



Traffic Engineering, Operations & Safety Manual

Chapter 16 Traffic Analysis and Modeling

Section 15 Highway Capacity Manual (HCM) – Deterministic Analysis

16-15-1 Basic Principles

September 2019

The Highway Capacity Manual (HCM) provides several analytical or deterministic tools that can estimate roadway or intersection capacity, delay, density, and other performance measures for various elements of the street and highway system. The HCM also includes procedures for evaluating bicycle, pedestrian, and transit facilities. In most cases, the HCM is the standard for traffic analysis in the US; its methods are generally reliable and have been well-tested through significant validation efforts. The Highway Capacity Manual, 6th Edition: A Guide for Multimodal Mobility Analysis (HCM6) (1) is the most current version of the HCM.

The HCM6 consists of the following four volumes:

- Volume 1: Concepts
- Volume 2: Uninterrupted Flow
- Volume 3: Interrupted Flow
- [Volume 4: Applications Guide](#) (a web-based document, requires a user account)

Each chapter within Volume 2 and Volume 3 of HCM6 has six or more sections covering the following topics: introduction, concepts, methodology, extensions to the methodology, applications, and references. The methodology section (typically Section 3) highlights the scope, strengths, and limitations of the applicable HCM methodology, and as such, serves as a good reference when determining whether use of the HCM methodology is appropriate. HCM6, Volume 1, Chapter 7 provides additional guidance as to when an alternative (non-HCM based) analysis methodology may be appropriate.

The HCM procedures are good for analyzing the performance of isolated and non-congested facilities but do have limitations. For example, the HCM models cannot account for interactions between network elements (e.g., they cannot reflect the effect of a queue backup at a ramp terminal on the adjacent freeway operations) and they may under-predict the extent of congestion in oversaturated conditions. Consider the strengths and limitations of the HCM methods when selecting the methodology to apply. Document the rationale for choosing the selected traffic analysis methodology (HCM-based, microsimulation, etc.) in the Traffic Analysis Tool Selection memoranda and submit to the WisDOT regional traffic staff for approval.

[TEOps 16-10](#) provides a brief description of when and how to apply the HCM methodologies and identifies the WisDOT-supported programs that implement the HCM methodology.

16-15-5 Signalized Intersections

November 2020

5.1 Introduction

WisDOT accepts the use of the HCM6, Chapter 19 methods for estimating the performance of a signalized intersection from the perspective of the motor vehicle, pedestrian, and bicycle modes. These procedures are applicable for three-leg and four-leg intersections that operate in isolation from nearby signals with a pre-timed, semi-actuated or fully-actuated controller. Signalized intersections that are not isolated, that operate in an actuated-coordinated manner, or are part of a system or corridor require the use of a combination of both the signalized intersection methods of Chapter 19 and the urban street segment procedures outlined in Chapter 18. For closely spaced signals, such as those found at freeway ramp terminals, the analyst *should* follow the methodology presented in Chapter 23 for interchange ramp terminals. If the project spans multiple contiguous urban street segments, consider applying the Chapter 16 urban street facilities methodologies.

The analyst *should* recognize and account for the methodological limitations of the signalized intersection methods. There are cases that may not fit within the analytical framework of the HCM, including but not limited to intersections with five or more approaches, those with more than two exclusive turn lanes on any approach or those with complex geometry or controller operations. When these, or similar limitations exists, the project manager *should* specify the use of an alternative tool such as microsimulation. See [TEOps 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported tools that implement the HCM methodology for signalized intersection analysis are:

- Highway Capacity Software (HCS), McTrans
- Synchro, Cubic|Trafficware
- Vistro, PTV Group (requires prior approval from WisDOT regional traffic engineer)

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

When conducting capacity analysis for signalized intersections, apply the basic signal parameters as outlined in the following section in conjunction with the HCM-based analysis methodologies.

5.2 Basic Parameters for Capacity Analysis

The Traffic Signal Design Manual, Section 3, Chapter 2-2 ([TSDM 3-2-2](#)) provides recommended parameters to use for the general analysis of state-owned signals; including minimum and maximum green times, pedestrian phase times and cycle lengths. The following provides updated direction for the use of right-turn on red (RTOR) and saturation flow rate. Unless noted otherwise, the policy within this section supersedes the guidance provided in [TSDM 3-2-2](#). If it is unclear which guidance to follow, contact BTO-TASU (DOTTrafficAnalysisModeling@dot.wi.gov) for clarification.

5.2.1 Right-Turn on Red (RTOR)

5.2.1.1 Background

Right-turns made while facing a red traffic signal indication, permitted under Wisconsin statute 346.37(1)(c)3, can have a beneficial effect on traffic flow and intersection capacity as they reduce the number of vehicles serviced during the green phase. The following section describes how to apply RTOR when conducting capacity analysis for signalized intersections.

5.2.1.2 Dedicated Right-Turn Lanes

Since vehicles making other movements (through or left-turns) may block right-turn access at shared left-through-right (LTR) or shared through-right (TR) lanes, WisDOT has only investigated RTOR volumes at locations with dedicated right-turn lanes. For the purposes of RTOR inclusion in capacity analyses, a dedicated right-turn lane is any lane that satisfies at least one of the following criteria:

- Pavement markings or signage clearly dedicate the lane for a right-turn only movement
- Field observations indicate that the lane functions as a de-facto right-turn only lane (requires approval from WisDOT regional traffic staff)
- Subject approach flares out at the intersection such that a right-turning vehicle can safely fit beside a through vehicle within the same lane and field observations show vehicles using the approach flare to make right turns (requires approval from WisDOT regional traffic staff)

Additionally, for RTOR inclusion to be applicable for capacity analysis, the following must exist:

- Right-turns on red are permissible (i.e., field signage does not prohibit this maneuver during the analysis period)
- Vehicle queuing from the adjacent lane does not prevent vehicles wishing to make a right-turn from accessing the dedicated (or de-facto) right-turn lane

For additional clarification, as to what constitutes a right-turn lane for purposes of capacity analysis at signalized intersections, contact the WisDOT regional traffic engineer or BTO-TASU.

5.2.1.3 RTOR Estimation

An estimate of the proportion of vehicles making RTOR from a dedicated right-turn lane is most accurate when derived from field counts taken at the intersection in question. As it is not always practical to gather this information, WisDOT developed the following recommendations regarding RTOR volumes (V_{RTOR}) in relation to total right-turn demand (V_{RT}):

- Single Right-Turn Lanes at Intersections: $V_{RTOR} = 0.38V_{RT}$ [Equation 5.1]
- Single Right-Turn Lanes at Interchange Off Ramps: $V_{RTOR} = 0.66V_{RT}$ [Equation 5.2]
- Dual Right-Turn Lanes (Intersections and Interchanges): $V_{RTOR} = 0.30V_{RT}$ [Equation 5.3]

Field studies conducted throughout Wisconsin in 2009 (2) and 2015 (3) guided the development of these recommendations. WisDOT has not studied RTOR at any other intersection configuration, such as shared lanes or triple right-turn lanes, thus unless intersection-specific field data is available to indicate otherwise, the analyst should assume that vehicles do not make RTOR movements at these locations. Obtain approval from WisDOT regional traffic staff prior to including RTOR volumes for triple right-turn lanes or shared lanes within the capacity

analysis.

Equation 5.2, is only applicable for single right-turn lanes exiting the off ramp at an interchange. For single right-turn lanes turning onto an on-ramp at an interchange, utilize Equation 5.1.

The analyst **shall not** use RTOR volumes in the analysis when field signage prohibits this maneuver during the analysis period.

5.2.1.4 RTOR Application

WisDOT supports the use of HCS for traffic signal analysis and supports the use of Vistro and Synchro for both traffic signal analysis and signal optimization (see [TEOpS 16-10](#)). Use and acceptance of Vistro for signal analysis and optimization, however, is up to the discretion of the WisDOT regional office. Due to limitations of the HCS optimization methodologies, WisDOT does not support the use of HCS for signal optimization.

Vistro uses the same module for both HCM-compliant analysis and for signal optimization. Synchro, however, uses two distinct modules – one which provides HCM-compliant analysis and another which provides signal optimization as well as non-HCM-compliant analysis. The later module uses a proprietary methodology to calculate intersection delay and other values. Changes made in one module do not necessarily transfer to the other module. Therefore, there are nuances in how to conduct HCM-compliant analysis and signal optimization in Synchro which are not present in Vistro.

