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WIND TURBINE DESIGN AND IMPLEMENTATION


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By



Bethany Kuhn



Julie Marquis



Hilary Rotatori

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Approved:

Professor Leonard D. Albano, Advisor


Abstract


This project examined the design of a land-based wind turbine considering various alternatives including soil and foundation type, turbine size and type, tower design, type of site, and wind speeds. In addition, a cost analysis of the chosen wind turbine design was completed. An integrated design sheet was produced for the development of a wind turbine. Once the primary factors in the design of a land-based turbine were determined, the implications for an offshore turbine were investigated. Based on research and the cost and structural analyses, conclusions and recommendations for appropriate designs were established.

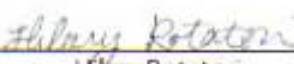
Authorship

All members of the project group, Bethany Kuhn, Julie Marquis and Hilary Rotatori, made an equal contribution to the major qualifying project. The following sections were written by the specified person:

Section	Author
Capstone Design	Hilary Rotatori
Introduction	Julie Marquis
Background	Bethany Kuhn, Julie Marquis, Hilary Rotatori
Methodology	Bethany Kuhn, Julie Marquis, Hilary Rotatori
Tower Design	Julie Marquis
Foundation Design	Hilary Rotatori
Project Analysis	Bethany Kuhn
Offshore Turbine Design	Bethany Kuhn, Julie Marquis, Hilary Rotatori
Conclusions	Julie Marquis, Hilary Rotatori


Bethany Kuhn


Julie Marquis


Hilary Rotatori

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Capstone Design Statement

This project addressed the design constraints set by the Accreditation Board for Engineering and Technology (ABET) in order to meet the requirement of capstone design experience for the Major Qualifying Project. As stated by ABET General Criterion 4, “Student must be prepared for engineering practice through the curriculum culminating in a major design experience based on the knowledge and skills acquired in earlier course work and incorporating engineering standards and realistic constraints that include most of the following considerations: economic; environmental; sustainability; manufacturability; ethical; health and safety; social; and political.”¹ In addition to these constraints, this project considered the American Association of Civil Engineers (ASCE) Commentary on “Engineering Design.”

Economic

One of the more important constraints on large-scale civil engineering projects is economic feasibility. The design of the project considered the cost of construction and the materials used. In order to ensure that the project is affordable, a cost analysis of alternative designs and materials was performed to help choose a design. A cost analysis of the final design was also conducted to determine the economic impact of the turbine once it is in operation.

Environmental

The environmental impacts of projects have been a rising concern in the civil engineering industry in recent years. It is important to protect the land surrounding the construction site and to investigate the affects of the wind turbine on wildlife. This project addressed environmental constraints by reviewing environmental codes and regulations regarding the construction and operation of the turbine. The codes that were reviewed involved studies that must be performed in order to obtain a permit for the wind turbine project. These studies include avian and bat interaction; wildlife, plants and wetlands studies; archaeological and historical reviews; stream crossing and soil disturbance.² This investigation allowed the group to review the typical regulations that should be considered before a turbine is constructed.

¹ UCSC CE Department's fulfillment of the Major Design Experience. abet.soe.ucsc.edu/design.html (2009, February 20).

² National Wind. (September 2009), www.nationalwind.com

Sustainability

Sustainability is an important aspect of the design of the wind turbine because the design should be durable as well as economical and environmentally-friendly. The installation of a wind turbine itself is sustainable as it is a source of renewable energy. This project addressed sustainability by considering the maintenance to be done on the turbine in the future. The availability of the materials used was also taken into account. By using innovative designs such as offshore turbines land can be preserved. In addition, a life-cycle cost analysis was conducted, including an estimation of the “payback” time for the construction of the turbine.

Constructability

The constructability of the project addressed the feasibility of the design and construction of the turbine. This project addressed this constraint by considering ease of construction when choosing building materials, and researching the most practical methods for the construction of a turbine tower and foundation. The group considered using standard dimensions for the tower height and cross-section and a standard turbine power so that the turbine could be easily constructed and replicated if successful. In addition, the tower was designed using a modular approach to facilitate the construction process and limit the necessary site operations.

Ethical

In order to address the ethical constraints, the group used the principles outlined by the American Society of Civil Engineers (ASCE) Code of Ethics which states, “Engineers uphold and advance the integrity, honor and dignity of the engineering profession by using their knowledge and skill for the enhancement of human welfare and the environment, being honest and impartial and serving with fidelity the public, their employers and clients, striving to increase the competence and prestige of the engineering profession, and supporting the professional and technical societies of their disciplines.”³ This project considered these principles in all aspects of the design and construction plans.

Health and Safety

In order to address health and safety considerations, a structural analysis of the tower and foundation design materials under different forces was performed. The research performed on zoning and location restrictions was considered in the design process to ensure that the turbine is an appropriate distance from its surroundings.

³ *Code of Ethics*, (September 2009), www.asce.org/inside/codeofethics.cfm

Social and Political

Since wind energy is a relatively new technology, society has not fully embraced the idea of wind turbines being constructed in their communities. This project addressed social and political constraints by considering the process of installing a wind turbine in a specific community and by researching the people and steps involved. Once these processes were understood, a flow chart of the process in terms of people or organizations who are involved was prepared. In addition, the group examined case studies in order to determine strategies by which wind turbines have been successfully integrated into various communities.

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Chapter 1: Introduction

As the demand for more environmentally-friendly energy resources grows, energy providers have recognized the importance of wind power and have invested in the development of wind turbines. In fact, wind energy is the only renewable resource that has grown faster than predicted.⁴ At the end of 2007, the wind energy generating capacity in the United States was 16,818 MW.⁵ In 2008 alone, 8,358 MW of wind energy was added⁶ and in 2009, an additional 9,922 MW of wind energy was added.⁷ As of the start of 2010, the wind generating capacity increased to 35,098 MW, more than doubling the wind energy capacity since 2007. The U.S. Department of Energy predicts that wind energy will provide at least 20 percent of the nation's electricity by 2030.⁸

The primary reason behind the recent peak of wind energy capacity is due to improved turbine technology.⁹ Several components of a wind turbine must be taken into account, including the tower, blades, nacelle, and foundation designs. New developments in the construction of taller structures using better, lightweight materials, and improved turbine design techniques have allowed today's taller turbines to tap better winds at higher elevations for reduced costs. As a result of the improved turbine designs using lighter-weight steel for the tower, smaller, lighter foundations can be used and in turn reducing costs. It is imperative to continue improving the design of the wind turbine in order to harvest better energy and optimize its cost.

Wind turbine towers and foundations must be designed to withstand heavy loads and moments due to extreme wind conditions to prevent failures, as well as other forces that are introduced with alternative site designs. The forces that the tower and foundation must resist are wind loads, ice loads, and the self-weight of the tower. The tower structure must also resist earthquake loads, which can be designed

⁴ Singh. *Concrete Construction for Wind Energy Towers*

http://www.icjonline.com/views/POV_Wind_energy_tower.pdf (November 2009)

⁵ *Wind Energy Fast Facts*, American Wind Energy Association, http://www.awea.org/newsroom/pdf/Fast_Facts.pdf (September 2009).

⁶ *Wind Energy Basics*, American Wind Energy Association, http://www.awea.org/newsroom/pdf/Wind_Energy_Basics.pdf (September 2009).

⁷ *U.S. Wind Energy Industry Installed Nearly 10,000 MW in 2009*, Energy Efficiency and Renewable Energy http://apps1.eere.energy.gov/news/news_detail.cfm/news_id=15794 (February 2010).

⁸ http://www.awea.org/newsroom/pdf/Wind_Energy_Basics.pdf

⁹ *U.S. Wind Resource Even Larger than Previously Estimated*, American Wind Energy Association, http://www.awea.org/newsroom/releases/02-18-10_US_Wind_Resource_Larger.html (October 2009).

based on checking resistance in the steel's plastic range.¹⁰ In addition, the soil has to have adequate bearing capacity to resist the loads on the tower and weight of the foundation.

The construction of a wind turbine is limited to specific site guidelines due to social, political, and environmental constraints; therefore there is an alternative need for sites other than land-based turbines, such as offshore applications. Offshore wind turbines are becoming increasingly favored because of the high wind speeds off the coast, and the fact that there is little or no effect in terms of noise because of their distance from the shoreline; however, aesthetics may pose an issue with area residents who feel the turbines adversely affect the natural seascape of an area. Additionally, offshore turbines must harvest more energy to offset increased construction costs due to large underwater support structures and foundations.¹¹

The purpose of this project was to design a land-based wind turbine as well as explore designs for offshore options for the energy needs of the chosen site. To begin studying the construction of wind turbines, a suitable site must be chosen based on a given criteria. The effect of loading conditions is evaluated on the wind turbine tower and foundation for its design. This project investigated and drew recommendations on the process of constructing two different wind turbine heights with alternative foundations as well as a cost analysis of the designs.

In addition to the design of the tower and foundation, one of the main deliverables for this project was an integrated spreadsheet, which was developed to effectively suggest a suitable tower and foundation design and the corresponding cost data. The creation of this spreadsheet allowed for the investigation of various design alternatives.

This report will contain an overview of wind energy and the current status of wind turbines as an effective producer of renewable energy, a methodology of how the project was completed, chapters detailing the tower design, foundation design, cost-estimation, and the investigation of offshore wind turbines, followed by a conclusion chapter, which will summarize the key findings and discuss limitations and suggestions for future work related to our research and analysis.

¹⁰ *Manual of Steel Construction LRFD, 3rd ed* (2001). American Institute of Steel Construction Inc.

¹¹ Malhotra, Sanjeev. "Design and Construction Considerations for Offshore Wind Turbine Foundations in North America". *Civil Engineering Practice*, Volume 24, Number 1. 2009.

Chapter 2: Background

As energy needs are continuously changing, the structural elements associated with the new energy sources need to be developed. An understanding of all the energy, mechanical, construction, and structural components related to the design, construction, and operation of wind turbines must be evaluated to develop an understanding for the design and management of this project.

Harvesting Wind Energy

Wind energy is defined as the process by which wind is used to generate mechanical power and electricity and is the world's fastest-growing energy source.¹² Figure 1: U.S. Wind Energy

Production Figure 1 below shows the wind energy capacity, or the amount of energy produced by the United States from 1999 to 2009.¹³

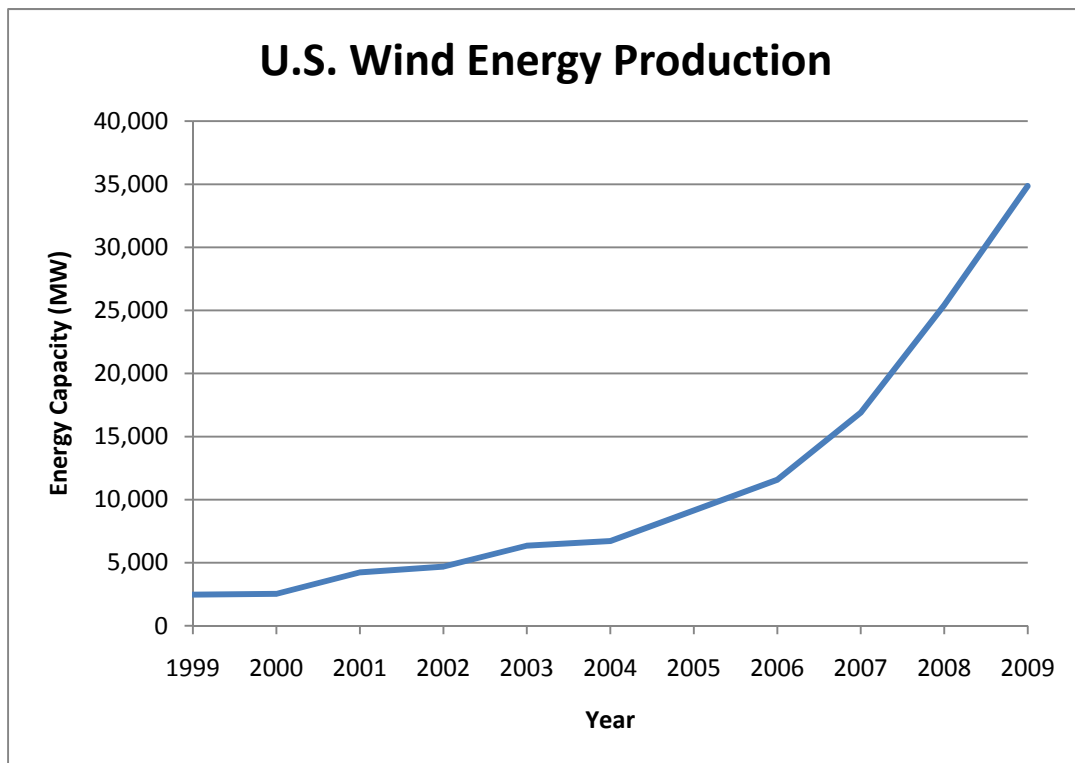


Figure 1: U.S. Wind Energy Production¹⁴

¹² US Department of Energy: Energy Efficiency and Renewable Energy. Wind & Hydropower Technologies Program. www1.eere.energy.gov/windandhydro/wind_technologies.html (October 2009).

¹³ Installed U.S. Capacity and Wind Project Locations. U.S. Department of Energy. http://www.windpoweringamerica.gov/wind_installed_capacity.asp (January 2010).

¹⁴ http://www.windpoweringamerica.gov/wind_installed_capacity.asp.

From 2007-2009 alone, wind energy production increased 109 percent. It is predicted that wind energy will provide at least 20 percent of the nation's electricity by 2030.¹⁵

Wind power is nondispatchable, which means the output cannot be controlled by operators and must be used at the time the wind is available; however, as long as a suitable site and design is selected, it serves as a reliable, renewable energy source.¹⁶

Wind turbines emerged as one of the most efficient ways of converting the kinetic energy in wind into mechanical power, aside from hydroelectric plants from flooding reservoirs and wave power from oceans.⁴ Wind turbine consumers have found many ways to use the wind energy harnessed by the turbines. Historically, turbines were used on remote sites that did not have access to the electricity grid, and are still used for that purpose today even though electricity is significantly more widely available.¹⁷ Some homeowners who do have access to the grid have chosen to use turbines to be more environmentally-friendly and save on energy costs. The use of large, utility-sized wind turbines or multiple wind turbines used in conjunction can create bulk energy to help supply electricity to the utility grid for residential, commercial, or industrial use.

Types of Turbines

Wind turbines are classified by the orientation of its axis and power output. The axis can be horizontally or vertically oriented, which have various affects on the design of the overall structure. The power output of the turbine can be selected based on the energy needs of the site and affect the overall size of the turbine.

Orientation of Axis

There are two main designs of wind turbines, the horizontal axis wind turbines (HAWT) and the vertical axis wind turbines (VAWT), both shown below in Figure 2. Horizontal axis wind turbines are more common and widely available. For these reasons and since more information is available on the construction of horizontal axis wind turbines, the HAWT model will be used in this project. Horizontal axis wind turbines usually have two or three tapered fiberglass-reinforced blades which are operated facing into the wind.¹⁸ The turbine unit, which includes the blades, rotor, nacelle, and additional

¹⁵ http://www.awea.org/newsroom/pdf/Wind_Energy_Basics.pdf

¹⁶ *Wind Power*. (2009, October). *Wind Power*. en.wikipedia.org/wiki/Wind_power(2009, October).

¹⁷ Gipe, P. (2004). *Wind Power: Renewable Energy for Home, Farm and Business*. White River Junction, VT: Chelsea Green Publishing Co.

¹⁸ *American Wind Energy Association*, (September 2009), www.awea.org/faq/wwt_basics.html

mechanical equipment, is attached to a tubular steel tower which supports a main rotor shaft and electrical generator at the top. Most horizontal axis wind turbines are equipped with furling and shutdown systems to protect the turbines from damaging high-speed winds. If the wind speeds are too high for the designed critical strength of the blades, a furling system will turn the wind turbine generator rotor away from the direct wind force while still producing maximum output. A shutdown system can be electrical or mechanical and is designed to shut down the generator completely, and the blades are unable to spin.¹⁹

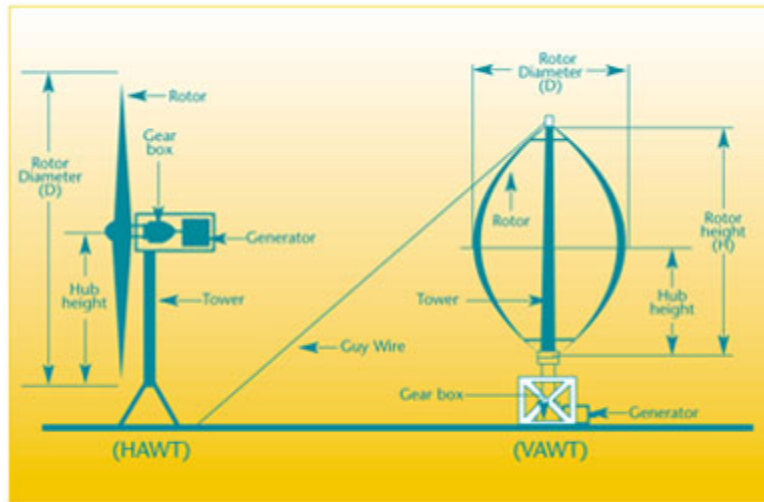


Figure 2: A Comparison of the Horizontal Axis Wind Turbine (HAWT) and Vertical Axis Wind Turbine (VAWT) Configurations

Source: <http://colonizeantarctica.blogspot.com/2008/01/vertical-axis-wind-turbines.html>

Power Outputs

Wind turbines are also classified by the amount of power that they can generate, and are directly correlated to the physical size of the turbine (larger wind turbines produce more energy). The three types of turbines in terms of power output levels are utility-, industrial-, and residential-scaled wind turbines. Table 1 displays the average size range and uses for each of the three scales of turbines. Utility-scaled turbines produce the highest amount of energy which is commonly sold back to the grid, while Residential-scaled turbines produce only 2 to 28 percent of the energy of a Utility-scaled wind turbine and are used for single buildings. Industrial-scaled wind turbines lie between the range of a

¹⁹ *Wind Turbine Concepts Defined and Explained*. (Retrieved September 2009) otherpower.com/otherpower_wind_tips.html#tower

Utility- and Residential-scaled turbine, and can either be applied for single buildings or communities or sold to the grid.

Table 1: Wind Turbine Classifications²⁰

Scale	Average Output Range	Uses
Utility	900 kW - 2 MW	<ul style="list-style-type: none"> •Generate bulk energy for sale in power markets •Commonly used in “wind farms”
Industrial	50 kW - 250 kW	<ul style="list-style-type: none"> •Remote grid production •Reduce consumption of higher cost grid power •May be sold if permitted by state regulations
Residential	400 W to 50 kW	<ul style="list-style-type: none"> •Remote power •Battery charging •Net-metering type generation

Mechanical Operation

In order to design the tower and foundation of a wind turbine, it is helpful to know the mechanical design and how it works. The force of the wind is applied to the blades of the turbine, which are angled to produce a rotation where the blades convene.²¹ The blades are connected to a shaft that also spins along with the rotation of the blades. The mechanical energy that is produced by the revolving shaft is converted to electricity through a generator at the base of the turbine. The equipment at the top of the tower rotates based on the direction of the wind, in which the blades are positioned perpendicular to the wind load to maximize the wind exposed surface area. When the force of the wind exceeds the force for which the turbine was designed, the furling and/or shutdown systems are activated to prevent failure.²²

Initial Site Assessment

Once an interest for constructing a wind turbine has been shown, a site must be chosen and evaluated based on the optimal conditions for installing a wind turbine, which are outlined in the following

²⁰ *Global Energy Concepts*. (September 2009), www.powernaturally.org/Programs/Wind/toolkit/9_windturbinetech.pdf

²¹ *Wind Turbine Design*. (2009, October Wikipedia, The Free Encyclopedia: en.wikipedia.org/wiki/Wind_turbine_design

²² *Wind Turbine Concepts* (2009, October).

sections. These steps are necessary to develop a wind turbine to produce the maximum profit return to the investor based on the expected value of the system.

