

STRUCTURAL DESIGN FOR RRC SERVICE RESERVOIR FOR NANDGAON KHANDESHVAR VILLAGE WATER SUPPLY SCHEME BY USING STADD PRO

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ABSTRACT

While observing the water supply scheme in this village we noticed that, this village situated near Amravati city, and there is lots of surrounding villages workers lived there in after doing their work in MIDC and some stayed permanently due to this increasing population and water demand of village day by day. Since 2003 water supply of village were dependent on flow of Bembla river, this source diminished day by day due to the little rainfall and greater demand of water therefore government implemented some sources there scarcity of water was present therefore the study undertaken.

Keywords: Elevated Service Reservoir, Cylindrical Dome, Structural Design, Staad Pro.

I. INTRODUCTION

Nandgaon Khandeshvar village is situated on Amravati Yeotmal Road about 35 kms away from Amravati This is taluka place and all the civic activities are managed by Village Panchayat. There is temple of Khandeshvar therefore village is known as Nandgaon Khandeshvar. There is pilgrim of God Khandeshvar every year and this village is known as great market place for surrounding villages. In the year 1976, water supply scheme costing Rs 5.50 lacs was commissioned by Maharashtra Jeevan Pradhikaran and handed over to village panchayat for day-to-day maintenance. At present there is acute scarcity of water in said village. Domestic water is a major problem in these areas Vidarbha region in Maharashtra, so as to solve this problem innovative design and strategies and solution to existing problem is essential, hence for that study of ESR is undertaking.

AIM OF THE PROJECT

Making this water reservoir will serve plenty of water to the families of the locality named Nandgaon Khandeshvar to fulfill their daily water demand and also keep in mind that there will be no scarcity of water if the population increases in future up to 2040.

SCOPE OF THE PROJECT

Similar type of structural design project can be prepared for other small Towns / villages with reference of this project by manually or by using software's e.g., stadd pro.

OBJECTIVES OF STUDY

- To reduce the water scarcity in said village
- To plan water supply project by manual and software
- To study of existing water supply project and apply technical concepts
- To estimate of all components using software

II. LITERATURE SURVEY

Mr. Manoj Nallanathel [1], Mr. Ramesh [2], Jagadeesh [3] (2018)-The shape of the tanks plays predominant role in the design of overhead and underground water tanks. Usage of Staad pro in design gives accurate results for shear force and bending moment than convenient method

Bugatha Adilakshmi¹, Paliki Suribabu², Reddi Ramesh³(2016)-They concluded that from chart-1, cost will reduce when bearing capacity of soil increases. From chart-2 they concluded that between 5 lakhs to 10 lakhs capacity there is no so much of cost variation, in this region cost not so much dependent upon soil bearing capacity. From chart-3 they concluded that for 5 lakh liter capacity of water tank, for the wind speed between 33-41 m/s, Cost is approximately constant. But in the same region for 10 lakhs to above capacity, cost will vary.

G.P. Deshmukh¹, Ankush. S. Patekhede² (2015) –In this project, emphasis is given on the study of the inbuilt feature of solving seismic coefficient method in STAAD PRO V8i. This method provides the values of time period and base shear, which are very much in agreement with the values of the manually calculated results

Manish N. Gandhi, Prof. A. Rajan (2014)-For basic staging overturning moment is highest as compared to the other staging patterns. The slender staging that results from the low design forces is a very unfavorable feature for seismic areas for elevated water tanks. The current design of RC shaft type circular staging for elevated water tanks are extremely vulnerable to lateral loads cause by earthquake

Neha. S. Vanjari, [2] Kritika. M. Savant,[3] Prashant. S. Sisodia,[4] S. B. Patil (2017) –Elevated Water Tanks provide head for supply of water. When water has to be pumped into the distribution system at high heads without any pumps for supply however pumps are necessary or pumping only till tank is filled.

III. METHODOLOGY

At present there is acute scarcity of water in Nandgaon khandeshwar village govt. of Maharashtra has given approval principally to prepare the detail water supply project. Therefore, the detailed survey including investigation of sources of water is undertaken. This required data is collected for design. Also carrying out the study of existing water supply arrangements and course of action to solve the problem.

