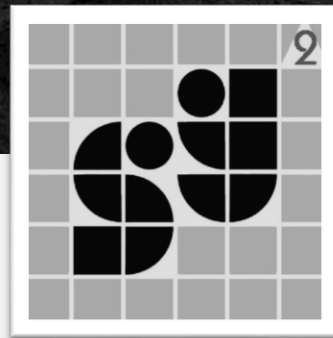


# Structural Design Practices in the Philippines

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Looking Back and Looking Beyond.

**Engr. Jose A. Sy**  
Sysquared & Associates, Inc.  
President/Ceo



# A G E N D A

- 01.** Marcos Regime
- 02.** August 21, 1983 : Ninoy Aquino's Assassination
- 03.** EDSA People Power Revolution
- 04.** 1987 : Key Projects
- 05.** 1990 – 1997 : Construction on the Rise
- 06.** 1997 : Asian Financial Crisis
- 07.** 2000 Onwards : Moving Forward
- 08.** Look Ahead



# Marcos Regime

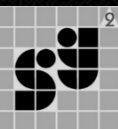


That time, no structural design code is available in the Philippines.

Seismic lateral design force is proportional to the building weight.

$$V = 0.10 W$$

The collapse of Ruby Tower, a six-story condominium building, in the magnitude 7.3 1968 earthquake started the call for better design and construction practice in the Philippines



# Marcos Regime

## National Structural Code for Buildings, 1<sup>st</sup> Edition

- Based on the Uniform Building Code (UBC 1970)
- Divided the Philippines into 3 zones with corresponding seismic zone factors

$$V = ZKCW$$

- Ductility requirements are still not included from 1971 San Fernando Earthquake

NATIONAL STRUCTURAL CODE  
FOR BUILDINGS

NSCB 1972



# Marcos Regime



Cultural Center of the Philippines (CCP)



Folk Arts Theatre



Philippine International Convention Center (PICC)

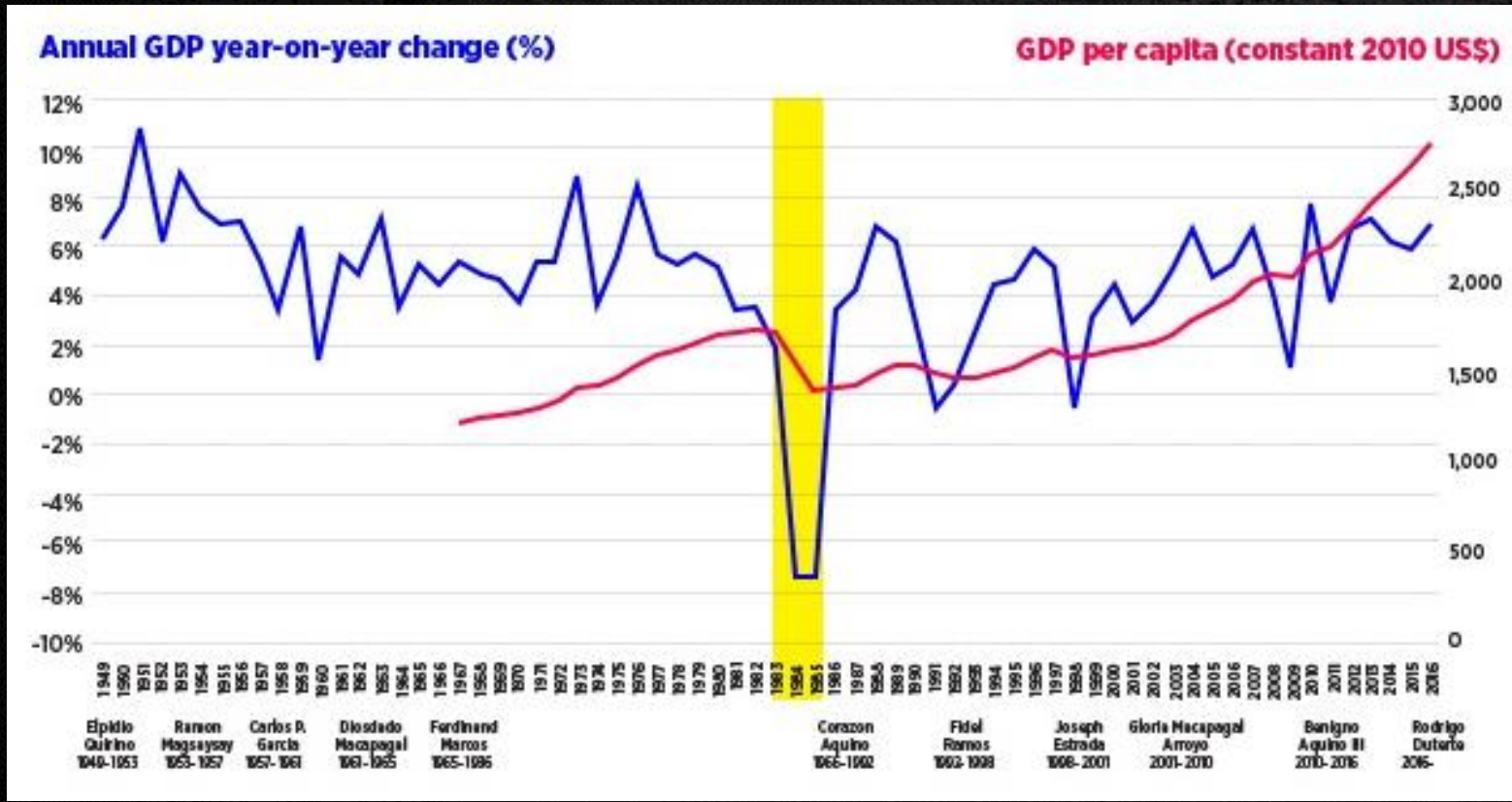


Philippine Heart Center

# AUGUST 21, 1983

## Ninoy Aquino Assassination

- Economic Status



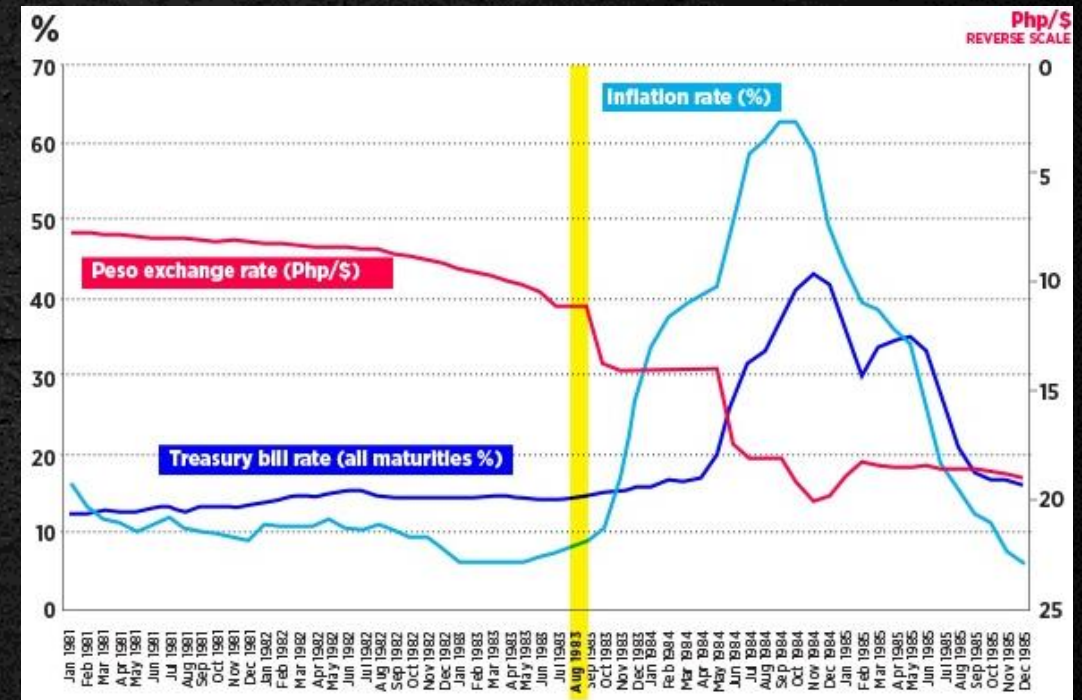
“The period shortly after the Aquino Assassination in August 1983 and the Debt Moratorium in October 1983 saw the severest economic contractions in the Philippines since after World War II.”

# AUGUST 21, 1983

## Ninoy Aquino Assassination

### ■ Dollar Exchange Rate

- The Philippines Peso to US Dollar Exchange rate was already suffering due to increasing debt, but it truly took a downfall almost immediately after the assassination.
- The peso plunged 30% in 1983, and a further 50% in 1984, **1 USD = 27 PHP**



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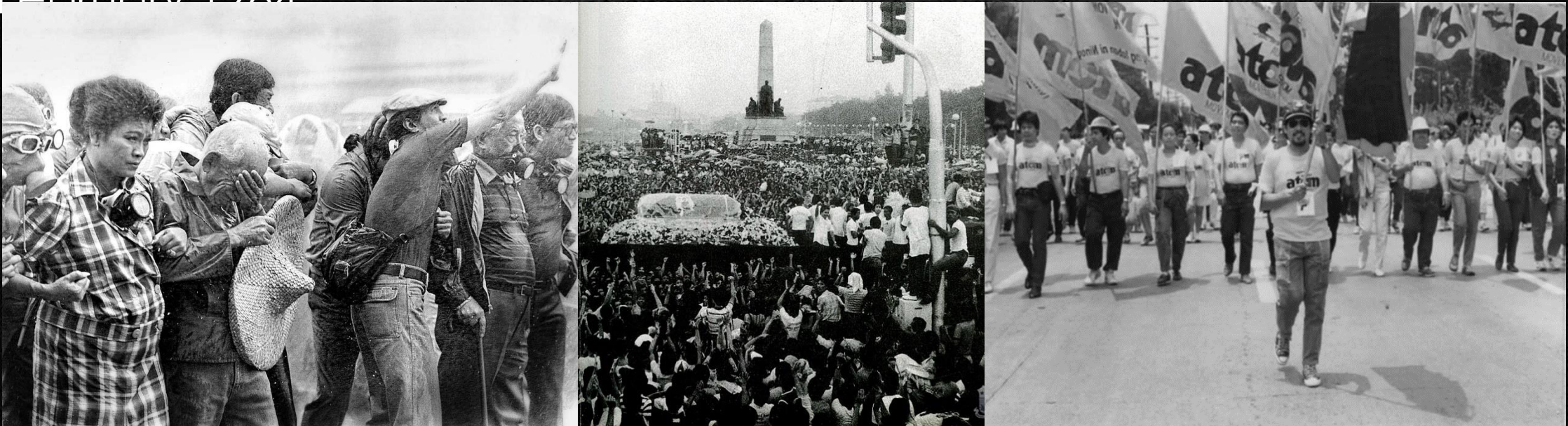
# AUGUST 21, 1983

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## Ninoy Aquino Assassination

- Rampant Protests and Rallies

165 rallies, marches, and other demonstrations took place between August 21 and September 30, 1983. Protest demonstrations continued into the following year, with more than 100 held between October 1983 and February 1984.





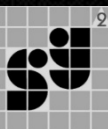
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# AUGUST 21, 1983

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## Ninoy Aquino Assassination

- As a result, no construction activities were carried out
  - Decrease in overall government spending
  - Restrictive monetary policies became detrimental, especially in the industrial sector
  - Majority of government led programs gave focus to the agricultural sectors only, failing to address the needs of other industries
  - In effect, construction activities were at a standstill



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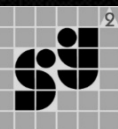
# AUGUST 21, 1983

## Ninoy Aquino Assassination



### Pacific Star (Nauru Building)

- Tallest building in the Philippines during the revolution
- 112.5 meters
- 29 stories with 4 basement levels



# EDSA People Power Revolution





# EDSA People Power Revolution

## IM kept vigil at Camp Crame

by REYM LAGOSIN

UNDAUNTED by the curfew imposed by Mr. Marcos Monday night, an estimated one million people kept their vigil at the barricaded area along E. Edsa, Santa Arce Avenue until this morning.

