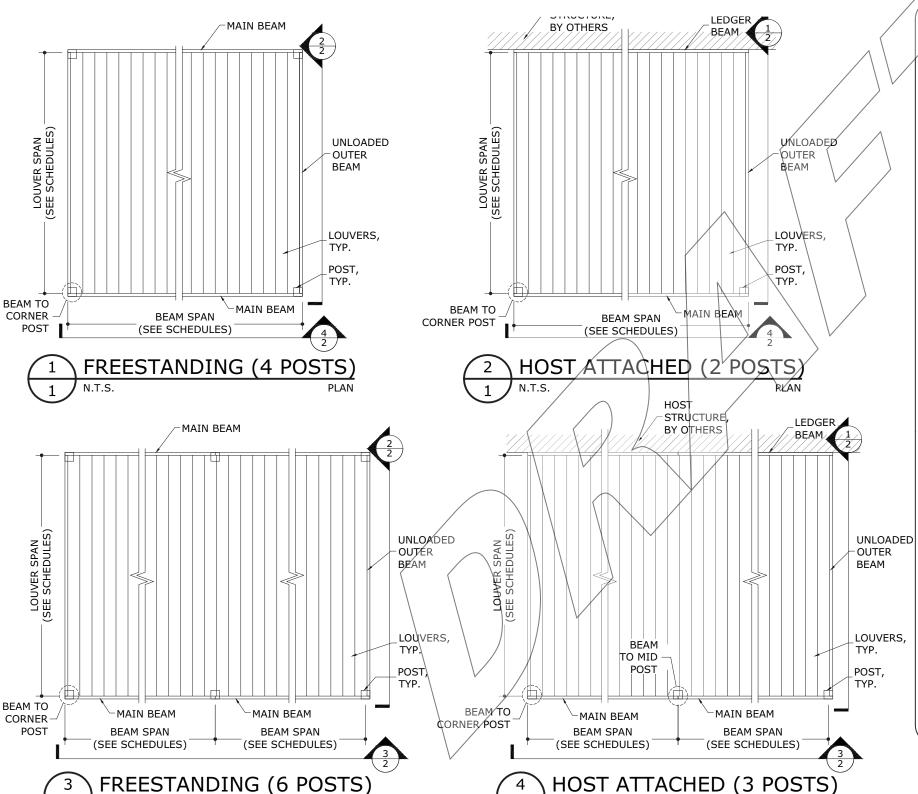
# SUNDANCE LOUVERED ROOFS, LLC ALUMINUM LOUVERED ROOF MASTER PLAN SHEET

ANY LOUVERED ROOF SYSTEM COMPONENTS/DETAILS NOT LISTED HEREIN SHALL BE PER MANUFACTURER'S RECOMMENDATIONS

NOTE: SINGLE-BAY AND DOUBLE-BAY PLAN VIEWS ARE SHOWN ON THIS PAGE, FOR MULTI-BAY OPTIONS, SEE PAGE 3.

OVERHANG OPTIONS



REOUIRED ALLOWABLE DESIGN PRESSURES USED IN COORDINATION WITH THIS DOCUMENT SHALL BE CALCULATED SEPARATELY PER THE STRUCTURAL REQUIREMENTS OF THE 2012, 2015 OR 2018 INTERNATIONAL BUILDING CODES & ASCE 7-16 OR ASCE 7-10 AS APPLICABLE USING ALLOWABLE STRESS DESIGN METHODOLOGY

THIS DOCUMENT SHALL NOT BE USED OR REPRODUCED WITHOUT THE ORIGINAL SIGNATURE & RAISED SEAL OF FRANK BENNARDO, P.E. ALTERATIONS, ADDITIONS OR OTHER MARKINGS TO THIS DOCUMENT ARE NOT PERMITTED AND INVALIDATE OUR CERTIFICATION. PHOTOCOPIES AND UNSEALED DOCUMENTS ARE NOT TO BE ACCEPTED.

3. NO 33-1/3% INCREASE IN ALLOWABLE STRESS HAS BEEN USED IN THE DESIGN OF THIS SYSTEM. WIND LOAD DURATION FACTOR Cd=1.6 HAS BEEN USED FOR WOOD ANCHOR DESIGN.

THE ARCHITECT/ENGINEER OF RECORD FOR THE PROJECT SUPERSTRUCTURE WITH WHICH THIS DESIGN IS USED SHALL BE RESPONSIBLE FOR THE INTEGRITY OF ALL SUPPORTING

THE SYSTEM DETAILED HEREIN IS GENERIC AND DOES NOT PROVIDE INFORMATION FOR A SPECIFIC SITE. FOR SITE CONDITIONS DIFFERENT FROM THE CONDITIONS DETAILED HEREIN, A LICENSED ENGINEER OR REGISTERED ARCHITECT SHALL PREPARE SITE SPECIFIC DOCUMENTS FOR USE IN CONJUNCTION WITH THIS DOCUMENT.

6. THE CONTRACTOR SHALL CAREFULLY CONSIDER POSSIBLE IMPOSING LOADS ON ROOF, INCLUDING BUT NOT LIMITED TO ANY CONCENTRATED LOADS WHICH MAY JUSTIFY GREATER O DESIGN CRITERIA. THIS ADDITIONAL ROOF LOAD CRITERIA SHALL BE PROPERLY ANALYZED BY A LICENSED ENGINEER OR REGISTERED ARCHITECT

CERTIFICATION.

8. LINEAR INTERPOLATION FOR MEMBER SPANS MAY BE USED FOR ALL LOUVER AND BEAM SCHEDULES.

9. ALL FASTENERS TO BE #14 OR GREATER SAE GRADE 5 UNLESS NOTED OTHERWISE. ASTENERS SHALL BE CADMIUM-PLATED OR OTHERWISE CORROSION-RESISTANT MATERIAL AND SHALL COMPLY WITH "SPECIFICATIONS FOR ALUMINUM STRUCTURES" BY THE ALUMINUM

DISTANCE AND 2.5xDIAMETER MIN. SPACING TO ADJACENT ANCHORS, UNLESS NOTED

RECOMMENDATIONS. MINIMUM EMBEDMENT SHALL BE AS NOTED HEREIN

12. ALL CONCRETE ANCHORS SHALL BE INSTALLED TO NON-CRACKED CONCRETE ONLY.

13. THE CONTRACTOR IS RESPONSIBLE TO INSULATE ALL MEMBERS FROM DISSIMILAR

15. ALL CONCRETE AND EPOXY TO REACH A MINIMUM COMPRESSIVE STRENGTH OF 3000 PSI

16. STEEL REINFORCEMENT: ALL REINFORCEMENT SHALL BE DEFORMED BARS OF INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO CURRENT REQUIREMENTS OF ASTM A615, GRADE 60 (U.N.O.), FREE FROM OIL, LOOSE SCALE AND LOOSE RUST: AND BENT, LAPPED, PLACED, SUPPORTED AND FASTENED ACCORDING TO THE "ACI DETAILING MANUAL" AND ACI 318. ALL STEEL SHALL BE SECURELY HELD IN PLACE DURING POURING OF CONCRETE. IF REOUIRED, ADDITIONAL BARS SHALL BE PROVIDED BY THE GENERAL CONTRACTOR TO FURNISH