IV. MODELING AND ANALYSIS USING SOFTWARE

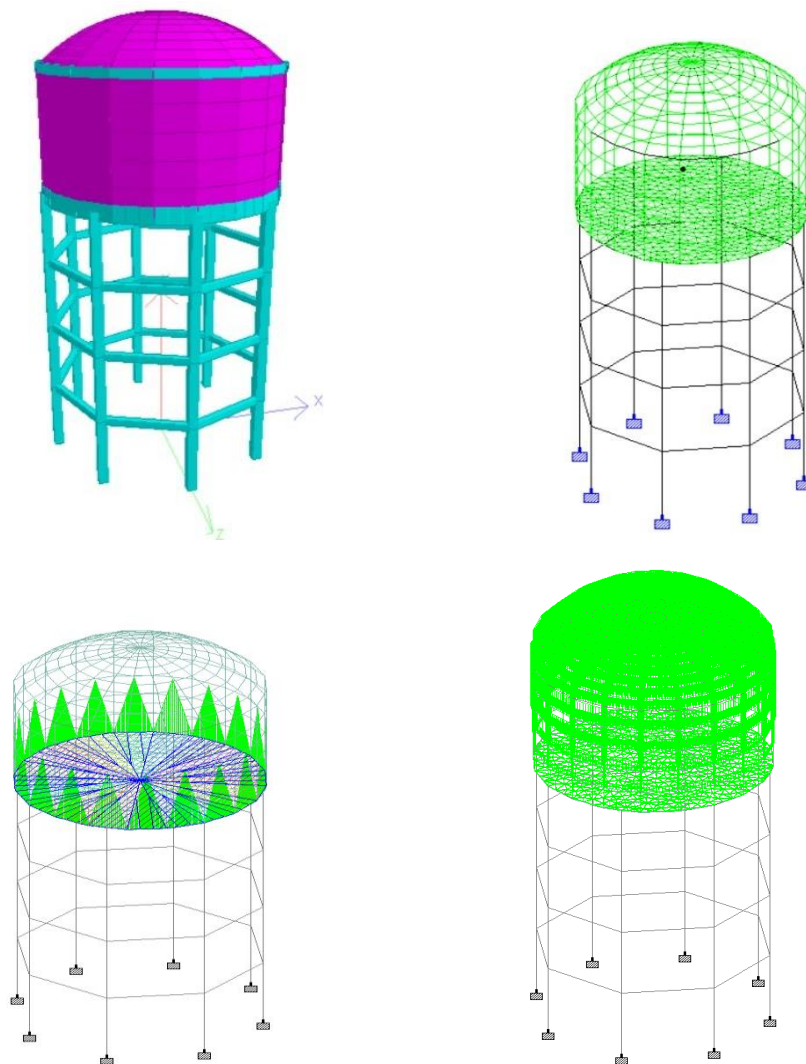


Figure: 3D view of elevated service reservoir.

V. RESULTS AND DISCUSSION

Followings are the design details of the water reservoir:-

Dimensions:

- No of columns: - eight No.
- Bracing Dimension: - 230 x 300 millimeter
- Column Dimension: - 350 x 350 millimeter
- Bottom Ring beam: - 300 x 690 millimeter
- high Ring beam: - 300 x 350 millimeter

Plate Thickness:

- Top dome: - 120 mm
- Cylindrical wall: - 150 mm
- Bottom slab :180mm
- Grade of concrete: M30
- Grade of steel: Fe 500

Clear Cover as per IS: 456, 2000[1]:

- Column: 40mm
- Beam: 25mm
- Plate: 25mm

Reinforcement Details:-

- **Column** :- 12 nos. of 12mm ϕ Fe-500 main bars @ 170 mm c/c ; 8mm ϕ Fe-500 bars as stirrup @ 300mm c/c.
- **Beam** :- 6 nos. of 12mm ϕ Fe-500 main bars ; 12mm ϕ Fe-500 bars as stirrup @ 300mm c/c.
- **Plate** :- 12mm ϕ Fe-500 main bars @ 125 mm c/c for base plate, bottom inclined plates & vertical plates.
10mm ϕ Fe-500 main bars @ 125 mm c/c for upper inclined plates & top most plate.

Load Cases: -

1. D.L \rightarrow Dead Load \rightarrow Self Weight
2. E.L \rightarrow Element Load \rightarrow Plate Loads \rightarrow Hydrostatic Load.
3. W.L.X \rightarrow Wind Load at X-direction.
4. W.L.Z \rightarrow Wind Load at Z-direction.