Groups from all over

Metro Manila have each secured their "quarters" beside the street islands and pedestrian sidewalks along the stretch of EDSA from Ortigas Avenue to Santolan Road, fronting Camp Crame and Agate-Ida.

To page 2

## Balanced news, fearless views

# PHILIPPINE DAILY INQUIRER

WEDNESDAY, February 26, 1986

★ P2

Vol. One No. 80

## Businessmen heed call of Cory

by EDGAR MALAY

FOREIGN and local businessmen will reopen their establishments today in answer to President Corason C. Aquino's call for cooperation in rebuilding the country's battered economy.

Local businessmen, particularly those who set in

the defunct Philippine Productivity Council (PPC), under former President Marcos, have declared that they will contribute to the efforts of the government — which is supported by the people.

Businessmen have remained closed since the

To page 2

# It's all over; Marcos flees!

By LUIS D. BELTRAN  
Editor-in-Chief



## Cory Aquino forms new government

OATH: President Corason Aquino takes her oath of office before SC Senior Justice Claudio Teehanee at Club Filipino. Witnessing among others (from left) are daughters Pasty, and Betty, and Sen. Aurora Aquino and a host of Opposition leaders.

THE PEOPLE of the Philippines who brought Ferdinand E. Marcos to the pinnacle of power two decades ago with their votes yesterday brought him crashing out of power with their bodies.

After three days of alternately blistering, between threatened attacks, just ineffectual verbal positions, deposed President Marcos last night ended 20 years of almost a constitutional and authoritarian rule by

accepting exile to the United States.

His departure wrote him in a sense drama that saw his vast army abandoning him in dozens and joining the ordinary Filipinos in opposing the most oppressive regime in Philippine history.

Pro-Cory MIA forces stopping

Marcos apparently mumbled his departure and talked hoaxes of angry Filipinos by an oath-

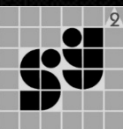
First destroying the legitimacy of Marcos' regime by denying him a new mandate last Feb. 7, the Filipino people then frustrated his attempts to stay in power by blocking his tanks with their bodies.

Marcos took power in 1965, was weakened in 1969 and perpetuated himself in power by declaring Martial Law in 1972 and holding successive mock referenda and elections — in gain new "mandates" from the Filipino people.

In the elections held last February 7, the first free elections since Mar-

was preceded by nationwide fears that he would trigger a bloodbath by making a last ditch stand and his departure has made official the success of a virtually bloodless revolution led by his former Defense Minister, Juan Ponce-Enrique, and his former Vice-Chief of Staff General Fidel Ramos.

Marcos has been

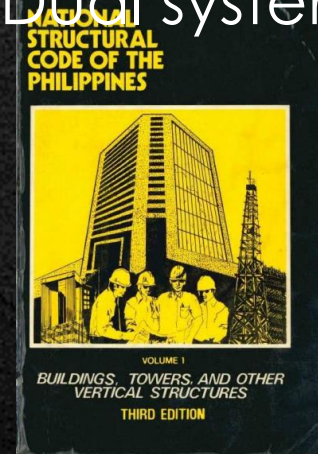


# 1987 : Key Projects

## Pacific Plaza



- Tallest building to be after the revolution
- 43 stories
- Designed using NSCP 1987 (based on UBC 1985)
  - Wind pressure,  $P = C_e C_q q_s I$
  - Earthquake force,  $V = ZIKCSW$
- First building in the Philippines to use 6000 psi concrete
- Dual system

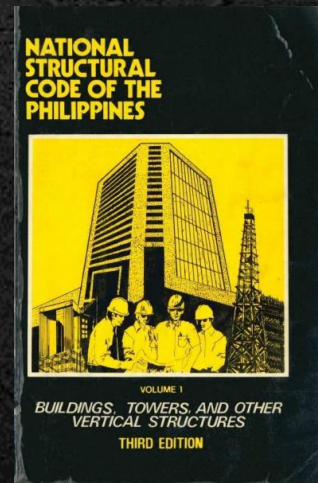


# 1987 : Key Projects

## SM Megamall



- Largest shopping mall in Asia in 1991
- 330,000 square meters total floor area
- Also designed using NSCP 1987
- Utilized post-tensioning for long span beams



# 1990 - 1997

The Magnitude 7.7 Luzon Earthquake in 1990 reminded us to further improve the structural design practice in the Philippines



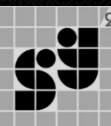
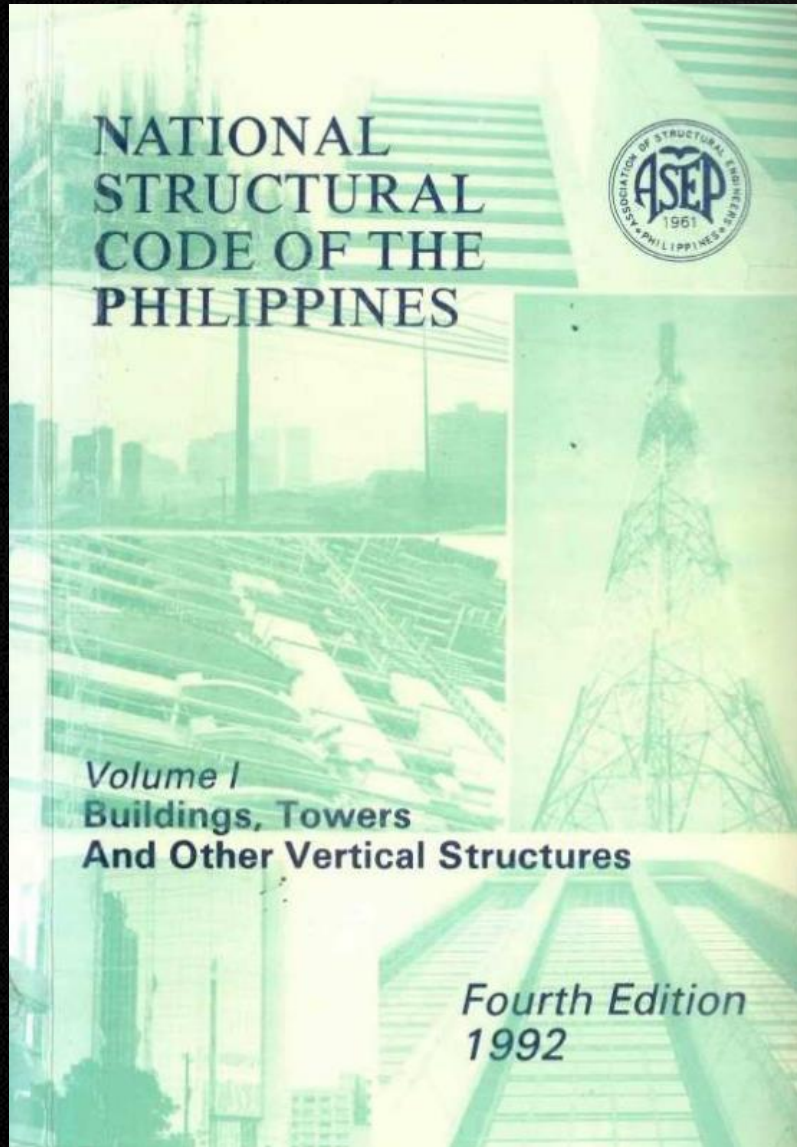


# 1990 - 1997

## National Structural Code for Buildings, 4<sup>th</sup> Edition

- Based on the Uniform Building Code (UBC 1988)
- Inclusion of Seismic Zone Maps (Zone 2 and 4)
- Introduction of response modification factors or “R-Factors”

$$V = (ZIC/R_w)W$$



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# 1990 - 1997

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## Construction On the Rise

The country saw a boom in construction projects from the early to mid-90s



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# 1990 - 1997

## Construction On the Rise



### **PBCom Tower**

- Tallest building in the Philippines
- 259 meters
- 52 storeys

### **GT International Tower**

- 217 meters
- 47 storeys



1990 - 1997

Construction On the Rise



**LKG Tower**

- 180 meters
- 38 storeys

**Ayala Tower One**

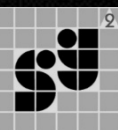
- 160 meters
- 35 storeys



# 1997 : Asian Financial Crisis



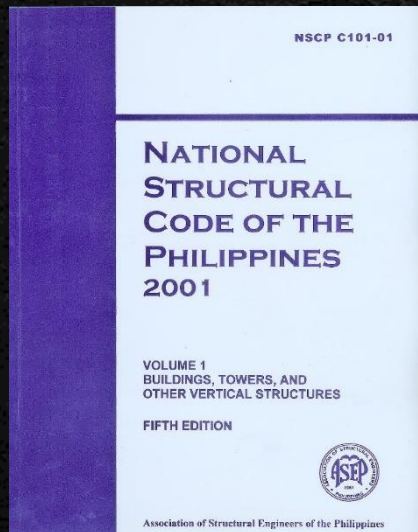
- The Peso dropped from 26.2 Pesos per US Dollar at the start of the crisis to 43.173 Pesos. in 1998, it was down to 54 Pesos per US Dollar.
- Investment in the Philippines declined as a result of the limitations of investors to accessing capital.
- In June 1997, office vacancy rates for Makati were extremely low at 0.9%, but by September 1998, the Makati area then had a vacancy rate of 5.8%.



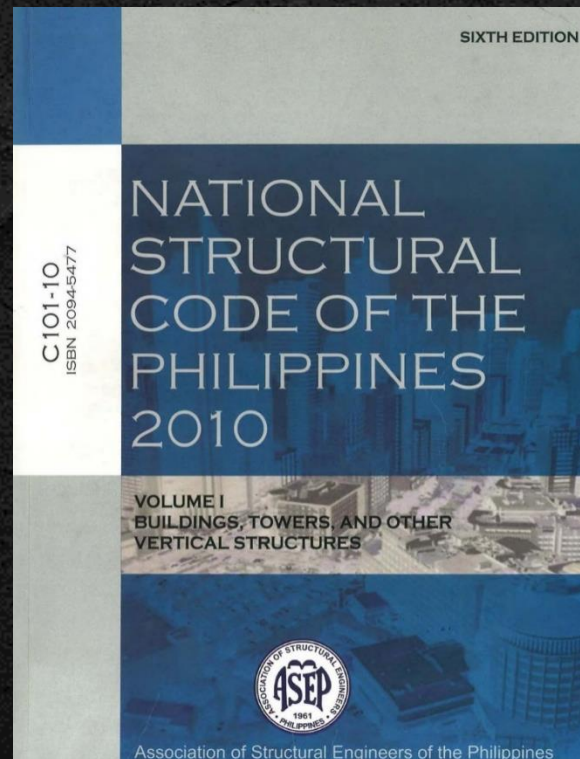
# 2000 Onwards

The National Structural Code of the Philippines continued to develop.