Feasibility and Permitting

The wind turbine process can begin in many different ways. If a person owns a great deal of land, they might investigate how to purchase and install a wind turbine and utilize the electricity on their own property. In order to do this, the property owner would need to have the need for such a great deal of electricity. Another possibility is that a person who owns sufficient land for a turbine can lease their land to a company who would install the turbine and maintain it. In this scenario, the company who owns the turbine would pay the landowner a monthly amount to lease the site. Also, a landowner can be approached by a company who believes their land is valuable and the land owner can either lease the land or can become part owners of the wind turbine. In all of these scenarios, the land is only useful if there is sufficient wind energy available.

Siting

There are several factors that must be considered when choosing an appropriate site for a wind turbine. The factors include:

- Land size
- Wind speeds
- Surrounding landscape and structures
- Social constraints
- Distance between the turbine site and the local power distributor/substation site
- Air zone constraints

There must be a sufficient amount of land for the turbine. Generally, there are specific laws on how much land area must be unoccupied surrounding the wind turbine tower. In Massachusetts, this land area is often defined as “one and a half times the overall blade tip height of the wind turbine from the nearest existing residential or commercial structure and 100 feet from the nearest property line and private or public way”²³.

²³ Massachusetts Division of Energy Resources. *Model Amendment to a Zoning Ordinance or By-law: Allowing Wind Facilities by Special Permit*, (November 2010), <http://www.mass.gov/Eoca/docs/doer/renew/model-allow-wind-by-permit.pdf>

Another variable that must be considered is the quality of the wind in the area. There must be a significant amount of wind at the site for it to be considered suitable for the use of a turbine. On average, any site with less than an average annual wind speed of 5.8 mph is not considered feasible in New England. The breakeven wind speed varies depending on location of the wind turbine due to the retail value of electricity. In some locations wind speeds of lower than 7.0 mph cannot be considered.²⁴ Initial wind speed data can be collected using online wind energy databases and local wind maps of the area.

The distance between the wind turbine site and the local power distribution or substation is also a factor to be considered. If this distance is too great, it is not practical to install a wind turbine on the site. This is because the electricity would have to travel such a great distance to be disbursed that it would not be economically beneficial. If there is the need for electricity “behind the grid,” however, this is another option for the turbine. This means that if there is a high need of electricity locally where the turbine is being installed, the electricity can go directly to these locations instead of having to travel a great distance to an electricity distribution center.

The use for the electricity must be analyzed and the most economically beneficial option should be chosen. If there are no beneficial options, a different site should be considered.

The limitations on the air zones should be considered next. For example, if there is an airport within a close proximity to the site the Federal Aviation Administration (FAA) may have laws prohibiting or limiting the construction of a very tall object. According to the United States Department of Energy, “Tower heights more than 200 feet, which include most utility-scale wind turbines, require Federal Aviation Lighting and the filing of the FAA form '7460-1 Notice of Proposed Construction or Alteration.”²⁵

Community Bylaws

After these first initial variables are considered, the next factor that should be researched is the community’s decision making process. This is important to the wind turbine process because it is information on the steps that must be taken to obtain project approval. For example, a town may have a wind energy bylaw already created. This bylaw would define all of the rules governing the wind turbine approval and installation process. If such a bylaw does not exist, then one would have to be written and proposed to the local government. This local government could also be run many different ways. There

²⁴ Brian Kuhn, Email, 1/28/2010

²⁵ *US Department of Energy*. (2009, May 21).

could be a mayor, or town selectmen that make up a town council, and/or there could be town meetings. Knowing how the local government works is essential to comprehending what steps must be taken to get the wind turbine approved.

In addition to community bylaws, one should analyze the probable community position towards the installation of wind turbines in the area. Many times if a community is opposed to wind turbines, it may postpone the permitting process for years. An example of this is the Cape Wind project off the coast of Cape Cod, Massachusetts. This project has been searching for approval for over five years but the lack of community support has made it almost impossible to get the necessary permits and approvals. Community support of wind turbines makes the entire process much easier.²⁶

Site Analysis

Once it is clear that the construction of a wind turbine is possible on a site, much more detailed factors must be analyzed in order to ensure the feasibility of constructing a wind turbine as well as collecting useful data for cost analysis.

Access to a site can often be a major issue. Many times, viable sites for wind turbines are at high altitudes where the wind energy is most profitable. These remote sites can be very hard to access, however. If the access road leading to the site is not large enough, a wider road would need to be created. A report prepared by Global Energy Concepts states that, "Developers look for sites with existing adequate roads that can handle the construction equipment requirements of delivering large turbine and tower components and the specialized crane to erect the turbine...The grade, turning radius, and loading criteria for construction require highly suited road access, or the ability to modify the road to accommodate the criteria."⁹ Depending on the distance to the nearest road, this could be a very costly project and could make the economic benefits of the turbine not sensible. Also, these sites may not even be available for construction purposes. The Global Energy Concepts report goes further to say that, "In New England, many of the higher elevation sites that are attractive for wind energy projects are under conservation easements or are state or federally owned lands that may not allow for wind development...Other windy sites, whether on mountain ridges or shorelines, are highly valued for their recreational or scenic purposes."⁹ This statement suggests that finding sites that are both constructible and not valued for other reasons is very rare.

²⁶ *Cape Wind*, (February 2010), en.wikipedia.org/wiki/Cape_Wind

In addition, foundation types will be a very important part of the construction process. If the site has a great deal of rocks or is on a rock bed, then rock anchors will need to be considered to stabilize the foundation. If there is no rock, but rather a silt/sand composition, then a larger foundation will need to be constructed to limit the bearing stresses exerted on the soil. This factor can change the initial cost of the turbine by tens of thousands of dollars.

The onsite electrical demand should then be researched more in depth. Electricity bills and finances from the past year could be gathered and analyzed to see just how much electricity could be used locally before the electricity enters the grid. This scenario would be beneficial to a project whose goal is to provide lower cost electricity for people in the local area. If the project's goal is to be as profitable as possible then selling all of the electricity that is generated to the local grid would be the most appropriate. Research on local utility sales (\$/kWh) in order to calculate the income from the grid sales would be necessary. Also, the cost of connecting to the local utility (grid) would need to be determined. By completing these steps the cost analysis will be more accurate. In this cost analysis phase all monetary factors must be considered. This includes the basic cost of the construction and materials of the turbine, shipping, installation, electricity sales, government benefits, and maintenance.

The profit made from selling the electricity that is generated could vary exponentially by location. Some locations have high costs of electricity, which makes wind energy more attractive; however they do not have high wind energies. While other areas have high wind energies, but the cost of electricity in that area is low enough that investment in wind turbines is not attractive. For example, Maine has high wind energy but has low cost of electricity, while southern Connecticut has high costs of electricity but little wind energy. Often times wind energy seems most practical when the local price for electricity exceeds 10 to 15 cents/kWh.²⁷ In addition to revenue from sale and electricity, there are many state and federal incentives available to people who install wind turbines, which be a great source of profit. These should be considered in the cost analysis portion of the project in order to make an accurate assessment of the potential profit.

The earlier research into the winds at the site was a very general investigation of the local winds. While the other factors are being considered, this stage of the process should be developed to give a more detailed and more accurate description of the local winds. For example, previously only the local wind maps of the area would have been analyzed. At this stage in the project, however, the specific wind on

²⁷ Database of State Incentives for Renewables and Efficiency: <http://www.dsireusa.org/>, (2009)

the site should be measured for at least one full year to give accurate wind values for each season²⁸. The data generated from the in-depth wind analysis will be used to determine more accurately the amount of electricity the wind turbine could generate. This step can be bypassed if a nearby site has been screened for its wind energy. Then the information can be correlated, and this new information would be sufficient for wind energy examination.

In order to obtain a permit for a wind turbine, a feasibility test is often necessary. This test includes many different factors including avian and bat interaction, wildlife/plants/wetlands studies, archaeological and historical reviews, stream crossing and soil disturbance, and local zoning²⁹. All of these studies must be done to prove that the turbine will not interfere with the local environment. The feasibility studies add time and cost to the overall construction of a wind turbine, and it is greatly influenced by the public. This part of the process allows the public to learn more about the project and also express their opinions on the construction of the turbine. Public involvement also educates the community about the benefits of wind turbines. This is extremely important in encouraging the societal acceptance of the structures, as well as to the adaption of more wind turbine projects in the future. If the community understands the benefits of wind energy, they are more likely to consider the construction of turbines on their own property.

Social Integration of Wind Turbines

While the technical aspects of wind turbines are a main focus, part of the process of implementing a wind turbine is the social acceptance of the turbines into the areas in which they are being constructed. Studies have been conducted focusing on three main social groups: the public, the community, and the stakeholders. There are different methods used to analyze the factors that influence each individual group.

Typically, researchers will obtain the opinion of the public through surveys and polls related to renewable energy in general as well as wind turbines. In the past, studies have shown that public acceptance does not have a direct correlation between favorable opinions about turbines and the presence of turbines.

Local controversies or issues in the community in which the turbine is being proposed can be looked into to help determine the likelihood of acceptance of wind energy. According to the “Not in My Back Yard”

²⁸ *Wind Resource Assessment Handbook*, AWS Scientific, Inc. (1997)

²⁹ National Wind, (September 2009), www.nationalwind.com

theory devised by Maarten Wolsink, “local opposition is often based on distrust, negative reactions to the actors (developers, authorities and energy companies) trying to build the turbines, and the way projects are planned and managed, and not the wind turbines themselves.”³⁰ Therefore, the “actors” play a large role in whether or not the community will trust these developers or companies to complete the project efficiently.

The stakeholders of the project must also be considered because they are often the policy makers and politicians in the area. Research shows that the financial status of the stakeholders as well as their planning capabilities and past political history can influence their opinions on a large-scale project such as the implementation of a wind turbine.

After the opinions of each group are explored, the most important factors to consider when attempting understanding the reactions of a community to a proposed installation of a wind turbine are:

- The physical and environmental characteristics and potential of the site including facts and figures on how successful a turbine could produce energy at a specific site.
- The individual and collective profile of the community such as their knowledge of wind energy and the demographic (age, gender, nationality, etc).
- The interaction between technology and society.

The most effective way to gather this information is to conduct surveys to establish the current views on wind energy and the past decision-making capabilities of the town and then distribute appropriate information to educate the community and stakeholders about wind energy and the benefits.³¹

Choosing a Turbine Size

The size of a turbine is also considered when establishing a location for the site based on the projected energy needs. The size is determined to be a Utility-, Industrial-, or Residential-scale wind turbine (see Table 1). The manufacturers that produce turbines in the area are researched, and a model of the turbine is chosen based on the size needed to support the site. Economically, it is advised a wind turbine with the largest feasible energy output is chosen for the constraints of the investor and the site because smaller wind turbines are more expensive per kW than larger wind turbines. Regardless of the

³⁰ *The Social Research on Wind Energy Onshore*, Accessed: January 2010, Wind Energy: The Facts: <http://www.wind-energy-the-facts.org/en/environment/chapter-6-social-acceptance-of-wind-energy-and-wind-farms/social-research-on-wind-energy-onshore.html>

³¹ *The Social Research on Wind Energy Onshore*, Accessed: January 2010

wind turbine size, the controls, electrical connection to grid and maintenance are a much higher proportion of the capital value of the system. Larger wind turbines are more successful in offsetting operating costs because of the larger energy output.

Planning Process of Wind Turbine Installation

There are various stages and people involved in the planning process of determining the feasibility of a wind turbine. This can involve social and political issues including permitting and approval through town meetings. Figure 3 is a flowchart which summarizes the general steps, individuals, and organizations involved in the planning process of the installation of a wind turbine.



Figure 3: Planning Process of Installing a Wind Turbine

Wind Turbine Design

The structural design of a wind turbine consists of two main parts: its tower and its foundation. The tower design is based primarily on wind and ice loads, loads acting from the rotor, nacelle, blades, and additional equipment at the top of the tower in addition to wind loads acting on the tower. The foundation is designed according to the moment and axial loads resulting from the tower design and the properties of the supporting soil.

Tower

The tower is designed to resist the loads acting on it based on the desired scale of the wind turbine and desired hub height. A taller tower translates to an exponential increase in the amount of wind that is being harvested from the wind turbine because wind speed increases as the harvest height increases. The “increase factor” of the wind speed increases exponentially as the height above a surface increases, as shown in Figure 4.³² From this graph, one can see that at low heights, the wind speed increases significantly at first, but at higher elevations the rate of increase is much less.

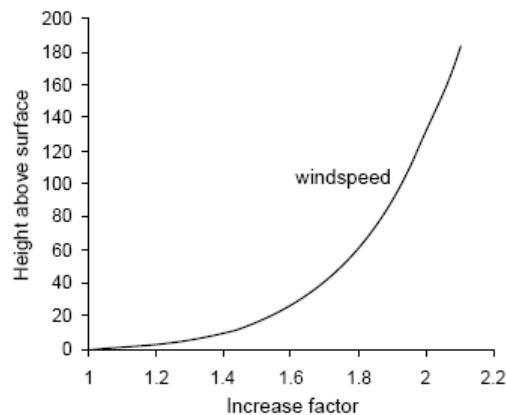


Figure 4: Increase Factor Due to Height of Tower

The wind speed will also vary due to the surrounding area.⁴ The increase factor of wind speeds is higher in open areas (such as over a body of water) than in highly vegetated areas (such as wooded or built-up areas). Through years of experience, turbine manufacturers and consultants have recommended that a turbine should be at least 30 feet higher than any object within 300 feet.³³ This rule of thumb is used to avoid rapid changes in wind velocity and direction, or turbulence. The wind speeds must be maximized

³² *Energy Efficiency and Renewable Energy Clearinghouse*, (September 2009), www.greenenergyohio.org

³³ Gipe, P. (2004).

and direction constant at the blades of the turbine in order to maximize the productivity of the wind turbine. A reduction in turbulent winds also reduces the wear and fatigue on the wind turbine.³⁴

The most widely used design of a wind turbine tower today is a freestanding, continuously tapered, cylindrical hollow steel monopole.³⁵ The conical shape is efficient in simultaneously increasing the tower strength and saving materials as well as reducing the exposed profile for wind forces at higher elevations.³⁶ The tower is constructed in sections from rolled steel at a desired thickness, and is transported on site and bolted and/or welded together to create the massive towers.

The design of a wind turbine tower is dependent on the criteria outline in Table 2. The codes used in conjunction with the design method are also listed. It is the responsibility of the engineer to design the wind turbine tower against local buckling, bending and combined axial and bending failures.

Table 2: Tower Design Criteria

Design Factor	Design Criteria	Design Codes
Dead Load	Consists of the self weight of the tower, the turbine equipment at the top of the tower, and ice loads	Massachusetts Building Code
Wind Load	Wind profile is approximated along small sections of the tower	TIA/EIA-222
Local Buckling	Considers a column's slenderness ration, resist failure due to high axial compressive loads	AISC
Flexural Failure	Resist failure due to high bending moments inducing tensile strength in a cross section that is greater than the yield strength	AISC
Combined Axial and Bending loads	Considers the combined failure due to combined axial and bending loads with a magnification factor on the bending stress, B_2	AISC

³⁴ Hansen, P. *Modular towers and the quest for stronger wind*, (2010, February 10). <http://www.windpowerengineering.com/tag/modular-towers/>

³⁵ Hansen

³⁶ *Wind Turbine Towers* (19 Sept 2003). Retrieved January 2010 from Danish Wind Industry Association: <http://guidedtour.windpower.org/en/tour/wtrb/tower.htm>

Foundation

The foundation of a wind turbine is designed for the worst-case loading. The turbine foundation has to resist great loads mainly because of the overturning moment due to wind and must simultaneously resist forces from geotechnical conditions.³⁷ Reinforced concrete spread footings are most common in design because they are applicable to a wide range of soil strengths, however for some weaker soils a deeper foundation is needed to stabilize the turbine tower.³⁸ The main components in designing a spread footing are its width, depth, amount of steel reinforcement, and construction requirements.

The essential foundation design criteria are stiffness, strength, stability, differential settlement, durability, and economy. These minimum values for foundations are usually provided by the turbine manufacturers' design specifications. Table 3 below lists the design factor, its design criteria, and any design codes or standards that are used in conjunction with the criteria.³⁹

Table 3: Foundation Design Criteria

Design Factor	Criteria	Design Codes/Standards
Strength	Withstand factored loads and fatigue loads	ACI
Stability	Resist excessive translational and rotational movement under extreme loads	ACI
Settlement	Considers soil criteria to ensure adequate bearing capacity	Geotechnical Standards
Durability	Resist environmental damage to concrete and maintain serviceability for the turbine's lifetime	ACI

³⁷ Morgan, K., & Ntambakwa, E. (2008, June). Retrieved September 2009, from AWEA Windpower Conference, Houston:
www.garradhassan.com/downloads/reports/Wind_Turbine_Foundation_Behavior_and_Design_Considerations.pdf

³⁸ *Wind Energy Basics*, American Wind Energy Association.

³⁹ *Installed U.S. Capacity and Wind Project Locations*. U.S. Department of Energy.

Chapter 3: Methodology

When designing a wind turbine, there are different alternatives to the design factors associated with the site that is chosen to build upon. As seen in Table 4 several alternatives of each design component were analyzed to explore the affect of each alternative on the overall design. The first column in Table 4 displays the specific design component, the second column displays alternatives to the design component to consider, and the third column displays what other design factors play into designing that particular part of the turbine.