AFFIRMATIONS ARE INTENDED. 19. ALTERATIONS, ADDITIONS, OR OTHER MARKINGS TO THIS DOCUMENT ARE NOT

#### ABOUT THIS DOCUMENT

& MORE INFORMATION CAN BE FOUND BY VISITING ENGINEERINGEXPRESS.COM/STORE OR BY SCANNING OR USING THE WEB ADDRESS FOUND HERE

THIS DOCUMENT NOT VALID WITHOUT ORIGINAL ENGINEER CERTIFICATION



# **DESIGN NOTES:**

## GENERAL NOTES:

2. THIS STRUCTURE HAS BEEN DESIGNED AND SHALL BE FABRICATED IN ACCORDANCE WITH THE STRUCTURAL PROVISIONS OF THE 2012, 2015, OR 2018 INTERNATIONAL BUILDING CODES. CONTRACTOR SHALL INVESTIGATE AND CONFORM TO ALL LOCAL BUILDING CODE AMENDMENTS WHICH MAY APPLY AND GOVERN. DESIGN CRITERIA OR SPANS BEYOND STATED HEREIN MAY REQUIRE ADDITIONAL SITE SPECIFIC SEALED ENGINEERING

SURFACES TO THIS DESIGN WHICH SHALL BE COORDINATED BY THE PERMITTING CONTRACTOR.

7. LOUVER SYSTEM HINGES AND OPERABILITY ARE OUTSIDE THE SCOPE OF THIS

ASSOCIATION, INC., & ANY APPLICABLE FEDERAL, STATE, AND/OR LOCAL CODES.

10. FOR ALUMINUM MEMBERS ALL ANCHORS SHALL BE SPACED WITH 2xDIAMETER END

11. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS

MATERIALS TO PREVENT ELECTROLYSIS. 14. ALL ALUMINUM SHALL BE 6063-T6 ALLOY AND TEMPER UNLESS NOTED OTHERWISE.

SUPPORT FOR ALL BARS.

17. ENGINEER SEAL AFFIXED HERETO VALIDATES STRUCTURAL DESIGN AS SHOWN ONLY. USE OF THIS SPECIFICATION BY CONTRACTOR, et. al. INDEMNIFIES & SAVES HARMLESS THIS ENGINEER FOR ALL COST & DAMAGES INCLUDING LEGAL FEES & APPELLATE FEES RESULTING FROM MATERIAL FABRICATION, SYSTEM ERECTION, & CONSTRUCTION PRACTICES BEYOND THAT WHICH IS CALLED FOR BY LOCAL, STATE, & FEDERAL CODES & FROM DEVIATIONS OF THIS PLAN. 18. EXCEPT AS EXPRESSLY PROVIDED HEREIN, NO ADDITIONAL CERTIFICATIONS OR

PERMITTED AND INVALIDATE THIS CERTIFICATION.

NGINEER-CERTIFIED ORIGINALS, VARIATIONS

ECALC.IO/2025436

RANK BENNARDO, P.E. ALID ONLY WITH ORIGINAL ENGINEER S

RANK BENNARDO, P.E. E# : SEE SEAL BLOCK

CORPORATE OFFICE:
160 SW 12th AVE, SUITE 106
DEERFIELD BEACH, FL 33442
P: (954) 334-066 P: (954) 334-043
OFFICE@ENGINEERINGEXPRESS.COM
ENGINEERINGEXPRESS.COM