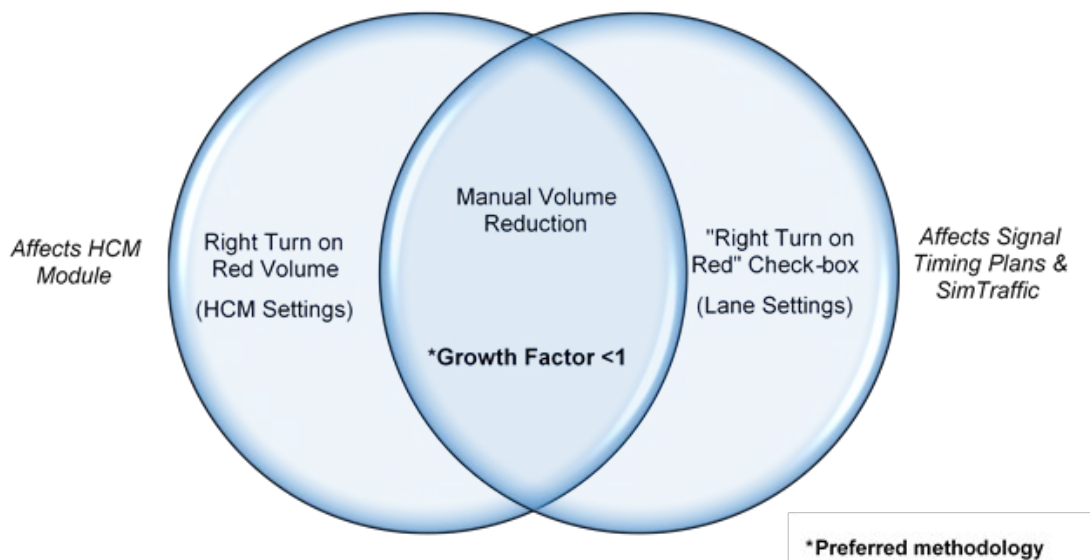
Figure 5.1 provides an overview of the various methodologies available for affecting RTOR in the two modules of Synchro. A subset of the methodologies, those which adjust demand, affect both Synchro modules. As noted in the figure, the “growth factor” method is the preferred methodology when the analyst is using Synchro to conduct HCM-compliant analysis and signal optimization. This methodology involves applying a growth factor of less than one to the right turn movements. Apply the following growth factors, derived from Equations 5.1 and 5.3, unless field data is available and supports otherwise:

- 0.62 for Single Right-Turn Lanes at Intersections
- 0.70 for Dual Right-Turn Lanes (Intersections and Interchanges)

Note that the above rates do not include a growth rate for Single Right-Turn Lanes at Interchange Off Ramps. Applying Equation 5.2 would yield a growth factor of 0.34 for this scenario; however, Synchro currently sets a floor of 0.5 for growth rates preventing the use of the 0.34. When dealing with Single Right-Turn Lanes at Interchanges, use the manual reduction method detailed below.

The other methodology to affect both modules in Synchro is to manually reduce the right-turn volumes by the V_{RTOR} . This is less transparent when conducting a peer review and is more prone to typographical error. Therefore, WisDOT prefers the use of the growth factor method where possible.

Figure 5.1 Synchro RTOR Adjustments Venn Diagram



5.2.1.4.1 HCM-Compliant Analysis

WisDOT provides the following guidance on incorporating RTOR volumes when conducting HCM-compliant

analysis. The RTOR volumes used may be based on field-collected values or the equations above (see Equations 5.1 – 5.3).

- **HCS:** Enter the V_{RTOR} , rounded to the nearest whole vehicle per hour (veh/h), into the “RTOR, veh/h” field for the relevant approaches. This field is at the bottom of the “Primary Input Data” within the HCS “Streets” module, which includes traffic signal analysis.
- **Vistro:** Check the “Right Turn on Red” boxes for the relevant approaches in the “Intersection Setup” tab. Enter the V_{RTOR} , rounded to the nearest whole vehicle per hour (veh/h), into the “Right-Turn on Red Volume (veh/h)” field in the “Volumes” tab.
- **Synchro:** Use the growth factor method outlined above. Checking the “Right Turn on Red” box in the “Lane Settings” area **does not** affect the HCM-compliant analysis.

Entering the V_{RTOR} value associated with the approach into the “Right Turn on Red Volume” field in the Synchro HCM module is also acceptable, though WisDOT does not prefer this method as it only affects the HCM module. The analyst **shall not** enter a volume other than the default of 0 into the “Right Turn on Red Volume” field in combination with the growth factor method, as it will lead to incorrect results.

5.2.1.4.2 Signal Optimization

In Synchro, changes to the “Right Turn on Red Volume” field in the HCM module do not affect the signal timings or optimization calculations. If the analyst checks a box to allow RTOR within the “Lane Settings” module (automatically checked by default), Synchro uses an algorithm to determine a “Saturated Flow Rate (RTOR)”. Synchro uses the “Saturated Flow Rate (RTOR)” value within the signal optimization function. The RTOR checkbox does not affect the HCM results. Synchro’s proprietary RTOR methodology, enabled via the RTOR checkbox, is not straightforward and is thus not a preferred methodology for developing signal timing plans. When optimizing signals, the analyst should uncheck the RTOR checkbox for all approaches.

WisDOT prefers the use of the growth factor method for conducting signal optimization in Synchro.

5.2.1.4.3 Microsimulation Analysis

WisDOT also currently supports two microsimulation software programs for traffic signal analysis: SimTraffic (associated with Synchro, affected by demand reductions but not by changes within the HCM module) and Vissim. The analyst should not dictate RTOR volumes within microsimulation programs, as the models should determine when these turns happen based on how the right-turning vehicles interact with other vehicles in the system.

Where right-turns at signals are critical movements, a good check for reasonableness could be comparing modeled RTOR volumes to field-collected ones. The analyst should direct any questions regarding how to model RTOR within a specific microsimulation software program to BTO-TASU

(DOTTrafficAnalysisModeling@dot.wi.gov).

5.2.2 Saturation Flow Rate

5.2.2.1 Background

One of the many variables that influence the performance of traffic signals is saturation flow (sat. flow) rate. The base saturation flow rate for a lane is the theoretical number of vehicles that could travel through the intersection during one hour of green time under ideal conditions. The saturation headway, or the average time between the front bumper of one vehicle and the front bumper of the vehicle behind it under ideal conditions, determines the saturation flow rate. The HCM6 default values for base saturation flow rate are:

- 1900 passenger cars per hour per lane (pc/h/lane) in metropolitan areas with population >250,000
- 1750 pc/h/lane otherwise

The HCM provides several factors to adjust these base saturation flow rates to account for prevailing conditions at the approach, including heavy vehicle percentages, grade, lane width, etc. More information on flow rate concepts is available in HCM6, Chapters 4 and Chapters 19.

Through movements at signalized intersections typically have high volumes relative to other movements, and have an oversized role in determining the signal timing and phasing plan, as well as level of service (LOS). Therefore, this policy focuses on the saturation flow rate for through lanes.

5.2.2.2 Saturation Flow Rate Methodology

A field saturation flow study at an intersection will provide the most accurate measure of experienced flow rates on its approaches. Given the expense, it may not be practical to conduct these studies, especially at locations that are operating significantly under capacity.

Since it is impractical to conduct field studies for every intersection and in an effort to gain a better understanding of the range of saturation flow rates, WisDOT funded a study in 2015 to evaluate saturation flow rates at various signalized intersections across the state (3). The study aimed to identify the variables, beyond those already accounted for by the HCM, which influenced the field saturation flow rates. The study followed the methodology laid out in the HCM and only collected data on the saturation flow rate for through lanes.

The 2015 WisDOT sat. flow study (3) found that the following three factors affect the base saturation flow rate of a through lane at a signalized intersection: the urbanized area or cluster population, the total number of approach lanes (left, through and right), and the posted speed limit of the approach. Accordingly, the base saturation flow rate may differ from one approach to the next at a given signalized intersection. The field conditions or traffic signal design dictate the total number of approach lanes and the posted speed limit of the approach. The urbanized area or cluster population information is available from either the table or map provided by the [2010 Census Bureau](#).

WisDOT used the results of this study to develop a methodology to estimate the base saturation flow rate for exclusive through lanes and shared through-right lanes at signalized intersections in Wisconsin. Since the methodology accounts for more variables and reflects Wisconsin-specific data, analysts should use the WisDOT sat. flow methodology as described below to estimate the base saturation flow rate for exclusive through lanes and shared through-right lanes at signalized intersections. If the WisDOT estimation methodology results in a sat. flow rate less than the relevant HCM default value, specifically if it is less than 1750 pc/h/ln, the analyst should consider completing a field study or using the HCM6 default values.

Coordinate with WisDOT regional traffic staff to determine the most appropriate methodology for calculating the base saturation flow rate for exclusive through and shared through-right lanes. Use the HCM default base saturation flow rates for exclusive left, shared left-through, shared left-through-right, and exclusive right turn lanes unless there is field data or other documentation supporting an alternative value or WisDOT instructs otherwise.

5.2.2.3 Saturation Flow Rate Estimation

Use the [WisDOT sat. flow spreadsheet](#) (a Microsoft Excel based spreadsheet) or the adjustment factors shown in [Table 5.1](#) to implement the WisDOT sat. flow methodology. The WisDOT sat. flow spreadsheet implements equations to apply the various site-specific adjustments in the same general form as HCM6 and calculates the base sat. flow rate for each lane group

In lieu of the WisDOT sat. flow spreadsheet, the analyst may use the adjustment factors shown in [Table 5.1](#) in conjunction with a starting saturation flow rate value of 1980 pc/h/ln (derived from the 2015 WisDOT sat. flow study (3)) and the following equation:

$$s_0 = 1980 \times f_{Pop} \times f_N \times f_{SL} \text{ [Equation 5.4]}$$

Where:

s_0 = Base saturation flow rate for exclusive through and shared through – right lanes

f_{Pop} = Adjustment factor for population

f_N = Adjustment factor for number of approach lanes

f_{SL} = Adjustment factor for speed limit of approach

As with the WisDOT sat. flow spreadsheet, apply the adjustment factors by approach to determine the base sat. flow rate for each through lane group. The lane adjustment factor (f_N) is dependent on the total number of lanes on the approach (i.e., includes all left, through, right and shared lanes) and the speed adjustment factor (f_{SL}) is based on the speed limit of the approach. Accordingly, the base saturation flow rate may differ from one approach to the next at a given signalized intersection (e.g., the base saturation flow rate for the eastbound through movement may be 1950 pc/h/ln while the base saturation flow rate for the northbound through lane may be 1825 pc/h/ln).

Due to rounding, use of the adjustment factors from Table 5.1 may result in a slightly different sat. flow rate than that calculated using the WisDOT sat. flow spreadsheet. The WisDOT sat. flow spreadsheet uses formulas to calculate the adjustment factors and does not round until after it computes the sat. flow rate, where the adjustment factor methodology utilizes rounded values from Table 5.1 to compute the sat. flow rate.