Note – Seismic load is not considered here as the location of the construction is situated under Zone II (low intensity) as per IS 1893 Part 1

Bending Moment:

	B	C	D	E	F	G	H	I	J	K	L	M	N	O	P	Q
1																
2																
3																
4	9	1	LOAD	Max +ve	3.444	19.785	0	0.769								
5				Max -ve	1.435	-7.741	3.444	-0.785								
6	2	LOAD	Max +ve	3.444	0.291	0	0.04									
7				Max -ve	0	-0.287	3.444	-0.041								
8	3	LOAD	Max +ve	3.444	26.906	0	3.738									
9				Max -ve	0	-26.555	3.444	-3.782								
10	4	WX	Max +ve	3.444	38.879	0	8.683									
11				Max -ve	0	-45.659	3.444	-3.733								
12	5	WZ	Max +ve	3.444	65.452	3.444	3.322									
13				Max -ve	0	-62.336	0	-6.449								
14	6	GENER	Max +ve	3.444	30.114	0	1.214									
15				Max -ve	1.435	-11.681	3.444	-1.239								
16	7	GENER	Max +ve	3.444	70.746	0	11.391									
17				Max -ve	0	-45.249	3.444	-5.471								
18	8	GENER	Max +ve	3.444	102.633	3.444	2.996									
19				Max -ve	0	-65.261	0	-6.768								
20	9	GENER	Max +ve	0	64.333	3.444	3.488									
21				Max -ve	3.157	-23.163	0	-9.449								
22	10	GENER	Max +ve	0	84.345	0	8.71									
23				Max -ve	3.444	-54.451	3.444	-4.978								
24	11	GENER	Max +ve	3.444	24.091	0	0.971									
25				Max -ve	1.435	-9.345	3.444	-0.991								
26	12	GENER	Max +ve	3.444	87.996	0	14.178									
27				Max -ve	0	-56.131	3.444	-6.777								
28	13	GENER	Max +ve	3.444	127.855	3.444	3.805									
29				Max -ve	0	-81.146	0	-8.52								
30	14	GENER	Max +ve	0	80.847	3.444	4.421									
31				Max -ve	3.157	-29.318	0	-11.871								
32	15	GENER	Max +ve	0	105.861	0	10.827									
33				Max -ve	3.444	-68.5	3.444	-6.162								
34	16	GENER	Max +ve	3.444	29.678	0	1.154									
35				Max -ve	1.435	-11.612	3.444	-1.178								
36	17	GENER	Max +ve	3.444	17.807	0	0.692									
37				Max -ve	1.435	-6.967	3.444	-0.707								
38	0	1	LOAD	Max +ve	0	19.765	3.444	0.763								
39				Max -ve	2.009	-7.725	0	-0.774								
40	2	LOAD	Max +ve	0	0.29	3.444	0.04									
41				Max -ve	3.444	-0.285	0	-0.04								
42	3	LOAD	Max +ve	0	26.796	3.444	3.713									
43				Max -ve	3.444	-26.335	0	-3.733								
44	4	WX	Max +ve	3.444	82.969	0	0.047									
45				Max -ve	0	-84.435	3.444	-5.754								
46	5	WZ	Max +ve	3.444	24.85	3.444	3.519									
47				Max -ve	0	-20.059	N/A	N/A								
48	6	GENER	Max +ve	0	30.082	3.444	1.204									
49				Max -ve	2.009	-11.656	0	-1.221								
50	7	GENER	Max +ve	3.444	109.157	N/A	N/A									
51				Max -ve	0	-77.257	3.444	-5.941								
52	8	GENER	Max +ve	3.444	39.414	3.444	5.186									
53				Max -ve	1.148	-9.676	0	-0.542								
54	9	GENER	Max +ve	0	125.388	3.444	7.868									
55				Max -ve	3.444	-89.968	0	-1.033								
56	10	GENER	Max +ve	0	48.136	N/A	N/A									
57				Max -ve	2.87	-23.088	3.444	-3.26								
58	11	GENER	Max +ve	0	24.066	3.444	0.963									
59				Max -ve	2.009	-9.325	0	-0.977								
60	12	GENER	Max +ve	3.444	136.873	N/A	N/A									
61				Max -ve	0	-97.005	3.444	-7.487								
62	13	GENER	Max +ve	3.444	49.695	3.444	6.423									
63				Max -ve	1.148	-12.242	0	-0.617								
64	14	GENER	Max +ve	0	156.3	3.444	9.775									
65				Max -ve	3.444	-112.033	0	-1.231								
66	15	GENER	Max +ve	0	59.735	N/A	N/A									
67				Max -ve	2.87	-28.577	3.444	-4.134								
68	16	GENER	Max +ve	0	29.647	3.444	1.144									
69				Max -ve	2.009	-11.588	0	-1.161								
70	17	GENER	Max +ve	0	17.788	3.444	0.686									
71				Max -ve	2.009	-6.953	0	-0.696								
72																