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FS

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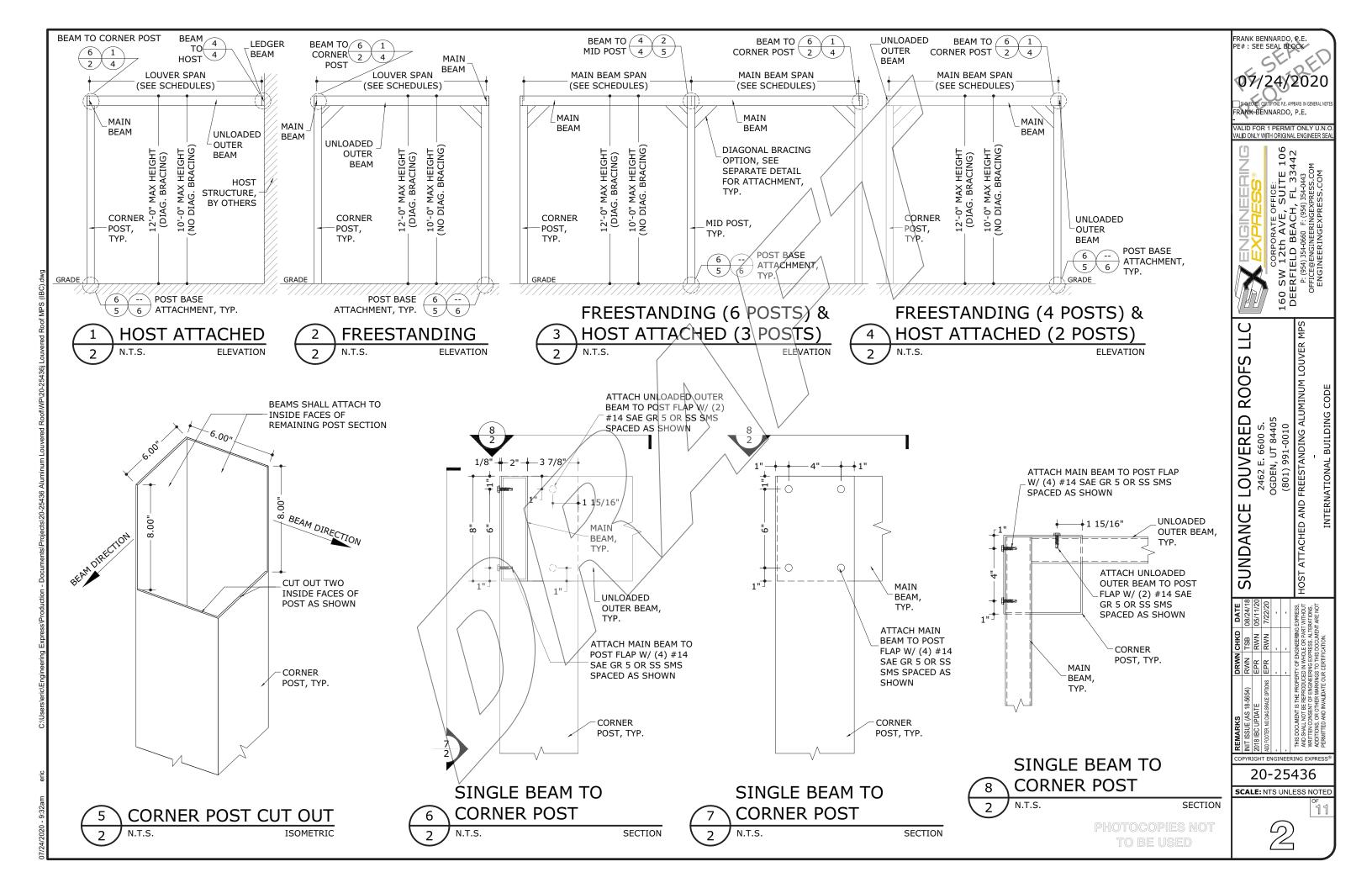
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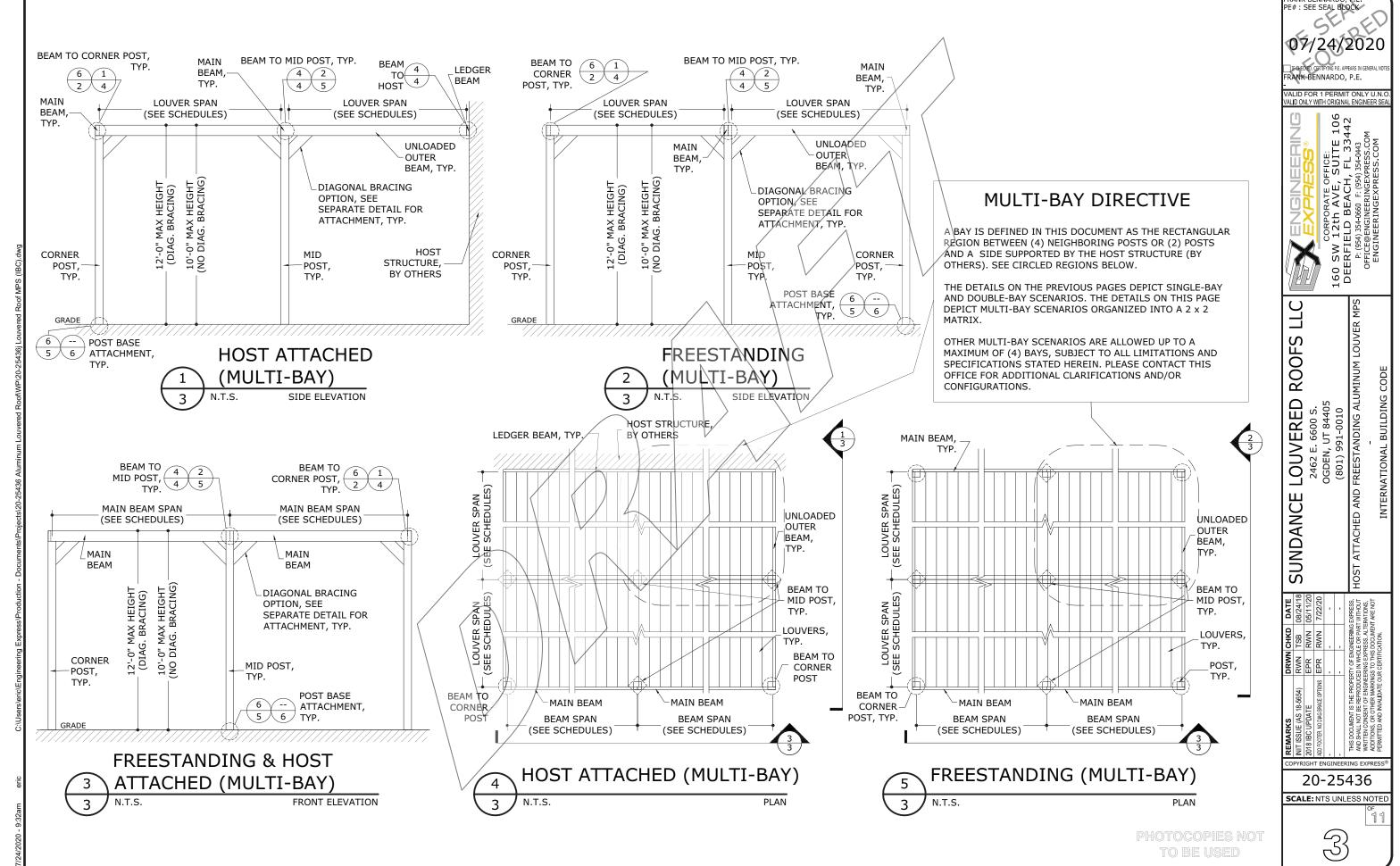
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2462 E. OGDEN, L