An example of how to apply the adjustment factors for saturation flow rate follows:

A signalized intersection is within an urbanized area that has a population of 29,000 ($f_{Pop} = 0.95$). Looking at an approach with a left-turn lane, two through lanes, and two right-turn lanes (five total approach lanes, $f_N = 0.97$) and a posted speed limit of 40 MPH ($f_{SL} = 1.00$), the resulting base saturation flow rate would be:

$$s_0 = 1980 \times 0.95 \times 0.97 \times 1.00$$

$$s_0 = 1825 \text{ pc/h/ln}$$

Use the resulting base saturation flow rate ($s_0 = 1825 \text{ pc/h/ln}$) for operational analysis of the two through lanes on this approach. Use the HCM default values for the left and right turn lanes unless there is field data or other documentation supporting an alternative value or WisDOT instructs otherwise. Calculate the base saturation flow rate for the through lane group on all other approaches in a similar manner.

Table 5.1 WisDOT Saturation Flow Adjustment Factors

| Population Adjustment Factor | | Lane Adjustment Factor | | Speed Adjustment Factor | |
|---------------------------------------|----------------------|------------------------------|----------------------|---|----------------------|
| Urbanized Area/ Cluster Population | Adjustment Factor | Total # Approach Lanes | Adjustment Factor | Posted Speed Limit of Approach (mph) | Adjustment Factor |
| < 2,000 | 0.91 | 1 | 0.88 | 25 | 0.94 |
| 2,000 - 4,499 | 0.92 | 2 | 0.94 | 30 | 0.96 |
| 4,500 - 8,999 | 0.93 | 3 | 0.96 | 35 | 0.98 |
| 9,000 - 18,999 | 0.94 | 4 | 0.97 | 40 | 1.00 |
| 19,000 - 39,999 | 0.95 | 5 | 0.97 | 45 | 1.02 |
| 40,000 - 82,999 | 0.96 | 6 | 0.98 | 50 | 1.04 |
| 83,000 - 170,499 | 0.97 | ≥7 | 0.98 | 55 | 1.07 |
| 170,500 - 347,499 | 0.98 | | | | |
| 347,500 - 704,499 | 0.99 | | | | |
| ≥ 704,500 | 1.00 | | | | |

Since the WisDOT sat. flow methodology calculates a Wisconsin, site-specific base saturation flow rate, the analyst should apply all other HCM adjustment factors (e.g., heavy vehicles, grade, lane width, Central Business District (CBD), etc.) as appropriate to calculate the final adjusted sat. flow rate. These adjustments are typically applied within the individual software program (HCS, Synchro, Vistro). It is important to note that the WisDOT sat. flow estimation methodology applies only to exclusive through lanes and shared through-right lanes, as these two types of through lanes were the only ones included in the 2015 study.

5.2.2.4 Saturation Flow Rate Application

5.2.2.4.1 HCM-Compliant Analysis and Signal Timing Plan Development

As detailed in [TEOpS 16-10](#), WisDOT currently supports three HCM-based software programs for traffic signal analysis, HCS, Vistro, and Synchro, although use of Vistro requires prior approval from the WisDOT regional traffic engineer. WisDOT provides the following guidance on entering base saturation flow rates generated from the WisDOT sat. flow methodology.

- **HCS:** Enter the base saturation flow rate, rounded to the nearest 5 pc/h/ln, into the “Saturation, pc/h/ln” field for the relevant movements. This field is in the “Traffic” section within the HCS “Streets” module, which includes traffic signal analysis.
- **Vistro:** Check the “Override Base Saturation Flow Rate per Lane” box for the relevant lane groups in the “Saturation Flow” area of the “Traffic Control” tab. Enter the base saturation flow rate, rounded to the nearest 5 pc/h/ln, into the “User Defined Base Saturation Flow Rate per Lane (pc/h/ln)” field.
- **Synchro:** In the HCM module, used to generate fully HCM-compliant results, enter the base saturation flow rate, rounded to the nearest 5 pc/h/ln into the “Ideal Satd. Flow (vphpl)” field for the relevant movements. Alternately, edit this field through the “Lane Settings” module – changes made there carry through to the HCM module.

Note that the resulting base saturation flow rate calculated for a lane group containing a shared through-right lane would also be applied to the right turn movement unless there is an exclusive right turn lane on the approach.

Although the terminology within Synchro indicates that the base saturation flow rate is in vehicles per hour per lane (vphpl), further investigations found that this value is actually representative of passenger cars per hour per lane (pc/h/ln). Therefore, the analyst should enter the base saturation flow rate as calculated above in pc/h/ln without further adjustment.

As noted above (TEOpS 5.2.2.2), the field data used to develop the WisDOT sat. flow methodology purposely minimized the impact from heavy vehicles to lessen the impact of using pc/h/ln versus using veh/h/ln. Further, any of the adjustment factors beyond those included in the 2015 WisDOT sat. flow study (3) that are incorporated into the HCM base saturation flow rate calculations (heavy vehicles, grade, lane width, CBD, etc.) will be applied on top of the WisDOT sat. flow rates within the software package used to calculate the final adjusted sat. flow rate in pc/h/ln.

5.2.2.4.2 Microsimulation Analysis

Capacity is not typically an explicit input within microsimulation programs, as it will vary based on vehicle interactions and various parameters. Since headway dictates saturation flow rate and because each microsimulation program has one or more adjustable parameters characterizing the concept of headway, adjustments to these settings will increase or decrease potential and realized capacities. The analyst should calibrate each signalized intersection, ensuring that the model meets the applicable validation thresholds and adequately replicates field behavior. Direct any questions regarding how to apply saturation flow rate within a specific microsimulation software program to BTO-TASU (DOTTrafficAnalysisModeling@dot.wi.gov).

16-15-10 Two-Way Stop-Controlled (TWSC) Intersections

September 2019

WisDOT accepts the use of HCM6, Chapter 20 methods for analyzing the performance of a two-way stop-controlled (TWSC) intersection from the perspective of the motor vehicle mode and the pedestrian modes. Currently, no specific methodology exists to assess the performance of bicycles at TWSC intersections. These methods are applicable to three-leg and four-leg intersections with stop-control only on the side street(s).

Analysts should recognize and account for the methodological limitations of Chapter 20 methods. Some of the limitations of the TWSC methodology include, but are not limited to, the following:

- Only applicable for TWSC intersections with up to three through lanes (either shared or exclusive) on each major-street approach and up to three lanes on each minor-street approach (max of one exclusive lane per movement)
- Limited to no more than four approaches
- Limited to one stop-controlled approach on each side of the major street

Additionally, apart from a TWSC intersection located between two signalized intersections, the HCM methodology typically does not account for the effects from other intersections. For TWSC intersections located on an urban street segment between two coordinated signalized intersections, to account for the interaction of the adjacent signalized intersections, the analyst should follow the methodologies presented in Chapter 18 for urban street segments. When these, or similar limitations exists, the project manager should specify the use of an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software programs for HCM-based TWSC intersection analysis are:

- *HCS, McTrans*
- *Synchro, Cubic|Trafficware*
- Vistro, PTV Group (requires prior approval from WisDOT regional traffic engineer)

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

16-15-15 All-Way Stop-Controlled (AWSC) Intersections**September 2019**

WisDOT accepts the use of HCM6, Chapter 21 methods for analyzing the performance of unsignalized intersections with stop control at all approaches (i.e., requires every vehicle to stop before entering the intersection). HCM6, Chapter 21 methodologies focus on the motor vehicle mode but do offer some guidance for how to assess the performance of pedestrian and bicycles. The procedure is applicable for typical AWSC configurations of three-leg and four-leg intersections with no more than four approaches and no more than three lanes on any given approach.

Analysts *should* recognize and account for the methodological limitations of Chapter 21 methods. There are cases that may not fit within the analytical framework of the HCM, including but not limited to queue interactions from adjacent intersections, or the impact of pedestrians. When these, or similar limitations exists, the project manager *should* specify the use of an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software programs for HCM-based AWSC intersection analysis are:

- *HCS, McTrans*
- *Synchro, Cubic|Trafficware*
- *Vistro*, PTV Group (requires prior approval from WisDOT regional traffic engineer)

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

16-15-20 Roundabouts**May 2021****20.1 Introduction**

WisDOT accepts the use of the HCM6, Chapter 22 methods for the analysis of isolated roundabouts with one-lane and two-lane entries, up to one yielding or non-yielding bypass lane per approach, and up to two circulating lanes. HCM6, Chapter 22 methodologies focus on the motor vehicle mode but do offer some guidance for how to assess the performance of pedestrian and bicycles.

Analysts *should* recognize and account for the methodological limitations of Chapter 22 methods. For roundabouts that are not isolated, are part of a system or corridor of roundabouts, or are located within the influence area of an adjacent signal, the analyst *should* use a combination of the roundabout methods of Chapter 22 and the urban street segment procedures outlined in Chapter 18. For closely spaced roundabouts, specifically those found at freeway ramp terminals, the analyst *should* follow the methodology presented in Chapter 23 for interchange ramp terminals.

See [TEOpS 16-15-20.4](#) for additional information on the use of supplemental tools for operational analyses and design.

20.2 Wisconsin-Calibrated Models**20.2.1 Driver Behavior**

Critical headway (also referred to as 'critical gap') and follow-up headway are the driver behavior parameters that influence the capacity of a roundabout approach and intersection. Critical headway is the smallest gap in circulating traffic that an entering driver would accept to enter the roundabout. Follow-up headway is the time between two successive entering vehicles accepting the same gap in circulating traffic. Figure 20.1 diagrams the concept of critical headway and Figure 20.2 diagrams the concept of follow-up headway.