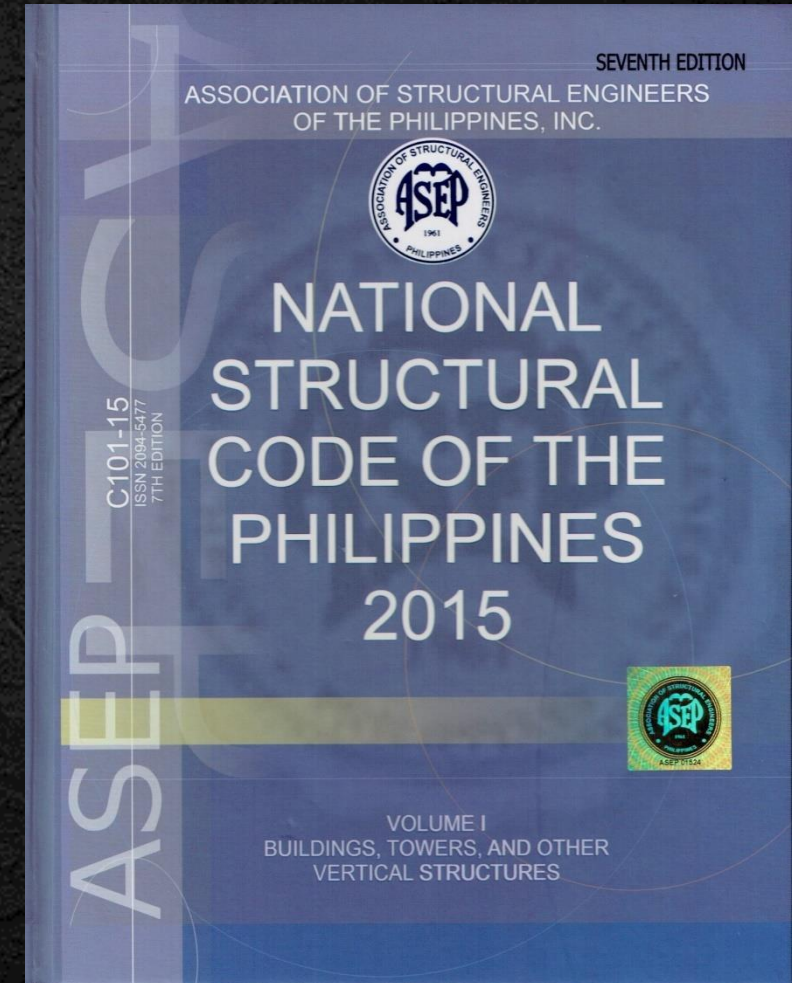
Provisions of UBC 1997 are still being adopted.



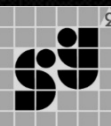
NSCP 2001



NSCP 2010



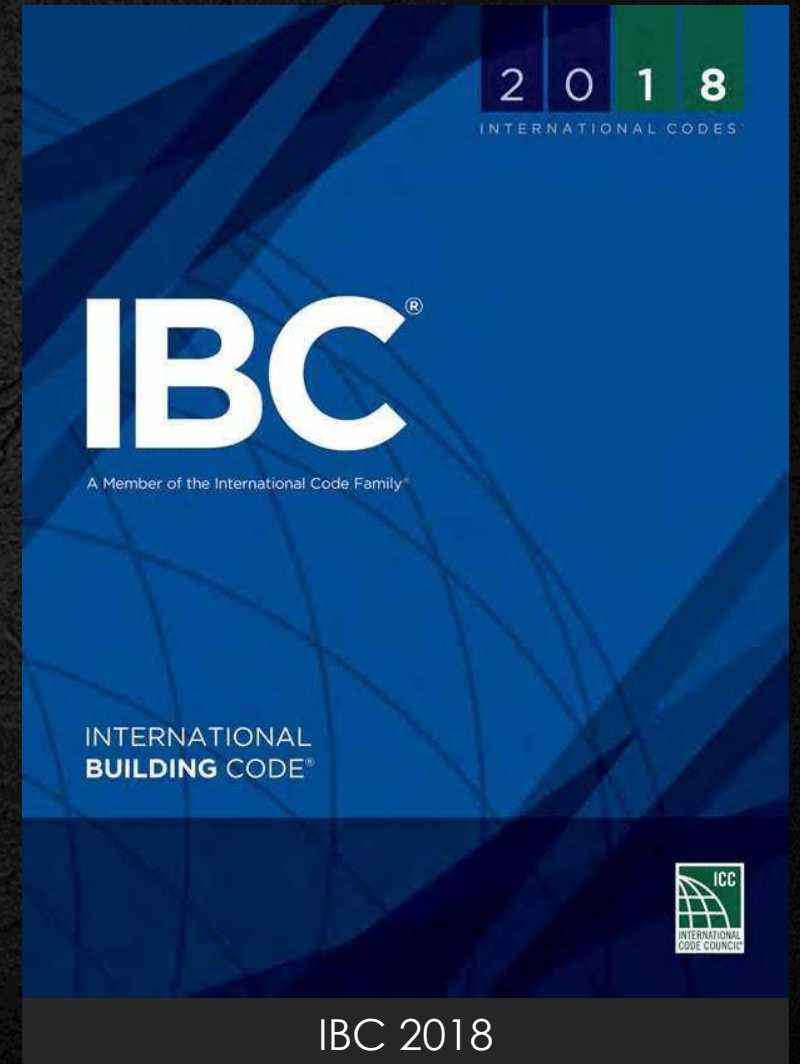
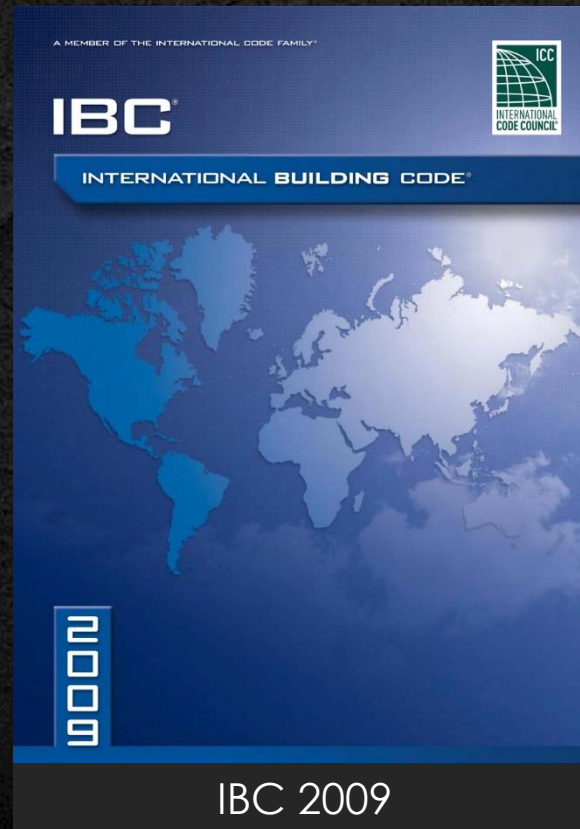
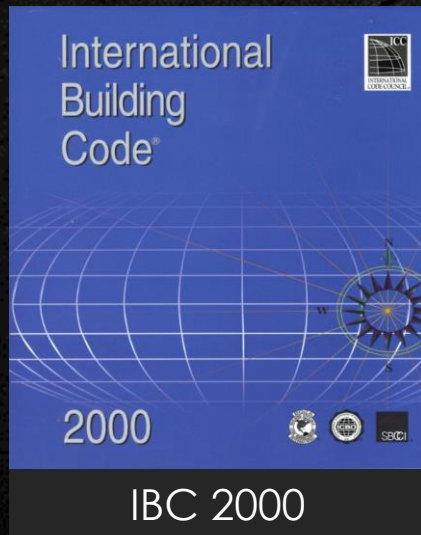
NSCP 2015



# 2000 Onwards

Meanwhile, international practice also advanced at a rapid pace.

Seismic design is now based on mapped spectral accelerations.



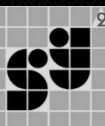
# 2000 Onwards

Building codes are intended to establish minimum requirements for providing safety to life and property from hazards.

- Specification of a *Prescriptive* Criteria
  - Acceptable materials for construction
  - Approved structural and nonstructural systems
  - Required minimum levels of strength and stiffness
  - Details of how a building is to be put together
- A certain level of performance is *implied* without assessing the actual performance capability of the building

*Code Compliance or Achievement of Performance Objective?*

*Is code compliance enough?*





# 2000 Onwards

The development of seismic design criteria is a continuous process.

1933 Long Beach Earthquake

1971 San Fernando Earthquake

1994 Northridge Earthquake  
buildings

Minimum levels of lateral strength

Ability to deform without failure (Ductility)

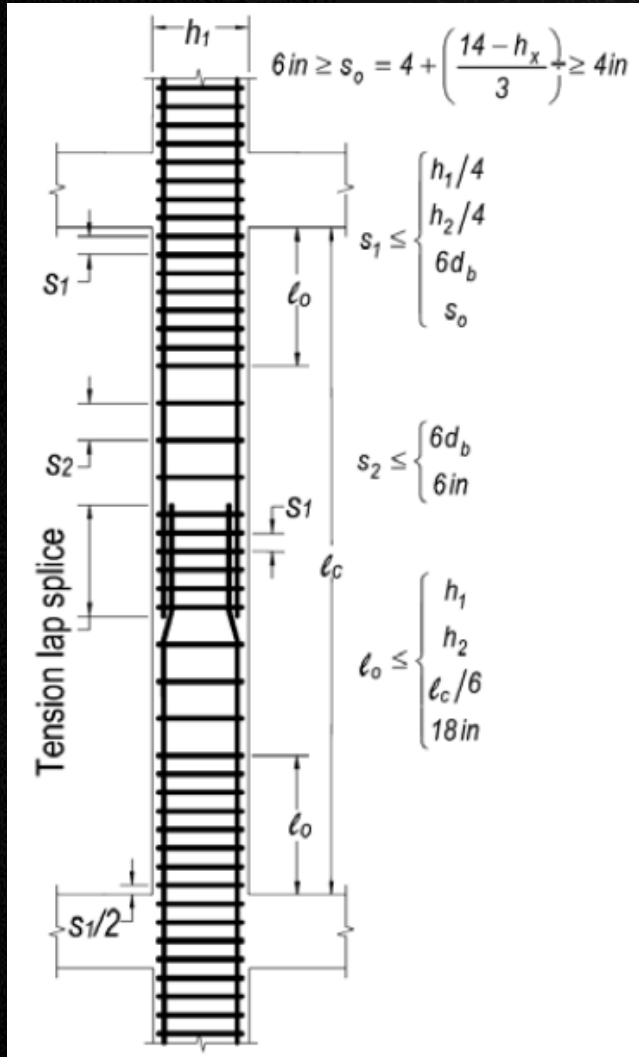
Damage can occur in code-compliant



7.1M Mexico Earthquake

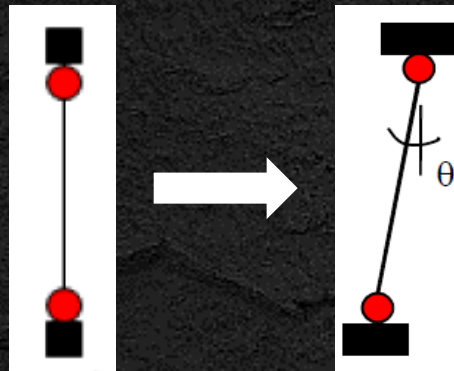
# 2000 Onwards

## Confining Reinforcement of Columns



## The Code Approach for Columns of Special Moment Frames

Specify ductility requirements at yield zones.



Specify maximum spacing of ties, say 100.

*Intent is confinement for adequate ductility and hysteretic behaviour.*

If we provide 125, our design is not code compliant.

*Does this mean failure?*

*Will this specific column yield?*

*Do we need to provide the same ductility for all columns?*

# 2000 Onwards

Performance is not guaranteed by code conformance.

Performance can be achieved without code conformance.

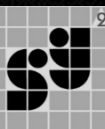
## 208.1.1 Purpose

The purpose of the succeeding earthquake provisions is primarily to design seismic-resistant structures to safeguard against major structural damage that may lead to loss of life and property. These provisions are not intended to assure zero-damage to structures nor maintain their functionality after a severe earthquake.

Explicit verification of performance is a must.