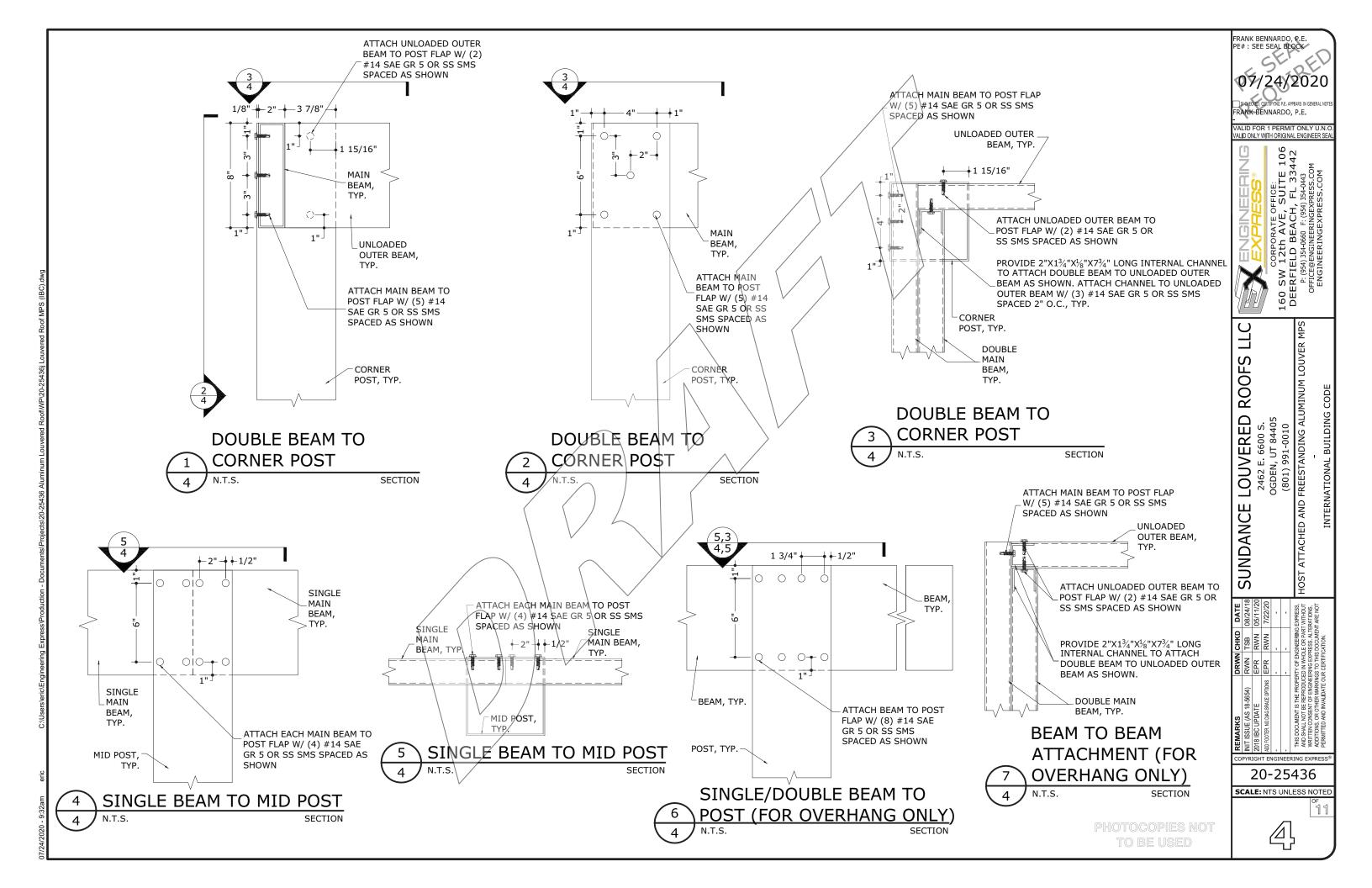
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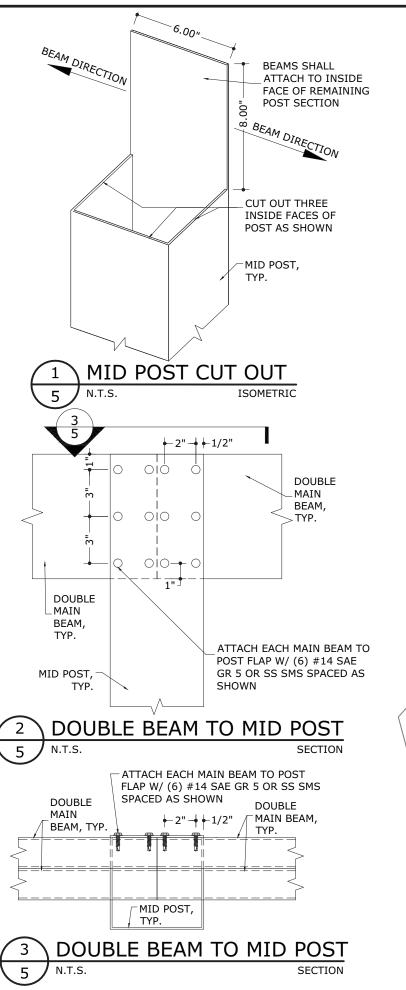
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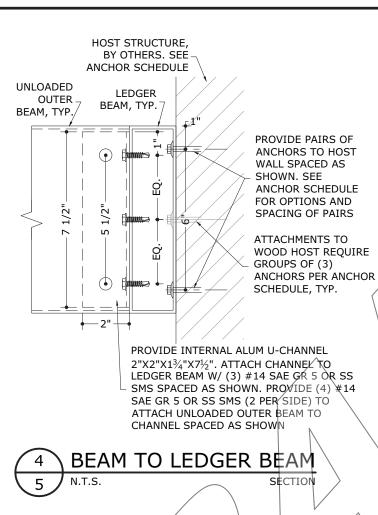




FRANK BENNARDO, P.E. PE# : SEE SEAL BLOCK







# ANCHOR SCHEDULE

## TO HOLLOW CONCRETE BLOCK (ASTM C-90):

1/4" ITW TAPCON (OR EQUIV.) W/ 11/4" MIN EMBEDMENT, 4' MIN. EDGE DISTANCE. SPACE PAIRS AT 10" O.C AND 5" FROM ENDS

## TO WOOD HOST STRUCTURE (G=0.55 MIN);

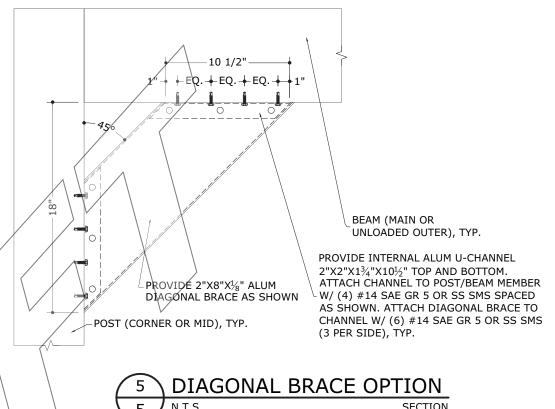
5/16" LAG SCREWS W/ 2" MIN. THREAD PENETRATION, 34" MIN. EDGE DISTANCE. SPACE GROUPS OF (3) AT 16" O.C. AND 6 MIN. FROM ENDS

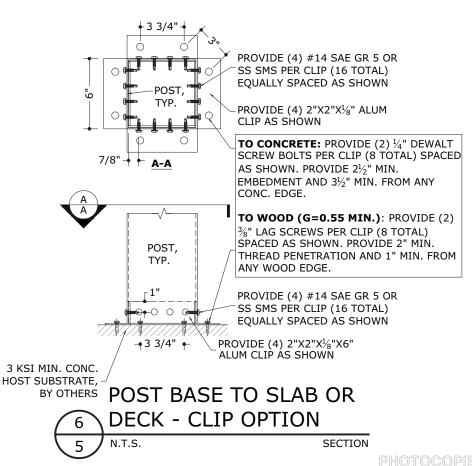
## TO CONCRETE (3000 PSI MIN):

• /1/4 ITW TAPCON (OR EQUIV.) W/ 13/4" MIN. EMBEDMENT, 2½" MIN. EDGE DISTANCE. SPACE PAIRS AT 16" Q.C. AND 6" MIN. FROM ENDS

# ANCHOR NOTES:

- 1. \ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS' RECOMMENDATIONS.
- WOOD HOST/STRUCTURE SHALL BE G=0.42 OR GREATER DENSITY AND SHALL NOT BE DIRECTLY EXPOSED TO WEATHERING ELEMENTS.
- 3. MINIMUM EMBEDMENT SHALL BE AS NOTED IN ANCHOR SCHEDULE. MINIMUM EMBEDMENT AND EDGE DISTANCE EXCLUDES STUCCO, FOAM, BRICK, AND OTHER WALL FINISHES.
- ALL CONCRETE ANCHORS SHALL BE INSTALLED TO NON-CRACKED CONCRETE ONLY.
- WHERE EXISTING STRUCTURE IS WOOD FRAMING, EXISTING CONDITIONS MAY VARY. FIELD VERIFY THAT FASTENERS ARE INTO ADEQUATE WOOD FRAMING MEMBERS, NOT INTO PLYWOOD.