Figure 20.1 Critical Headway

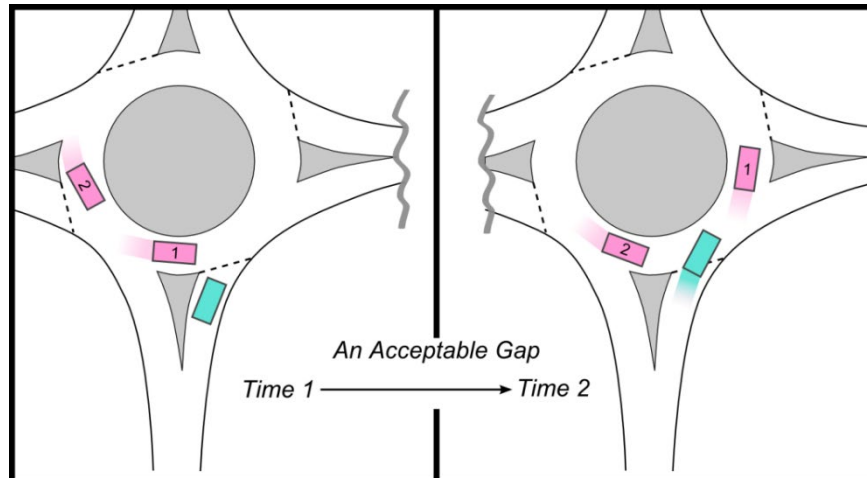
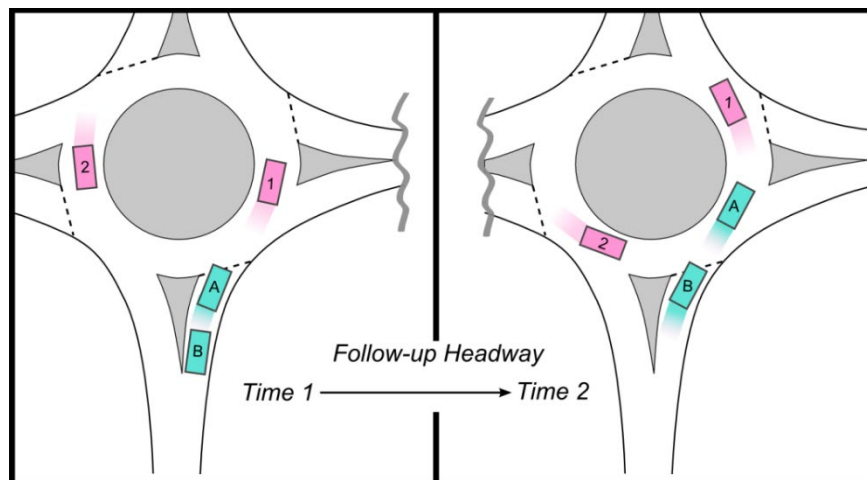


Figure 20.2 Follow-up Headway



As part of the National Cooperative Highway Research Program (NCHRP) Report 572 (4) and the 2015 Federal Highway Administration (FHWA)-sponsored report FHWA-SA-15-070 (5), researchers collected and analyzed critical and follow-up headways at several roundabouts across the US. The NCHRP Report 572 (4) and FHWA-SA-15-070 (5) report reflect 2003 and 2012 data collection efforts, respectively. Both these research efforts found that an exponential gap-acceptance theory combined with field determined headway values could provide an acceptable empirical capacity equation for estimating the operations of a U.S. roundabout (see HCM6 Chapter 22 and HCM6 Supplemental Chapter 33 for additional details). This method of analyzing roundabouts is the basis for Wisconsin's driver behavior-based approach to analyzing roundabout operations. The general form of the capacity equation for a roundabout follows below in Equation 20.1 - Equation 20.3:

$$c_{pce} = Ae^{(-Bv_c)} \quad \text{[Equation 20.1]}$$

$$A = \frac{3,600}{t_f} \quad \text{[Equation 20.2]}$$

$$B = \frac{t_c - (t_f/2)}{3,600} \quad \text{[Equation 20.3]}$$

where

c_{pce} = entry lane capacity, adjusted for heavy vehicles (pc/hr/ln)

v_c = lane capacity, adjusted for heavy vehicles (pc/hr)

t_c = critical headway (s), and

t_f = follow-up headway (s)

Adjusting the critical and follow-up headways allow the capacity equation in Equation 20.1 to be calibrated to reflect local site conditions. The HCM6 provides default capacity equations based on observations of critical and follow-up headways made at US roundabouts in 2012 (5)

In an effort to calibrate the HCM capacity equations to reflect conditions in Wisconsin, in 2020, WisDOT completed a research project to observe headways at Wisconsin roundabouts (6). [Table 20.1](#) lists the recommended headway values and the corresponding parameters A and B that were developed based on the findings of the study. The analyst shall use the values listed in [Table 20.1](#) for roundabout capacity analyses statewide. Refer to [Attachment 20.1](#) for an illustration of the roundabout lane configurations associated with the headway values in [Table 20.1](#).

The values shown in [Table 20.1](#) represent the headway numbers based on the 2020 Wisconsin research (6). Consult with the WisDOT regional traffic contact or contact BTO-TASU for information on or potential use of headway numbers based on older Wisconsin research.

Table 20.1 Recommended Headway Values¹

| | Critical Headway, t_c | Follow-up Headway, t_f | Parameter A | Parameter B |
|--|-------------------------|--------------------------|-------------|-------------|
| Single Circulating (Conflicting) Lane | | | | |
| Single Lane Entry (1 - 1) | 4.7 | 2.6 | 1385 | 0.000944 |
| Two Lane Entry | | | | |
| Left Lane (L2 - 1) | 4.7 | 2.5 | 1440 | 0.000958 |
| Right Lane (R2 - 1) | 4.4 | 2.5 | 1440 | 0.000875 |
| Three Lane Entry | | | | |
| Left Lane (L3 - 1) | 4.6 | 2.3 | 1565 | 0.000958 |
| Center Lane (C3 - 1) | 4.4 | 2.6 | 1385 | 0.000861 |
| Right Lane (R3 - 1) | 4.4 | 2.4 | 1500 | 0.000889 |
| Slip/Bypass Lane, all entry lane cases (R-bypass - 1) | 4.0 | 2.3 | 1565 | 0.000792 |
| Multi-Circulating (Conflicting) Lanes² | | | | |
| Single Lane Entry (1 - 2) | 4.8 | 2.6 | 1385 | 0.000972 |
| Two Lane Entry | | | | |
| Left Lane (L2 - 2) | 4.6 | 2.6 | 1385 | 0.000917 |
| Right Lane (R2 - 2) | 4.3 | 2.6 | 1385 | 0.000833 |
| Three Lane Entry | | | | |
| Left Lane (L3 - 2) | 4.6 | 2.5 | 1440 | 0.000931 |
| Center Lane (C3 - 2) | 4.4 | 2.4 | 1500 | 0.000889 |
| Right Lane (R3 - 2) | 4.6 | 2.5 | 1440 | 0.000931 |
| Slip/Bypass Lane, single entry lane case (R-bypass - 2)³ | 4.8 | 2.8 | 1286 | 0.000944 |

¹ Refer to the TADI 2020 research study for details on how the recommended headway values were calculated (6).

² Values are based on observations of roundabouts with two circulating (conflicting) lanes but are assumed to be similar for roundabouts with three circulating (conflicting) lanes. For roundabouts with more than two circulating (conflicting) lanes, consider conducting field investigations to verify values.

³ The TADI 2020 research study (6) did not evaluate bypass lanes for multi-circulating lane roundabouts with more than one entry lane. In absence of field data, for multi-circulating lane roundabouts with two or three lane entries, assume the slip/bypass lane has the same headway values as the right lane.

20.2.2 HCM Analysis

The WisDOT-supported traffic engineering software programs for HCM-based roundabout analysis are:

- HCS, McTrans
- SIDRA (HCM mode only), Akcelik & Associates

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. Use of the WisDOT-supported software requires calibration with the recommended Wisconsin headway values listed in [Table 20.1](#).

20.2.2.1 HCS

Consistent with the limitations of the HCM6, HCS is limited to no more than four approaches and two entry lanes with one or more bypass lanes. Partial right turn bypasses are restricted to single lanes. For other roundabout lane configurations, use of SIDRA or a supplemental tool (see [TEOpS 16-15-20.4](#)) is required.

20.2.2.2 SIDRA

SIDRA can analyze roundabouts with multiple models. When analyzing Wisconsin roundabouts, the analyst shall use the HCM6 capacity and delay models. The limitations of the HCM methodology on lane configuration has been expanded by SIDRA (U.S. mode) and the analysis can be used for all roundabouts but is specifically required for evaluating roundabouts with three entry lanes, dual partial right turn bypass lanes, or five or more approaches. SIDRA applies the basic HCM procedures and provides essentially the same results as HCS. Verify the SIDRA results for three-lane entries and dual partial bypass lanes with one of the supplemental design-aid tools discussed in [TEOpS 16-15-20.4](#).

Within SIDRA, there is the option to apply an HCM Roundabout Capacity Model extension to address unbalanced flow conditions. Additionally, SIDRA has an Extra Bunching parameter, that when checked, adjusts the proportion of platooned vehicles in the traffic stream according to the proximity of and level of queuing at an upstream signalized intersection. Prior to utilizing either the unbalanced flow model extension or the extra bunching parameter for operational analysis, the analyst *should* verify the appropriateness of their use with the WisDOT regional traffic engineer or BTO-TASU.

In addition to the HCM mode, SIDRA has its own roundabout capacity model (i.e., SIDRA Standard) which is based on Australian and international research. The analyst may use the SIDRA Standard model as a design-checking tool, but this mode is not acceptable for demonstrating that the roundabout provides sufficient capacity. See [TEOpS 16-15-20.4](#) for additional information on the use of supplemental design-aid tools.