Table 4: Design Factor Alternatives for Consideration

Design Component	Alternatives to Consider	Design Factors to Consider
Soil Profile	<ol style="list-style-type: none"> 1. Sandy-Gravel 2. Clayey (2000 psf) 3. Rocky (6500 psf) 	Strength of Soil
Foundation Type	<ol style="list-style-type: none"> 1. Spread 2. Monopile (Offshore) 	Soil Profile, Turbine Size, Number of Blades, Tower Design, Wind Speed, Tower Location
Turbine Size	<ol style="list-style-type: none"> 1. Small Scale 2. Mid-scale 3. Large Scale 	Type of Site, Distribution
Number of Blades	<ol style="list-style-type: none"> 1. Two Blades 2. Three Blades 	Wind Speed, Tower height
Tower Design	<ol style="list-style-type: none"> 1. Width 2. Height 3. Shape 	Wind Speed, Surrounding Terrain, Turbulence
Distribution	<ol style="list-style-type: none"> 1. Behind the Grid 2. Sell to the Grid 	Energy needs of the Site

The following flow chart in Design Figure details the process of designing the wind turbine:

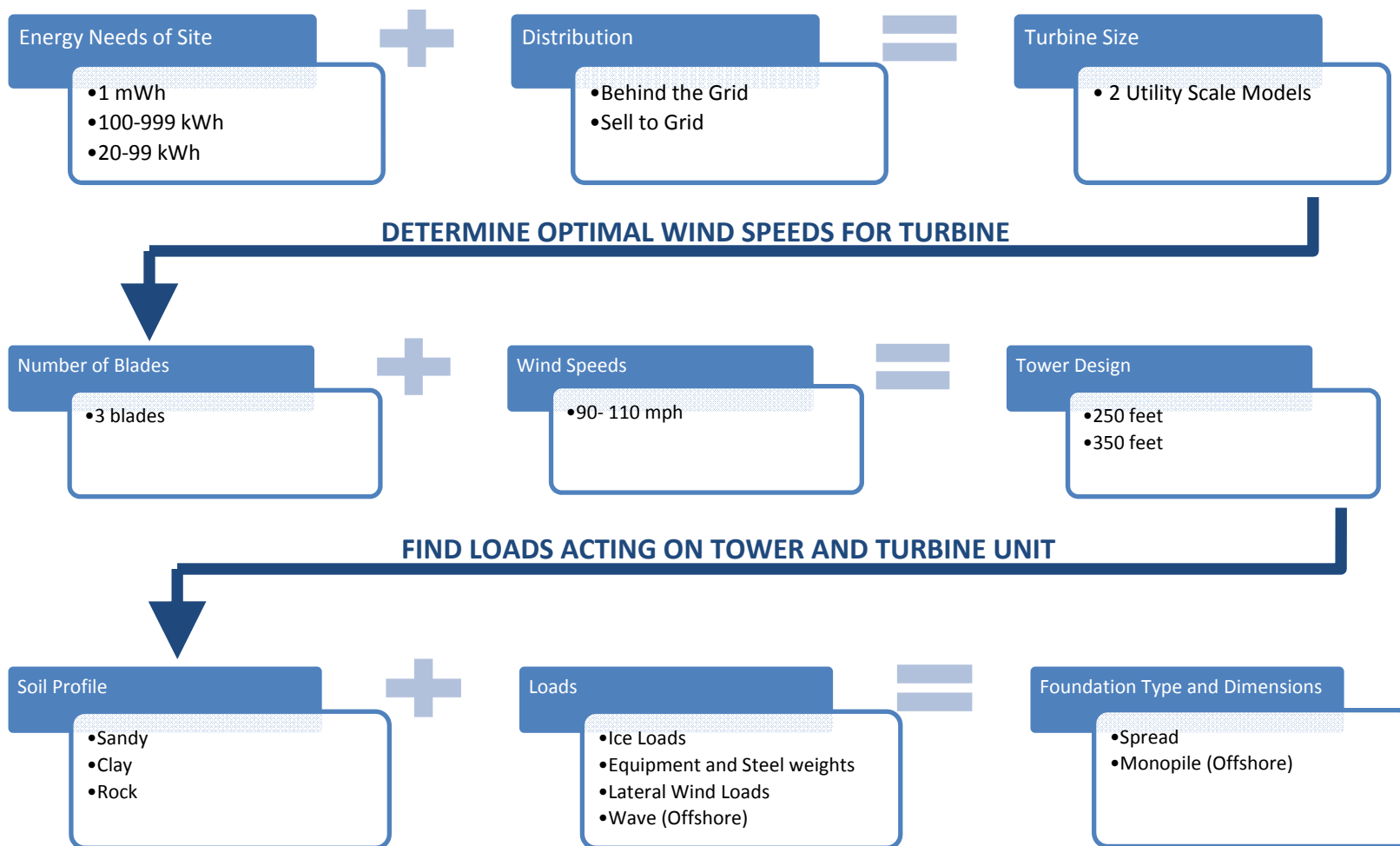


Figure 5: Flow Chart of Wind Turbine Design

Through research and calculations, this project determined how the different alternatives to the design components affected the overall design of the turbine and is discussed in the subsequent sections. The following three sections were outlined: the tower design, foundation design, and the project analysis. Each section was researched individually and eventually was compared with the other two sections.

The individual design sheets for the tower design, foundation design, and project analysis were developed individually throughout the course of this project. When the design sheets were finalized, they were linked together in order to create one completely integrated spreadsheet which produced output sheets that summarized important information regarding Energy Output, Financial Analysis, Foundation Design, and Tower Design. With little knowledge about the specific calculations, a user can use this spreadsheet simply by inputting common information outlined in a user friendly main page and check if their wind turbine project is feasible. The following chapters will elaborate on the methodologies of the tower design, foundation design, and project analysis.

Chapter 4: Tower Design

The tower was designed in accordance with the latest edition of the *Massachusetts State Building Code (780 CMR)* and *AISC Steel Construction Manual*. The material used to design the tower is A36 steel plates, which can be formed and welded in sections to create the tower. Loading conditions on the tower were determined through local wind speed data from *780 CMR*, and dimensions of the tower were later established based on Load and Resistance Factor Design (LRFD). Two tower heights were chosen for design, one at 250 feet and another at 350 feet (standard heights for utility-scale wind turbines).

Loading Conditions

Since there are no regulations specific to loading conditions for wind turbine towers in the *Massachusetts State Building Code*, the tower was modeled per *780 CMR 3108.0* for Radio and Television Towers. It is an industry standard to use this section of the *Building Code* because the shape of a tapered, cylindrical steel radio or television tower is comparable to that of a wind turbine tower.⁴⁰

Section 780 CMR 3108.0 details *Location and Access* (3108.2), *Construction* (3108.3), *Loads* (3108.4), and *Grounding* (3108.5). For structural design purposes, the section that is of most concern is *Loads* and takes into consideration both dead loads and wind loads.

Axial Loads

It is stated in *CMR 3108.4.1* that “towers shall be designed for the dead load plus the ice load in regions where ice formation occurs,” which is the case for New England. The dead load components of a wind turbine are summarized in Table 5 below:

Table 5: Axial Load Components

Axial Load Component	Weight	Method/Standard
Equipment at top of tower	400 to 700 kips for utility-scale turbine	Research from existing turbines
Steel Tower	$[V_{\text{steel}} (\text{ft}^3)] * [\gamma_{\text{steel}} = 0.490 \text{ k/cf}]$	Basic geometric principles for determining volumes
Ice	$[V_{\text{ice}} (\text{ft}^3)] * [\gamma_{\text{ice}} = 0.056 \text{ k/cf}]$	ASCE-7-05 Design Formulas

The equipment at the top of the tower includes the rotor, blades, nacelle, and all other mechanical components. The thickness of ice is determined through ASCE-7-05 design equations as follows:

⁴⁰ Lagadimos, F. (2009, December 8).

$$t_d = 2.0tI_i f_z (K_{zt})^{0.35}$$

where:

the nominal thickness of ice t is obtained from the design maps in ASCE-7-05 Chapter 10; ranges from 0.75" to 1" for Massachusetts,

importance factor I_i is 0.8 for category I structures,

$f_z = (\frac{z}{33})^{0.1}$, z is the mid-height of the tower section, and

K_{zt} does not apply, assuming no wind speed up.

This design ice thickness is then used in the following equation to obtain the cross-sectional area of ice,

$$A_i = \pi t_d (D_c + t_d)$$

where D_c is the diameter of the tower. The use of design spreadsheets were used to determine the axial loads through repetitive calculations. These dead loads will act vertically downward on the tower and supporting foundation.

Lateral Loads

CMR 3108.4 specifies that the tower "shall be designed to resist wind loads in accordance with TIA/EIA-222," the *Structural Standards for Steel Antenna Towers and Antenna Supporting Structures*. A horizontal wind force was applied on the tower, and a separate horizontal wind force was applied to the equipment at the top of the tower, which was considered a discrete appurtenance. The wind loading along the tower height was defined in 1' sections to obtain an accurate wind profile and resultant load (due to the varying section height from the ground, and also average section diameter because of the taper).

The horizontal force (F, lbs) applied to each section along the tower height was calculated from the following equation: ⁴¹

$$F = q_z G_H [C_F A_E + \sum (C_A A_A)] \leq 2q_z G_H A_G$$

To use the force equation, the following values were obtained from the provisions of TIA/EIA-222:

⁴¹ TIA/EIA-222 *Structural Standard for Antenna Supporting Structures and Antennas* (June 1996). Telecommunications Industry Association.

velocity pressure $q_z = 0.00256K_zV^2$ for wind speed V in mi/h,

gust response factor G_H is 1.69 for tubular towers,

force coefficient C_F is 0.59 for tall cantilevered tubular pole structures,

effective projected area of structural components in one face A_E is equal to the tower's height multiplied by its diameter,

$\sum(C_A A_A)$ applies to linear appurtenances (does not apply to wind turbine tower),

and A_g is the gross area of one tower face in ft^2 (for tubular tower, $A_g = \text{diameter} \times \text{height}$).

For the velocity pressure q_z , the exposure coefficient $K_z = \left[\frac{z}{33}\right]^{2/7}$ where $1.00 \leq K_z \leq 2.58$, V (mi/h) is the basic wind speed measured in the fastest mile, and z is the height above the average ground level to the midpoint of the section. The basic wind speed was obtained from the *Massachusetts State Building Code (780 CMR)*, where the basic wind speeds are listed for each individual city in Massachusetts in units of 3-second gusts. The wind speed must be converted from 3-second gust to fastest mile using the equation $V_{FM} = V_3 \frac{V_t/3600}{1.53}$. First, the gust duration $t = (3600 \text{ s}/mi)/V_3$ was calculated where $V_3 = 3$ -second gust speed in miles/hour. Then, Vt was found using the design chart in Figure 6 from ASCE-7-05 by matching t along the x-axis, drawing a vertical line to the durst line, and from there drawing a horizontal line over to the y-axis to find the corresponding $V_t/3600 \text{ sec/hr}$ value to use in the equation.

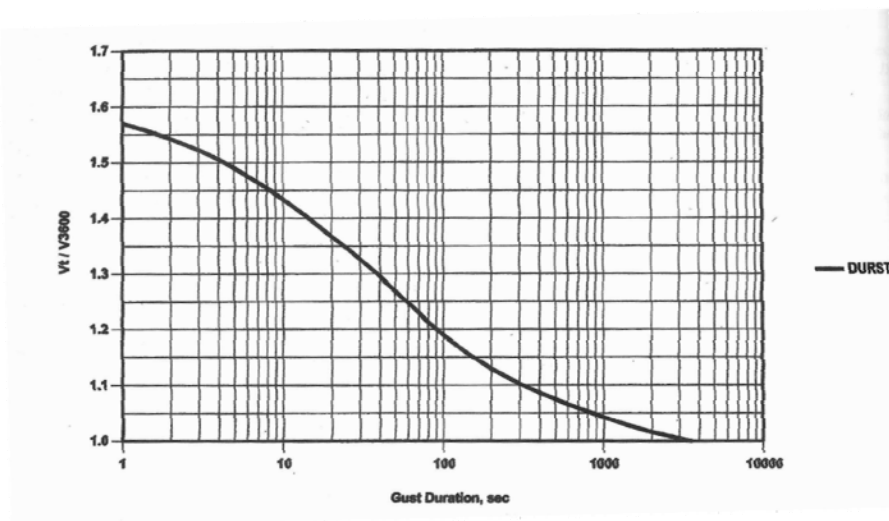


Figure 6: Wind Speed Conversion Table, *Massachusetts State Building Code (780 CMR)*

For instance, the specified 3-second gust speed for Worcester is 100 mph, which corresponds to a wind speed V_t equal to 87 mph. When using a tower height of 250 feet, a constant diameter of 10 feet, and a design wind speed of 87 mph in Worcester, the design wind profile in one foot increments is shown in Figure 7.

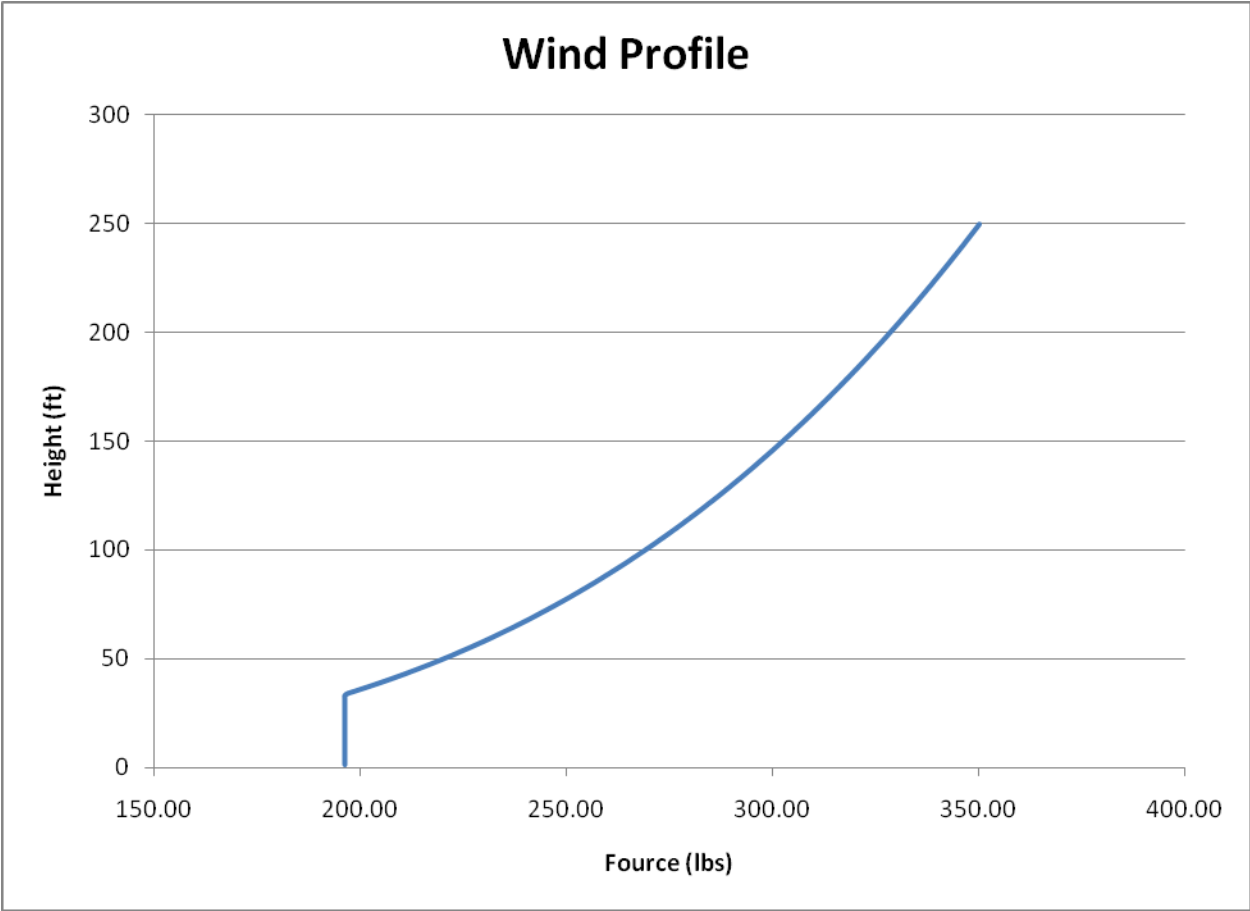


Figure 7: Example Wind Profile Graph

The point force F_c for the discrete appurtenance will act on the center of the hub (normal to the plane of the spinning blades). The design wind load F_c is calculated from the following equation: ⁴²

$$F_c = q_z G_H [\sum C_A A_c]$$

where the velocity pressure q_z and gust response factor G_H uses the same calculations as stated above for the force on the tower and $\sum C_A A_c$ takes into account all elements related to the appurtenance (in

⁴² TIA/EIA-222 (June 1996).

the ultimate strength of the tower:⁴³

$$\phi R \geq \gamma Q$$

In the previous equation, R is the resistance of the steel, Q is the applied load, ϕ'_b is a strength reduction factor and γ is a load factor. Table 6 below summarizes the design equations that were used for the design of the wind turbine tower and were based on the *Manual of Steel Construction LRFD, 3rd Edition*⁴⁴.

Table 6: Tower Design Equations for Available Strength

Design Component	Design Equation	Description
Axial Strength	$P_u \leq \phi F_{cr} A_g$	Factored axial loads must not exceed the reduced axial capacity, which is the critical load multiplied by the gross area.
Flexural Strength	$M_u \leq \phi F_y Z$	Check strength: ultimate flexure must not exceed the steel strength multiplied by its section modulus
	$M_u \leq \phi \left[\frac{0.021E}{\frac{D}{t} F_y} + 1 \right] F_y S$	Check local buckling (for non-compact members only): the flexural strength should be greater than the required moment strength, takes slenderness into account
Combined Axial and Flexural Strength	$\frac{P_r}{P_c} + B_2 \frac{8 M_r}{9 M_c} \leq 1.0$	Checks that member under combined forces will not fail by taking ratio of required strength to available strength. Moment is multiplied by magnification factor of B_2 that considers lateral stability.
	$\frac{P_r}{2P_c} + B_2 \frac{8 M_r}{9 M_c} \leq 1.0$	

The stresses in the tapered tower were analyzed for five different sections to account for the varying cross-sectional geometry. The magnification factor B_2 found in the combined axial and flexural strength equations takes into account the shear loads on the structure lateral deflection of the tower, which was

⁴³ *Manual of Steel Construction LRFD, 3rd ed (2001)*. American Institute of Steel Construction Inc.

⁴⁴ *Manual of Steel Construction LRFD, 3rd ed.*

found through using ANSYS, a computer analysis program. The tower was entered as a beam with five separate sections to assume different properties. The results of the ANSYS analysis can be found in Appendix C.

Flexure Strength		Combined Loads	
Factored Load	83472.00 in-kips	Combined Loading	0.8621 ≤ 1.0 OK
Reduced Strength	220467.83 in-kips	Pu/Pc	0.5133
Yielding Mn	244964.25	For Pu/Pc ≥ 0.2	0.8621
Local Buckling	216128.81	For Pu/Pc < 0.2	0.6054
Check for Compact section:		Magnification Factors	
D/t	128.00	B2	1.04
λ_p	56.39	Pnt	774.40
λ_c	249.72	Pe2	22100.00
non-compact		ΣH	52.00
Flexure Strength:	OK	ΔH	6.00
		L	3000.00

Figure 9 below shows the checks for the axial, flexural, and combined strength using the design equations listed above.