M9 Great Tohoku  
EQ

Changes in the state of knowledge and development of new analytical tools should translate to evolution in structural design practice.



# 2000 Onwards

## Alternative Systems and Methods of Analysis

### 101.4 Alternative Systems

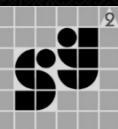
The provisions of this code are not intended to prevent the use of any material, alternate design or method of construction not specifically prescribed by this code, provided any alternate has been permitted and its use authorized by the Building Official (see Section 102).

Sponsors of any system of design or construction not within the scope of this code, the adequacy of which had been shown by successful use and by analysis and test, shall have the right to present the data on which their design is based to the Building Official or to a board of examiners appointed by the Building Official or the project owner/developer. This board shall be composed of competent structural engineers and shall have authority to investigate the data so submitted, to require tests if any, and to formulate rules governing design and construction of such systems to meet the intent of this code. These rules, when approved and promulgated by the Building Official, shall be of the same force and effect as the provisions of this code.

### 104.3 Analysis

Any system or method of construction to be used shall be based on a rational analysis in accordance with well-established principles of mechanics that take in to account equilibrium, general stability, geometric compatibility and both short-term and long-term material properties.

Members that tend to accumulate residual deformations under repeated service loads shall have included in their analysis the added eccentricities expected to occur during their service life. Such analysis shall result in a system that provides a complete load path capable of transferring all loads and forces from their point of origin to the load-resisting elements. The analysis shall include, but not be limited to, the provisions of Sections 104.3.1 through 104.3.3.



# 2000 Onwards

## Alternative Systems and Methods of Analysis

No provisions for other structural systems appropriate for tall buildings

### 208.4.8.4 Alternative Procedures

#### 208.4.8.4.1 General

Alternative lateral-force procedures using rational analyses based on well-established principles of mechanics may be used in lieu of those prescribed in these provisions.

#### 208.4.8.4.2 Seismic Isolation

Seismic isolation, energy dissipation and damping systems may be used in the analysis and design of structures when approved by the building official and when special detailing is used to provide results equivalent to those obtained by the use of conventional structural systems.



Outriggers



Diagrids



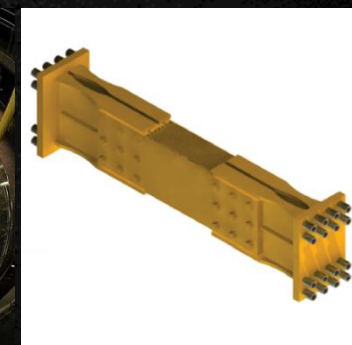
Base Isolators



Viscous Dampers



Tuned-mass Dampers



Viscoelastic Dampers

# 2000 Onwards

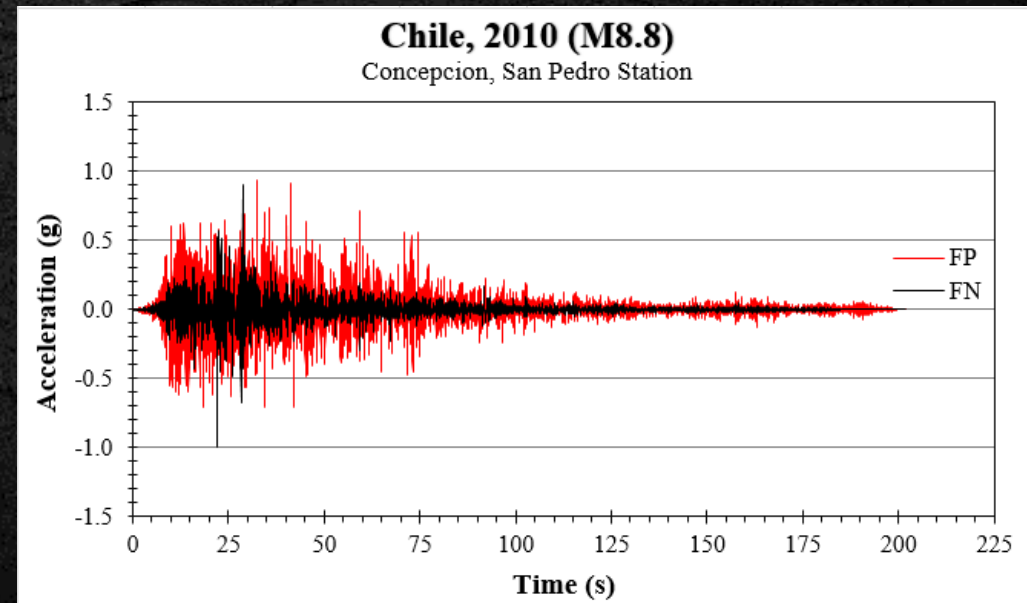
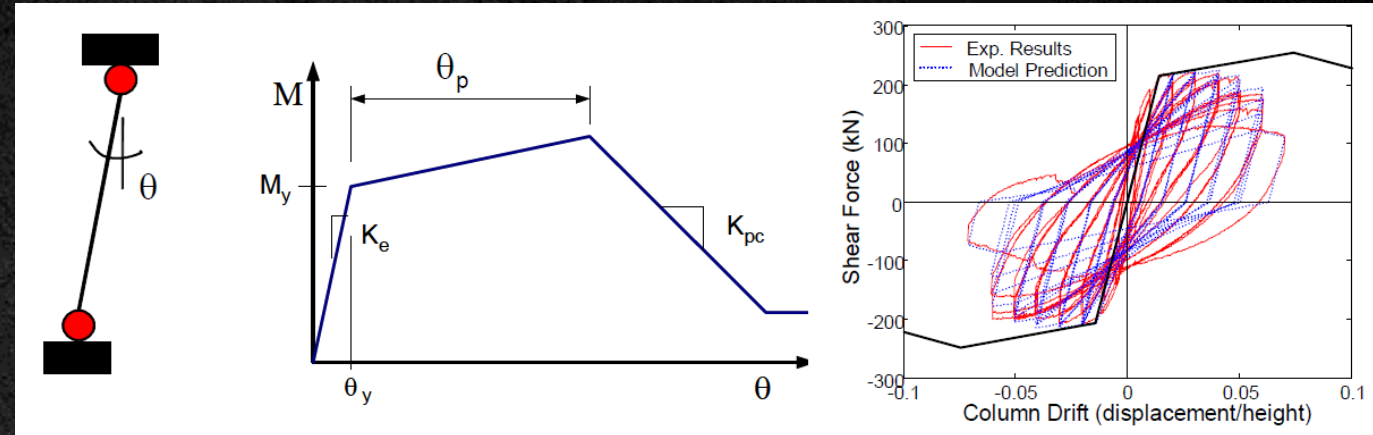
## Alternative Systems and Methods of Analysis

### 208.5.3.6.3.1 Nonlinear Time History Analysis

Nonlinear time history analysis shall meet the requirements of Section 208.4.4, and the time histories shall be developed and results determined in accordance with the requirements of Section 208.5.3.6.1. Capacities and characteristics of nonlinear elements shall be modeled consistent with test data or substantiated analysis, considering Importance Factor. The maximum inelastic response displacement shall not be reduced and shall comply with Section 208.6.5.

Time-history analysis provides an evaluation of dynamic structural response under loading which may vary according to the specified time function.

The nonlinearity comes from material and geometric nonlinearity, including P-delta and large-displacement effects.



# 2000 Onwards

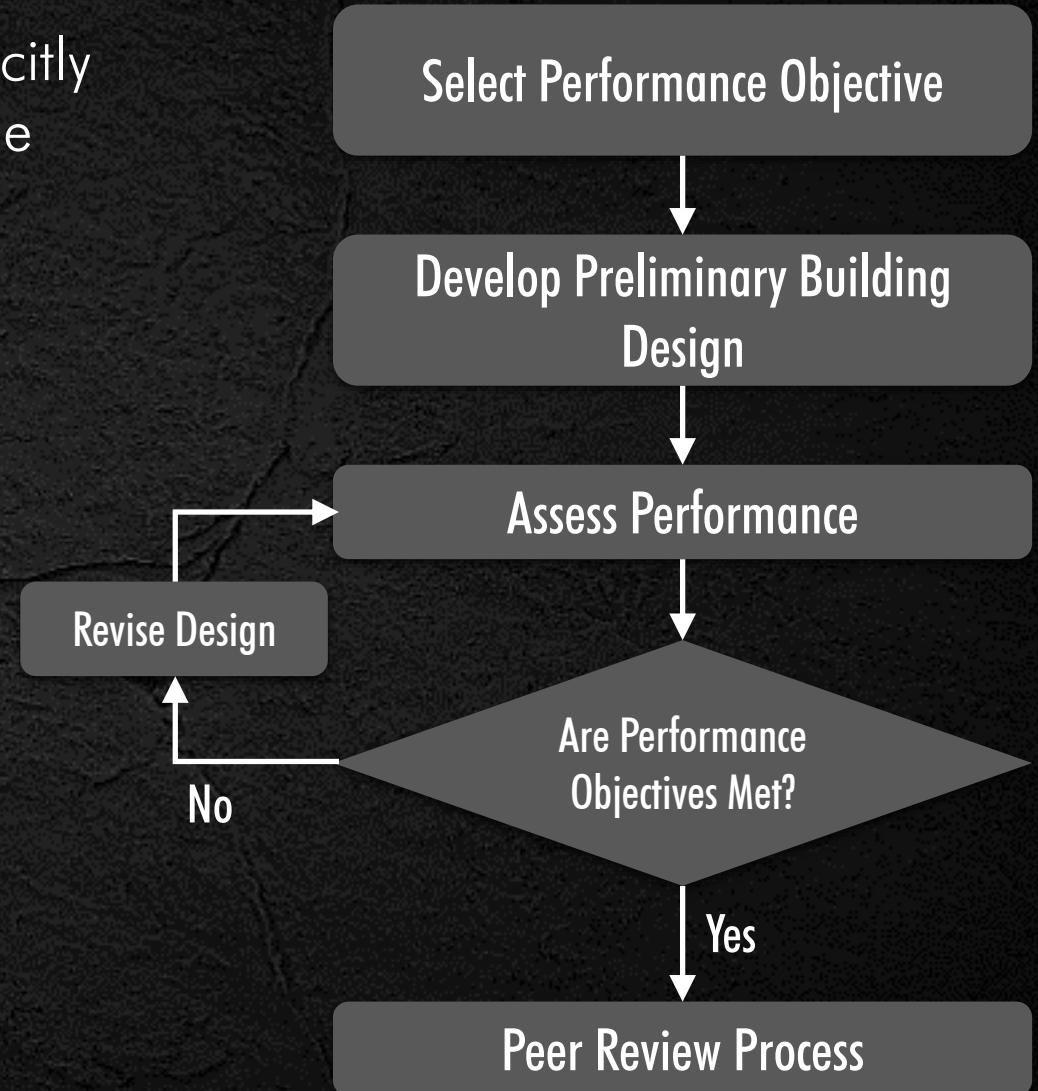
The performance-based seismic design process explicitly evaluates how a building is likely to perform given the potential hazard it is likely to experience.

Assess performance of buildings under multiple seismic events

Provide higher performance for critical structures than intended by the building code.

Deliver standard performance at a reduced cost

Can be applied to both new and existing buildings



# 2000 Onwards

For Risk Category II tall buildings, two performance objectives are investigated explicitly:

Level of Earthquake	Performance Objective
<b>Service Level Earthquake (SLE):</b> 50% probability of exceedance in 30 years (43-year return period)	Negligible damage in once-in-a-lifetime earthquake. Structure to remain essentially elastic with minor damage to structural and non-structural elements.
<b>Maximum Considered Earthquake (MCE):</b> 2% probability of exceedance in 50 years (2,475-year return period)	Response without loss gravity-load-carrying capacity; without inelastic straining of important lateral-force-resisting elements; and without experiencing excessive permanent lateral drift or development of global structural instability.