20.2.2.3 Selecting the Appropriate Analysis Tool

Use [Table 20.2](#) as guidance in choosing the most appropriate WisDOT supported analysis tool for the specific roundabout lane configuration under consideration. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

Table 20.2 Choosing the Appropriate Analysis Tool

| Analysis Tool | Appropriate Situations |
|--------------------|---|
| HCS | One or two-lane entries, single lane partial bypasses, no more than four approach legs |
| SIDRA Intersection | One, two or three lane entries, one or two-lane partial bypasses, up to 8 approach legs |

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version of HCS and SIDRA Intersection that WisDOT currently supports.

20.2.3 Calibrating the HCM Model

20.2.3.1 HCS

In order to calibrate the HCM model within HCS, the analyst will need to enter the headway values (both critical and follow-up) for each travel lane under the Roundabout Traffic Tab. Critical and follow-up headway values shall match the accepted Wisconsin headways listed in [Table 20.1](#). The headway values entered depend on the number of entry lanes and the number of lanes circulating (conflicting) past a given entry, see [Attachment 20.1](#). Refer to the HCS Users Guide for additional details on how to modify the critical and follow-up headway values.

20.2.3.2 SIDRA

To calibrate the model within SIDRA Intersection, the analyst, after entering the intersection geometry, will need to revise the default Parameter A and Parameter B values (located under the Roundabouts – Site Input - HCM6 Data tab) to reflect the Wisconsin-specific values shown in [Table 20.1](#) (see [Attachment 20.2](#) for a screenshot of the SIDRA Roundabout HCM6 Model Parameter data entry). Note that as of October 2020, SIDRA only allows you to enter the Parameter A and Parameter B values for the following roundabout configurations:

- Single Lane Circulating, Single Lane Entry (1-1)
- Multi Lane Circulating, Single Lane Entry (1-2)
- Multi Lane Circulating, Two Lane Entry – Right (Dominant) Lane (R2-2)
- Multi Lane Circulating, Two Lane Entry – Left (Subdominant) Lane (L2-2)

For all other lane configurations, the analyst will need to enter the headway values (both critical and follow-up) for each travel lane (located under the Roundabouts – Site Input – Gap Acceptance Data tab) making sure to specify the use of “Input” values rather than the default “Program” values (see [Attachment 20.3](#) for a screenshot of the SIDRA Roundabout Gap Acceptance data entry).

SIDRA does allow for the entry of Parameter A and Parameter B values for a roundabout with a single circulating lane and two entry lanes; however, it assumes that both the right and left lane have the same values and would not allow for the use of the Wisconsin-specific values shown in [Table 20.1](#). Therefore, analysis of the single-lane circulating, two-lane entry roundabout requires the user to enter the headway values as described above.

Future releases of SIDRA may allow the use of the Parameter A and Parameter B values for additional lane configurations, eliminating the need to modify the gap acceptance data. See the SIDRA User Guide for additional details on how to modify the Parameter A and Parameter B values and Gap Acceptance Data.

20.2.3.3 Reporting

Remember to revise the headway values or Parameter A and B values if the number of entry or circulating lanes change. Once the analysis is complete, print the formatted report(s). The format for results should follow the intersection control evaluation (ICE) FDM policy ([FDM 11-25-3](#)) and the [Traffic Impact Analysis \(TIA\) guidelines](#) for reporting on operational analysis. Include the analysis files as attachments and report all queues in feet.

Contact the regional traffic engineer or BTO-TASU (DOTTrafficAnalysisModeling@dot.wi.gov) for specific guidance on how to conduct the roundabout operational analysis within any of the WisDOT supported analysis tools.

20.3 Calibration for Compact Roundabouts

HCM 6 methodology does not account for geometric parameters, such as the inscribed circle diameter (ICD), as factors in determining the operations of a roundabout. As international research has shown, changing the size of the ICD has an impact on overall operations. Therefore, without additional calibration, the current HCM 6 single-lane roundabout methodologies will not correctly estimate the operations of a compact roundabout, generally overestimating the capacity.

Currently, both the presence of compact roundabouts and subsequently the research on compact roundabouts is minimal in the US. As such, as an interim approach until more research can be completed on in-service compact roundabouts operating at or near capacity, WisDOT studied the effects of known volume-based operational parameters of larger sized roundabouts (ICDs greater than 120 ft) on the varying ICDs and volume flow rates of compact roundabouts.

The analyses, completed with WisDOT-supported software (HCS and SIDRA), resulted in the development of volume calibration factors (VCFs) for two size ranges of compact roundabouts.

20.3.1 Volume Calibration Factor (VCF) Selection

Select an initial scaling factor based on the proposed ICD size, as shown in [Table 20.3](#).

Table 20.3 Initial Volume Scale Factor

| ICD | Initial VCF |
|--------------|-------------|
| 80 – 99 ft | 30% |
| 100 – 119 ft | 35% |

If the entering AADT for the proposed intersection is less than or equal to 15,000, the initial VCF will be the final VCF to use in the operational analyses. However, if the entering AADT is greater than 15,000, the following adjustments need to be made:

1. Find the percent difference between the expected entering AADT and 15,000.
2. Subtract the percent difference from the Initial VCF for the desired ICD.
3. Apply the result as the VCF in the final analyses.

Example:

An intersection has an expected entering AADT of 18,000, with a proposed ICD of 110 ft.

1. Find the percent difference from the 15,000 AADT baseline: $[(18,000-15,000)/15,000]*100 = 20\%$
2. Subtract the difference from the Initial VCF: $35\% - 20\% = 15\%$
3. The VCF to use in the final analyses is **15%**

20.3.2 Application of VCFs using HCS

Manually scale the design year turn movement volumes by the appropriate VCF and enter them into the Traffic tab. Enter the PHF and Percent Heavy Vehicles as determined by the field count data. Enter the appropriate Wisconsin calibrated headway values as shown in [Table 20.1](#).

20.3.3 Application of VCFs using SIDRA

The following outlines the four-step process for applying the VCFs within SIDRA.

1. Start by setting up a site in SIDRA based on the instructions in [TEOpS 16-15-20.2.2.2](#) and [TEOpS 16-15-20.2.3.2](#) and the associated attachments: Attachment 20.2 and Attachment 20.3. Enter the volumes, PHF, and Percent Heavy Vehicles as determined by the field count data.
2. Set the Roundabouts Site Input to use the US HCM 6 Roundabout Capacity Model.
3. To apply the VCF, open the Volumes window and activate the Volume Factors tab. Enter the VCF into the Flow Scale (Constant) cells under each movement, for every leg of the site. The new Flow Scale will be the VCF + 100, as shown in [Figure 20.3](#).
4. Process the site as normal.

Figure 20.3 VCF Input as the Flow Scale

Vehicle Volumes | Volume Factors

Import Volume Data | Quick Input | View Display

Approach Selector

Volume Factors

| From South to Exit: | W | N | E |
|------------------------|---------|---------|---------|
| | ↶ L2 | ↶ T1 | ↷ R2 |
| Peak Flow Factor | 76.0 % | 76.0 % | 76.0 % |
| Flow Scale (Constant) | 135.0 % | 135.0 % | 135.0 % |
| Growth Rate (per year) | 2.0 % | 2.0 % | 2.0 % |

NB STH 80

Movement Class

All Movement Classes
 Light Vehicles (LV)
 Heavy Vehicles (HV)

Dialog Tips

Help | OK | Cancel | Apply | Process Site

20.4 Supplemental Tools for Operational Analysis and Design

When performing roundabout operational evaluations, analysts should recognize and account for the methodological limitations of the HCM. There are cases that may not fit within the analytical framework of the HCM, including but not limited to: volume-to-capacity ratio exceeding 0.80, high level of pedestrian or bicycle activity, priority reversal under extremely high flows, and flared entry lanes. When the volume-to-capacity ratio exceeds 0.80, the analyst should carefully consider predicted queues and delays and perform additional sensitivity analysis. Further analysis with microsimulation or design-aid tools such as SIDRA Standard, Rodel, or ARCADY can supplement the study if the effort is justifiable based on the site conditions.

Microsimulation is capable of system level analysis and allows the analyst to adjust roundabout designs indirectly. Additionally, microsimulation that provides for animation and visualization of operating predictions is useful for assessing lane utilization and capacity, especially when considering closely spaced roundabouts. Analysis with microsimulation may help identify lane imbalances or lane use problems within a series of intersections allowing for a more robust design of any single roundabout. Microsimulation may also prove beneficial for public outreach. Since microsimulation requires significantly more time, resources and effort than HCM-based analysis, it is not appropriate to use for all roundabout analysis or design. Refer to [TEOpS 16-10](#) for additional guidance on determining whether the use of a microsimulation tool would be appropriate. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

SIDRA Standard, Rodel, ARCADY and any other tool that designers have available to assist them in the design process can prove beneficial for the final geometric design of the roundabout. These programs provide for geometric sensitivity testing, allowing the user to test the effects of size and key geometric parameters (i.e., inscribed circle diameter, entry radius, phi angle, lane width and flared entry) along with varied flows on an existing or proposed roundabout design.

Rodel and ARCADY apply UK research producing a model that relates geometry to capacity, for roundabout capacity calculations. SIDRA Intersection, when used in Standard mode, implements a capacity estimation method that assumes a dependence of gap acceptance parameters on multiple factors. Roundabout geometry, circulating flows, entry lane flows, and model designation of dominant or subdominant lanes all influence gap acceptance parameters to account for lane-by-lane capacity variation. SIDRA Standard utilizes what they call the Environment Factor as one of the main parameters to calibrate the capacity model. The recommended Environment Factor for U.S. roundabouts is 1.05 for one-lane roundabouts (approach road or circulating road has one lane) and 1.2 for multi-lane roundabouts (both approach road and circulating road have two or more lanes). See the SIDRA Intersection User Guide Calibration Parameters for Roundabout Capacity Models for details on how to apply the Environment Factor in the SIDRA capacity model.