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160 SW 12th AVE, SUITE 106
DEERFIELD BEACH, FL 33442
P: (954) 354-060 F: (954) 354-043
OFFICE@ENGINEERINGERRESS.COM
ENGINEERINGERRESS.COM ROOFS

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2462 E. 6600 S. OGDEN, UT 84405 (801) 991-0010

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# POST BASE ATTACHMENT -FOOTER OPTION

SEE DETAILS ON THIS PAGE

# FOOTER SCHEDULE

# TABLE 1: FOOTING ALLOWABLE UPLIFT

ALLOWABLE UPLIFT FOR AVERAGE **ON-CENTER COLUMN SPACING, LB** (SEE CALCULATIONS ON THIS PAGE)

FOOTING	UPLIFT
TYPE	PER FOOTER
PAD H	479
PAD I	1619
J PAD	2999

#### **SINGLE BEAM:**

POST TYPE	DESIGN PRESSURE (UPLIFT)	UPLIFT PER FOOTER
	16 psf	2560 lb
	18 psf	2880 lb
MID	20 psf	3200 lb
	25 psf	4000 lb
	30 psf	4800 lb
CORNER	16 psf	1280 lb
	18 psf	1440 lb
	20 psf	1600 lb
	25 psf	2000 lb
	30 psf	2400 lb

### **DOUBLE BEAM:**

POST TYPE	DESIGN PRESSURE (UPLIFT)	UPLIFT PER FOOTER
	16 psf	3072 lb
	18 psf	3456 lb
MID	20 psf	3840 lb
	25 psf	4800 lb
	30 psf	5760 lb
	16 psf	1536 lb
	18 psf	1728 lb
CORNER	20 psf	1920 lb
	25 psf	2400 lb
	30 psf	2880 lb

NOTE: UPLIFT PER FOOTER FORCES SHOWN ABOVE INCLUDE A 50% REDUCTION IN TOTAL FORCE TO ACCOUNT FOR LOUVER SURFACE AREA EXPOSED TO WIND WHEN IN THE FULLY OPENED POSITION. TO UTILIZE THESE VALUES, LOUVERS SHALL BE FULLY OPENED REFORE ANY NAMED STORM OR SIMILAR HIGH-WIND EVENT. IF NOT, THE UPLIFT PER FOOTER FORCES SHOWN ABOVE SHALL BE DOUBLED BEFORE USE.

## PAD FOOTING LAYOUT / DETAILS

HOST STRUCTURE

## TABLE 1 DESIGN NOTES

- 1. F.S. AGAINST UPLIFT OF 1.67 HAS BEEN APPLIED TO ALL LOADS.
- TABLES CAN ALSO BE USED WITH INTERMEDIATE CARRY BEAM. USE TOTAL ROOF SPAN WHEN SOLVING FOR INTERMEDIATE FOOTING & SPAN FROM CARRY BEAM TO END OF ENCLOSURE FOR OUTER **FOOTING**
- AT 2000PSF SOIL BEARING PRESSURE, GRAVITY DOES NOT GOVERN IN ANY CASE. ENSURE PROPER COMPACTION & EMBEDMENT FOR FROST & HEAVE REQUIREMENTS.

SITE SPECIFIC ENGINEERING REQUIRED FOR ANY VALUES, SIZES, CONCRETE TYPES, OR SOIL BEARING PRESSURES NOT COINCIDING WITH LIMITS OF CORRESPONDING MASTER PLAN SHEETS AND THIS MASTER PLAN.

## (SOIL, PAVERS, OR OTHER SURFACE) CONCRETE PAD PER TABLE REQUIRED UNDER ALL COLUMNS B SPAN3= 1/2(B) COLLIMN SPACING A (VALUES ARE TO OVERHANG) THIS OPTION TYPICALLY USED FOR OPEN STRUCTURES

#### **FOOTER SCHEDULE NOTES:**

- FOOTERS SHALL BE 3000 PSI MINIMUM CONCRETE, BEARING ON SOIL WITH A MINIMUM BEARING CAPACITY OF 2000 PSF AS VERIFIED BY OTHERS.
- IN LIEU OF THE FOOTERS DETAILED HEREIN, POSTS MAY BE ANCHORED TO EXISTING CONCRETE \$LAB\$ DESIGNED AND VERIFIED BY OTHERS TO SUPPORT THE CANOPY SYSTEM AS DETAILED HEREIN.
- FOOTERS SHALL HAVE THE MINIMUM REINFORCEMENT INDICATED IN THE FOOTER REINFORCEMENT DETAIL
- THESE SCHEDULES PROVIDE ONLY THE REQUIRED FOOTER SIZES, BEAM SPANS AND PURLIN SPANS SHALL NOT EXCEED THE MAXIMUM ALLOWABLE SPANS NOTED IN THEIR RESPECTIVE DESIGN SCHEDULES.
- ALL REINFORCEMENT SHALL BE DEFORMED BARS OF INTERMEDIATE GRADE NEW BILLET STEEL CONFORMING TO CURRENT REQUIREMENTS OF ASTM A615, GRADE 60 (U.O.N.), FREE FROM OIL, LOOSE SCALE AND LOOSE RUST AND BENT, LAPPED, PLACED, SUPPORTED AND FASTENED ACCORDING TO THE "ACI DETAILING MANUAL" (\$P-66) AND THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318).
- 6. MINIMUM REINFORCING STEEL CLEAR COVERAGE SHALL BE AS FOLLOWS. (PER ACI 318)
- FOOTING: TOP, SIDES, BOTTOM = 1 ½ MIN. INSTALLED ON A SMOOTH LEVEL SURFACES ONLY. MAINTAIN 3" IF NOT IN SMOOTH