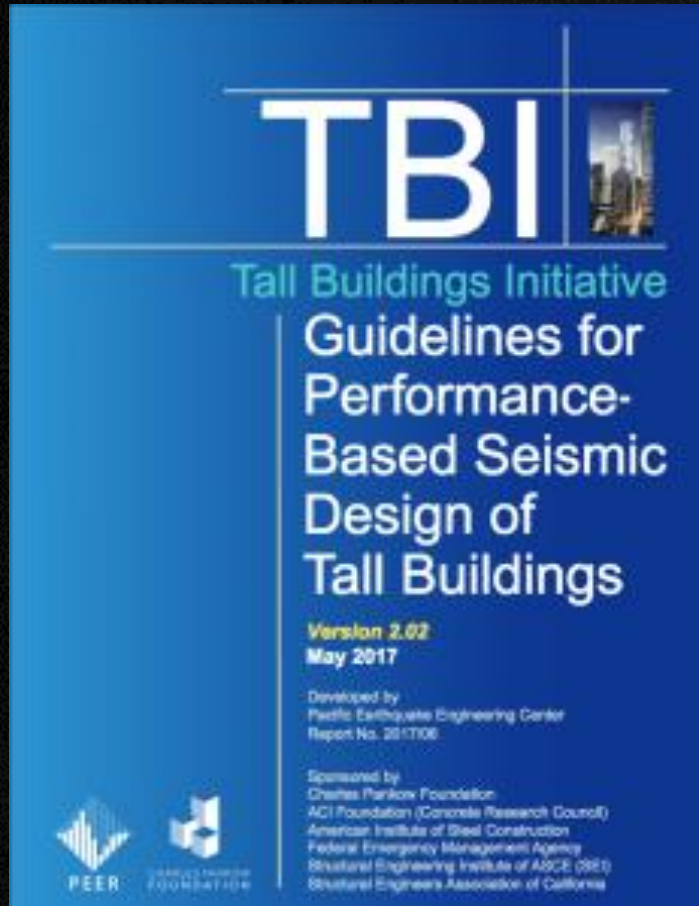


# 2000 Onwards

## Standard Structural Performance Objectives

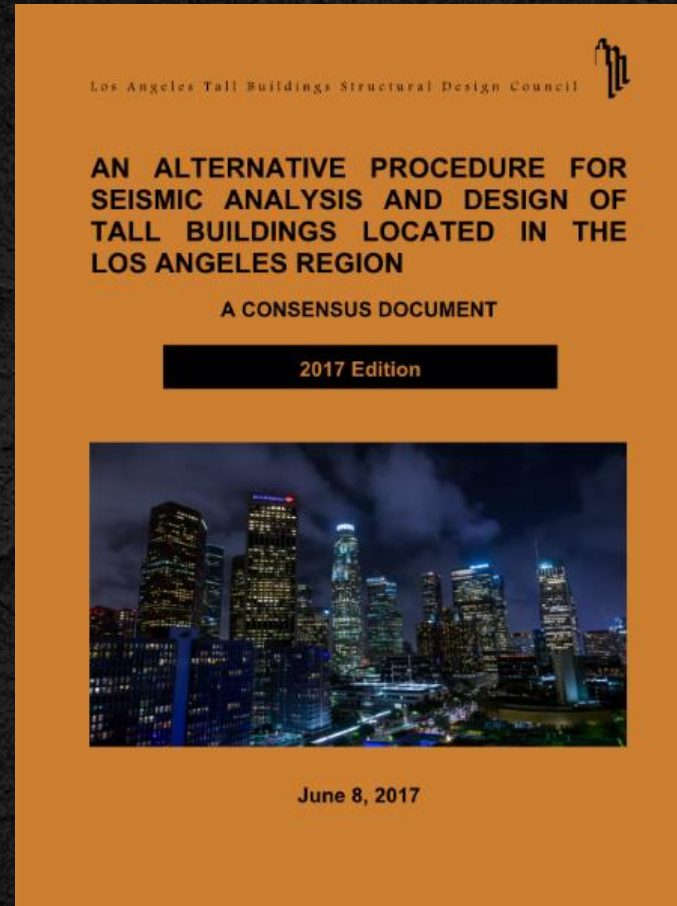
Earthquake Level	Performance Level			
	Operational	Immediate Occupancy	Life Safety	Collapse Prevention
Frequent (43 years)	Basic Objective	Unacceptable	Unacceptable	Unacceptable
Occasional (72 years)	Essential Objective	Basic Objective	Unacceptable	Unacceptable
Rare (475 years)	Safety-Critical Objective	Essential Objective	Basic Objective	Unacceptable
Very Rare (2475 years)	Not Feasible	Safety-Critical Objective	Essential Objective	Basic Objective

# 2000 Onwards



**TBI**  
Tall Buildings Initiative  
**Guidelines for Performance-Based Seismic Design of Tall Buildings**  
Version 2.02  
May 2017  
Developed by  
Pacific Earthquake Engineering Center  
Report No. 2017-06  
Sponsored by  
Charles Pankov Foundation  
ACI Foundation (Concrete Research Council)  
American Institute of Steel Construction  
Federal Emergency Management Agency  
Structural Engineering Institute of ASCE (SEI)  
Structural Engineers Association of California

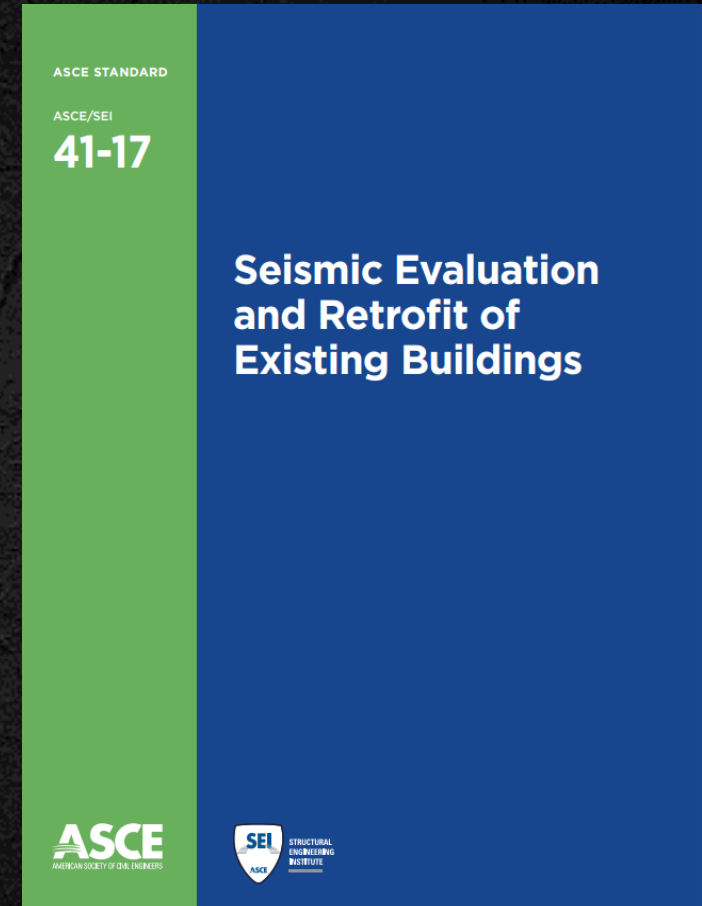
PEER TBI



Los Angeles Tall Buildings Structural Design Council

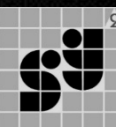
**AN ALTERNATIVE PROCEDURE FOR SEISMIC ANALYSIS AND DESIGN OF TALL BUILDINGS LOCATED IN THE LOS ANGELES REGION**  
A CONSENSUS DOCUMENT  
2017 Edition  
June 8, 2017

LATBSDC



ASCE STANDARD  
ASCE/SEI  
**41-17**  
**Seismic Evaluation and Retrofit of Existing Buildings**  
ASCE  
AMERICAN SOCIETY OF CIVIL ENGINEERS  
SEI  
STRUCTURAL ENGINEERING INSTITUTE

ASCE 41



# 2000 Onwards

## Why PBD?

Integrating Aesthetics, Integrity, and Economy

Aesthetics

*The art that exist within the built environment*

Integrity

*The backbone of structural engineering  
Stability, Strength, and Stiffness*

Economy

*Enhancing the quality without sacrificing cost*



# 2000 Onwards

Why PBD?

Buildings are getting taller.



Stratford Residences



Trump Tower

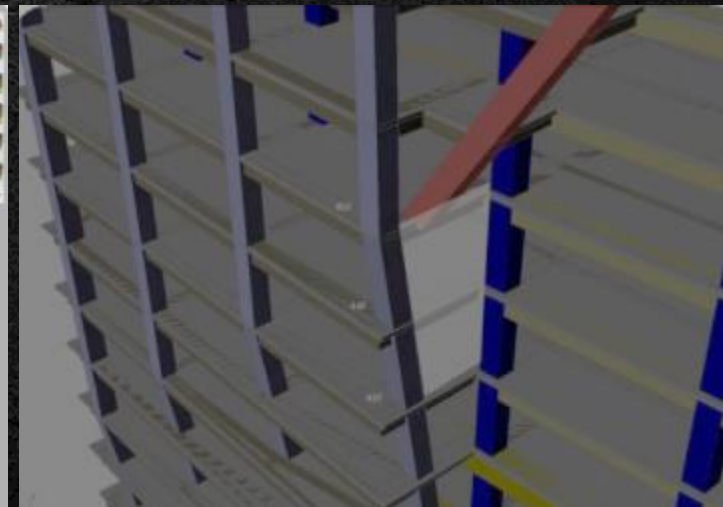
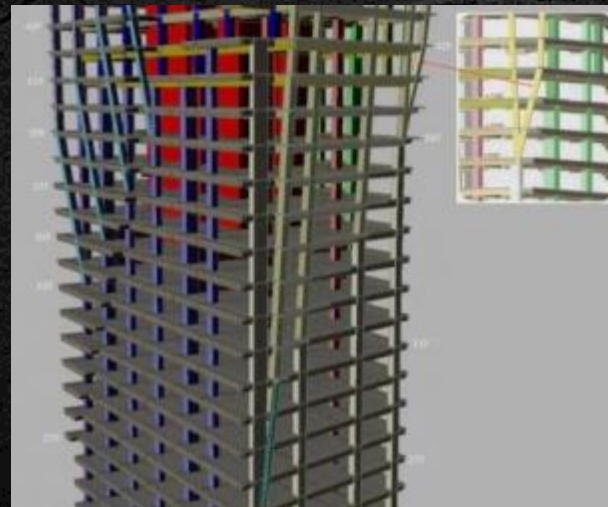
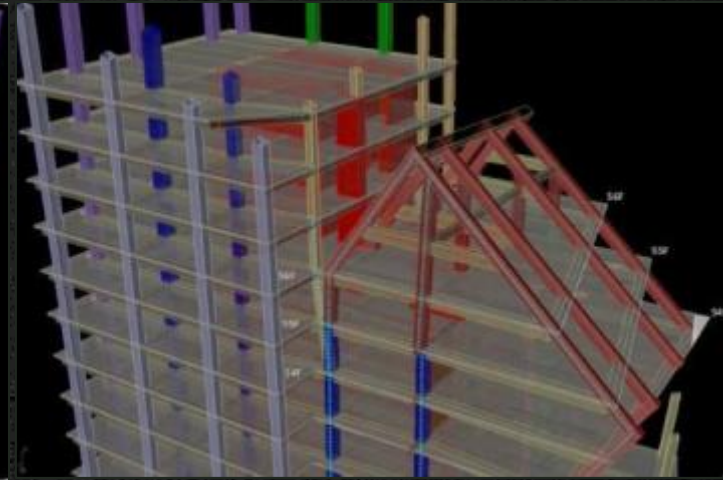
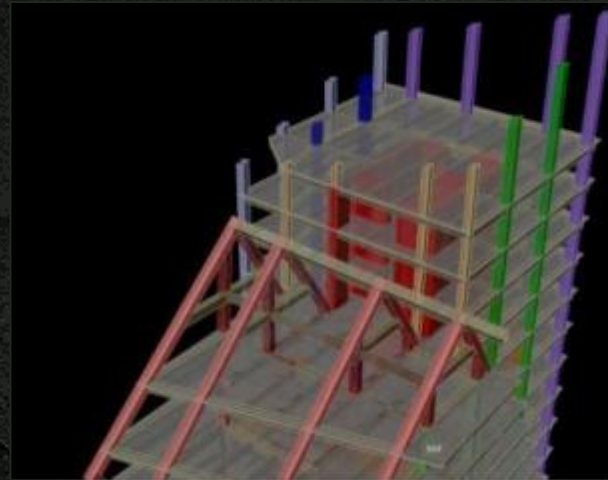


Gramercy Residences

# 2000 Onwards

Why PBD?