Available Strength

User Input	
Overall height	250 ft in
Diameter at Base	8 ft in
Shell Thickness	0.75 in
Strength of Steel	36 ksi
Modulus of Elasticity	29000 ksi

Section Properties	
ID	94.50
S	5302.76
Z	6804.56
r	33.68
I	254532.37

Axial Strength	
Factored Load	774.40 kips
Reduced Strength	1508.54 kips
Pn	1774.75
Fcr	7.91
KL/r	178.16
4.71*sqrt(E/Fy)	133.68
Fe	9.02
Fcr for KL/r ≤ 4.71*sqrt(E/Fy)	6.77
Fcr for KL/r > 4.71*sqrt(E/Fy)	7.91
Ag	224.43
Check for Compact section:	
D/t	128.00
λp	
λc	88.61
noncompact	
Axial Strength:	OK

Flexure Strength	
Factored Load	83472.00 in-kips
Reduced Strength	220467.83 in-kips
Yielding Mn	244964.25
Local Buckling	216128.81
Check for Compact section:	
D/t	128.00
λp	56.39
λc	249.72
non-compact	
Flexure Strength:	OK

Combined Loads	
Combined Loading	0.8621 ≤ 1.0 OK
Pu/Pc	0.5133
For Pu/Pc ≥ 0.2	0.8621
For Pu/Pc < 0.2	0.6054
Magnification Factors	
B2	1.04
Pnt	774.40
Pe2	22100.00
ΣH	52.00
ΔH	6.00
L	3000.00

Figure 9: Available Strength Design Sheet

This design sheet allows the user to modify the diameter and shell thickness of the wind turbine tower to determine an acceptable design. The spreadsheet shows the previously calculated required strength, shown in orange, and compares it to the calculated available strength, in blue. If the dimensions entered satisfy the design criteria (required strength does not exceed the available strength), the design is deemed “OK” shown in the yellow boxes. If the dimensions entered do not satisfy the design criteria (required strength exceeds the available strength), the design is deemed “N.G.” for no good, and either the thickness of the shell or the diameter must be increased.

Flange Design

The design of the tower flange at the base was also considered in order to effectively attach the tower to its foundation. Since 1 ½" diameter rods were used in the anchor bolt design (see Chapter 5: Foundation Design for equations used to design the connections), the minimum edge distance was based on 12 times the diameter of the rod, which is 7 ½" from the edge, or 15" centered on the flange. The flange width was checked for flexural strength using the equation $M_u \leq \phi F_y Z$, and also checked for shear in yield and ultimate strength. Finally, the thickness was calculated based on the nominal moment at the outside edge of the tower base using the equation $\geq \sqrt{\frac{6M_u}{\phi_b F_y}}$, where ϕ_b is 0.9. Note that the flange design was not included in the design sheets because it is very case-specific based on the anchor bolt design. Supporting calculations can be found in Appendix C.

Design Summary

The designs of the 250 foot tower are as follows. Figure 10 shows the overall tower dimensions, which includes the height, and base and hub dimensions. It also shows the steel tower with reasonably-sized turbine equipment attached. Figure 11 depicts the base detail, including the flange design. The bolt design is also shown, which is later designed in Chapter 5: Foundation Design.

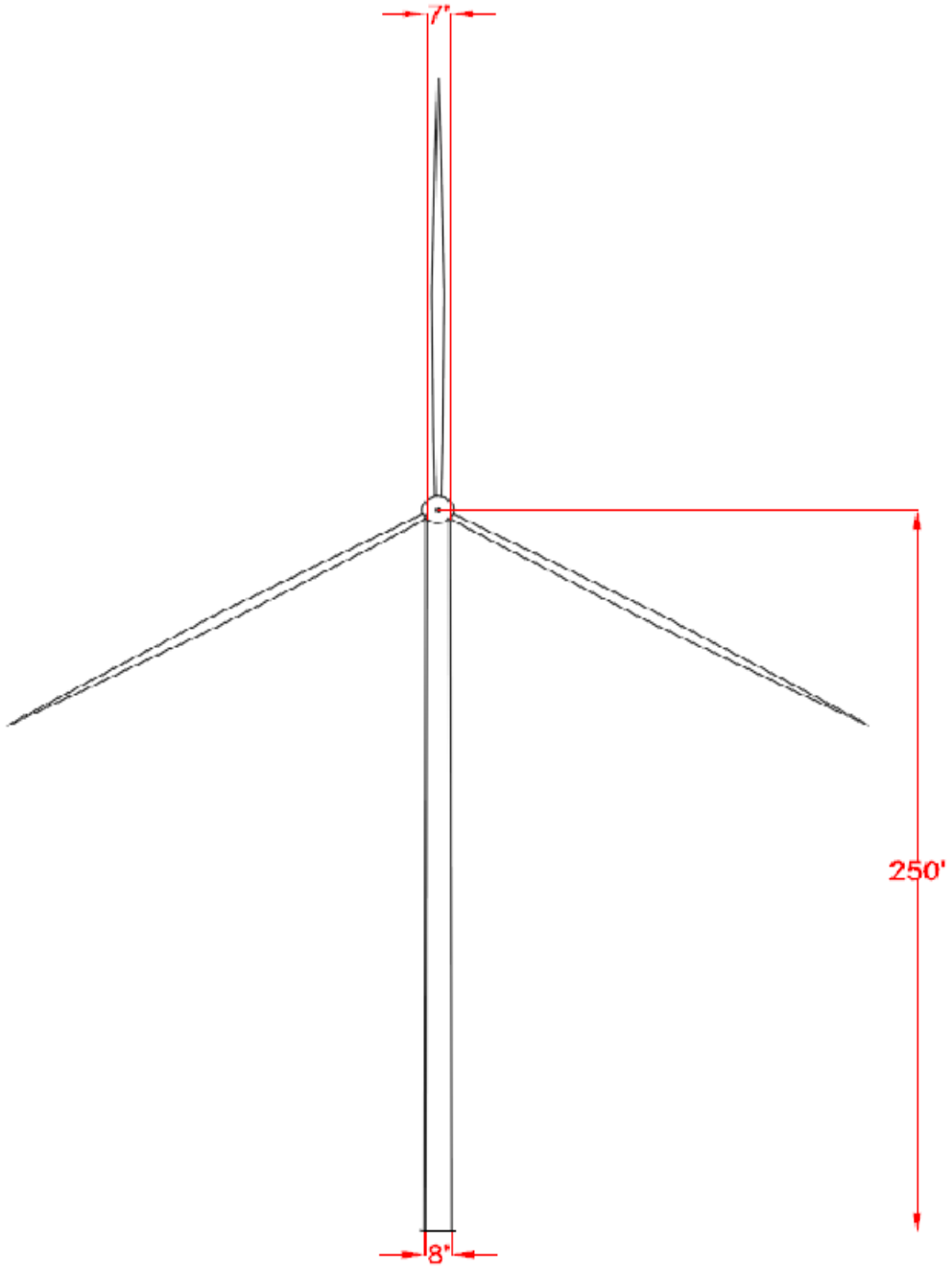


Figure 10: Overall Tower Dimensions

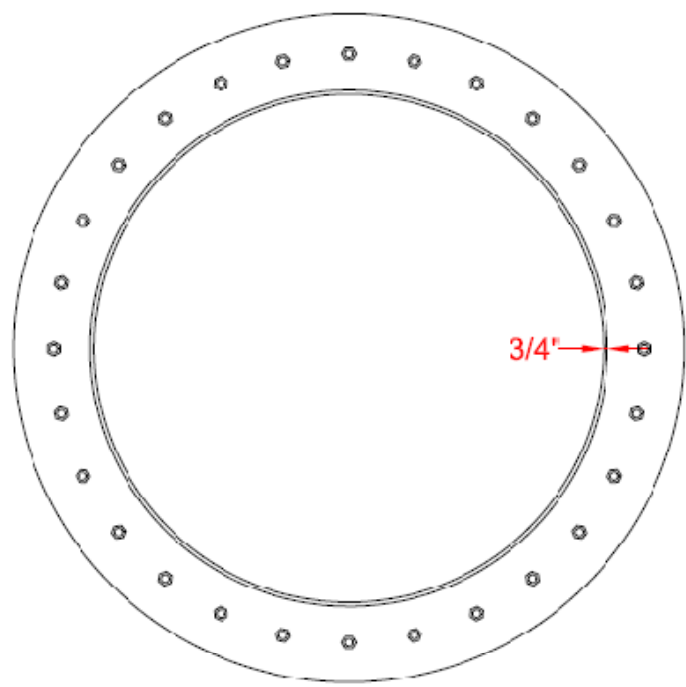
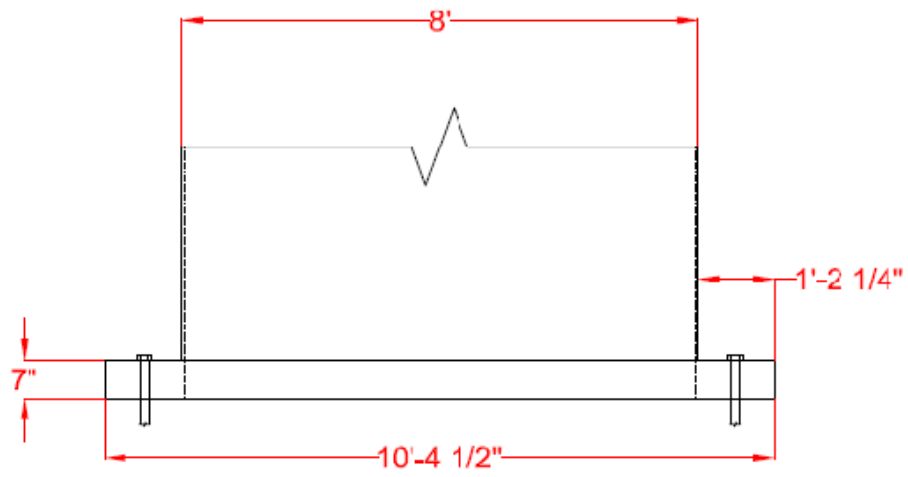


Figure 11: Tower Base Detail

Discussion of Data

The design loads and materials are summarized for a 250 foot and 350 foot tower in the following table. All supporting calculations can be accessed in Appendix C: Supporting Calculations, including the

associating design sheets to compare to the hand calculations. The foundation design and project analysis of the 250 foot and 350 foot towers will also be examined in the appropriate chapters for consistency. Table 7: Tower Design Data for Varying Heights Table 7 shows the criteria that were used for the varying tower heights.

Table 7: Tower Design Data for Varying Heights

Design Criteria	250' Tower	350' Tower
Diameter at base	8 feet	12 feet
Diameter at hub	7 feet	10 feet
Shell thickness	3/4 in.	3/4 in.
Strength of steel	36 ksi	36 ksi
Modulus of Elasticity	29,000 ksi	29,000 ksi
Total axial load acting at tower base	774.4 kips	1206.9 kips
Total moment acting at tower base	6956.0 ft-kips	21637.7 ft-kips
Total shear load acting at tower base	52.0 kips	115.5 kips

By examining this table, it is important to note that with a tower height increase of 40 percent, the axial load acting at the tower base increases by just over 40%; however the moment on the tower increases by over 200%. Therefore, the design will most likely be governed by the required flexural strength.

In order to determine the effects of different wind speeds on a turbine design, Table 8 was produced for the design data of three different towns in Massachusetts with varying wind speeds and a constant tower height of 250 feet, steel strength of 36 ksi, and modulus of elasticity of 29,000 ksi.

Table 8: Tower Design Data for Varying Windspeeds

Design Criteria	Lee	Belmont	Yarmouth
Design Windspeed (3-sec gust)	90 mph	105 mph	120 mph
Diameter at base	8'-3"	8'-3"	8'-6"
Diameter at hub	7'-6"	7'-6"	7'-6"
Shell thickness	1/2 in.	3/4 in.	3/4 in.
Total axial load acting at tower base	669.7 kips	757.7 kips	748.7 kips
Total moment acting at tower base	5587.9 ft-kips	7878.0 ft-kips	10553.0 ft-kips
Total shear load acting at tower base	41.5 kips	58.5 kips	78.8 kips

The supporting design sheet calculations are referenced in Appendix D. The loads acting on the wind turbine have a minimal increase by increasing the design windspeeds by 15 miles per hour. In turn,

there is little change in the design of the tower. However, as noted in the background section, a small increase in windspeed produces an exponential increase in power output and in turn increasing profit.⁴⁵

⁴⁵ *Energy Efficiency and Renewable Energy Clearinghouse*, (September 2009),

Chapter 5: Foundation Design

When designing the foundation of a wind turbine, there are many factors that engineers must take into account including loading and geotechnical conditions. Spread, drilled, and anchored footings can be used for turbine foundations. Spread footings are some of the most common foundation designs for modern turbines.⁴⁶ Several different shapes of spread footings are used for turbine foundations including circular, hexagonal, octagonal, and square. Since there are no specific codes for wind turbine foundation design, ACI standards were used in developing the procedure. The textbook, *Reinforced Concrete Design*, was used as a major source for the steps and equations involved in the design process.

The following chapter will explain the procedure used for designing a square spread footing as well as the directions for the use of the spreadsheet to aid in the design process. Hand calculations supporting the spreadsheet can be found in Appendix E. Figure 12 shows the general procedure for designing a square spread footing.

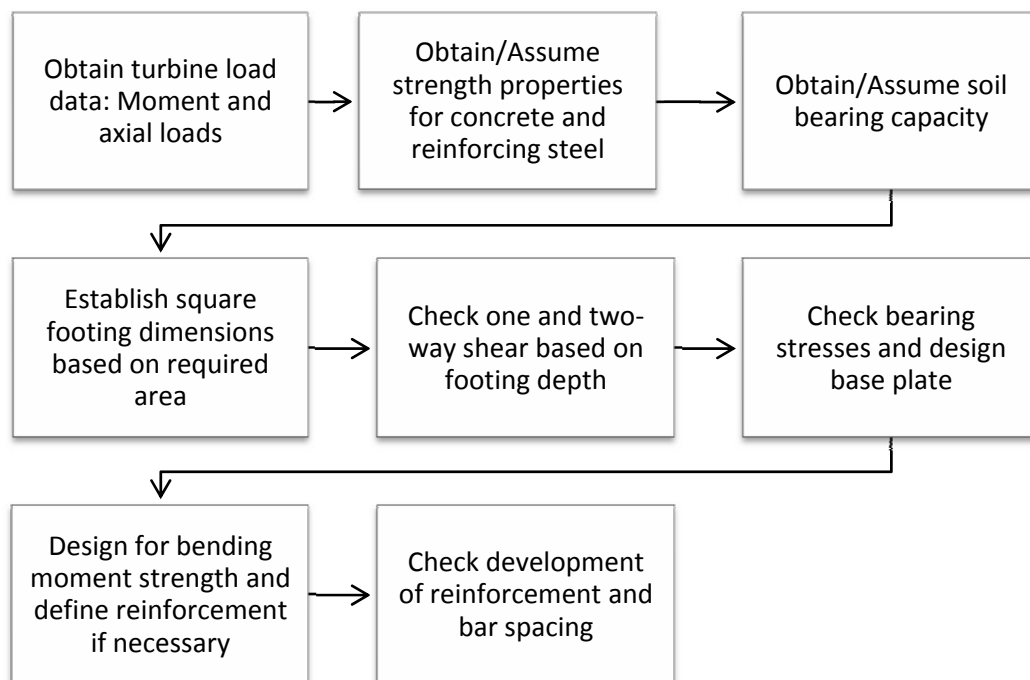


Figure 12: Steps for the Design of a Square Spread Footing

⁴⁶ Ischebeck, D.-I. E. (n.d.). *Combination Spread Footing with Micropiles as Foundation for Wind Turbines*. Retrieved November 2009, from www.contechsystems.com/Articles/CSFM.pdf

Footing Dimension Design

The wind turbine foundation can be designed using the data from the analysis of the tower. This information includes the axial load, lateral moment, and the diameter at the base of the tower. Other design factors, such as the bearing capacity of the soil and the concrete strength properties used for the foundation, influence the design and can either be obtained from actual site data or assumed for initial calculations.

After the tower data and the soil and concrete data are established, they can be used to estimate the pressure of the footing and the required area. The pressure of the footing must also be calculated using a trial size for the depth of the footing and an assumed unit weight of concrete. For the purposes of this project, the standard value of 150 pounds per cubic foot was used and the average footing depth for wind turbine spread footing foundations typically do not exceed 6 feet.⁴⁷ The axial load of the column and the net pressure of the soil, found by subtracting the pressure of the footing from the allowable soil pressure, are used in the equation below for the required area:

$$\text{required } A = \frac{Pu}{p_{net}}$$

Where Pu is the factored axial load in kips from the tower load data and p_{net} is the net soil pressure in ksi. Once the required area is calculated, the square root of this number can be used as the trial size for the square footing. Figure 13 below shows depicts the forces acting on the footing.

⁴⁷ RAM/STAAD Solution Center. Bentley: *RAM/STAAD Solution Center*. (December 2009), ftp://ftp2.bentley.com/dist/collateral/Web/Building/STAADPro/Wind_Turbine_Footing_Design.pdf

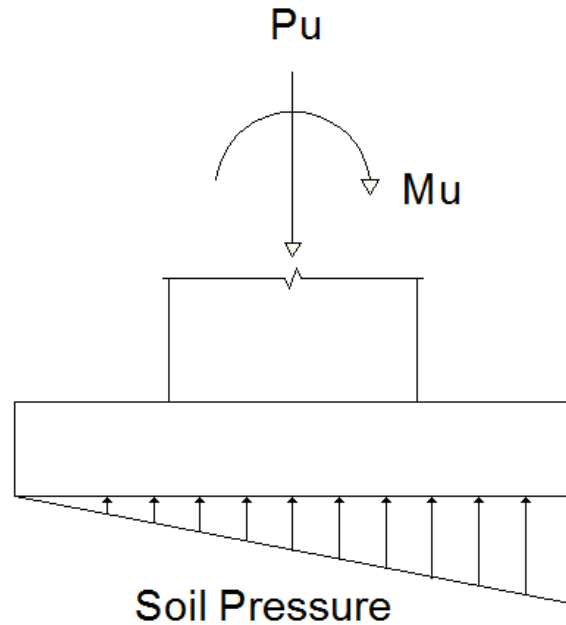


Figure 13: Loading Model and Soil Pressure on Footing

The eccentricity, e , must be calculated in order to determine if the suggested side footing length, B , is adequate by satisfying the following inequality:

$$e = \frac{M_u}{P_u + W_f} \leq \frac{B}{6}$$

Where W_f is the weight of the footing in psf. This check of eccentricity is done to verify that the resultant force on the footing acts within the kern of the cross section and ensures that there are only compressive stresses between the foundation and the soil. In addition, the equivalent soil bearing pressure, q_{equiv} , must be checked to ensure that it is less than the allowable bearing pressure, q_a , using the equations below⁴⁸:

$$q_{equiv} = \frac{P_u + W_f}{B^2} < q_a$$

If the aforementioned conditions are satisfied, then the chosen footing side length is adequate for the bearing capacity of the soil and the resultant is within the middle third of the footing.

⁴⁸ Coduto, D. (2001). *Foundation Design: Principles and Practices*.

In the design spreadsheet, the axial load, moment, and column base diameter from the tower design automatically fills in the appropriate cells and the concrete and soil properties are chosen by the user. The user input includes the values for the allowable bearing stress of the soil, q_a , compressive strength of the concrete, f'_c , the yield strength of the concrete, f_y , and trial depth of the footing, B .

In Figure 14, the red bracket shows the auto-fill cells and the blue bracket identifies the user input. The outcome of this section of the spreadsheet, shown by the green arrow, will suggest a side length for the square foundation to the user. For constructability purposes, the value for the side length dimension is rounded up to the nearest foot. The “OK” to the left of the yellow star indicates that the bearing strength of the soil can withstand the axial load and weight of the foundation with the chosen side dimension. The “OK” to the left of the red star indicates that the suggested side length is adequate for eccentricity, meaning that the resultant falls within the middle third of the footing. If the conditions fail, “NG” for “no good” will appear in the check box and a larger footing size must be chosen. It should be noted that the side length is a trial size and may need to be altered later to satisfy other conditions later in the design process.

