    - $e$  decreases due to consolidation.
    - Stress applied so slow no excess pwp

## Triaxial Tests

---

- UU – Unconsolidated Undrained
- UD – Unconsolidated Drained
- CU - Consolidated Undrained
- CD - Consolidated Drained

## Undrained Soil Strength Parameters ( $S_u, \phi = 0$ )



Note: Always test soil at void ratio on the field

## Skempton's Porewater Coefficients

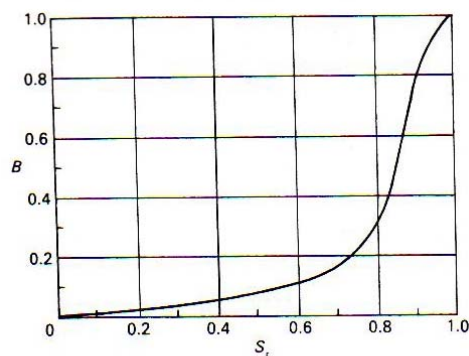
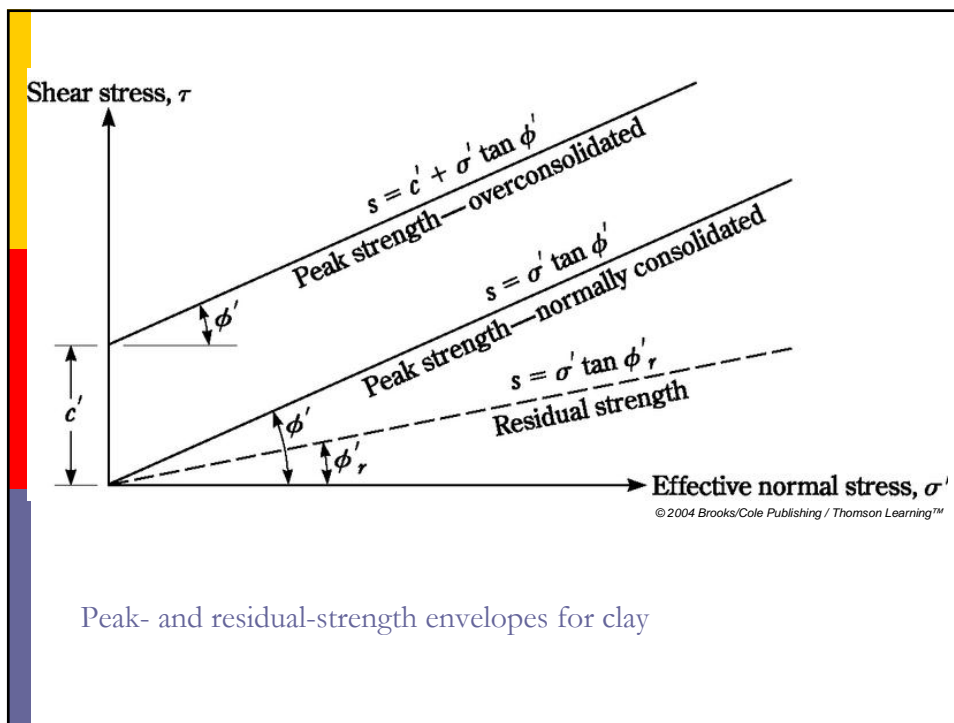
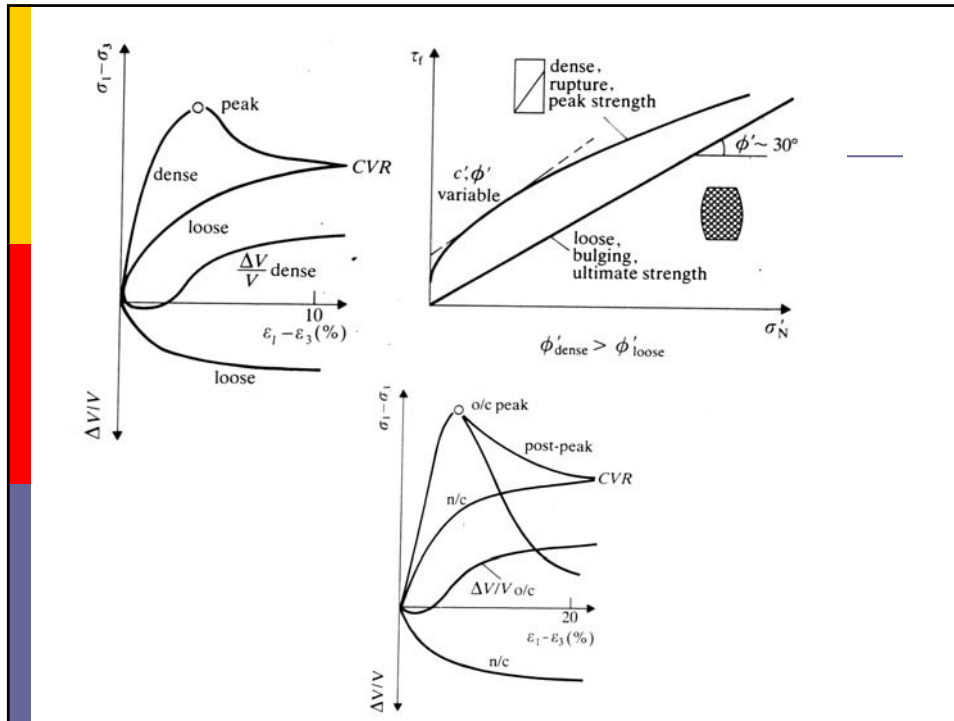