Buildings are getting more complicated.



# 2000 Onwards

Why PBD?

Buildings are getting complex.



The Proscenium



Ayala Triangle Gardens



Park Central Towers

# 2000 Onwards

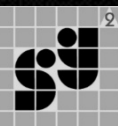
## 80 PBD Projects

Beacon Tower-2  
Beacon Tower-3  
Park Terraces Tower-1  
Park Terraces Tower-2  
Gramercy Residences  
Milano Residences  
Niagara Tower  
Anchor Skysuites  
One Shangri-la Place  
Admiral Baysuites  
Stratford Residences  
Blue Residences  
Ascott Residences  
Discovery Primea  
Edades Tower  
One World Place Building  
BDO Tower  
M Place @ Ortigas  
Shang Salcedo Place  
Knightsbridge Residences

Green Residences  
Sequoia at Two Serendra  
The Suites  
Kirov Tower  
Sakura Tower  
Lorraine Tower  
Lincoln Tower  
Proscenium Tower  
Dettifoss Tower  
Trump Tower  
Sutherland Tower  
Garden Tower-1  
Garden Tower-2  
Spire Tower  
Shangri-la at the Fort  
Livingstone Tower  
Two Roxas Triangle  
Iguazu Tower  
The Rise  
East Gallery Place

128 Nivel Hills Tower 1  
128 Nivel Hills Tower 2  
128 Nivel Hills Tower 3  
Air Residences  
Anchor Grandsuites  
Sonata Premier Residential Tower  
Sonata Premier Hotel Tower  
Olive Residences  
Ayala Triangle Garden Hotel  
Ayala Triangle Garden Office  
Fame Residences  
Portico Tower 1  
Menarco Tower  
West Gallery Place  
Nova Manila  
Emerald North Tower  
Emerald South Tower  
Two Serendra Building  
Azure North  
Mandani Bay Ph. 1 Tower 1

Mandani Bay Ph. 1 Tower 2  
STRC Apartment Ridge  
The Connor  
Glas Office Development  
Mandani Bay Ph. 2 Tower 1  
Mandani Bay Ph. 2 Tower 2  
Mandani Bay Ph. 2 Tower 3  
Mandani Bay Ph. 2 Tower 4  
Sixth Tower at Proscenium  
Proscenium Performance Hall  
Shang Residences  
Landmark BGC Office Tower  
NAIA Terminal I  
GSIS Building  
Alphaland Makati Tower  
Vertis North  
Estancia  
Frontera Verde  
Benpres  
Innoland Makati Tower



# 2000 Onwards

- 179 meters
- 53 storeys
- Designed using displacement-based design approach
- Utilized Tuned Liquid Column Sloshing Dampers to resist wind

Pacific Plaza Twin Towers





# 2000 Onwards

Designed more 250m using the Performance-Based Design approach

Shangri-La at the Fort



Trump Tower



Gramercy Residences

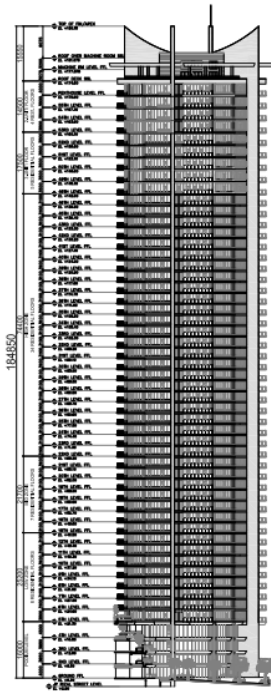


# 2000 Onwards

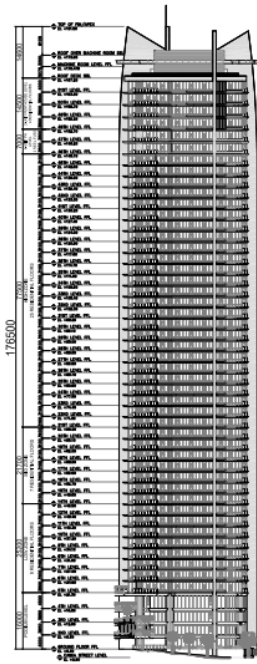
The Proscenium



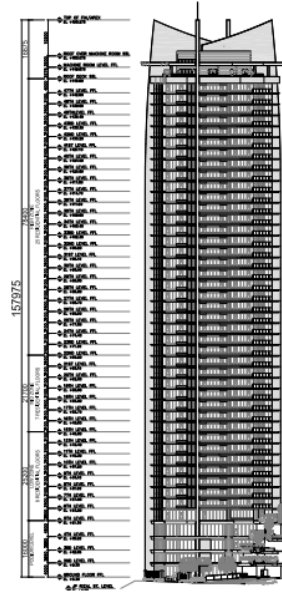
**KIROV**  
 52 PHYSICAL FLOORS  
 (GF- LAST REST. FLOOR)  
 5 PODIUM LEVELS  
 NO 13TH, 14TH, 44TH & 47TH  
 PH FLOOR (56TH)  
 (LEVEL NAME OF LAST REST. FLOOR)  
 1120 SQM (TYP)  
 5 ELEVATORS  
 4 PE + 1 SE/PE  
 18 (6/6) (AVS)  
 APEX: 190.15M ANSL  
 BUILDING HEIGHT (GF-APEX): 164.85M  
 PH HEIGHT: 15.20M  
 4BR (48 SQM) 6  
 3BR (22-33 SQM) 172  
 TOTAL: 178 UNITS



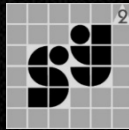
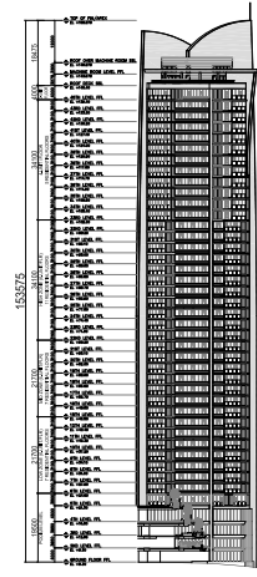
**SAKURA**  
 50 PHYSICAL FLOORS  
 (GF- LAST REST. FLOOR)  
 5 PODIUM LEVELS  
 NO 13TH  
 51ST FLOOR  
 (LEVEL NAME OF LAST REST. FLOOR)  
 1056 SQM (TYP)  
 5 ELEVATORS  
 4 PE + 1 SE/PE  
 20 (6/6) (AVS)  
 APEX: 161.20M ANSL  
 BUILDING HEIGHT (GF-APEX): 170.50M  
 PH HEIGHT: 14.00M  
 4BR (43-43 SQM) 8  
 3BR (19-20 SQM) 124  
 2BR (11-12 SQM) 80  
 TOTAL: 212 UNITS



**LINCOLN**  
 44 PHYSICAL FLOORS  
 (GF- LAST REST. FLOOR)  
 5 PODIUM LEVELS  
 NO 13TH, 14TH & 44TH  
 47TH FLOOR  
 (LEVEL NAME OF LAST REST. FLOOR)  
 1074 SQM (TYP)  
 5 ELEVATORS  
 4 PE + 1 SE/PE  
 30 (6/6) (AVS)  
 APEX: 163.275M ANSL  
 BUILDING HEIGHT (GF-APEX): 190.47M  
 PH HEIGHT: 16.97M  
 2BR (9-11 SQM) 159  
 1BR (5-6 SQM) 210  
 ST (3-4 SQM) 130  
 TOTAL: 499 UNITS



**LORRAINE**  
 42 PHYSICAL FLOORS  
 (GF- LAST REST. FLOOR)  
 5 PODIUM LEVELS  
 NO 13TH, 14TH & 44TH  
 45TH FLOOR  
 (LEVEL NAME OF LAST REST. FLOOR)  
 949 SQM (TYP)  
 5 ELEVATORS  
 4 PE + 1 SE/PE  
 15 (5/5) (AVS)  
 APEX: 158.875M ANSL  
 BUILDING HEIGHT (GF-APEX): 153.67M  
 PH HEIGHT: 15.47M  
 4BR (29-30 SQM) 2  
 3BR (12-20 SQM) 132  
 2BR (9-12 SQM) 74  
 1BR (5 SQM) 1  
 TOTAL: 109 UNITS



# 2000 Onwards

The Proscenium



## THE PROSCENIUM RESIDENCES

54 PHYSICAL FLOORS  
(GF-LAST REST. FLOOR)

4 PODIUM LEVELS

NO 3RD, 13TH, 14TH & 44TH

59TH FLOOR

(LEVEL NAME OF LAST FLOOR)

1262 SQM +13 SQM (SCENIC ELEV.)

4 ROOF DECK LEVELS

2 LEVEL RESTAURANT AND HELIPAD

4 ELEVATORS

6 PE + 1 RESE + 1 SCENE

25 WALKWAY (20 WALKWAY @ 2.5M WALKWAY)

APEX: 302.718M ANGL

BUILDING HEIGHT (GF-APEX): 311.03M

FIN HEIGHT: 16.03M

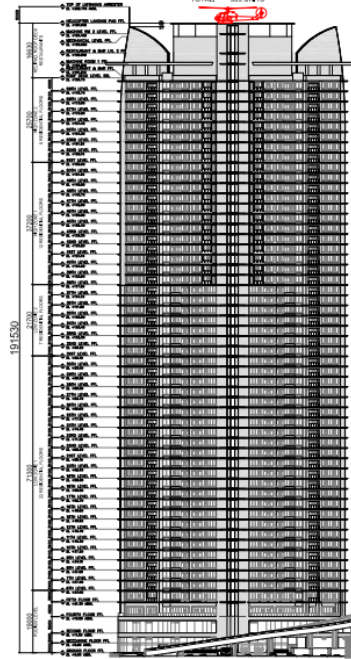
3BR (147 SQM) 100

2BR (94-91 SQM) 301

1BR (60-62 SQM) 60

ST (10-30 SQM) 100

TOTAL: 963 UNITS



### SIXTH TOWER

20 PHYSICAL FLOORS  
(GF-LAST REST. FLOOR)

1 PODIUM LEVEL

NO 4TH, 13TH & 22ND

23RD FLOOR

(LEVEL NAME OF LAST OFFICE FLOOR)