## 7. ALLOW 3 DAYS CURE TIME BEFORE INSTALLING ANCHORS INTO CONCRETE.

- ALL CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS AND RECOMMENDATIONS OF ACI 318 "SPECIFICATION FOR STRUCTURAL CONCRETE FOR BUILDINGS". CONCRETE HAVE A MINIMUM COMPRESSIVE STRENGTH OF 3,000PSI AT 28 DAYS. A CERTIFICATE OF MANUFACTURER'S MIX AND STRENGTH IS TO BE PROVIDED TO THE INSPECTOR UPON REQUEST. NO WATER TO BE ADDED AFTER TRUCK LEAVES PLANT WITHOUT APPROVAL OF ENGINEER OR PLANT ENGINEER. PLANT CONTROL IS REQUIRED. MAXIMUM MIX TIME AT POINT OF DEPOSIT IS 90 MINUTES.
- THESE TABLES REPRESENT EMPIRICAL UPLIFT CALCULATIONS FOR FOOTING SIZES BASED ON STANDARD WEIGHT (150PCF) CONCRETE. THEY NEGLECT THE BENEFIT OF SKIN FRICTION AND OTHER BENEFICIAL SURCHARGE LOADING. NO WARRANTY, EXPRESSED OR IMPLIED, IS CONTAINED HEREIN AS TO ANY INFORMATION OTHER THAN THAT WHICH IS PUBLISHED HEREIN.

#### FOR ADDITIONAL FOOTING OPTIONS:

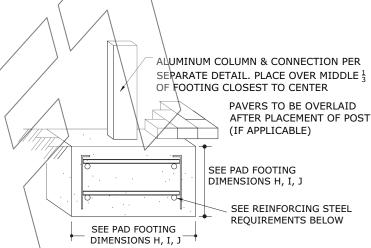
## VISIT ECALC.IO/FOOTING

FOR ENGINEER-CERTIFIED ORIGINALS, HELPFUL rools & resources & site-specific ordering

VISIT ENGINEERINGEXPRESS.COM/STORE FOR MORE MANUFACTURER MASTER PLANS TO USE WITH THIS DOCUMENT

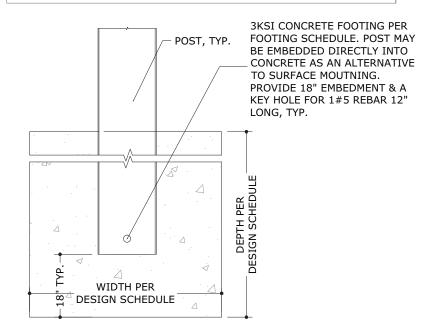
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PAD	FOOTING DE	SIGNATIONS H, I, J	VOLUME, CUFT
H	DIMENSIONS: HORIZONTAL STEEL: VERTICAL STEEL:	24" X 24" X 16" DEEP (2) #5 BARS EACH WAY T & B (4) #3 STIRRUPS	5.33
ı	DIMENSIONS: HORIZONTAL STEEL: VERTICAL STEEL:	36" X 36" X 24" DEEP (3) #5 BARS EACH WAY T & B (6) #3 STIRRUPS	18.00
J	DIMENSIONS: HORIZONTAL STEEL: VERTICAL STEEL:	40" X 40" X 36" DEEP (4) #5 BARS EACH WAY T & B (8)#3 VERTICAL STIRRUPS	33.33

**NOTE:** SIMILAR EQUIVALENT FOOTING SIZES MAY BE USED PROVIDED CUFT, ACI STEEL PLACEMENT, & FOOTING DEPTHS ARE AS SPECIFIED/FOLLOWED & FROST DEPTH IS MET. TOP OF FOOTINGS SHALL BE AT GRADE OR BELOW





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CORPORATE OFFICE:
160 SW 12th AVE, SUITE 106
DEERFIELD BEACH, FL 33442
P: (954) 354-0660 F: (954) 354-0443

ROOFS

2462 E. 6600 S. OGDEN, UT 84405 (801) 991-0010

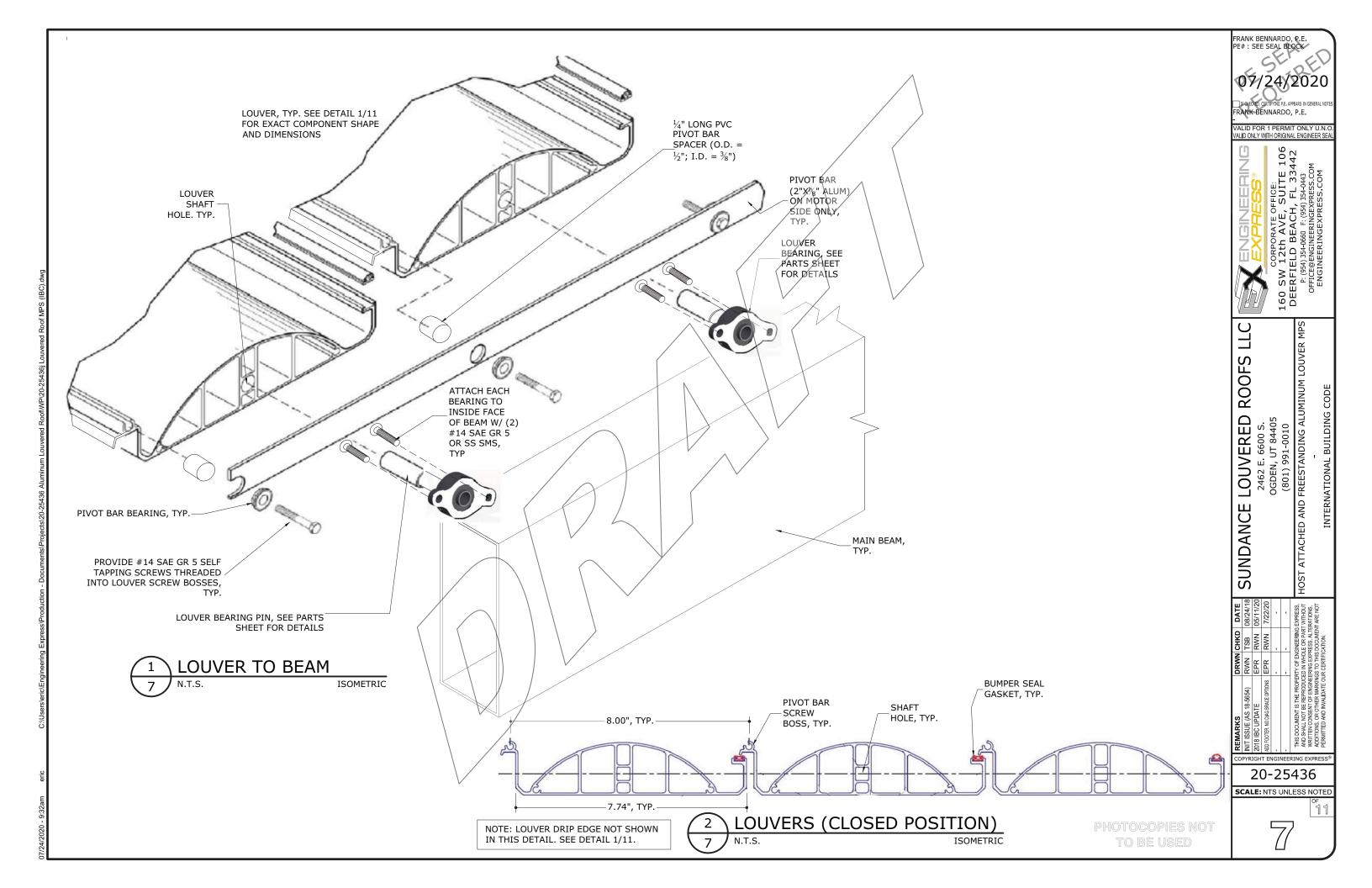
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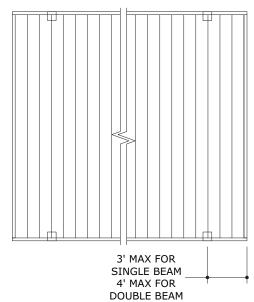
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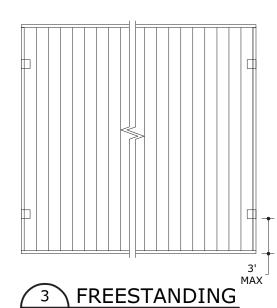