Fig. 4.9 Typical  $B/S_r$  relationship

$$B = \frac{\Delta\mu}{\Delta\sigma_3}$$

Developed "B" for a measure of degree of Water saturation



Peak- and residual-strength envelopes for clay

## Skempton's Porewater Coefficients

- Developed "A" parameter for indication of sample OCR

$$A = \frac{\Delta\mu_i}{(\sigma_1 - \sigma_3)_i}$$

where:

$$\Delta\mu_i = (\mu_i - \mu_o)$$

$\mu_o$  = initial pore pressure

$\mu_i$  = pore pressure at point of interest

$(\sigma_1 - \sigma_3)_i$  = deviatoric stress at point of interest

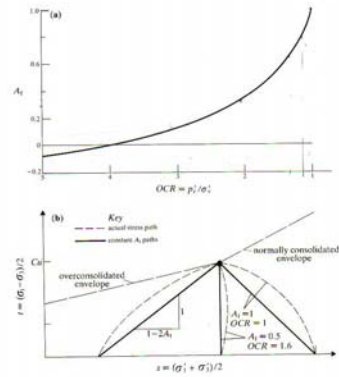
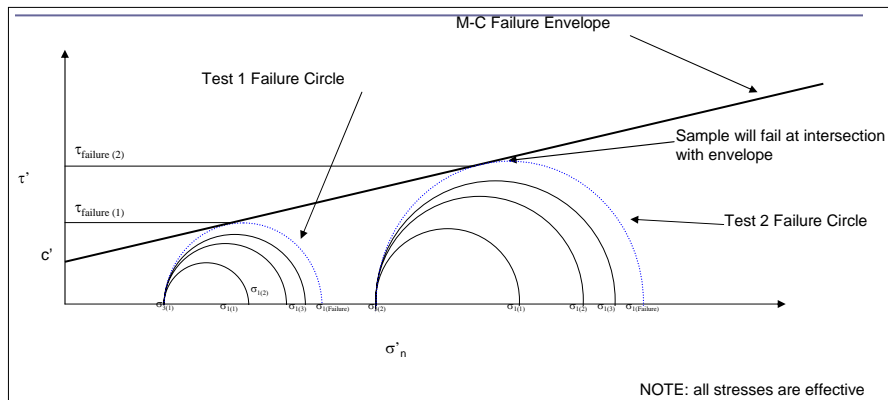


Figure 2.4 (a) Pore-water pressure  $A_i$  and (b) undrained strength of fine-grained soils.