724 SQM (TYP)

4 ELEVATORS

2 PE + 1 RESE

35.4 WALKWAY

APEX: 317.708M ANGL

BUILDING HEIGHT (GF-APEX): 311.03M

FIN HEIGHT: 16.03M

18-30 + 201 SQM @ 2 UNITS/FLOOR

387 + 548 SQM @ SINGLE TENANT/FLOOR

### PERFORMANCE HALL

7 PHYSICAL FLOORS

1 MUSEUM/RETAIL FLOOR

6 THEATER FLOORS

4 ELEVATORS

2 FOH + 1 BOH + 1 FREEHOLD

APEX: 41.03M ANGL

BUILDING HEIGHT (GF-APEX): 33.03M

FIFTH FLOOR: BALCONY

FOURTH FLOOR: ORCHESTRA

THIRD FLOOR: STAGE LEVEL

FIFTH FLOOR: BALCONY

FOURTH FLOOR: ORCHESTRA

THIRD FLOOR: STAGE LEVEL

SEAT COUNT

TOTAL: 756 SEATS

ORCHESTRA: 588 SEATS

FOH & BOH: 8 SEATS

BALCONY: 160 SEATS

TOTAL: 688 SEATS

ORCHESTRA: 300 SEATS

FOH & BOH: 200 SEATS

FOH & BOH: 8 SEATS

BALCONY: 100 SEATS

### NORTH GARDEN VILLAS

5 PHYSICAL FLOORS  
(GF-LAST REST. FLOOR)

3 OFFICE LEVELS

3 RESIDENTIAL LEVELS

APEX: 32.00M ANGL

BUILDING HEIGHT (GF-APEX): 27.30M

FIN HEIGHT: 4.00M

4BR (438 SQM) 3

TOTAL: 3 UNITS



### EAST/WEST GARDEN VILLAS

5 PHYSICAL FLOORS  
(GF-LAST REST. FLOOR)

3 OFFICE LEVELS

3 RESIDENTIAL LEVELS

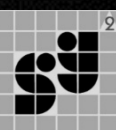
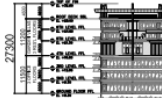
APEX: 32.00M ANGL

BUILDING HEIGHT (GF-APEX): 27.30M

FIN HEIGHT: 4.00M

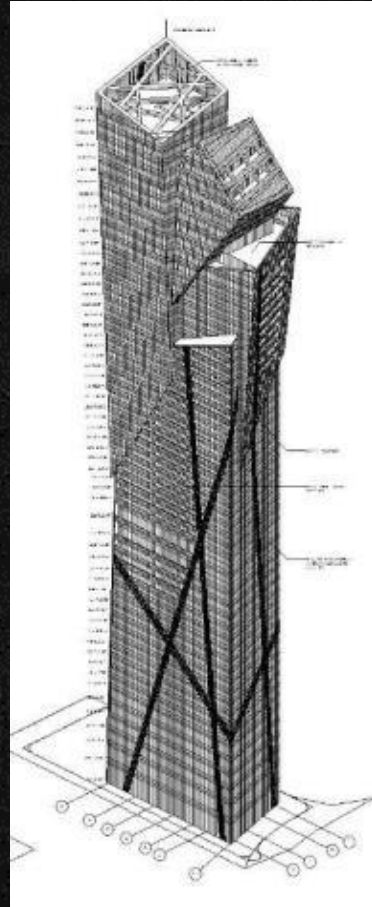
3BR (312 SQM) 2

TOTAL: 2 UNITS

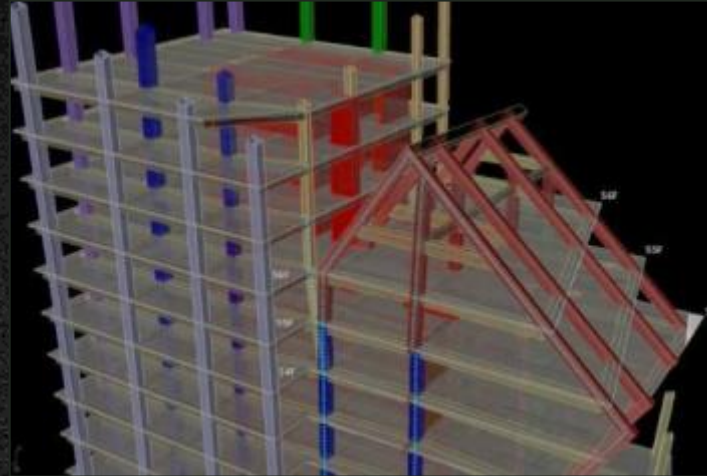


# 2000 Onwards

The Spire

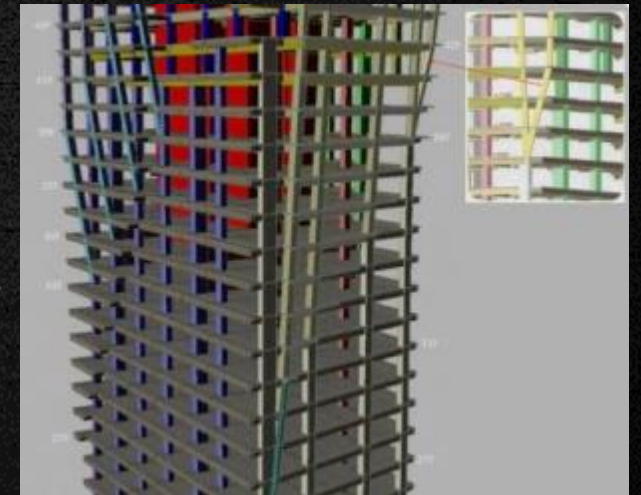


Cladding Axometric



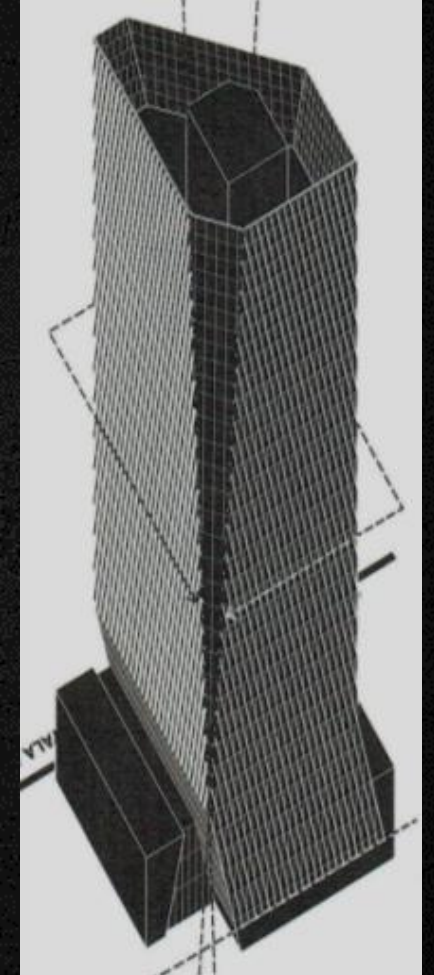
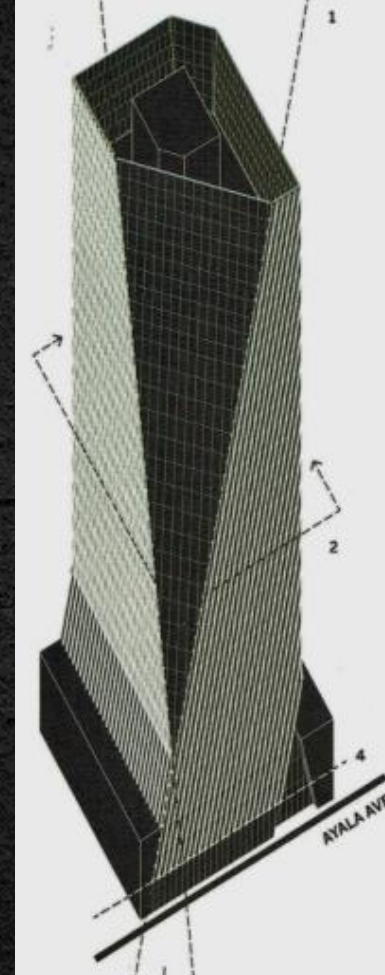
**Vertical Irregularities**  
Asymmetrical Framing  
System of the Crown

**Vertical Irregularities**  
Mushrooming Column  
Diagonally Outward at  
Different Levels



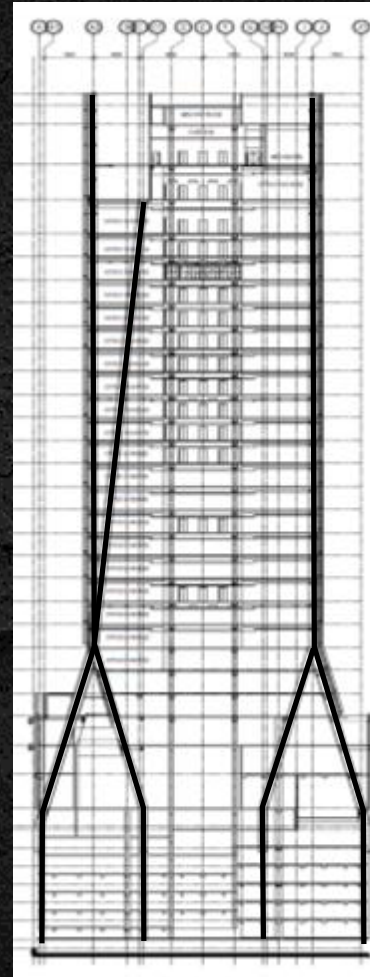
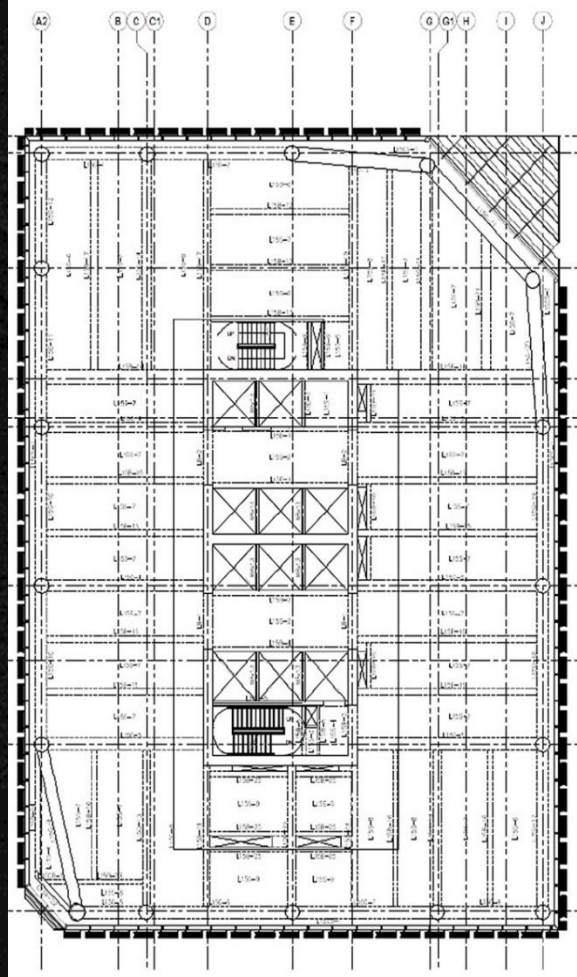
# 2000 Onwards