SF calculation:-

STAAD.Pro Query Bending and Shear Results

Bending about Z for Beam 30

Load Case: 1:LOAD CASE 1DL



Dist.m	Fy(kN)	Mz(kip-in)
0.000000	3.8842	32.9609
0.287013	3.4165	23.6881
0.574025	2.9488	15.6033
0.861038	2.4811	8.7066
1.148051	2.0134	2.9979
1.435064	1.5457	-1.5227
1.722076	1.0780	-4.8553
2.009089	0.6104	-6.9998
2.296102	0.1427	-7.9563
2.583115	-0.3250	-7.7247
2.870127	-0.7927	-6.3051
3.157140	-1.2604	-3.6974
3.444153	-1.7281	0.0984

Load calculation:-

Wind intensity -

Intensity	Height
1.5	19
1.2	15
1	12
0.75	10

Exposure factor - 1

1. Dead Load - 1) Self wet
- 2) Paraphet wall=0.125x20x1.3=3KN/m
2. Live Load =Floor Load =1KN/m
3. Hydrostatic Pressure= δgh
= 1000x9.81x5
=49.050 KN/m
4. Wind load x =Factor =1
Z = Factor =1
5. Load combination = Indian Standard Code
6. Material = concrete IS 456


$F_c = 3000 \text{ KN/m}$

$F_y \text{ min} = 50000 \text{ KN/m}$

$F_y \text{ sec} = 50000 \text{ KN/m}$

Max min = 16 mm

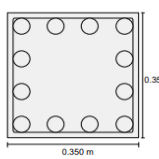
Max sec = 12mm

		Job No	Sheet No	1	Rev
Software licensed to		Part			
Job Title		Ref			
Client		By	Date	18-Apr-22	Chd
		File	wwater.stl	Date/Time	20-Apr-2022 13:05

STAAD.Pro Query Concrete Design

Beam no. 57

Design Code: IS-456



Design Load

Load	4
Location	End 1
Pu(Kns)	-0.070000
Mx(Kns-M)	4.830000
My(Kns-M)	0.000000

Design Results


Fy(Mpa)	415
Fc(Mpa)	30
As Reqd(mm ²)	980.000000
As (%)	1.108000
Bar Size	12
Bar No	12

Print Time/Date: 20/04/2022 00:17

STAAD.Pro CONNECT Edition 22.10.00.153

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CONCRETE DESIGN FOR COLUMNS

		Job No	Sheet No	1	Rev
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Job Title		Ref			
Client		By	Date	18-Apr-22	Chd
		File	wwater.stl	Date/Time	20-Apr-2022 13:05

Job Information

Engineer	Checked	Approved
Name:		
Date:	18-Apr-22	

Project ID

Project Name

Structure Type

SPACE FRAME

Number of Nodes	882	Highest Node	902
Number of Elements	128	Highest Beam	1526
Number of Plates	1364	Highest Plate	1508

Number of Basic Load Cases	5
Number of Combination Load Cases	12

Included in this printout are data for:

All The Whole Structure

Included in this printout are results for load cases:

Type	L/C	Name
Primary	1	LOAD CASE 1DL
Primary	2	LOAD CASE 2LL
Primary	3	LOAD CASE 3FLUID
Primary	4	W X
Primary	5	W Z
Combination	6	GENERATED INDIAN CODE GENRAL_S1
Combination	7	GENERATED INDIAN CODE GENRAL_S1
Combination	8	GENERATED INDIAN CODE GENRAL_S1
Combination	9	GENERATED INDIAN CODE GENRAL_S1
Combination	10	GENERATED INDIAN CODE GENRAL_S1
Combination	11	GENERATED INDIAN CODE GENRAL_S1
Combination	12	GENERATED INDIAN CODE GENRAL_S1
Combination	13	GENERATED INDIAN CODE GENRAL_S1
Combination	14	GENERATED INDIAN CODE GENRAL_S1
Combination	15	GENERATED INDIAN CODE GENRAL_S1
Combination	16	GENERATED INDIAN CODE GENRAL_S1
Combination	17	GENERATED INDIAN CODE GENRAL_S1