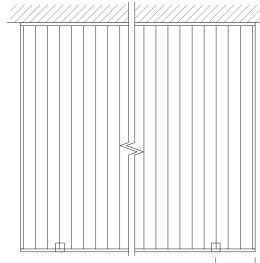


(SEE SHEETS 4 AND 5 FOR BEAM TO POST ATTACHMENT DETAILS)



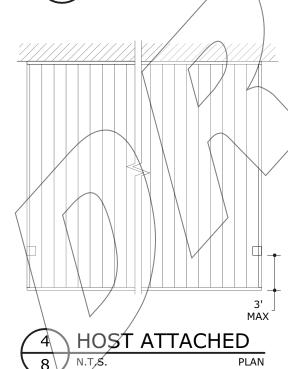






3' MAX FOR SINGLE BEAM 4' MAX FOR DOUBLE BEAM

**HOST ATTACHED** 



# LOUVER SPAN SCHEDULE

	ALLOWABLE LOUVER SPAN		
DESIGN PRESSURE	L/60 DEFLECTION LIMIT	L/80 DEFLECTION LIMIT	L/180 DEFLECTION LIMIT
20 PSF	192.0"	192.0"	155.1"
25 PSF	192.0"	188.7"	144.0"
30 PSF	192.0"	177.6"	135.5"
35 PSF	185.7"	168.7"	128.7"
40 PSF	177.6"	161.3"	123.1"
45 PSF	170.7"	155.1"	118.4"
50 PSF	163.3"	149.8"	114.3"
55 PSF	155.7"	145.1"	110.7"
60 PSF	149.0"	141.0"	107.6"

## LOUVER SPAN SCHEDULE NOTES:

DEFLECTION REQUIREMENT SHALL BE DETERMINED BASED ON THE INTERNATIONAL BUILDING CODE ALONG WITH ANY LOCAL JURISDICTION REQUIREMENTS.

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20-25436 SCALE: NTS UNLESS NOTED

# MAIN BEAM SPAN SCHEDULE (SINGLE) END BEAM

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	240.00"
	108"	240.00"
	120"	240.00"
	132"	240.00"
± 20 PSF	144"	240.00"
	156"	231.75"
	168"	223.32"
	180"	215.75"
	192"	208.90"
	96"	240.00"
	108"	240.00"
	120"	236.34"
	132"	225.34"
± 25 PSF	144"	215.75"
	156"	207.29"
	168"	199.75"
	180"	192.97"
	192"	186.84"
	96"	240.00"
	108"	227.42"
	120"	215.75"
	132"	205.71"
± 30 PSF	144"	196.95"
	156"	189.23"
	168"	182.34"
	180"	176.16"
	192"	170.57"

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	223.32"
	108"	210.55"
	120"	199.75"
	132"	190.45"
± 35 PSF	144"	182.34"
	156"	175.19"
Í	168"	168.82"
İ	180"	163.09"
	192"	157.91"
	96"	208.90"
	108"	196.95"
	120"	186.84"
	132"	178.15"
± 40 PSF	144"	170.57"
	156"	163.87"
	168"	157.91"
	180"	152.56"
	192"	147.71"
	96"	196.95" \
	108"	185.69" \
	120"	176.16"
	132"	167.96"
± 45 PSF	144"	160.81"
	156"	154.50"
	168"	148.88"
	180"	143.83"
	192"	139.27"
		/

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	186.84"
	108"	176.16"
	1/20"	167.12"
	132"	159.34"
± 50 PSF	144"	152.56"
	156"/	146.57"
/	168'(	141.24"
	180"\	136.45"
	192" \	132.12"
	96"	178.15"
	108"	167.96"
	120"	159.34"
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	132"	151.93"
± 55/PSF	\ 144"	145.46"
	\156"	139.75"
	168"	134.67"
	180"	130.10"
	192"	125.97"
	96''	170.57"
V /	108	160.81"
	120"	152.56"
	132"	145.46"
± 60 PSF	144"	139.27"
	156"	133.80"
\	168"	128.94"
\	180"	124.56"
\	192"	120.61"

# MAIN BEAM SPAN SCHEDULE (SINGLE) INTERIOR BEAM

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	208.90"
	108"	196.95"
	120"	186.84"
	132"	178.15"
± 20 PSF	144"	170.57"
	156"	163.87"
	168"	157.91"
	180"	152.56"
	192"	147.71"
	96"	186.84"
	108"	176.16"
	120"	167.12"
	132"	159.34"
± 25 PSF	144"	152.56"
	156"	146.57"\
	168"	141.24"
	180"	\136.45"
	192"	132.12"
	96"	70.57"
	108"	160.81"
	120"	152.56"/
	132"	145\46"
± 30 PSF	144"	139.27"
	156"	133.80"
	168"	128.94"
	180"	124.56"
	192"	120.61"

			\	
DESIGN PRESSURE	LOUVER SPA	AN	ALLOWABLE I	ВЕАМ
	96"		157.91,"	
	108"		/148.88"	
	120"		141.24"	
/ / \	132"		134.67"	
± 35 PSF	144"		128.94"	
\ /	156"		123.88"	
\ /	168"		119.37"	
V	180		115.32"	
\	192"		111.66"	
	96" /	*	147.71"	
	108"/		139.27"	
\	120"		132.12"	
\ \	132"		125.97"	
40 PSF \	144"		120.61"	
\ \	156"		115.88"	
\ \	168"		111.66"	
	180"		107.87"	
\ /	192"		104.45"	
	96"		139.27"	
	108"		131.30"	
	120"		124.56"	
	132"		118.77"	
± 45 PSF	144"		113.71"	
	156"		109.25"	
	168"		105.28"	
	180"		101.71"	
	192"		98.48"	