20.5 Special Considerations

Lane designation or lane assignments are critical to the success of the roundabout lane configuration and design. Conditions can be very complex with subtle problems that can reduce capacity and cause severe lane imbalance. Great care and sensitivity are required to achieve lane utilization balance. Supplementary software is especially suited to these situations.

- Unbalanced Conflicting Flows:

At a roundabout with unbalanced conflicting flow patterns, a traffic stream with a low flow rate enters the roundabout having to yield to a circulating stream with a high flow or visa-versa.

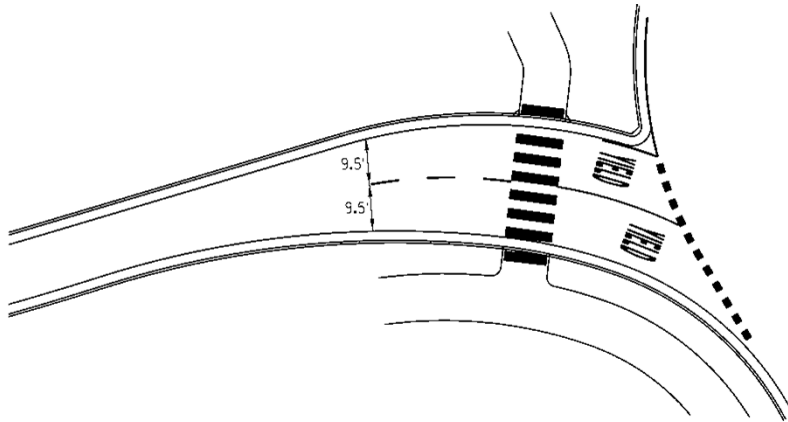
Unbalanced circulating flows highlight an operational condition that traffic engineers and designers should understand and interpret by taking into consideration all aspects including but not limited to the results of the analysis, the existing and future field conditions, and traffic patterns in order to better inform the findings on the analysis. The SIDRA Standard capacity model is sensitive to the ratio of entering to circulating flow, and therefore may be able to reflect expectations of capacity when unbalanced flow conditions are expected. A microsimulation model can also supplement the analysis, but the level of data and effort to calibrate this model can be significant and may not be appropriate for an isolated roundabout analysis.

- Capacity Considerations of Flared Entries:

In some situations, the use of appropriate lane arrows can encourage balanced lane use, thus improving capacity. Traffic often has a bias towards the right-most lane. Lane arrows either can encourage this bias or can encourage lane balance. [Figure 20.4](#) shows the preferred pavement marking scheme to encourage balanced lane demand. It is important for the analyst not to assume that flared entries at roundabouts will always provide for balanced lane use and therefore add capacity to that entry as HCS and SIDRA will predict. This scenario may occur on the approach to a roundabout that has little to no conflicting circulating traffic (e.g., a roundabout at an interchange

ramp or any roundabout with a one-way street). The suitable marking for an approach will depend on the turning volume proportions. A methodology similar to that described in [FDM 11-26-20.4.3](#) is used to assess lane designation alternatives.

Figure 20.4 Capacity Considerations of Flared Entries



In addition, assessment of the potential for one lane to fill and block the flared lane is necessary to achieve the predicted levels of service, (i.e., the geometry must be effective to match the capacity prediction). Lane starvation is a primary failure mechanism for flared entries. Microsimulation models have various forms of lane-by-lane simulation features, which allow the analyst to test alternative lane configurations with visualization of the simulated flows accumulating and filling the flared lanes.

LIST OF ATTACHMENTS

- [Attachment 20.1](#) Roundabout Lane Configurations
- [Attachment 20.2](#) SIDRA Roundabout Parameters Screenshot
- [Attachment 20.3](#) SIDRA Gap Acceptance Screenshot

16-15-25 Alternative Intersections

September 2019

Alternative intersections separate out one or more of the turning movement conflicts (typically left-turns) by rerouting them away from the center of the intersection to a secondary junction. Alternative intersections may be signalized or stop-controlled on the minor street movements. Examples of alternative intersections include, but are not limited to, the following:

- Restricted Crossing U-Turn (RCUT), also known as the J-Turn or superstreet,
- Median U-Turn (MUT), also known as the Michigan left turn or modified J-Turn, and
- Displaced Left Turn (DLT), also known as the continuous-flow intersection

Refer to [FDM 11-25 Attachment 3.3](#) for a brief description, summary of the key elements to consider, and some of the potential benefits/concerns associated with these alternative intersections.

By rerouting one or more of the turn movements away from the center of the primary intersection, alternative intersections result in two or more closely spaced intersections that are operationally dependent on one another. Thus, the analyst should treat these intersections as a single unit.

WisDOT accepts the use of HCM6, Chapter 23 to assess the performance of the RCUT, MUT, and DLT from the perspective of the motor vehicle, pedestrian, and bicycle modes. Note that the Chapter 19 signalized methodology for pedestrians and bicycles is typically applicable for the minor street crossings at a signalized RCUT and for all crossings at the signalized MUT. The HCM6, Chapter 23 methodology provides a means to measure experienced travel time and considers the control delay experienced at each intersection plus the additional travel time needed to travel from the primary/center intersection to the secondary junction and back to the primary/center intersection.

Analysts should recognize and account for the methodological limitations of the HCM methodology. Specifically, the analyst should bear in mind that the analysis methodology is relatively new. Additionally, the HCM Chapter 23 methodology is only applicable to the RCUT, MUT, and DLT. Consider using microsimulation analysis tools for those alternative intersections that do not fit within the methodological limitations of the HCM. See [TEOpS 16-20](#)

for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software for HCM-based analysis of alternative intersections is:

- HCS, McTrans

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

Trafficware has not yet implemented the HCM methodology for alternative intersections within Synchro; however, the analyst may be able to manipulate the coding within Synchro to analyze these intersections in accordance with the HCM6 methods. Confirm with the WisDOT regional traffic engineer whether it is appropriate to utilize Synchro for the analysis of alternative intersections.

16-15-30 Interchange Ramp Terminals

September 2019

The close spacing and interdependency of most ramp terminals requires that the operational analysis consider all ramp terminals within the interchange as a single unit. WisDOT accepts the use of HCM6, Chapter 23 for the analysis of interchange ramp terminals. As no specific methodologies for pedestrian and bicycle operations at interchange ramp terminals currently exist, the HCM6, Chapter 23 methodologies for interchange ramps focus on the motor vehicle mode. Chapter 23, however, does provide some guidance for addressing bicycles and pedestrians at interchanges.

The HCM6, Chapter 23 methodology addresses the following conventional interchange designs:

- Diamond interchanges,
- Partial cloverleaf (parclo) interchanges, and
- Interchanges with roundabouts.

Additionally, the HCM6, Chapter 23 methodology addresses the following alternative interchange designs:

- Diverging diamond interchanges (DDIs) and
- Single-point interchanges (SPI).

Refer to [FDM 11-25 Attachment 3.3](#) for a brief description, summary of the key elements to consider, and some of the potential benefits/concerns associated with each of these interchange designs.

The HCM6, Chapter 23 methodology calculates the control delay experienced at each ramp terminal plus any additional travel time associated with driving between ramp terminals within the interchange. This allows for an equal comparison of the various interchange designs.

The analysts should recognize and account for the methodological limitations of the HCM6, Chapter 23 methods. Specifically, the analyst should bear in mind that the analysis methodology is not applicable for freeway-to-freeway or system interchanges. Additionally, the methodology does not cover interchanges with TWSC intersections or interchanges consisting of both a signalized and roundabout intersection. Consider using microsimulation analysis tools for those interchanges that do not fit within the methodological limitations of the HCM. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software programs for HCM-based analysis of interchange ramp terminals are:

- HCS, McTrans
- Synchro, Cubic|Trafficware (conventional ramp terminals only)

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

Trafficware has not yet implemented the HCM methodology for the alternative interchange ramp terminals (e.g., DDI, SPI) within Synchro; however, the analyst may be able to modify the coding within Synchro to analyze these types of interchange ramp terminals in accordance with the HCM6 methods. Confirm with the WisDOT regional traffic engineer whether it is appropriate to utilize Synchro for the analysis of the alternative interchange ramp terminals.

16-15-35 Urban Street Facilities**September 2019**

WisDOT accepts the use of the HCM6, Chapters 16 and 18 for an integrated multimodal analysis of an urban street facility, including the intersections and segments that make up the facility. The methodology provides the analytical framework to assess the automobile, pedestrian, bicycle, and transit modes by calculating delay and other performance measures by mode for each direction of travel along each segment of the given urban street facility, in addition to mid-block access points and other study intersections. The analyst *should* also consider the methods for TWSC, AWSC, roundabouts, and signalized intersections to the extent that those facilities exist along the subject roadway.

For intersections along an urban arterial or collector street that do not operate in isolation (i.e., the operation of one intersection influences the operation of the adjacent intersection), follow the Chapter 18 Urban Street Segment methodology. If the project spans multiple contiguous urban street segments, consider applying the Chapter 16 Urban Street Facilities methodologies. The Chapter 16 Urban Street Facilities methods allow the analysis of corridors of coordinated signalized intersections to capture average-phase-duration and other analytical components related to progression and vehicular platooning. If travel time reliability performance measures are of interest, consider using the urban street reliability methodologies in HCM6, Chapter 17. For additional information on incorporating travel-time reliability into the analysis, contact BTO-TASU (DOTTrafficAnalysisModeling@dot.wi.gov).