Nova Manila



# 2000 Onwards

Nova Manila



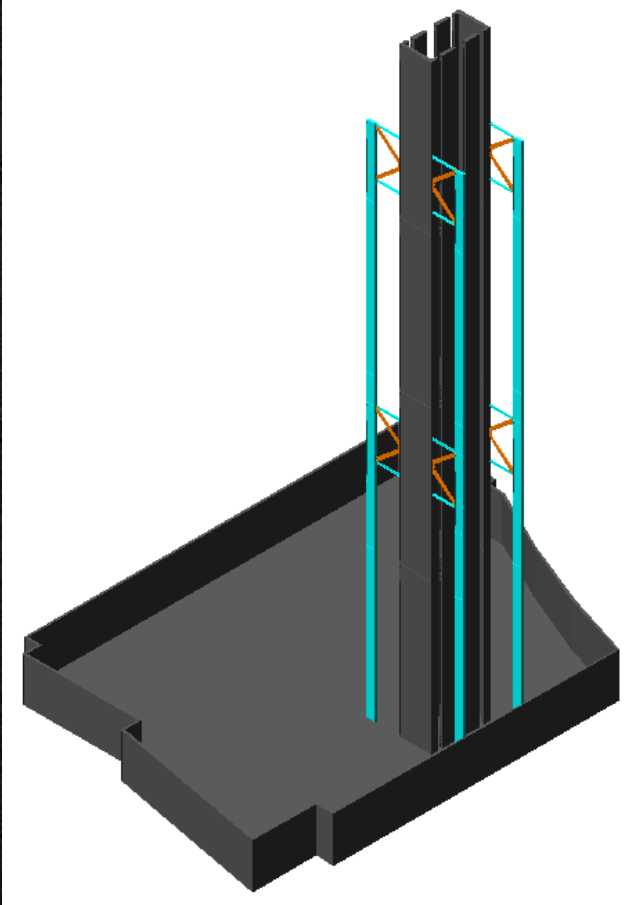
# 2000 Onwards

Started the application of energy-dissipating devices such as BRBs

Park Terraces

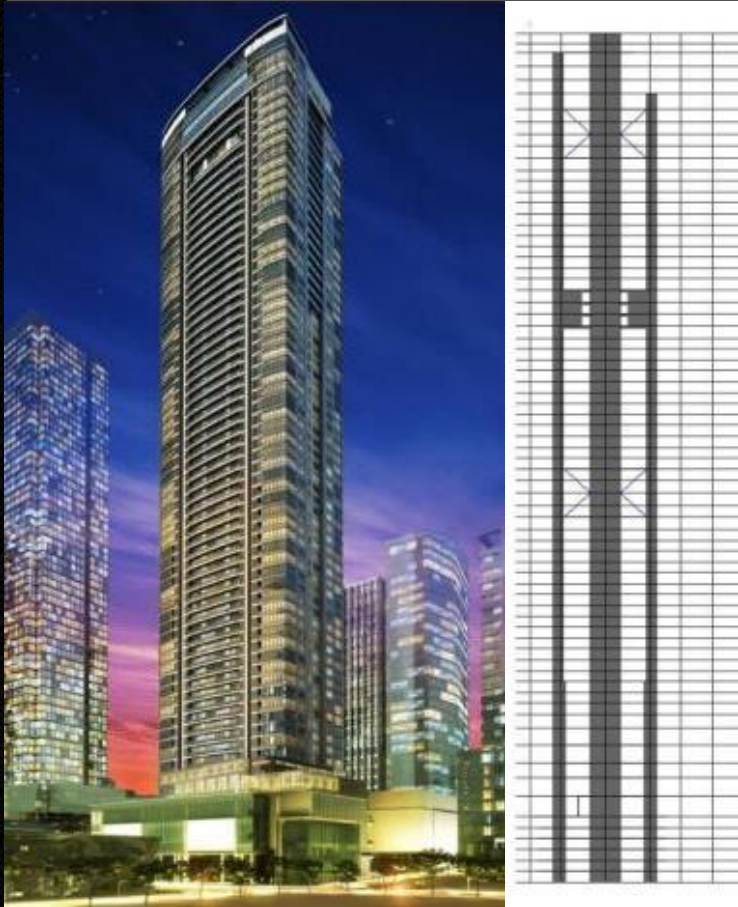


First Application of Buckling-Restrained Brace (BRB) in the



# 2000 Onwards

The Suites



Sonata Premier



East Gallery Place





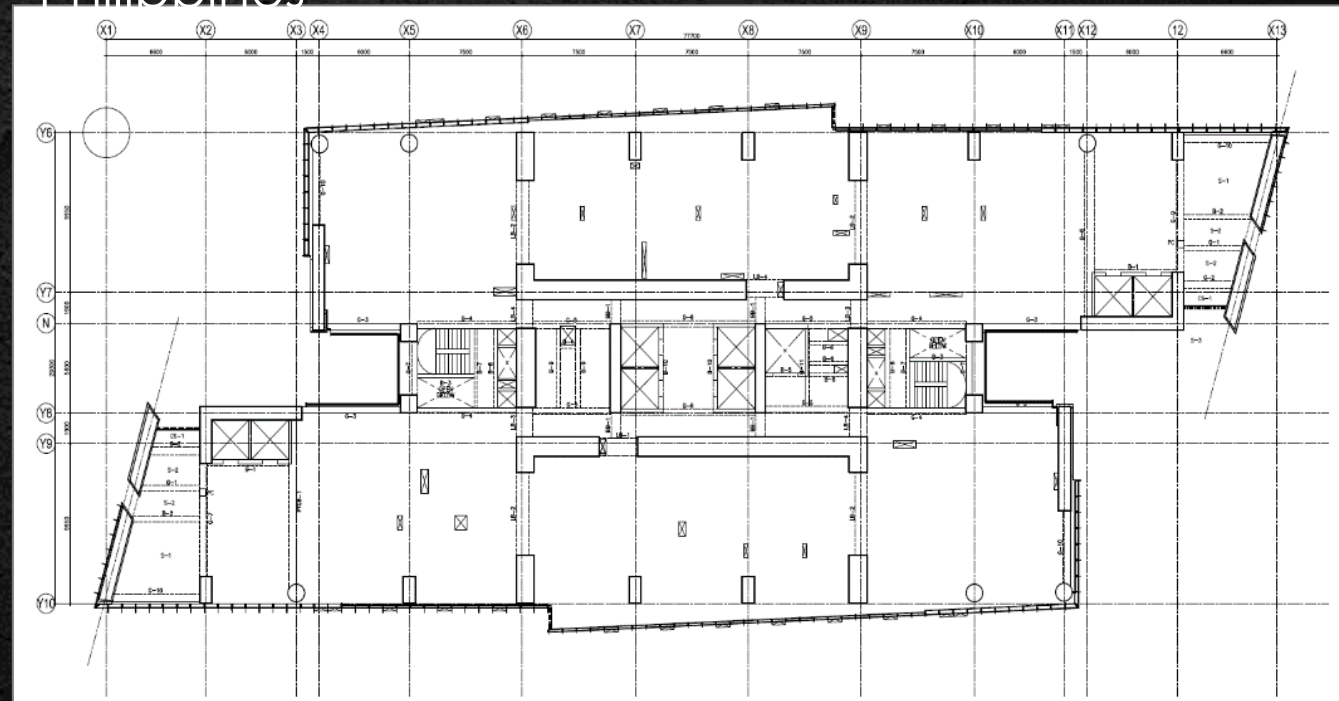
# 2000 Onwards

... until Viscoelastic Coupling Damper (VCD) was introduced in 2016.

Park Central Towers  
(Emerald)



- Consists of Two High-Rise Residential Towers
- South Tower (70-story, 235m) & North Tower (58-story, 197m)
- First application of Viscoelastic (VE) dampers in the Philippines



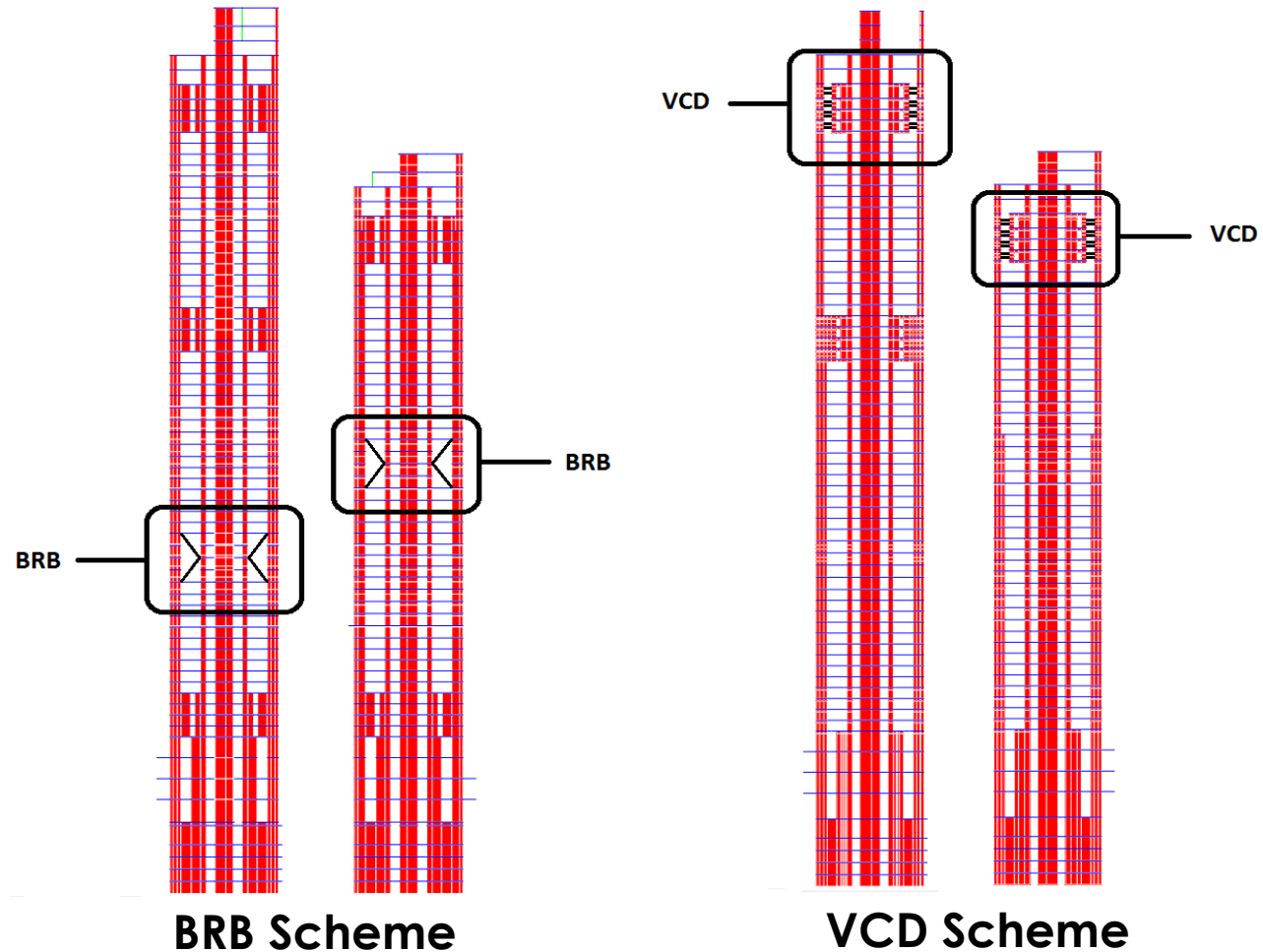
# 2000 Onwards

... until Viscoelastic Coupling Damper (VCD) was introduced in 2016.

Park Central Towers  
(Emerald)



First application of viscoelastic (VE) damper in the



# 2000 Onwards

... until Viscoelastic Coupling Damper (VCD) was introduced in 2016.

Park Central Towers  
(Emerald)

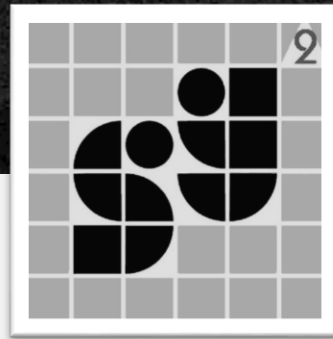


# Look Ahead

- The Philippines has the same earthquake risk as California, Japan, New Zealand, Haiti, etc. However, our practice of earthquake engineering is not at par with theirs.
- We believe that we should not be bounded by the limitations of the codes.
- We believe that we should take advantage of available powerful hardware and software in doing alternative approaches to ensure better performing buildings against earthquakes.
- Code compliance is not enough.
- Innovative approach is a must.



**Challenges Inspire Innovations.**



**THANK YOU**

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