What about negative values of  $A_f$

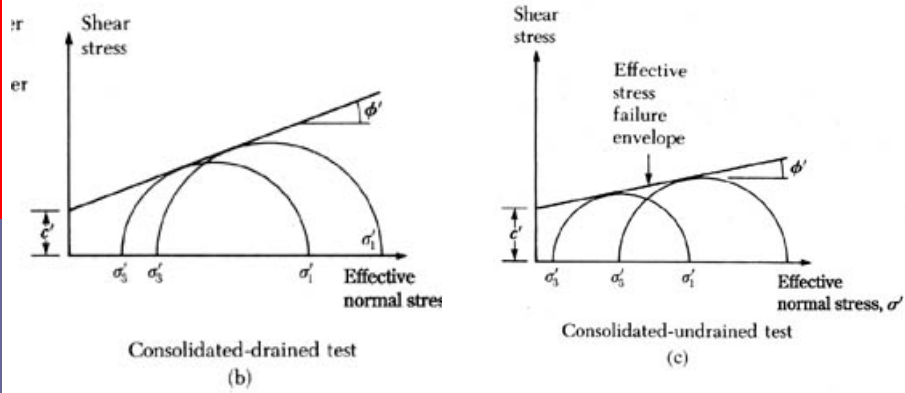
## Soil Strength



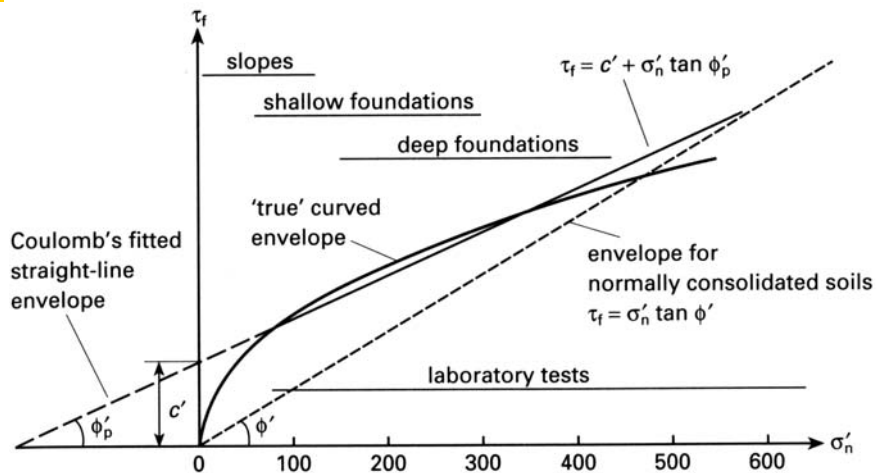
Each test is performed at a set void ratio



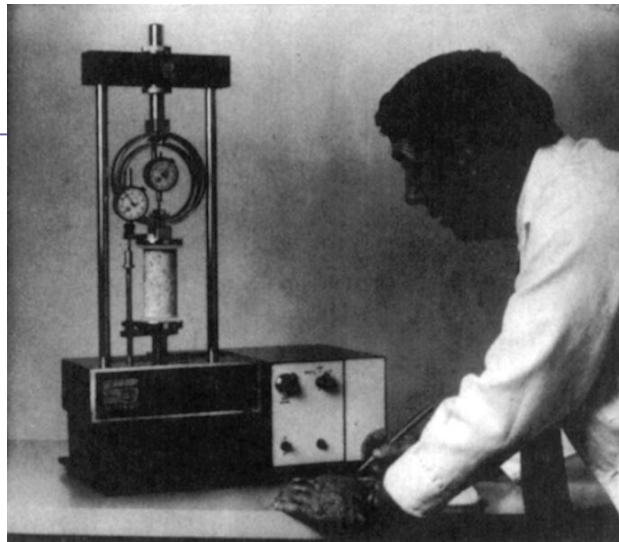
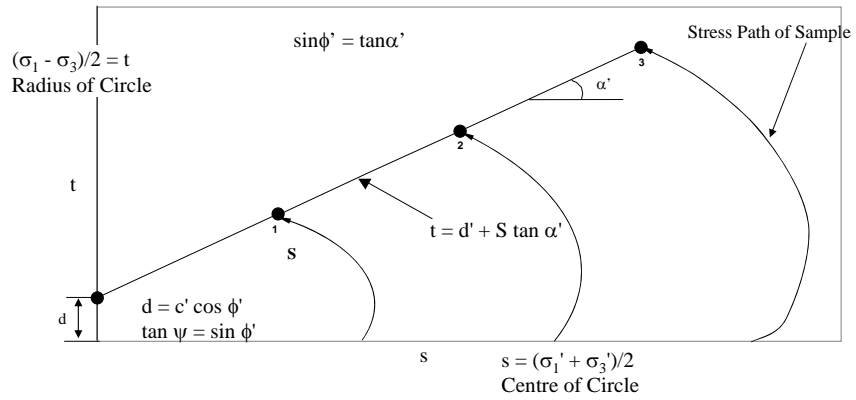
## Drained Soil Strength Parameters ( $c'$ , $\phi'$ )



## Soil Strength Envelope



# Soil Strength



Unconfined compression test in progress (courtesy of Soiltest, Inc., Lake Bluff, IL)