Bentley Software licensed to CONNECTED User: User is not logged in to CONNECTION Client		Job No Sheet No: 2 Rev
Job Title		Part
Client		Ref By Date: 18-Apr-22 Chd File: wwater.sld Date/Time: 20-Apr-2022 13:05

Number	Name	Type
1	LOAD CASE 1DL	Dead
2	LOAD CASE 2LL	Live
3	LOAD CASE 3FLUID	Fluids
4	W X	Wind
5	W Z	Wind

Comb.	Combination L/C Name	Primary	Primary L/C Name	Factor
6	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.50
		2	LOAD CASE 2LL	1.50
7	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.20
		2	LOAD CASE 2LL	1.20
		4	W X	1.20
8	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.20
		2	LOAD CASE 2LL	1.20
		5	W Z	1.20
9	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.20
		2	LOAD CASE 2LL	1.20
		4	W X	-1.20
10	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.20
		2	LOAD CASE 2LL	1.20
		5	W Z	-1.20
11	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.20
		2	LOAD CASE 2LL	1.20
12	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.50
		4	W X	1.50
13	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.50
		5	W Z	1.50
14	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.50
		4	W X	-1.50
15	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.50
		5	W Z	-1.50
16	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	1.50
17	GENERATED INDIAN CODE GENERAL S*	1	LOAD CASE 1DL	0.90

VI. CONCLUSION

The project can be useful for field engineers while implementing other water supply projects. By knowing the current population of village i.e., 16857 souls with rate of water supply 40LPCD water demand are 674320 liters (0.7 MLD) as we considered expected population in 2040 will be 32500 souls then with the same rate demand in liters will increase twice i.e. (1.4 MLD).

ACKNOWLEDGEMENTS

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VII. REFERENCES

- [1] American Concrete Institute, ACI 350.3. (2006) Seismic Design of Liquid Containing Concrete Structures, Farmington Hills, MI. Chen, J. Z. (2010). Generalized SDOF System for Dynamic Analysis of Concrete Rectangular Liquid Storage Tanks, Ph.D. thesis, Ryerson University, Toronto, Canada
- [2] Chen, J. Z. and Kianoush, M. R. (2009). Generalized SDOF System for Seismic Analysis of Concrete Rectangular Liquid Storage Tanks, Journal of Engineering Structures 31, 2426-2435.
- [3] Housner, G. W. (1963). The Dynamic Behavior of Water Tanks, Bulletin of the Seismological Society of American 53:2.
- [4] Ghaemian, M., Kianoush, M. R. and Mirzabozorg, H. (2005). Time Domain Dynamic Analysis of Rectangular Liquid Containers in Three-Dimensional Space. Journal of European Earthquake Engineering XIX (2), 3-9. E3S Web Conf. Volume 309, 2021
- [5] 3rd International Conference on Design and Manufacturing Aspects for Sustainable Energy (ICMED-ICMPC 2021)
- [6] European Committee for Standardization (CEN). (2006). Design Provisions for Earthquake Resistance of Structures, Part I – General Rules and Part 4 – Silos, Tanks and Pipelines, Eurocode 8, Brussels, Belgium

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- [7] Mr. Manoj Nallanathel, Mr. B. Ramesh, Mr. L. Jagadeesh (Design and Analysis of Water tanks using STADD-PRO, International Journal of Pune and Applied Mathematics, Issue 2018) (Page no 3021-3028)
- [8] Neha S. Vanjari, Kritika M. Sawant, Prashant S. Sisodia, S. B. Patil (Design of Circular Overhead Water Tanks, International Journal of Engineering Research in Mechanical and Civil Engineering vol 2, Issue, July 2017) (Page no 69-81).
- [9] Mr. Sandip L. Gongale, Mr. K. Ghumde (Analysis of Elevated Service Reservoir by using commercial software,) (Dec-2016).