Analysts *should* recognize and account for the methodological limitations of the HCM urban streets methods. Accordingly, limitations of the individual intersection methods are also limitations of the urban street methods. For urban street facilities that do not fit within the analytical framework of the HCM, including but not limited to cases involving turn-lane spillover, mid-block parking maneuvers, or capacity constraints between intersections, the project manager *should* specify the use of an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software programs for HCM-based urban streets analysis are:

- HCS, McTrans
- Synchro, Cubic|Trafficware

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

16-15-40 Freeway Facilities**September 2019**

WisDOT accepts the use of the HCM6 analysis methods in Chapter 10 for a combined freeway facility, Chapter 11 for freeway reliability analysis, Chapter 12 for basic freeway segments, Chapter 13 for freeway weaving segments and Chapter 14 for freeway merge and diverge segments. Analysts *should* use these methods to assess uninterrupted flow facilities that typically have restricted access and consist of higher-speed roadways through rural, suburban, and urban areas. Since there is no pedestrian/bicycle traffic on freeways, the HCM methodology focuses on the vehicular travel mode of travel. For additional information on incorporating travel-time reliability into the analysis, contact BTO-TASU (DOTTrafficAnalysisModeling@dot.wi.gov).

Analysts *should* recognize and account for the methodological limitations of the HCM methods for freeway analysis. The methodology does not account for off-ramp or surface street conditions affecting the performance of the freeway. In those cases, the project manager *should* specify the use of an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software for HCM-based freeway analysis is:

- HCS, McTrans

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

16-15-45 Multilane Highways**September 2019**

WisDOT accepts the use of the HCM6, Chapter 12 methods for the analysis of an expressway or multilane highway. The methodology provides the analytical framework to assess the automobile and bicycle modes of travel. The analyst *should* use these methods to assess uninterrupted flow on multilane highway facilities with

free-flow speeds between 45 and 70 mph, and two miles or more between traffic signals. These facilities may be divided, undivided, or have a two-way left-turn lane (TWLTL).

Many multilane highways will have periodic signalized intersections that are more than two miles apart. In these cases, the analyst *should* evaluate the highway segment portion using the Chapter 12 method and evaluate the isolated intersection using the signalized intersection analysis tools outlined in [TEOpS 16-10-5](#).

Analysts *should* recognize and account for the methodological limitations of the multilane highway methods. For multilane highway conditions that do not fit within the analytical framework of the HCM, including but not limited to: effect of lane drops and lane additions at the beginning or end of the multilane highway segment, queuing impacts at transition areas (i.e., transitions from a multilane to two-lane highway), significant presence of on-street parking, or significant pedestrian activity, the analyst *should* use an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software for HCM-based multilane highway analysis is:

- HCS, McTrans

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

16-15-50 Two-Lane Highways

September 2019

WisDOT accepts the use of the HCM6, Chapter 15 methods for the analysis of a two-lane highway. The methodology provides the analytical framework to assess the automobile and bicycle modes of travel. Use these methods to assess uninterrupted flow (i.e., there are no traffic control devices that interrupt traffic) on two-lane highways that have one lane in each direction. Passing takes place on these facilities in the opposing lane of traffic when there is adequate sight distance and safe gaps in the opposing traffic. The two-lane highway methodology also includes a procedure for predicting the effect of passing and truck climbing lanes on two-lane highways.

In general, this analysis includes any segments that have signalized intersections spaced two or more miles apart. Classify two-lane highways with signalized intersections spaced closer than two miles apart as an urban street or arterial and apply the methodologies of HCM, Chapter 16 as appropriate. Further, analyze any major signalized or unsignalized intersections within the two-lane highway corridor using the appropriate tools as outlined in [TEOpS 16-10-5](#).

Analysts *should* recognize and account for the methodological limitations of the two-lane highway methods. The HCM-methodology does not model counter-directional passing, and thus the analyst *should* only use the HCM-methodology for two-lane highway analysis if passing maneuvers are infrequent in the study area. If counter-directional passing is critical within the study area, the analyst *should* consider using an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis. (Note that currently Vissim is the only WisDOT-supported microsimulation tool that considers counter-directional passing.)

The WisDOT-supported traffic engineering software for HCM-based two-lane highway analysis is:

- HCS, McTrans

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

16-15-60 Pedestrian and Bicycle Facilities

September 2019

60.1 Mid-Block Pedestrian Crossings

WisDOT accepts the use of the methods outlined by the HCM6, Chapter 20 (pages 20-37 through 20-44) for one-stage and two-stage unsignalized mid-block pedestrian crossings, with or without a median refuge area, which are not located at an intersection. Assess the operations of mid-block pedestrian crossings by calculating seconds of delay per pedestrian or pedestrian-group.

Wisconsin-state law requires motorists to yield to pedestrians at designated mid-block pedestrian crossings. Motorist compliance, however, can vary. Implementation of pedestrian crossing treatments that are proven safety countermeasures (e.g., high visibility crosswalk markings, median refuges, and rectangle flashing beacons or

pedestrian hybrid signals) have shown to increase motorist compliance rates and reduce pedestrian crashes. In the absence of local data, and subject to professional judgment, use the default motorist-yield-rates as recommended in the HCM6, Chapter 20 (Exhibit 20-24) for the analysis of mid-block pedestrian crossings.

Analysts should recognize and account for the methodological limitations of the mid-block pedestrian crossing methods (i.e., TWSC pedestrian mode method). For mid-block pedestrian crossings that do not fit within the analytical framework of the HCM, including but not limited to, signalized mid-block crossings or cases where the impact on the major street vehicular traffic is relevant, the project manager should specify the use of an alternative tool such as microsimulation. See [TEOpS 16-20](#) for additional details on performing microsimulation analysis.

The WisDOT-supported traffic engineering software for HCM based mid-block pedestrian crossing analysis are:

- HCS, McTrans
- Synchro, Cubic|Trafficware

Refer to the [BTO Traffic Analysis, Modeling and Data Management Program area webpage](#) for the version and build of the above software that WisDOT currently supports. See [TEOpS 16-10-5](#) for additional guidance on how to select the most appropriate traffic analysis tool for a specific project.

60.2 Off-Street Pedestrian and Bicycle Facilities

WisDOT accepts the use of the HCM6, Chapter 24 methods for the analysis of off-street pedestrian and bicycle facilities (i.e., non-motorized vehicle usage only). The methodology provides the analytical framework to assess the capacity and LOS for the following types of facilities:

- Walkways: pedestrian-only paved facilities (paths, ramps, and plazas) typically located more than 35 feet from an urban street
- Shared-use paths: paths, separated by a physical barrier from highway traffic, dedicated for the shared-use of all forms of non-motorized (pedestrian, bicyclists, runners, inline skaters, etc.)
- Exclusive off-street bicycle paths: separated by a physical barrier from highway traffic, dedicated for bicycle-only traffic

Analysts *should* recognize and account for the methodological limitations of the HCM. For off-street pedestrian and bicycle facilities that do not fit within the analytical framework of the HCM, the project manager *should* specify the use of an alternative tool.

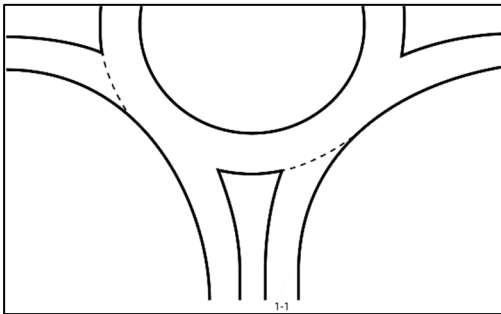
WisDOT has not currently identified a specific analysis tool for analyzing off-street pedestrian and bicycle facilities. Direct any specific questions regarding the analysis of off-street pedestrian and bicycle facilities to the WisDOT regional and statewide bicycle and pedestrian coordinators.

16-15-70 References

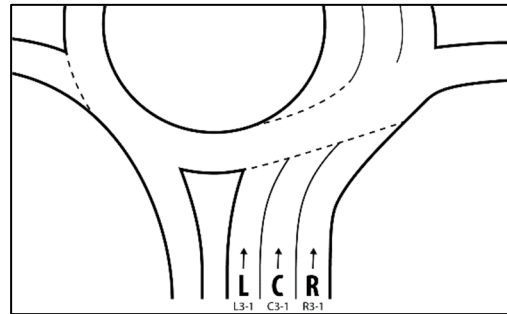
November 2020

1. **Transportation Research Board.** *Highway Capacity Manual, 6th Edition: A Guide For Multimodal Mobility Analysis.* Washington, D.C. : National Academy of Sciences, 2016. ISBN 978-0-309-36997-8.
2. **R.A. Smith National.** *Right Turn on Red Methodology Evaluation.* 2009.
3. **TranSmart Technologies, Inc.** *Signalized Intersection Capacity Data Collection: A Statewide Evaluation of Saturation Flow Rate and Right Turn on Red.* 2015.
4. **NCHRP 572, Roundabouts in the United States.** 2007.
5. **Rodegerdts, L.A., A. Malinge, P.S., Marnell, S.G. Beard, M.J. Kittelson, and Y.S. Mereszczak.** *Assessment of Roundabout Capacity Models for the Highway Capacity Manual: Volume 2 of Accelerating Roundabout Implementation in the United States.* Washington, D.C. : Federal Highway Administration, Sept. 2015. Report FHWA-SA-15-070.
6. **Traffic Analysis and Design, Inc. (TADI).** *Statewide Roundabout Traffic Operations Analysis.* March 2020.