Initial Inputs		
Axial Load, Pu	744.43 k	} Input from Tower Spreadsheet
Moment, Mu	6956.04 ft-k	
Column Base Diameter, D	8.00 ft	
Allowable Bearing Stress, qa	4500.00 psf	} User Input
f'c	3000.00 psi	
fy	40000.00 psi	
Determine Footing Size		
Footing Depth	3.00 ft	
Weight of Footing	450.00 psf	
p net	4050.00 psf	
Required Area	183.81 ft^2	
Suggested side length	13.56 ft	← Outcome: Trial Side Footing Dimension
Chosen side length	14.00 ft	OK q equiv < qa ★
Check Eccentricity		
Eccentricity, e	0.08 ft	
B/6	2.33 ft	OK ★

Figure 14: Footing Dimension Spreadsheet Output and Side Length Checks

Design for Shear Loads

According to ACI standards, foundations must be checked for two modes of shear failure, two-way shear and one-way shear. The footing design of this project does not address footings designed with shear reinforcement to simplify the design for constructability purposes. Therefore, if it was found that the chosen dimensions required shear reinforcement, the depth of the footing was increased.

Two-Way Shear

For two-way shear, an initial footing thickness must be assumed and modifications must be made for bar spacing and bar diameter. The result is used to determine the critical sections for shear. The equation below is used to calculate the two-way shear in the footing:

$$V_u = p_{net} * a_{effective}$$

Where a_{net} is the effective area of the foundation, which accounts for the perimeter of the column, D , and calculated effective depth, d . Supporting calculations for these values can be found in Appendix E. The critical section for two-way shear in a square footing must be calculated to investigate the adequacy of the footing thickness as shown in Figure 15 the shaded area. The area “ $D+d$ ” in Figure 15 is the diameter of the base of the column, D , plus the diameter of the effective depth, d .

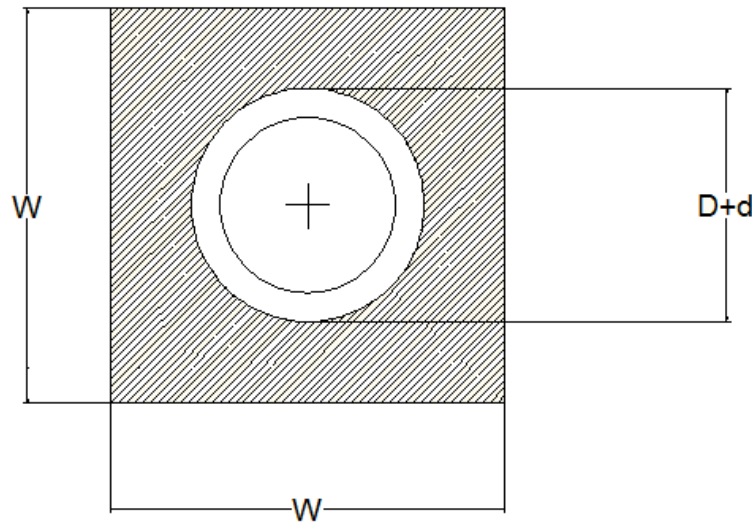


Figure 15: Critical section for two-way shear in square footing

According to ACI-11.12.2.1, when no shear reinforcement is used in the footing, the nominal punching shear strength is V_c . Although the column is circular, ACI code provisions for four-sided critical section were modified and used to compare the perimeter of the critical section, b_o , and the effective depth of the slab, d . ACI-11.12.2.1 states that $\beta_c \leq 2$ and $b_o/d \leq 20$ where β_c is the footing aspect ratio. Since the foundation is square, β_c will be 1, which satisfies the requirement of the ratio of long side to short side of the loaded area. ACI formulas 11-33 through 11-35 below were used to identify the smallest value for V_c .

$$V_c = \left(2 + \frac{4}{\beta_c}\right) \sqrt{f'_c} b_o d \quad \text{ACI Formula (11-33)}$$

$$V_c = \left(\frac{\alpha_s}{b_o/d} + 2\right) \sqrt{f'_c} b_o d \quad \text{ACI Formula (11-34)}$$

$$V_c = 4\sqrt{f'_c} b_o d \quad \text{ACI Formula (11-35)}$$

Where α_s is a provided modifier for two-way shear action, which is assumed to be 40 for an interior column. The smallest value obtained from these three equations was then used to check the allowable two-way shear, ϕV_c , against the actual shear, V_u , in the equation:

$$\phi V_c > V_u$$

Where the strength reduction factor, ϕ , is equal to 0.75. If this equation is satisfied, no shear reinforcement is required for two-way shear.

One-Way Shear

For one-way shear, the actual shear, V_u , is calculated at the critical section located at a distance, d , from the face of the column as shown in Figure 16.

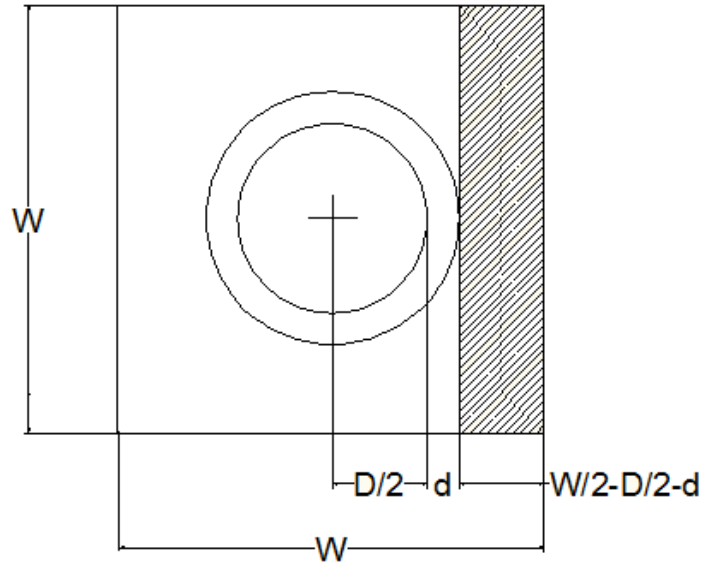


Figure 16: Critical Section for One-Way Shear in a Square Footing

According to ACI-11.12.1.1 and 11.3.1.1, the following equation can be used to calculate the one-way shear capacity, V_c , at the critical section:

$$V_c = 2 \sqrt{f'_c} b_w d$$

Where b_w is equal to the width of the footing. As with two-way shear, the allowable one-way shear was checked against the actual shear, with Φ equal to 0.75. If the equation is satisfied, then no shear reinforcement is required for one-way shear. Thus, the assumed thickness for the footing would be adequate for shear.

For the portion of the design spreadsheet dealing with shear capacity, the user will have already entered the required information shown in the red bracket in Figure 17. The blue bracket shows the checks which satisfy ACI-11.12.2.1. The outcome of the one and two-way shear test of the spreadsheet informs the user if the depth of the footing is adequate for shear. The blue boxes in the spreadsheet indicated by the green bracket show that when the design requires “No Shear Reinforcement” the check is “OK.” As mentioned previously, if the spreadsheet determines that shear reinforcement is needed, then a greater foundation depth should be chosen until the design is adequate for shear. The intent is to avoid the use of shear reinforcement as a strategy for controlling construction costs. When computing the average d , a 3-inch cover and a 2-inch bar diameter is assumed as a conservative approach since none of the selected bars will have a diameter greater than 2 inches.

Determine Depth based on Shear			
average d	31.00 in		} Assumed or Calculated Values
Vu	793.80 k		
bo	271.01 in		
Bc	1.00 (for squares)		
Alpha s	40.00 (for columns)		
Choose smallest of:			
Vc	2760.92 k		} Ratio Checks for Four-Sided Critical Section
Vc	3025.75 k	OK	
Vc	1840.61 k	OK	
Governing Vc	1840.61 k	OK	
Check Two-Way Shear			} No Shear Reinforcement
Bc	1.00	OK	
bo/d	8.74	OK	} Outcome: Footing Depth Adequate for
Shear Capacity (phi c, Vc)	1380.46 k	OK	
Check One-Way Shear			
Vu	23.63 k		} Outcome: Footing Depth Adequate for
Vcu	570.51 k		
Phi*Vc	427.88 k	OK	

Figure 17: Design and Checks for Shear Loading in Spreadsheet

Design for Bearing Strength and Base Plate

In accordance with ACI-15.8.2.1, all forces acting at the base of the tower must be transferred into the footing. Since the footing is subject to extreme loading, the column load cannot be transferred by bearing alone. It was determined that a base plate would be the most effective way of transferring the loads from the tower to the foundation.

The design of the base plate requires checks the ultimate capacity of the concrete in bearing against the axial load on the footing due to the column. In order to do this check, the ultimate capacity of the concrete is computed using the equations below:

$$\varphi_c P_p = \varphi_c * 0.85 f'_c A_1 \sqrt{\frac{A_2}{A_1}} \leq \varphi * 1.7 f'_c A_1$$

Where A_1 is the area of the base plate, using an estimated side length greater than the diameter of the column base, but less than the side length of the footing. A_2 is the area of the footing using the trial side length. In the above equation, $\varphi_c * 0.85 f'_c A_1 \sqrt{\frac{A_2}{A_1}}$ the value on the left governs unless it exceeds the upper-bound value given on the right side of the equation. This value is then compared to the

ultimate axial load, P_u , obtained from the tower calculations to ensure that the concrete has adequate bearing capacity.

The bearing strength in the base plate must be checked against the combined axial and moment forces transferred by the column to the base plate. The equation below is used to compute the maximum stress that occurs on each end of the bearing plate:

$$f = -\frac{P_u}{A_1} \pm \frac{6M_u}{bd^2}$$

The result of this equation was compared to the ultimate bearing stress on the base plate to ensure that the trial width of the base plate was adequate.

In order to design the thickness of the base plate, the flexural strength of the base plate must be investigated using the moment to right at center of the right flange, M_u , in the equation below:

$$t \geq \sqrt{\frac{6M_u}{\phi_b F_y}}$$

Where t is the thickness of the flange, ϕ_b is equal to 0.9, and F_y is the strength of the steel used for the base plate.⁴⁹

In the spreadsheet for the design of the base plate, a trial value for the width of the base plate is entered and checked to ensure that it is larger than the diameter at the base of the column, but smaller than the side length of the footing, as stated earlier. This check is indicated by the green arrow in Figure 18. The yellow star indicates that the concrete has enough bearing capacity to withstand the axial load of the column. The red star shows that the maximum stress that occurs on each end of the bearing plate is less than the bearing capacity. The blue star shows that the chosen value for the plate thickness exceeds the minimum calculated value, t_{min} .

⁴⁹ McCormak, Jack C., *Structural Steel Design*, 4th Ed. Pearson Prentice Hall, 2008.

Design of Baseplate			
Try base plate width	128.00 in	OK	← Trial Width and Check
Check Bearing Pressure:			
$\Phi_c P_p$	32901.12 >	744.43	OK ★
$\Phi_c * 0.85 * f'_c * A_1 * \text{sqrt}(A_2/A_1)$	32901.12 k		
$1.7 * f'_c * A_1$	50135.04 k		
fmax	-0.28 ksi	OK	★
Mu	256.00 in-k		
tmin	6.89 in		
Choose t	7 in	OK	★

Figure 18: Base Plate Design Spreadsheet

Design for Bending Moment Strength and Reinforcement

The foundation is subject to large overturning moment forces and therefore must be designed for bending moment strength. The critical section for the bending moment using sample values can be seen in Figure 19 below.

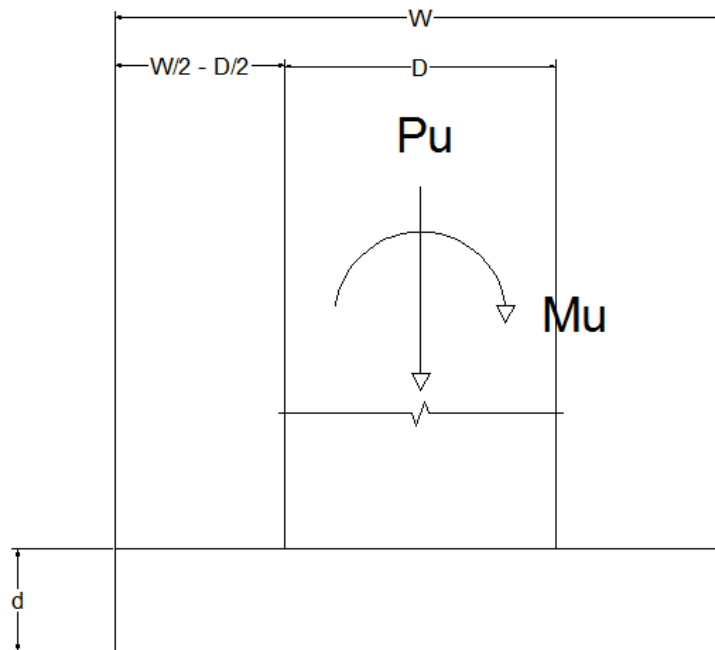


Figure 19: Critical Section for Bending Moment

For the purpose of the project, the bending moment value will be previously determined from the tower design. When the side footing length was selected, the eccentricity was checked to ensure that the

loading was within the middle third of the footing. Thus, the dimension of the side of the square footing and the moment value from the tower design will be used along with the footing dimensions and reduction factor to compute the required coefficient of resistance for strength design, R_n , in the equation below:

$$\text{Required } R_n = \frac{M_u}{\phi b d^2}$$

Where ϕ is equal to 0.9. This value is used in the equations for the reinforcement ratio, ρ , to find the required and minimum required area of steel, A_s , as specified by ACI- 10.5.4, for the development of reinforcement for the critical section seen below:

$$\rho = \frac{1}{m} \left(1 - \sqrt{1 - \frac{2mR_n}{f_y}} \right)$$

Once the value of ρ is computed, it is necessary to check to ensure that it is less than or equal to 75% of the reinforcement for a balanced condition, p_b , which is calculated in the equation below:

$$p_b = \frac{0.85f'_c}{f_y} * \beta_1 * \left(\frac{87,000}{87,000 + f_y} \right)$$

Where β_1 is equal to 0.85 for a concrete compressive strength of 3000 psi and the values for f'_c and f_y are taken from the initial assumptions. Once this check is satisfied, the reinforcement ratio can be used to compute the total area of reinforcing steel required and the minimum required area of steel shown below:

$$\text{required } A_s = \rho b d$$

$$\text{minimum required } A_s = 0.002 b d$$

Where m is equal to $f_y / 0.85 f'_c$. The result of this equation can be used to choose the number and

diameter of bars for reinforcement. In the design spreadsheet, the input values can be chosen by the user based on the computed A_s . The bar sizes can be chosen from Table 9 below.

Table 9: Size, Diameter, and Area of Reinforcing Bars 50

Bar Size	Nominal Diameter (in.)	Bar Area (in²)
3	0.375	0.11
4	0.5	0.2
5	0.625	0.31
6	0.75	0.44
7	0.875	0.6
8	1	0.79
9	1.128	1
10	1.27	1.27
11	1.41	1.56
14	1.693	2.25
18	2.257	4

⁵⁰ *Chu-Kia Wang, C. G. (2007). Reinforced Concrete Design. John Wiley & Sons, Inc.*

As shown in Figure 20, when the user selects a bar size as indicated by the green arrow, the values for the nominal diameter and the area for one bar are auto-filled from Table 9 as shown by the red bracket. The calculated reinforcement ratio will be checked determine if it satisfies the requirement for a balanced condition as indicated by the "OK" to the left of yellow arrow. The outcome of this portion of the spreadsheet, shown by the blue arrow, is that it suggests a number of bars based on the size chosen by the user.

Design for Bending Moment Strength			
Required Rn	574.47 psi		
m	15.69		
p	0.016		
Check balanced condition			
pb	0.028	OK	★
Required As	85.91 in ²		
Min Required As	12.10 in ²		
Bar Size, #	9.00	←	User selection
Diameter, db	1.13 in	}	Auto-fill from Table
Area, a	1.00 in ²		
Suggested # of bars, As/a	85.91		
Chosen # of Bars	86.00	←	Outcome: Number and Size of Bars
As	86.00 in ²		

Figure 20: Design for Bending Moment Strength in Design Spreadsheet

After the number of bars and the area of steel are selected, the bar spacing and moment values must be checked. The spacing is calculating by dividing the side length of the footing by the number of bars. According to ACI Code provisions, the spacing must be at least 1 in between the bars. The new bar diameter is calculated using the same equation as the initial calculation below:

$$new\ d = d - 3(cover) - 1\ bar\ diameter$$

The nominal bending capacity must also be found using the area of steel provided by the bars in the following series of equations:

$$C = 0.85f'_c b a$$

Where C is the compressive stress and a is the depth of the stress block. This equation is set equal to the equation for tension:

$$T = A_s f_y$$

The result is a value for a which is used to compute the nominal bending capacity, ϕM_n using the equation below:

$$\phi M_n = \phi T [new\ d - 0.5a]$$

Where ϕ is equal to 0.9. This value is compared to the actual moment value, M_u , resulting from the tower calculations. The suggested value of bars does not necessarily result in adequate bending capacity since the values of the required area of steel and the actual area of steel can be very close. Therefore, if “NG” is displayed in the check for the moment capacity, the user must increase the amount of bars used. In addition, the spacing of the bars will automatically default to at least one inch if the calculated value is less than one inch. This means that the side length of the footing may need to be increased or larger bars can be chosen. Although the exact values for the bar spacing are displayed in the spreadsheet, for a more realistic design, the bar spacing would most likely be rounded. Figure 21 shows the checks for bar spacing and adequate bending strength.

Check Bar Spacing			
Calculated bar spacing	1.95 in/bar	OK	★
Actual bar spacing	1.95 in/bar		
New d	31.87 in		
Compressive Strength, C	428.40 a		
Tensile Strength, T	3440.00 k		
a	8.03 in		
Phi Mn	7187.12 ft-k	OK	★

Figure 21: Check for Bending Moment Strength and Bar Spacing

If the chosen reinforcement passes all bending moment strength and bar spacing tests then ACI Formula 12-1 below can be used for the development of reinforcement.

$$L_d = \left(\frac{3}{40} \frac{f_y}{\sqrt{f'_c}} \frac{\psi_t \psi_e \psi_s \lambda}{\left(\frac{c_b + k_{tr}}{d_b} \right)} \right) d_b$$

Where c_b is the smaller of the two values from the following equations:

$$\text{bottom and side cover} = 1.5 (\text{clear}) + \text{bar radius} = c_b$$

$$\text{one - half center - to - center spacing} = c_b$$

The values for ψ_e, ψ_s , and λ are equal to 1.0 because for the purpose of this project, the bar sizes will typically need to be #7 or larger due to spacing constraints. The value for ψ_t is equal to 1.3 for top bars, which according to ACI-12.2.4 are horizontal reinforcement which greater than 12 inches of concrete below them. Since footings for turbines are deeper than 12 inches, a value of 1.3 can be assumed in the calculations and design spreadsheet. The value k_{tr} is equal to zero if there are no stirrups. The maximum value is 2.5, so if $\frac{c_b}{d_b}$ is greater than that value, then 2.5 must be used. These substitutions simplify the equation to:

$$L_d = \left(\frac{3}{40} \frac{f_y}{f'_c} \frac{1.3}{\left(\frac{c_b}{d_b}\right)} \right) d_b \quad \text{ACI 12.1}$$

Once this value is computed, it is compared against the actual embedment which is about two inches of cover subtracted from the width of the footing.