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	132.12"
	108"	124.56"
	120"	118.17"
	132"	112.67"
± 50 PSF	144"	107.87"
	156"	103.64"
	168"	99.87"
	180"	96.49"
	192"	93.42"
	96"	125.97"
	108"	118.77"
	120"	112.67"
	132"	107.43"
± 55 PSF	144"	102.85"
	156"	98.82"
	168"	95.22"
	180"	92.00"
	192"	89.07"
	96"	120.61"
	108"	113.71"
	120"	107.87"
	132"	102.85"
± 60 PSF	144"	98.48"
	156"	94.61"
	168"	91.17"
	180"	88.08"
	192"	85.28"

#### MAIN BEAM SPAN SCHEDULE (SINGLE) NOTES:

- A MAIN BEAM SHALL BE CONSIDERED AN INTERIOR BEAM IF THERE ARE LOUVERS ON BOTH SIDES OF THE BEAM (BEYOND THE ALLOWED OVERHANG ON SHEET 8). OTHERWISE, THE BEAM IS CONSIDERED AN END BEAM.
- 2. DEFLECTION LIMIT OF L/90 USED FOR DETERMINATION OF ALLOWABLE BEAM SPANS PER THE INTERNATIONAL BUILDING CODE.

SUNDANCE LOUVERED ROOFS LLC 2462 E. 6600 S. OGDEN, UT 84405 (801) 991-0010

20-25436 SCALE: NTS UNLESS NOTED



# MAIN BEAM SPAN SCHEDULE (DOUBLE) **END BEAM**

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	288.00"
	108"	288.00"
	120"	288.00"
	132"	288.00"
± 20 PSF	144"	288.00"
	156"	288.00"
	168"	288.00"
	180"	288.00"
	192"	288.00"
	96"	288.00"
	108"	288.00"
	120"	288.00"
	132"	288.00"
± 25 PSF	144"	288.00"
	156"	288.00"
	168"	282.48"
	180"	272.90"
	192"	264.24"
	96"	288.00"
	108"	288.00"
	120"	288.00"
	132"	288.00"
± 30 PSF	144"	278.53"
	156"	267.60"
	168"	257.87"
	180"	249.13"
	192"	241.22"

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
± 35 PSF	96"	288.00"
	108"	288.00"
	120"	282.48"
	132"	269.34"
	144"	257.87"
	156"	247.75"
	168"	238.74"
	180"	230.65"
	192"	223.32"
	96"	288.00"
± 40 PSF	108"	278.53"
	120"	264.24"
	132"	251.94"
	144"	241.22"
	156"	231.75"/
	168"	223.32"
	180"	215.75"
	192"	208.90"
± 45 PSF	96"	278.53"
	108"	262.60
	120"	249.13"
	132"	237.53"
	144"	227.42"
	156"/	218.50"
	168"	210.55"
	180"	203.41"
	102"	108 05"

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	264.24"
	108"	249.13"
	120"/	236.34"
/	13/2"	225.34"
± 50 PSF	/144"	215.75"
	156"	207.29"
	168"	199.75"
	\180"	192.97"
. \	192"	186.84"
	96"	251.94"
	108"	237.53"
\ \ \ \	120"	225.34"
	132	214.86"
± 55 PSF	144"\	205.71"
	156"	197.64"
	168"	190.45"
/ \	180"	183.99"
)	1,92"	178.15"
± 60 PSF	96"	241.22"
	108"	227.42"
	120"	215.75"
	132"	205.71"
	144"	196.95"
	156"	189.23"
	168"	182.34"
	180"	176.16"
	192"	170.57"

# MAIN BEAM SPAN SCHEDULE (DOUBLE) INTERIOR BEAM

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
± 20 PSF	96"	288.00"
	108"	278.53"
	120"	264.24"
	132"	251.94"
	144"	241.22"
	156"	231.75"
	168"	223.32"
	180"	215.75"
	192"	208.90"
	96"	<b>2</b> 64.24"
	108"	249.13"
	120"	236.34"
	132"	225.34"
± 25 PSF	144"	215.75"
	156"	207.29\"
	168"	199.75"
	180"	192.97"
	192"	186.84"
± 30 PSF	96"	\241.22"
	108"	227.42"
	120"	2 15.75"/
	132"	205.71"
	144"	196.95"
	156"	189.23"
	168"	182.34"
	180"	176.16"
	192"	170.57"

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	223.32"
	108"	210.55"
	120'	199.75"
	132"	190.45"
₹ 35 PSF	144"	182.34"
\ / /	156"	175.19"
	168"	168.82"
	180"	163.09"
	192"	157.91"
	96"	208.90"
	/108"	196.95"
\	120"	186.84"
\	132"	178.15"
\ ± 40 PS  <del> </del>	144"	170.57"
\ \	156"	163.87"
	168"	157.91"
\ /	180"	152.56"
\/	192"	147.71"
V	96"	196.95"
± 45 PSF	108"	185.69"
	120"	176.16"
	132"	167.96"
	144"	160.81"
	156"	154.50"
	168"	148.88"
	180"	143.83"
	192"	139.27"

DESIGN PRESSURE	LOUVER SPAN	ALLOWABLE BEAM SPAN
	96"	186.84"
	108"	176.16"
	120"	167.12"
	132"	159.34"
± 50 PSF	144"	152.56"
	156"	146.57"
	168"	141.24"
	180"	136.45"
	192"	132.12"
	96"	178.15"
	108"	167.96"
	120"	159.34"
	132"	151.93"
± 55 PSF	144"	145.46"
	156"	139.75"
	168"	134.67"
	180"	130.10"
	192"	125.97"
	96"	170.57"
	108"	160.81"
	120"	152.56"
	132"	145.46"
± 60 PSF	144"	139.27"
	156"	133.80"
	168"	128.94"
	180"	124.56"
	192"	120.61"

### MAIN BEAM SPAN SCHEDULE (DOUBLE) NOTES:

- 1. A MAIN BEAM SHALL BE CONSIDERED AN INTERIOR BEAM IF THERE ARE LOUVERS ON BOTH SIDES OF THE BEAM (BEYOND THE ALLOWED OVERHANG ON SHEET 8). OTHERWISE, THE BEAM IS CONSIDERED AN END BEAM.
  2. DEFLECTION LIMIT OF L/90 USED FOR DETERMINATION OF ALLOWABLE BEAM SPANS PER THE INTERNATIONAL BUILDING CODE.

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SCALE: NTS UNLESS NOTE