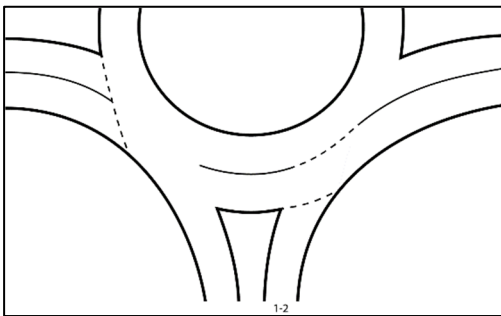
One-Lane Entry Conflicted by One Circulating Lane (1-1)



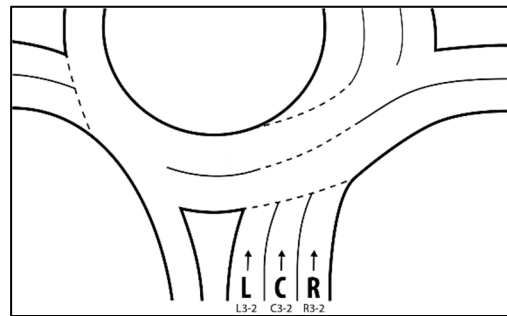
Three-Lane Entry Conflicted by One Circulating Lane (L3-1/C3-1/R3-1)



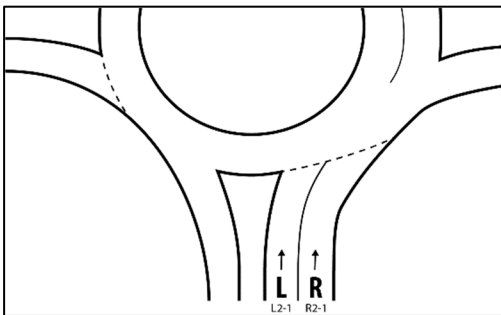
One-Lane Entry Conflicted by Two Circulating Lanes (1-2).



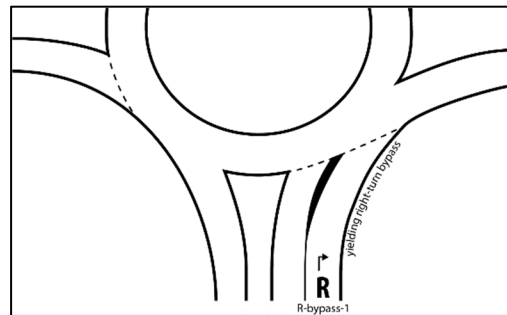
Three-Lane Entry Conflicted by Two Circulating Lanes (L3-2/C3-2/R3-2)



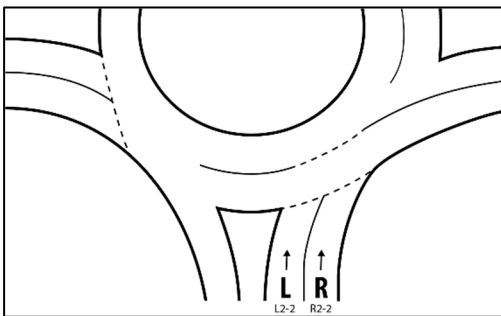
Two-Lane Entry Conflicted by One Circulating Lane (L2-1/R2-1)



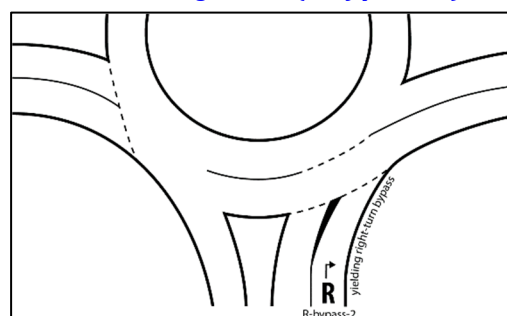
Right Yielding Bypass Lane Conflicted by One Exiting Lane (R-bypass - 1)



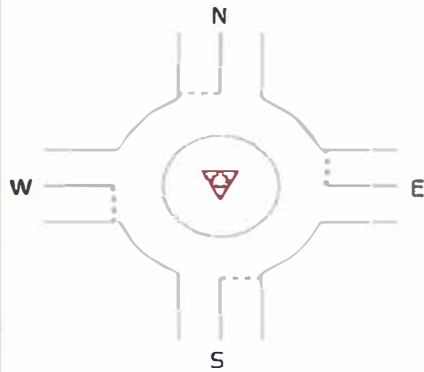
Two-Lane Entry Conflicted by Two Circulating Lanes (L2-2/R2-2).



Right Yielding Bypass Lane Conflicted by Two Exiting Lanes (R-bypass - 2)



Site Display



Current Capacity Model:
US HCM 6

For easy calibration, use Model Calibration Factor (HCM 6).

Geometry data in the Roundabout Data tab are not applicable when the HCM 6 roundabout capacity model is selected in the Options tab.

HCM 6 Roundabout Capacity Model Parameters

| Approach: | S | E | N | W |
|---|----------|----------|----------|----------|
| Single Lane Circulating: Single Lane Entry | | | | |
| Parameter A | 1385 | 1385 | 1385 | 1385 |
| Parameter B | 0.000944 | 0.000944 | 0.000944 | 0.000944 |
| Single Lane Circulating: Multi-Lane Entry | | | | |
| Parameter A | 1440 | 1440 | 1440 | 1440 |
| Parameter B | 0.000908 | 0.000908 | 0.000908 | 0.000908 |
| Multi-Lane Circulating: Single Lane Entry | | | | |
| Parameter A | 1385 | 1385 | 1385 | 1385 |
| Parameter B | 0.000972 | 0.000972 | 0.000972 | 0.000972 |
| Multi-Lane Circulating: Dominant Lane | | | | |
| Parameter A | 1385 | 1385 | 1385 | 1385 |
| Parameter B | 0.000833 | 0.000833 | 0.000833 | 0.000833 |
| Multi-Lane Circulating: Subdominant Lane | | | | |
| Parameter A | 1385 | 1385 | 1385 | 1385 |
| Parameter B | 0.000917 | 0.000917 | 0.000917 | 0.000917 |

This assumes the same values for all entry lanes and is not applicable for the WI-Calibrated model. For this configuration, modify the Gap Acceptance (Critical Gap and Follow-up Headway) values.

By default, the right lane is considered to be the dominant lane. For three entry lanes, modify the Gap Acceptance (Critical Gap and Follow-up Headway) values.

By default, the left lane is considered to be the subdominant lane. For three entry lanes, modify the Gap Acceptance (Critical Gap and Follow-up Headway) values.

HCM 6 Roundabout Model Calibration

| Approach: | S | E | N | W |
|----------------------------------|------|------|------|------|
| Model Calibration Factor (HCM 6) | 1.00 | 1.00 | 1.00 | 1.00 |
| Entry/Circ Flow Adj (HCM 6) | None | None | None | None |

The calibration parameters shown in this table apply to the HCM 6 roundabout capacity model only and do not apply to the SIDRA Standard model.

Modify the default Parameter A and B values (highlighted in yellow above) to reflect the Wisconsin-specific values shown in TEOpS 16-15-20, Table 20.1.

Dialog Tips

Help

OK

Cancel

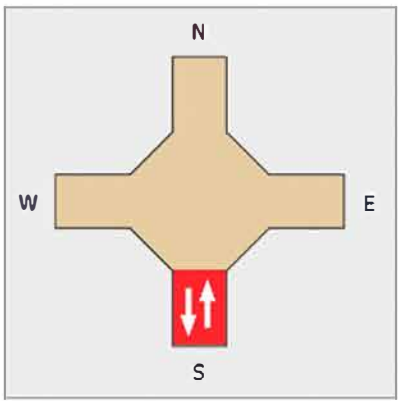
Apply

Process Site

Gap Acceptance Data

Reset to Defaults Quick Input View Display

Approach Selector



South Leg

Gap Acceptance Data

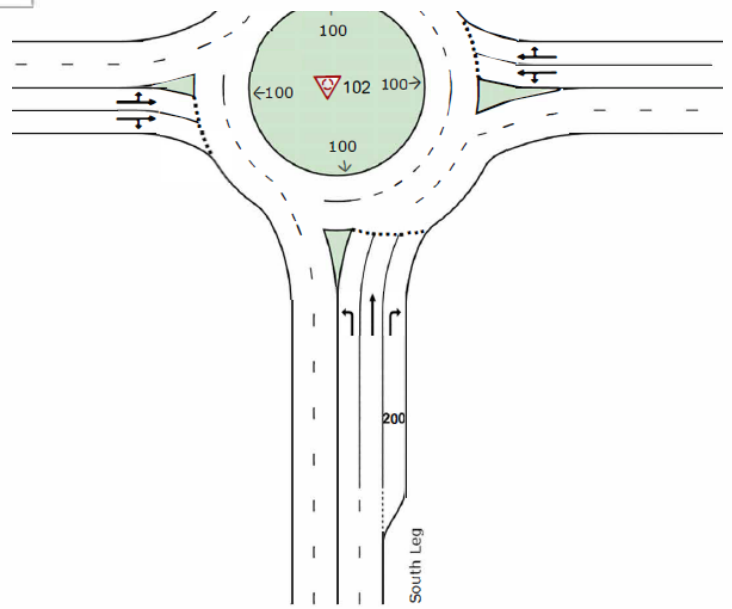
| From South to Exit: | W | N | E |
|---|-----------|-----------|-----------|
| | L2 | T1 | R2 |
| Vehicle Movements Opposing | | | |
| | Input | Input | Input |
| Critical Gap | 4.60 sec | 4.40 sec | 4.60 sec |
| Follow-up Headway | 2.50 sec | 2.40 sec | 2.50 sec |
| Minimum Departures (vehicles per minute) | 2.5 | 2.5 | 2.5 |
| Exiting Flow Effect | 0 % | 0 % | 0 % |
| Percent Opposed by Nearest Lane Only | 0.0 % | 0.0 % | 0.0 % |
| Main Crossing Pedestrians Opposing | | | |
| Opposing Peds (Unsig) | Prg(Flow) | Prg(Flow) | Prg(Flow) |

Select "Input"

Enter WI-Specific Critical Gap Value

Enter WI-Specific Follow-up Headway Value

South Leg Geometry:
 2 Circulating (Conflicting) Lanes
 3 Entry Lanes



Dialog Tips

Help OK Cancel Apply Process Site