As seen in Figure 22 below, the actual embedment must be greater than the development length. In this example the required embedment length is too large resulting in a “NG” message to the left of the yellow star. This check will cause the design spreadsheet to display the “Increase footing width” message underneath the check. This requires that all hand calculations affected by the footing length need to be updated, however, with the spreadsheet, the side length of the footing can be increased until the check is “OK” as long as all of the other requirements in the design spreadsheet remain satisfied.


Development of Reinforcement		
bottom and side cover	2.06 in	
one half c-c spacing	0.98 in	
cb	0.98 in	
Cb/db	0.87	OK
Actul Cb/db	0.87	
Ld	92.76 in	
Actual Embedment	70.00 in	NG 
		Increase footing width

Figure 22: Failed Embedment Length Check

Using the numbers in this example, the footing needed to be increased from 14 feet to 16 feet in order for the embedment length to be adequate as seen in Figure 23. As mentioned above, all other checks were satisfied and the number of bars was adjusted to reflect the increase in footing width.

Chosen side length	16 ft	OK	$q \text{ equiv} < q_a$
Development of Reinforcement			
bottom and side cover	2.064 in		
one half c-c spacing	1.129412 in		
cb	1.129412 in		
Cb/db	1.001252	OK	
Actul Cb/db	1.001252		
Ld	80.21764 in		
Actual Embedment	94 in	OK	★

Figure 23: Embedment Length Check "OK" After Increase in Footing Width

The final step in the design of the footing is to update the shear calculations using the new value for d , which was updated for the actual spacing and bar diameters. The procedure is the same as before as seen in Figure 24 from the design spreadsheet below with the red bracket signifying the calculated values including the new value for d .

Check Shear with New d			
average d	31.87 in		} Assumed or Calculated Values
Vu	1036.80 k		
bo	270.13 in		
Bc	1.00 (for squares)		
Alpha s	40.00 (for columns)		
Choose smallest of:			
Vc	2829.44 k		} Ratio Checks for Four-Sided Critical Section
Vc	3168.71 k	OK	
Vc	1886.30 k	OK	
Governing Vc	1886.30 k	OK	
Check Two-Way Shear			
Bc	1.00	OK	} No Shear Reinforcement
bo/d	8.48	OK	
Shear Capacity ($\phi c, V_c$)	1414.72 k	OK	
Check One-Way Shear			
Vu	87.09 k		} Outcome: Footing Depth Adequate for
Vcu	670.35 k		
Phi*Vc	502.76 k	OK	

Figure 24: Updated Shear Check with New d value

Since the updated shear check is “OK”, and no reinforcement is needed, this means that the dimensions, base plate, and reinforcement for the footing are adequate for the given tower including loading and soil conditions.

Foundation Dimensions

Based on the tower axial load, moment, and column base diameter, obtained from the tower design, the footing was designed using the method explained throughout the chapter. Table 10 displays the user input values were used for the soil and concrete conditions for the design examples:

Table 10: Assumed User Input Values for Design Examples

Soil and Concrete Condition	Value Used in Design Examples
Bearing Strength of the Soil, q_a	4500 psf (sandy-gravel)
Compressive Strength of Concrete, f'_c	3000 psi
Yield Strength of Concrete, f_y	40,000 psi

The results of the calculations involving the assumed user input values from Table 10 and the characteristics listed below in Table 11, the following results for the foundation design were produced.

Table 11: Summary of Foundation Design Results for Various Tower Heights

Design Criteria	250' Tower	350' Tower
Diameter at base	8 feet	12 feet
Total axial load acting at tower base	774.4 kips	1206.9 kips
Total moment acting at tower base	6956.0 ft-kips	21637.7 ft-kips
Total shear load acting at tower base	52.0 kips	115.5 kips
Footing Dimensions	16'x16'x3'	24'x24'x6'
Base Plate Dimensions	128"x128"x7"	180"x180"x8"
Reinforcing Steel	85 #9 @ 2.25"	113 #9 @ 2.55"

By observing these results, it can be determined that the 100 foot increase in tower height and consequent increase in axial loads and moment forces resulted in a 350% increase in the area of footing required. It was found that for both tower heights, the embedment length was the governing factor in the width of the footing. The bar size, #9, was kept consistent to show the increase in bars between the two footing sizes, which was 28 bars, and the spacing was increased 0.30 inches. Since the base plate

calculation is checked with shear, the designs were similar because the shear strengths of the 250' and 350' towers were similar as seen in Table 11. Hand calculations and the corresponding design sheets for the 250 foot tower can be found in Appendix E.

In addition to the variations in the loads from the tower, three different soil types were investigated using the dimensions and load data from the 250-foot tower. The soil types included clayey soil, sandy-gravel, which was used as an assumed user input value in the previous examples, and rocky soil. The foundation results are summarized in Table 12 below:

Table 12: Summary of Foundation Design Results for Various Soil Types

Design Criteria	Clayey Soil	Sandy-Gravel	Rocky Soil
Diameter at Base	8 feet	8 feet	8 feet
Allowable Bearing Strength ⁵¹	2000 psf	4500 psf	6500 psf
Footing Dimensions	22'x22'x3'	16'x16'x3'	14'x14'x4'
Base Plate Dimensions	128"x128"x7"	128"x128"x7"	128"x128"x7"
Reinforcing Steel	82 # 9 @ 3.22"	85 #9 @ 2.25"	58 #9 2.90"

By observing these results, it can be determined that the changes in the soil type affect both the width and the depth of the foundation. In the clayey soil, the footing width needed to be larger in order to compensate for a loss in bearing capacity in comparison to the assumed user value of 4500psf. When the allowable soil capacity was increased to 6500 psf for rocky soil, the footing width was able to be decreased; however, the depth of the foundation needed to be increased to satisfy the requirements for shear strength. The bar size, #9, was kept consistent to show the fluctuation in the number of bars between the different footing sizes and soil types. The results show that a significant decrease in the number of bars occurred in the rocky soil because of the combination of the increase in the depth and the decrease in the width of the footing. The design spreadsheets for the clayey and rocky soil can be found in Appendix E.

Anchor Bolt Design

Another important aspect of the design of the wind turbine footing and base plate is the anchor design. Since the anchor bolt design very case-specific and dependent on many different factors, it was not included in the design spreadsheet. The sample calculations for the anchor bolt design are referenced in

⁵¹ Pole Barns, (November 2009), www.pole-barn.info/soil-bearing-capacity.html

Appendix G. An anchor bolt design was created for the 250-foot tower in with the characteristics in Table 13:

Table 13: Anchor Bolt Design Tower and Foundation Characteristics

Design Criteria	250' Tower
Diameter at base	8 feet
Total axial load acting at tower base	774.4 kips
Total moment acting at tower base	6956.0 ft-kips
Total shear at tower base	52.0 kips
Footing Dimensions	16'x16'x3'
Base Plate Dimensions	128"x128"x7"
Reinforcing Steel	85 #9 @ 2.25"

There are a large variety of rods available for use as anchor bolts. In the design for the 250-foot tower, the rods used for the anchor bolt design are threaded ASTM A490 rods with a tensile capacity of 119 ksi and a shear capacity of 60 ksi. The rods were designed for tensile strength based on ASCE 7-05 Table 7-2. The bolts were designed in a circular configuration around the base of the tower flange. The bolt strength for combined shear and tension was considered to ensure that the bolts had adequate strength to resist those forces. The final design for the anchor bolts is listed in Table 14. Figure 25 shows the anchor bolt design and dimensions.

Table 14: Anchor Bolt Design for 250' Tower

Anchor Bolt Characteristic	Result
Number and Type of Rods	28 1-½" ASTM 490 rods
Embedment Length	18 "
Edge Distance	7.5"
Spacing	17 1/8"

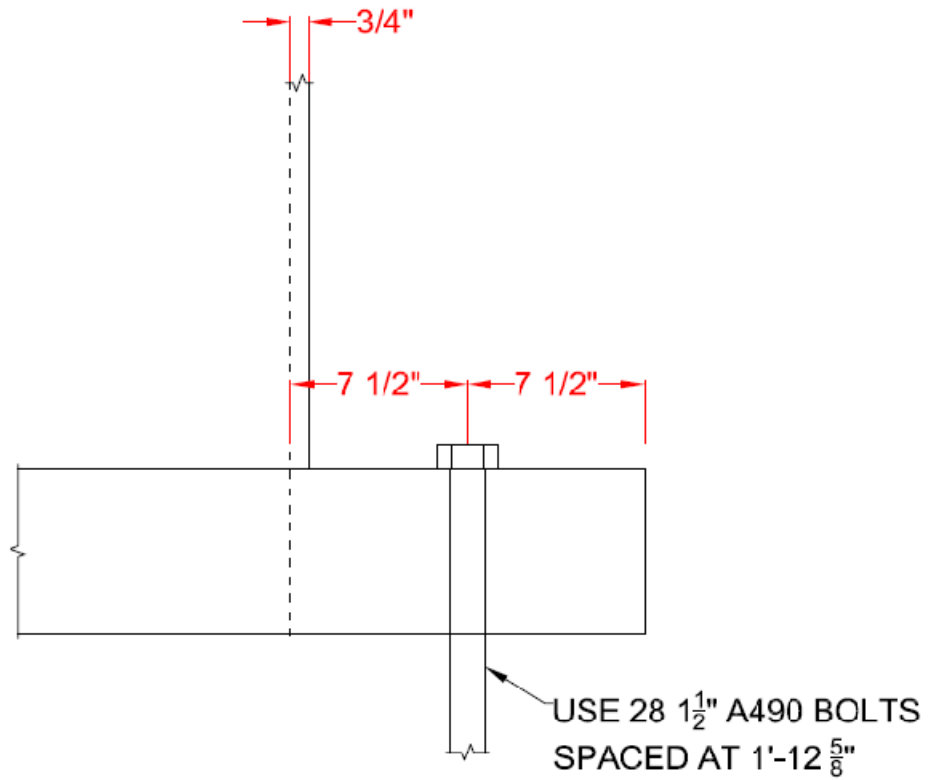


Figure 25: Anchor Bolt Design Detail

AutoCAD Drawings

AutoCAD Drawings were prepared for the design alternative in Table 15. Figure 26 and Figure 27 show the final dimensions for the design criteria.

Table 15: Design Criteria for AutoCAD Drawings

Design Criteria	250' Tower
Diameter at base	8 feet
Total axial load acting at tower base	774.4 kips
Total moment acting at tower base	6956.0 ft-kips
Footing Dimensions	16'x16'x3'
Base Plate Dimensions	128"x128"x7"
Reinforcing Steel	85 #9 @ 2.25"

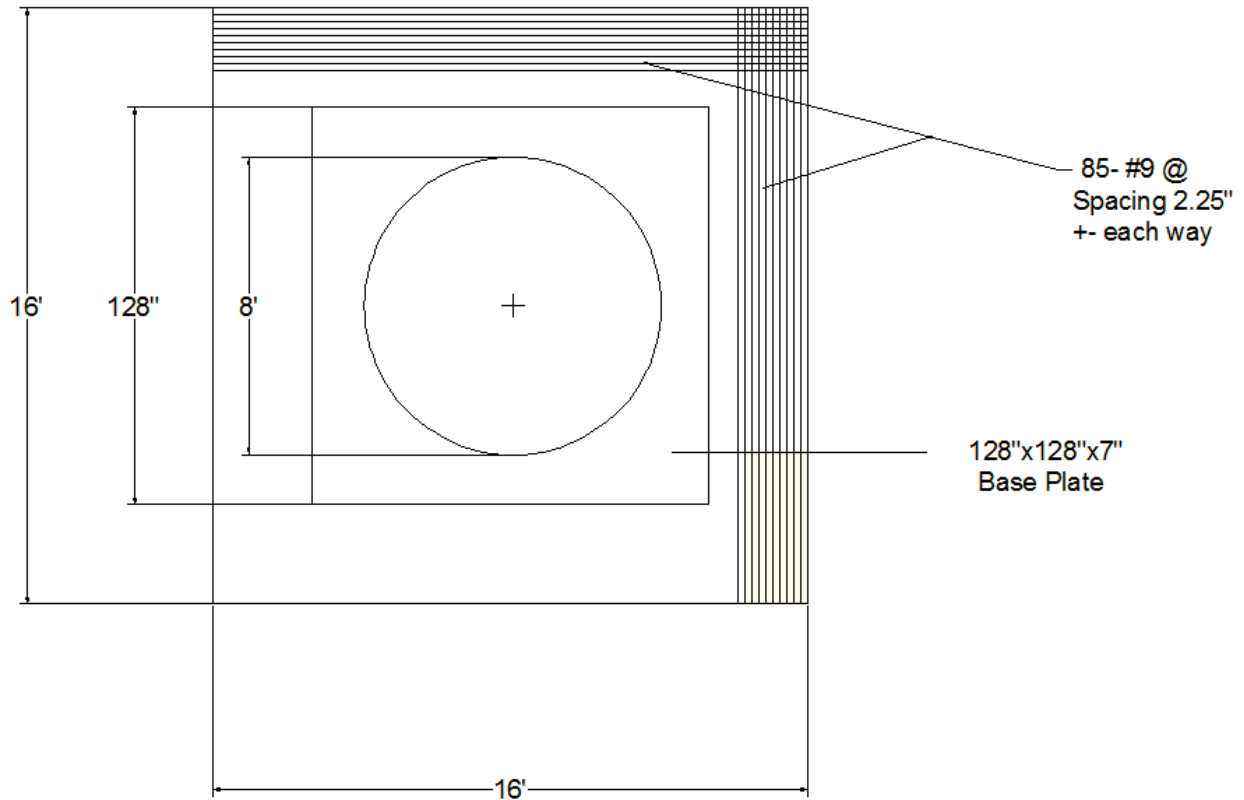


Figure 26: Footing and Base Plate Dimensions with Established Reinforcement (Top View)

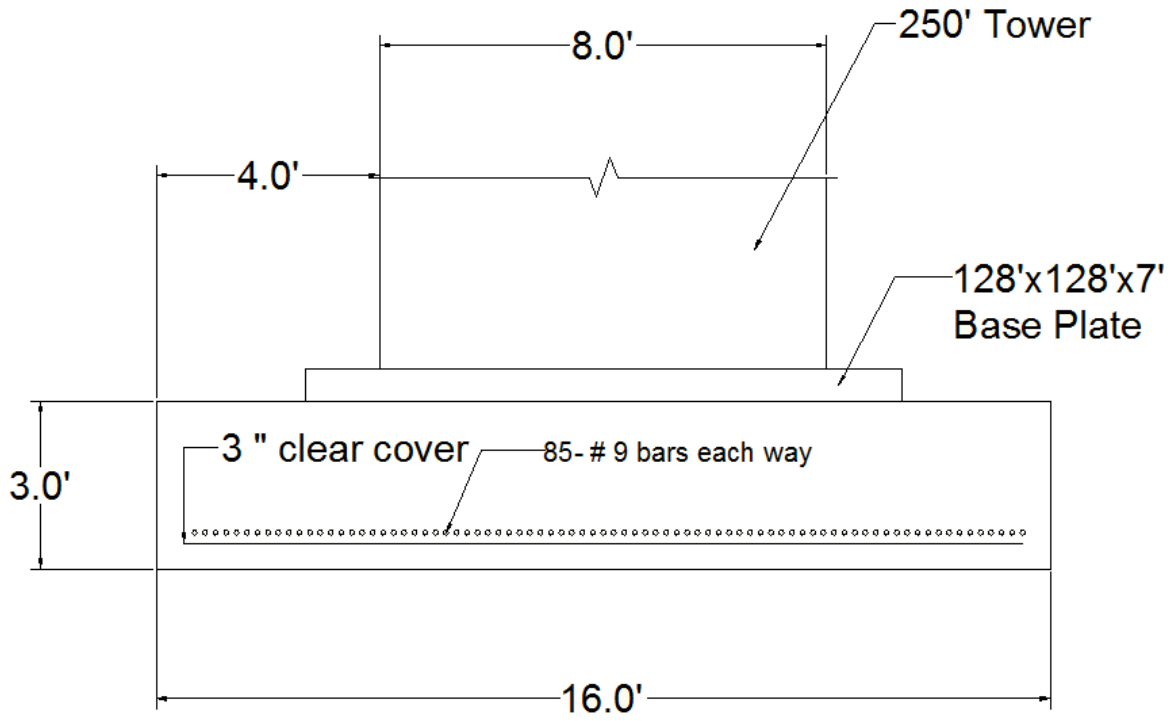


Figure 27: Footing and Base Plate Dimensions with Established Reinforcement (Side View)

Chapter 6: Project Analysis

The economics, energy output, permitting and scheduling all play a big role in the overall understanding of a project. In order to analyze these topics, an Excel spreadsheet was created which uses key information entered by a person involved in the project and calculates various necessary pieces of information needed to analyze many aspects of the project. This spreadsheet can calculate an estimate of the electricity output, the revenue earned, the expenses owed, the overall cash flow, and even the IRR of investing in a wind turbine. These are all important pieces of information for anyone who is researching the benefits of installing a wind turbine. This spreadsheet does many calculations and takes many things into consideration; all of which will be explained in this chapter. While this spreadsheet details the energy output, economic feasibility, and even helps design the tower and foundation, permitting and the overall process of installing a turbine were also researched for this project. These two topics are left out of the spread sheet, but are explained in this report.

Permitting

One of the first steps in the scheduling process is permitting the turbine. The process of obtaining a permit for a wind turbine project varies by state and county according to local laws. This report will focus on the general permitting process in the state of Massachusetts, where the guidelines are well developed. The Massachusetts Division of Energy Resources developed a “model amendment” for all Massachusetts towns which is show in the document “Model Amendment to a Zoning Ordinance or By-law: Allowing Wind Facilities by Special Permit.” This model explains the General Standards and Permitting Application. The permitting application is required to prove that a project is abiding by the general standards outlined in the beginning of the amendment.

In the general standards section of the amendment many topics are addressed. These topics range from insurance to turbine design; and from lighting to environmental impacts. Specifically, the following topics are addressed: Liability Insurance, Site Control, Height, Setbacks, Color, Finish, Lighting, Signage/Advertisement, Utility Connections, Appurtenant Structures, Shadow/Flicker, Noise, Habitat Impact, Facility Maintenance, Removal/Abandonment and Financial Security. Details about each of these topics can be found in the model amendment on The Official Website of the Commonwealth of Massachusetts. Significant details that should be considered are: insurance must be owned for the turbine; the tower must be set back at least 1.5 times the height of the turbine from any nearby inhabited structures, overhead utility lines and public roads; a financial surety is sometimes required of

turbine projects in the case that the turbine becomes abandoned and must be removed; and the wind turbine should not adversely affect neighboring properties. An application must be submitted by the project owners in order to prove they are abiding by all of the above standards.

The permit application process is a lengthy step in a wind turbine project. This can last upwards of many years if there is any opposition to the turbine installation. In this application the owner must include several pieces of information regarding their project. Included in this packet must be: General Information, Descriptions of the Property, Operation and Maintenance Plans and Compliance Documents. Each of these sections details a list of necessary documents that must be submitted in order to be considered for a permit.

The General Information section of the permit application is a very simple section. In this section the owner must supply the applicant's name, address, phone number, and signature. They must also provide the name, address, phone number, and signature of any agent who is representing the applicant. Lastly, in this section, documentation is needed to prove that the applicant has a legal right to use the site.

The Description of Property section is a much lengthier section of the application. First a locus map (1:25,000) showing the area within at least two miles of the facility is needed along with the planned wind turbine location. Next, a map of the proposed facility site at the scale of 1"=200' is needed that shows the contour intervals, property lines, existing buildings, roads, tree cover, and proposed ground equipment. Also needed are pictures which show what the structure will look like once it is installed. These pictures "should be representations from populated areas or public ways within a 2 mile radius of the wind facility." Lastly, a landscape plan for the site is needed.

The next section is the Operation and Maintenance Plan. This section requires a plan to maintain all of the access roads and storm water controls on the site. Also, a plan that outlines the procedures for operating and maintaining the wind facility is needed. These procedures are often provided by the manufacturer of wind turbines, so it will be beneficial to have the wind turbine model chosen prior to this permitting step.

The last section needed for the permit application is the Compliance Documents section. In this section, the owner must demonstrate to the town that the project is complying with the general standards outlined earlier. A financial surety must be provided here, as well as proof of liability insurance. A certification must be provided showing that the height of the structure is within the limitations

addressed in the General Conditions and if it is not within these limitations that it has been documented that the turbine has permissible reasons to be taller. These reasons that will allow the tower to be taller are “the applicant demonstrates by substantial evidence that such height reflects industry standards for a similarly sited wind facility; such excess height is necessary to prevent financial hardship to the applicant, and the facility satisfies all other criteria for the granting of a special permit under the provisions of this section.⁵²” And lastly, documentation regarding existing and maximum project noise levels is necessary to show that the noise is lower than the allowed maximum detailed in the general standards section.

This permitting process, while specific to Massachusetts, is similar across the country and often focuses on the same topics (see other state government websites). It is necessary to obtain a permit for a wind turbine project in order for construction to begin, and it is often this stage which holds up undesirable projects. Permitting is one of the first steps in the process to installing a wind turbine facility and must be taken seriously.

Installation

The next step in the schedule of a wind turbine project is the installation of the turbine. In order for this step to occur, the permitting, cost analysis, and manufacturing of the turbine must all have taken place. The first step of the installation process is clearing the site. This may be a large step if the site has many trees and other vegetation. After this step, the construction of an access road might be necessary, especially where the site is remote and has no existing roads. This road is needed in order to transport the wind turbine to the site, as well as any necessary construction equipment. After these two steps have been completed, the excavation for the foundation can take place. The entire installation of the cast-in-place foundation (and often design) is completed by a separate company who must be hired for the job in advance. The foundation can then be formed and installed. This step can be affected by the weather. Thus, enough time should be allotted for concrete placement and curing. After the foundation is cured the wind turbine can be installed. In order to complete this step a crane is needed to lift the tower segments, nacelle, and blades into place. This step is often done by companies suggested by the manufacturer. Wind speeds should not be excessive during installation of the tower in order to steer clear of all possible problems. Erection in rain or snow conditions also impedes the installation process.

⁵² *Massachusetts Division of Energy Resources. (n.d.). (November 2009),*
<http://www.mass.gov/Eoca/docs/doer/renew/model-allow-wind-by-permit.pdf>

Once the turbine is completely installed, it is connected to the local utility and can start generating electricity.

Maintenance

Once the wind turbine has been installed, some maintenance is required to keep it operating. The turbine should be inspected at least once every year; however, twice is recommended. Most turbines will come with a maintenance schedule and procedure when they are purchased from the manufacturer. These procedures detail how the blades, tower, foundation, and mechanics are inspected and maintained. Maintenance cannot be done by the average person. Often times, manufacturers have companies in many areas whom they train in maintenance in order to perform services in their region. The types of companies who perform these services are becoming more popular around the country. It costs, on average, \$10,000-\$15,000 every year in order to keep a turbine well maintained⁵³. This cost is relatively small compared to the cost of repairing a large problem on the turbine. Annual maintenance increases the overall life of the turbine and ensures that it runs smoothly.

Project Analysis Spreadsheet

As previously mentioned a project analysis spreadsheet was created to calculate values that would be needed to analyze the finances and design of the project. This analysis of the finances of the project is crucial to a successful project. Investors depend on this cost analysis when deciding whether or not an investment in the project is beneficial or not. Many different factors must be considered when calculating these final numbers, and often times these factors vary from project to project. It is most beneficial to have a spreadsheet that can be altered according to each different project. Major factors that must be considered are the average wind speed of a site, the size of the wind turbine, the location of the turbine, which influence ice loads and maximum wind forces, and the financing structure of the project.

The four main sections of the spreadsheet are Energy Output, Economic Analysis, Tower Design, and Foundation Design. The document is user friendly and only requires data to be typed into one main "General Information" page as shown in Figure 28. This input information is used in many different equations and eventually important turbine information is summarized in the output reports. Data is also calculated in the input tab of the spreadsheet in order to show the user various pieces of

⁵³ Kuhn, B. (2009, October 23). Vice President Marketing.

information that can be useful in the design of the project. This information includes checks of different equations that show whether or not design dimensions entered will be adequate.

Project Information		User Input
Project Name		Auto Fill
Street Address		Drop Down
Town, Zip Code		
Site Information		
Average Wind Speed (m/s):	9	
Altitude (m):	150	
Average Temperature (°C):	16	
Land Costs (\$/ac):	\$0	
Altitude Correction:	0.99	
Temperature Correction:	1.00	
Land Area (acres):	2	
Clear Area or Wooded?:	Clear	
Road Needed?:	No	
Length of Road Needed (ft):		
Financing Information		
Initial Costs (NPV):	\$3,301,449	
Down Payment (\$):	\$3,301,449	
Years of Payback (N):	15	
Salvage Value (%):	25%	
5-Year Escalation Rate:	5%	
Initial Investment:	\$3,301,449	
Coupon Payments:	\$200,000	
Internal Rate of Return:	5.29%	
Grants Received (\$):	\$100,000	
Tower Designed Specific or from Mfg?:	Designed Specifically	
Electricity Information		
Retail Value (\$/kWh):	\$0.128	
Value of RECs (\$/kWh):	\$0.035	
Value of PTCs (\$/kWh):	\$0.021	
Weibull K Factor:	2	
Wind Shear Exponent:		
Availability:	0.96	
Turbulence Intensity Loss:	0.03	
Safety Margin Loss:	0	
Electricity Generated/Year (kWh/yr):	2,601,073	
Turbine Information		
Turbine Model:	Aeronautica Windpower 47-750	
Turbine Cost:	\$1,265,000	
Rated Power (kW):	750.0	
Rotor Diameter (m):	47.0	
Swept Area (m²):	1734.9	
Tower Height (m):	76.2	
Hub Height (m):	77.7	
Tower Information		
Town:	Worcester	
Overall height (ft):	250	
Diameter at Base (ft):	8	
Diameter at Hub (ft):	7	
Shell Thickness (m):	0.75	
Strength of Steel (ksi):	36	
Modulus of Elasticity (ksi):	29,000	
Weight of Steel (lbs/ft³):	490	
Weight of Equipment (kip):	400	
Axial Strength (Check):	OK	
Bending Strength (Check):	OK	
Combined Loading (Check):	OK	
Overall Tower Design (Check):	OK	
Price of Steel (\$/lb):	\$0.30	
Price of Steel Fabrication (\$/lb):	\$2.70	
Foundation Information		
Allowable Bearing Stress, q _a (pcf):	4,500	
Compression Strength of Concrete (f _c) (psi):	3,000	
Yield Strength of Concrete (f _y) (psi):	40,000	
Footing Depth (ft):	3	
Footing Width (ft):	16.0	
Base Plate Width (in):	128	
Bar Size, Moment (k):	9	
Number of Bars:	85	
Bar Spacing (in/bar):	2.26	
One-Way Shear Reinforcement Needed?:	No	
Two-Way Shear Reinforcement Needed?:	No	
Footing Depth (Check):	OK	
Reinforcement (Check):	OK	
Base Plate Width (Check):	OK	
Bearing Pressure (Check):	OK	
"i" (Check):	OK	
Overall Foundation Design (Check):	OK	

Actual Load	<	Design Load
744.4 ft-k	<	1508.5 ft-k
83472.5 ft-k	<	220467.8 ft-k
0.8 ft-k	<	1.0 ft-k

Figure 28: Sample General Information Page

Energy Output

The electricity output of the wind turbine is the key revenue generator for the project. Because of this, in order to reliably analyze the economics of the project, the electricity output must be predicted using a probabilistic model which account for many different uncertainties in the wind. In order to do this, many different factors must be considered and analyzed. The key to this analysis is the power curve for the specific wind turbine. The power curve depicts the amount of electricity that the given wind turbine can generate at varying wind speeds. Because the energy contained in the wind raises exponentially for every 1 mile per hour increase in wind velocity, the electricity output of the machine also raises exponentially. The manufacturer generally produces these power curves for each of their products and provides these to the general public so that they can show how much electricity their product could produce.

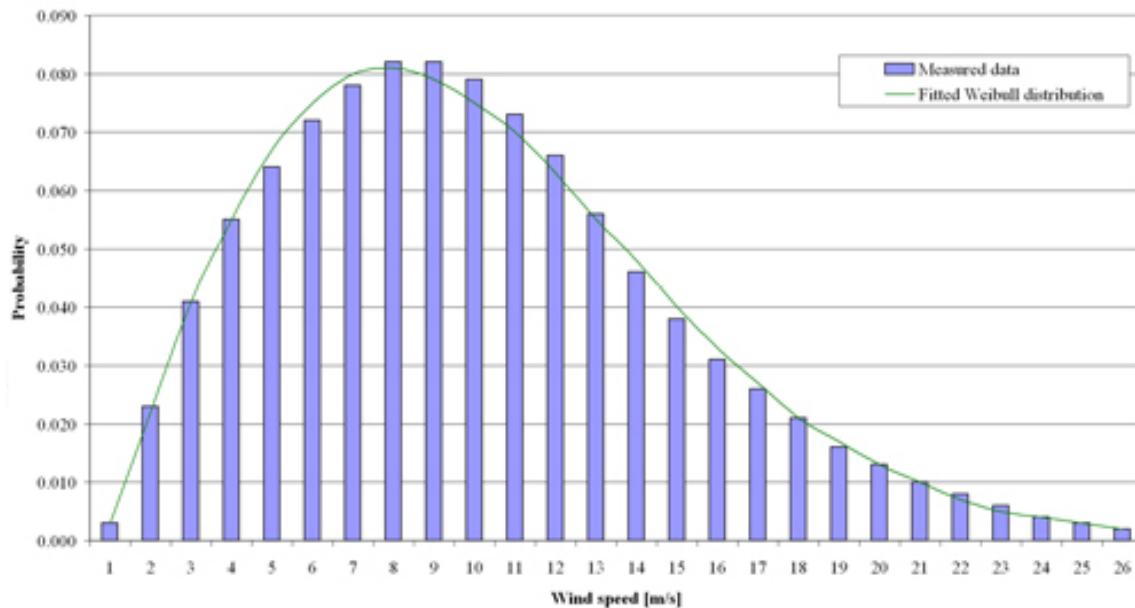


Figure 29: Wind Speed Distribution (www.wind-energy-the-facts.org)

The probability curve above in Figure 29 estimates the amount of time the wind will be blowing at any given wind speed at a specific location. In order to do this, the average wind speed for a site is used and is evaluated using a statistical formula to model the probability density of the distribution of wind speeds. The curve that defines the assumed distribution of wind speeds is generated using a certain factor that is common in the wind energy community. This factor is called the Weibull K factor, and for a

curve that is predicting wind speeds the factor is assumed to be 2⁵⁴. This factor defines the shape of the parabola being used as shown in Figure 29. Once this parabola is developed, other factors must be taken into consideration in order to more accurately derive an expected value for the amount of electricity that will be generated by the wind turbine in the specific location.

The altitude and temperature of a site also affect the expected value for the overall electricity the turbine generates. Both of these factors play a direct role in the density of the air in a given area, and the density of the air changes the amount of energy it contains. Equations are used to evaluate the change in energy contained in the wind colliding with the turbine, and these equations are incorporated into the spreadsheet. Often, these factors play a small role in affecting the electricity produced by the machine; however it still must be analyzed to produce results which consider all factors. The equations below are for the altitude and temperature correction in evaluating the energy in the wind coming in contact with the wind turbine:

$$\frac{1.2252 - (0.0001194 \times A)}{1.2252} = X_a$$

A = Altitude (m)

$$\frac{1013250}{2870000 \times (273.15 + T)} \times \frac{1000}{1.2252} = X_t$$

T = Temperature (°C)

The efficiency of the wind turbine is an attractive fact to know. This can be calculated by dividing the power curve energy (kW) for a given wind speed by the energy actually contained in the wind (kW) at that speed. The actual energy contained in the wind is calculated in the equation below. The entirety of this energy cannot be harvested by a wind turbine, and the energy that can be harvested is defined by the power curve of the turbine given by the manufacturer. By comparing these two numbers the efficiency of the turbine (at a given wind speed) is found:

⁵⁴ J.V.Seguro, T. (2000). Modern estimation of the parameters of the Weibull wind speed distribution for wind energy analysis. *Journal of Wind Engineering and Industrial Aerodynamics* , 75-84.

$$E_w = \frac{(W_s^3 * (1.2252/2) * A_s)}{1000}$$

W_s= Wind Speed (mph)

E_w= Energy in the Wind

A_s= Swept Area of Blades

The times that the wind turbine will not be operating are also considered when analyzing the overall energy output of the machine for an entire year. These outages are usually due to the annual maintenance that must occur for the upkeep of the machines. There are also outages due to extreme wind speeds when the turbine must shut down to prevent damage. These outages usually cause a very small percentage of lost revenue, but are vital to the quality of the estimate for the overall energy output. In order to take these times into consideration, a location is available in the Project Analysis spreadsheet to input the limiting factor that will be used. This factor can be cautious or bold, depending on the owner's approach to risk. If cautious, this number could be as low as 0.7⁵⁵, to allow for any possibility of low energy generation from the turbine, which would occur if there was not enough wind, there was a great deal of maintenance needed, and the turbine was not performing to maximum capacity. If bold, this number could be as high as 1.0, assuming that the turbine will always perform as expected for the entire year.

All of these variables are taken into account when calculating the expected value for the overall energy output of a given wind turbine. They are shown in Table 16 to demonstrate how each different wind speed produces a different output, which is summarized in the final section of the page.

⁵⁵ (Kuhn, Vice President Marketing, 2009)

Table 16: Sample Energy Output Analysis

Wind Speed Group (m/s)	Gross Wind Power (kW)	Turbine Power Curve	Efficiency (%)	Corrected Turbine Power (kW)	Weibull Freq. Distribution	Turbine Power (kW)	Hours/Year or Wind Blows at this	Gross Wind Energy (kWh/yr)	Turbine Avg. Energy Output
1	1.062826	0	0	0.0	0.0318	0.00	279	296	0
2	8.502609	0	0	0.0	0.0606	0.00	531	4515	0
3	28.69631	4	0.1393908	4.2	0.0839	0.35	735	21080	3,100
4	68.02087	24	0.3528329	25.3	0.0998	2.53	875	59497	22,146
5	132.8533	52	0.3914093	54.9	0.1079	5.92	945	125588	51,857
6	229.5704	91	0.3963925	96.0	0.1084	10.41	950	217995	91,159
7	364.5494	152	0.4169531	160.4	0.1025	16.44	898	327316	143,973
8	544.167	234	0.4300151	246.9	0.0919	22.69	805	438155	198,764
9	774.8002	332	0.4284975	350.2	0.0786	27.51	688	533199	241,026
10	1062.826	440	0.4139906	464.2	0.0642	29.80	562	597764	261,064
11	1414.622	540	0.3817275	569.7	0.0503	28.65	441	623261	250,986
12	1836.564	635	0.3457544	669.9	0.0378	25.34	331	608629	221,997
13	2335.029	714	0.3057778	753.2	0.0274	20.61	240	559614	180,518
14	2916.395	740	0.2537379	780.7	0.0190	14.87	167	486490	130,222
15	3587.038	750	0.2090861	791.2	0.0128	10.10	112	401173	88,488
16	4353.336	750	0.1722817	791.2	0.0083	6.53	72	314638	57,184
17	5221.665	750	0.1436324	791.2	0.0051	4.07	45	235207	35,639
18	6198.402	750	0.1209989	791.2	0.0031	2.45	27	167891	21,431
19	7289.924	750	0.1028817	791.2	0.0018	1.42	16	114602	12,438
20	8502.609	750	0.0882082	791.2	0.0010	0.80	9	74903	6,970
					Total	230.48		5911815	2,018,963

Expected production (kWh) in one year

The expected value for the output electricity, measured in kilowatt hours (kWh), is crucial to the cost analysis and decisions about the feasibility of the project. This electricity can be sold, and its revenue accounts for the majority of revenue acquired by the project.

Once all of these factors are taken into consideration, the Project Analysis spreadsheet summarizes all of the important information regarding the estimated Energy Output and displays this information in a user friendly printout page. This page is shown in Figure 30.

Energy Generation



<i>Project Name</i>	
<i>Street Address</i>	
<i>Town, ZipCode</i>	
Wind Turbine Model:	<i>Aeronautica Windpower 47-750</i>
Rated Power:	750 kW
Average Wind Speed:	7.5 m/s
Electricity Generated/Year:	1,990,504 kWh/year
Weibull K Factor:	2

Theoretical Wind Power = $W_{avg}^3 * (1.225/2) * (A_{swept}/1000)$

Power Curve for Turbine (Advised Power Generated): Given by Manufacturer

Efficiency = Theoretical Power in Wind/Advised Power Generated

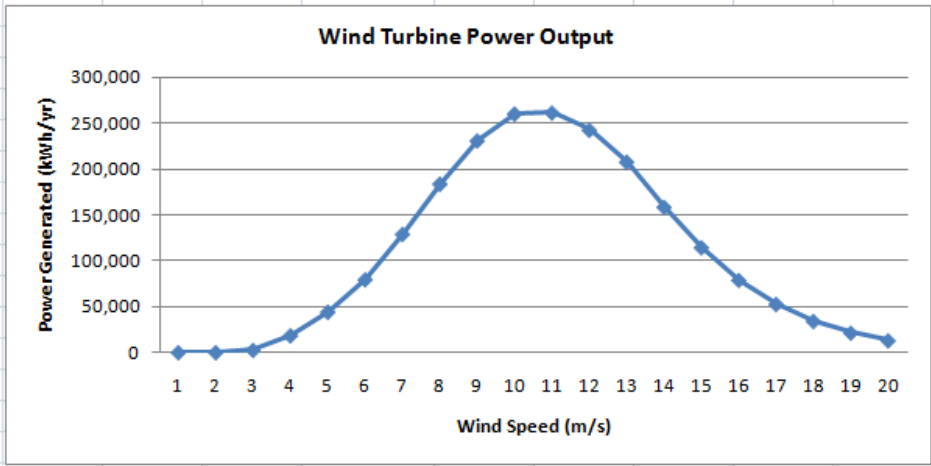
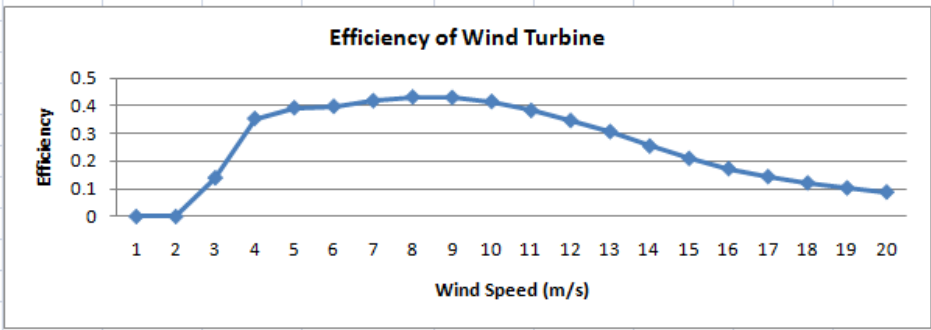


Figure 30: Sample Energy Generation Summary Sheet

Financial Analysis

Installing a wind turbine requires a large, lump sum of money in the beginning of the project.

Approximately 70% of the capital that is needed for the project is needed in the first year. This can be seen in the financing information in project analysis spreadsheet. Because of this, the owners of projects must often receive financing from an outside source. It is very rare that an owner can fund the entire project without the use of an outside lender or investor.

There are many ways capital can be obtained in the beginning of a project. The owner can apply for a loan from the bank, similar to a mortgage, and pay back monthly payments on the loan. Another option is to have outside financiers invest their money in the project. In order to do this, the owner needs to provide them with the expected return on their investment, and prove to them that the investment is a good financial decision. Similar to investors, the owner could partner with one other person who has a large sum of money and they could receive capital from that individual. This would be similar to a business partnership, and the partner with the large sum of money would see the financial returns quickly.

In order to demonstrate to investors that their investment would be a good economic decision, the owner of a project must analyze the expected revenues versus the expected costs. Various expenditures must be considered when analyzing the expected cost of the entire project. These main factors include feasibility studies, permitting costs, machine costs, installation, maintenance, financing, and insurance. Out of these main costs, feasibility studies, permitting costs, machine costs and installation costs are all required in the very beginning of the project. It is safe to analyze these costs as initial costs to the project; however, the costs of maintenance, financing, and insurance all have more costs due to the fact that they are affected by time value of money because they are needed years later.

Analyzing the finances for a project is a necessary step in order to verify that the project will be financially feasible. In analyzing these finances, many different factors must be considered that can affect the overall revenue of the project. The escalation rate of money, time value of money, internal rate of return, and safety factors all should be analyzed.

The escalation rate of money must be considered when analyzing future costs and revenue. This rate can fluctuate from year to year and a safe estimate of the average for past years may be determined. In this project, it was assumed that a 5-year escalation rate was sufficiently accurate and would be acceptable for the research. This was decided because the escalation rate varies only slightly from year

to year, and analyzing the rate every five years gives a balanced rate that accounts for the slight fluctuation from year to year. This escalation rate can then be applied to the annual cost analysis in order to consider how the money in the future is affected.

Time value of money is also a key factor that must be analyzed. The time value of money is the value of money including a given amount of interest earned over a certain period of time. For example, if someone was to invest \$1.00 at a 5% interest rate then they would have \$1.05 in one year. The value of the \$1.00 today and the \$1.05 in one year is the same to the investor at T=0, however at T=1 year the investor would rather have \$1.05. The interest rate for this investment must be calculated using the present value (the \$1.00, or in the wind turbine situation, the initial value of the investment) and also the future value (the \$1.05, or in the wind turbine situation, the final amount of money the project is left with).

Another factor that the analyst might want to consider is the safety percentage of the financial investment. This percentage takes into consideration the safety factor in the project's financial analysis. This is to consider the fact that the project may not make as much money as anticipated, because the actual energy output was less than the expected. When doing an analysis, one should make sure that they will definitely have a positive investment and not lose any money. This percentage can be as high or low as the investor wants, it merely depends on how cautious the investor would like to be.

The internal rate of return is a calculated value that is used to show investors how much profit their investment will produce. This value shows the investor the quality (or yield) of their investment. By calculating this number the owner of the project is more likely (or less likely, if the IRR is small) to obtain willing investors. This value must be as accurate as possible in order to eliminate the potential conflicts in the future. The IRR can be calculated using the net present value equation shown in the equation below for net present value. In this equation, the IRR is the "r" value.

$$NPV = \sum_{n=0}^N \frac{C_n}{(1+r)^n} = 0$$

C_n = Net Yield in Year "n"

Many portions of the wind turbine installation process cost money which is not considered in the price of the turbine from the manufacturer. These added expenditures include the foundation, surveying,

land clearing, and roadway installation (if necessary). RSMeans is a publisher of reference books that offers construction cost data that can be used to estimate costs of various construction activities. These books were used to estimate the cost of the portions of wind turbine installation process. The cost data was used to estimate the cost/unit of each activity. The foundation is estimated in \$/CY, surveying and land clearing is estimated in \$/acre, and roadway installation is measured in \$/LF. This is integrated into the Project Analysis spreadsheet in such a way that the operator only has to enter the cubic yards of foundation needed (specifically the dimensions of the foundation and the CY is calculated), acreage of land on the site, and the linear feet of roadway needed, and the spreadsheet will calculate and output the costs of each of the activities.

All of the revenues and expenses of the project are analyzed in the Project Analysis spreadsheet. This analysis gives the expected profits of the owner and the investor for various projects. This information is also summarized in an output sheet titled "Economic Analysis" from which the user can easily see important financial information about the project. A sample of this output sheet is shown in Figure 31.

Economic Analysis



Project Name
Street Address
Town, ZipCode

Wind Turbine Model: *Aeronautica Windpower 47-750*

Turbine Cost (w/ Installation):	\$3,301,449
Retail Value of Electricity:	\$0.128
Owner's Profit (20 Year Value):	\$2,787,662
Investor's Profit (20 Year Value):	\$1,701,122

Revenue:	
Retail Revenue:	\$332,937
<small>(Electricity \$/kWh x kWh Generated per Year)</small>	
REC Revenue:	\$91,038
<small>(RECs \$/kWh x kWh Generated per Year)</small>	
PTC Revenue:	\$54,623
<small>(PTCs \$/kWh x kWh Generated per Year)</small>	
Grant Revenue:	\$100,000
<small>(Sum of All Grants)</small>	
Total Revenue:	\$578,597

Expenses:	
Turbine Cost:	\$1,725,500
<small>(Turbine Cost, Shipping, Foundation)</small>	
Pre-Construction:	\$134,990
<small>(Site Investigation, Legal Work, Permitting)</small>	
Construction:	\$1,405,250
<small>(Site Upgrades, Electrical Tie-In, Labor)</small>	
Yearly Expenses:	\$91,000
<small>(O&M, Insurance, Land Leasing)</small>	
Total Expenses:	\$3,265,740

Financing Structure:			
Loan Type:	Investor	5 Year Escalation Rate:	5%
Loan Amount:	-\$3,301,449	Break-Even Point (Owner):	13
Coupon Payment:	\$250,000	Break-Even Point (Investor):	10
Years of Payback:	10	Internal Rate of Return:	4.40%

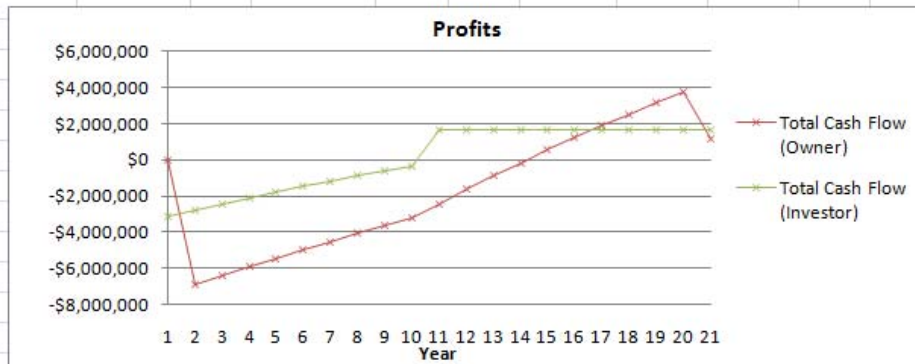


Figure 31: Sample Economic Analysis Summary Sheet

This summary sheet shows the user the financial outcomes based off of the project information they entered into the General Information tab of the Project Analysis spreadsheet. This specific output sheet shows the results of a positive cash flow project, which uses an investor type of investment. In this output sheet the user can reference the break-even point for the investor and the owner, which is given in the number of years it takes for the cash-flow to be positive. It also shows the overall profit of the investor and the owner, in 20-year values (i.e. the future value of their investments). The internal rate of return is given which shows the investor that the investment will (or will not) make money and at what degree they will do so.

Sample Project Analysis

This output sheet gives values for a ten year loan, paying \$250,000, on a 750kW Aeronautica Wind Turbine. The tower is designed specifically and the cost of the tower is taken from the cost estimation tab. The break-even point for the investor is year ten, and for the owner it is year 13. This example shows that with these values, both the investor and the owner have positive profits from the project. Table 17 shows financial information for the same 250 foot wind turbine that was designed in Chapters 4 and 5.

Table 17: Summary of Financial Data for 250' Turbine

Wind Turbine Model:	Aeronautica Windpower 47-750
Turbine Cost (w/ Installation):	\$3,301,449.24
Retail Value of Electricity:	\$0.12
Owner's Profit (20 Year Value):	\$1,626,386.20
Investor's Profit (20 Year Value):	\$3,692,746.17

Chapter 7: Offshore Turbine

In addition to land-based wind turbines, wind turbines located in the ocean, or offshore wind turbines, have emerged as an efficient source of renewable energy. In comparison to land-based wind turbines, offshore turbines are subject to higher wind speeds and are made in a larger scale. Thus, offshore turbines have the potential to produce more energy in a more efficient manner. This chapter will investigate the implications of an offshore wind turbine including the tower and foundation design, and modifications to the construction and energy delivery process.

Offshore Wind Turbine Background

As indicated throughout this report, a main factor in the feasibility and potential energy production of a wind turbine is the wind speeds in the location of the turbine. For this reason, offshore wind turbines appear to be a promising source of renewable energy for the future due to stronger wind speeds. While the investigation of offshore wind turbines in Europe began in the early 1980's, the design of offshore wind turbines is a relatively new technology to the United States.⁵⁶ The reason for this is because the United States has had more success with wind energy on land than European countries due to the climate and geography of the country. However, with increasing energy demands and costs, there has been a recent focus on offshore wind in the United States. At the end of 2008, it was determined that worldwide, there was 120,800 MW of wind generating capacity versus 1,471 MW from European offshore projects.⁵⁷ The wind generating capacity from offshore projects is predicted to rise to over 11,000 MW as a result of projects planned throughout Europe, Asia, the United States, and Canada in 2010.⁵⁸

As with land-based wind turbines, there are several characteristics and variables that that need to be investigated when selecting a suitable site for the offshore turbine. The following characteristics should be examined:

- Distance from the shore
- Proximity to power demand sites, local electric companies, local airports
- Interference with telecom installations and line of site from onshore
- Interference with wildlife and avian population

⁵⁶ European Wind Energy Association. EWEA. (October 2009), from www.ewea.org

⁵⁷ European Wind Energy Association

⁵⁸ *OCS Alternative Energy and Alternate Use Programmatic EIS: Information Center*. (n.d.). R(February 2010), Offshore Wind Energy: ocsenergy.anl.gov/index/index.cfm

- Existence of under-sea gas lines and cables
- Impact on existence on shipping routes

In terms of assessing the effects of the surrounding environment on the performance of the wind turbine, the existing wind conditions, the depth to bottom of the ocean, and wave conditions are critical. Additionally, currents, tides, ice loads, and geotechnical properties associated with the sea-floor soil must be taken into consideration.⁵⁹

Offshore towers are typically larger than land-based turbines because of cost implications. The massive and unique loading conditions on an offshore turbine drive the construction and foundation costs.⁶⁰ Therefore, larger turbines with greater energy outputs are being produced to offset initial construction expenses.

Offshore Tower Design

The offshore tower design was based on methods used for the land-based turbine design and included additional loads due to the effects of wave, shown in Figure 32.

⁵⁹ Malhotra, S. (2009). *Design & Construction Considerations for Offshore Wind Turbine Foundations in North America*. Civil Engineering Practice , 9.

⁶⁰ Malhotra, 11

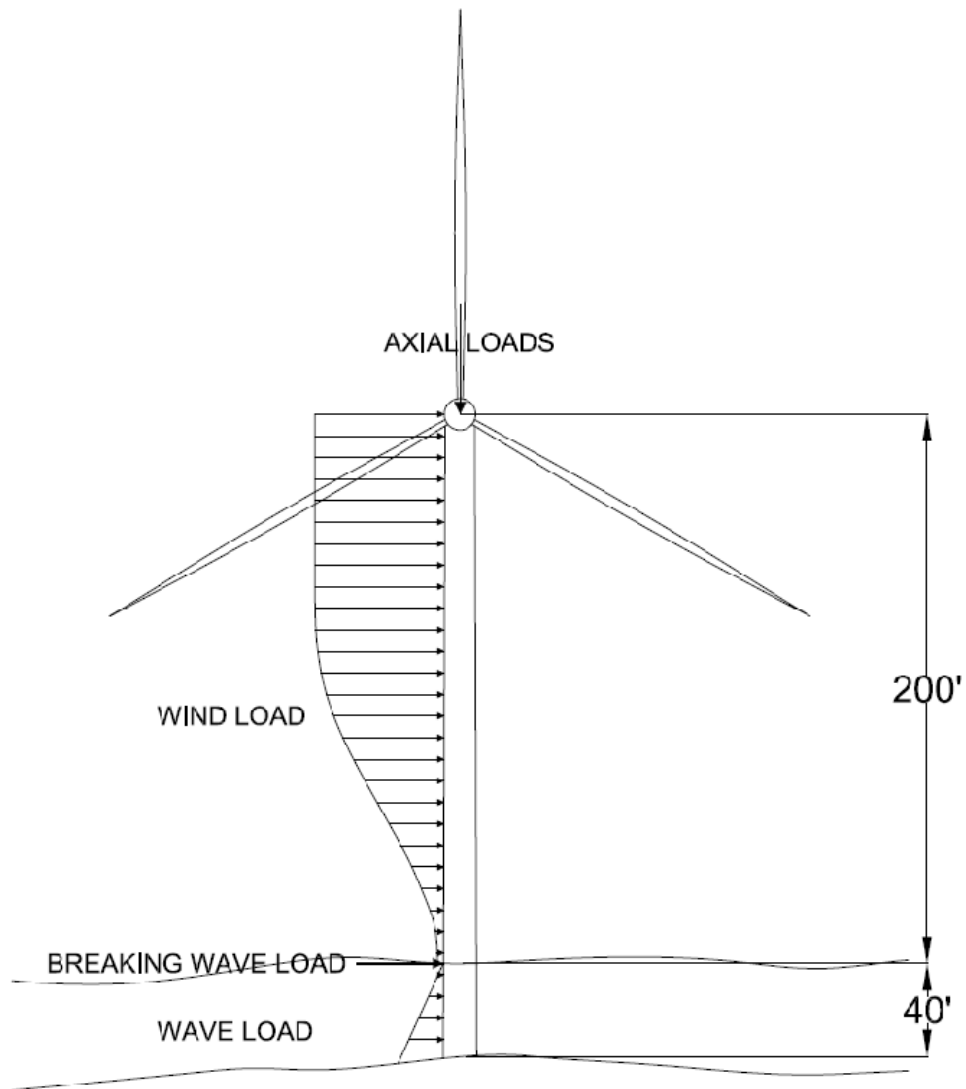


Figure 32: Loads Acting on Tower

The portion of the tower that remains above the sea level is designed with the same axial and lateral loads as a land-based turbine as described in Chapter 4: Tower Design. At sea level, a breaking wave load is introduced as follows:⁶¹

$$F_D = 0.5\gamma_w C_D D H_b^2$$

where

⁶¹ ASCE-7-05 (2005). American Society of Civil Engineers.

γ_w is the density of sea water, 64 pcf,

C_D is 1.75 for round columns,

D is the diameter of the tower at the sea level height,

and H_b is the water depth times a reduction factor of 0.78.

The hydrodynamic loads below the water surface are calculated by multiplying the water prism, $F_{water} = \frac{1}{2}\gamma_w h^2 D$ by an impact factor of two. In the water prism equations, h is the water depth. The forces on a 240 foot tower with 40 feet submerged underwater are summarized in Table 18 and Table 19 below. All supporting calculations and design sheets can be found in Appendix H.

Table 18: Forces Due to Axial Loads

Force	Axial
Equipment	700 kips
Steel	359.2 kips
Ice	47.03 kips

Table 19: Forces Due to Lateral Loads

Force	Moment	Shear
Wind	15370 ft-k	106.7 kips
Breaking Wave	27256 ft-k	681.4 kips
Wave	17066.7 ft-k	1280 kips

The tower was designed for axial, bending, and combined stresses in the same manner as the land-based wind turbine. The design summary data is in Table 20 below.

Table 20: Offshore Turbine Design Data

Design Criteria	240' Tower
Diameter at base	12.5'
Diameter at hub	11'
Shell thickness	1 in.
Strength of steel	36 ksi
Modulus of Elasticity	29,000 ksi
Total axial load acting at tower base	1318.1 kips
Total moment acting at tower base	59693 ft-k
Total shear load acting at tower base	2068.1 kips

Offshore Foundation Design

The design chosen for this tower is the driven steel monopole deep foundation, which is acceptable for depths up to 165 feet.⁶² Since the wind turbine is subjected to such excessive lateral forces, the foundation must be designed to resist it. The method used to design the diameter and depth of the foundation was Evan and Duncan's Charts, which is based on the p-y method. The p-y method limits the design in terms of lateral load deflection, which must not exceed $\frac{1}{2}$ ".⁶³ In the equations used to calculate the characteristic shear and moment loads, the diameter of the pile is assumed and checked for deflection. The depth of the pile is determined by multiplying its diameter by 35 so that it is considered fixed at its bottom for a correct analysis. The diameter chosen for this design was 12 $\frac{1}{2}$ feet, and the minimum depth of the pile should be 437 $\frac{1}{2}$ feet. All equations and calculations can be found in Appendix H.

Construction of Offshore Wind Turbines

The construction of offshore wind turbines can be summarized in three main phases. First, the transportation phase depends on the type of turbine and supporting structure being used. Most often the pieces of the turbine are transported to barges located at the site of construction. Sometimes pieces can be transported together after they are already assembled (i.e. the nacelle attached to the tower with the rotor also attached), but sometimes the turbine has to be shipped with the pieces separate. The barges at the site are used as a construction platform for the second phase of construction, the erection. The barges are most commonly available, however they lack the stability needed to construct the turbine. An alternative to this platform is a "jack-up" rig which is stable enough platform for

⁶² Malhotra, 11

⁶³ Coduto

installing the nacelle, rotors, and tower as well as for driving the piles needed for the foundation. These jack-up rigs lack the amount of maneuverability needed to install the turbine efficiently. A platform with a good balance of both stability and maneuverability is a “ship-shaped vessel” which carries a rotating crane. The last phase of construction needed for the installation of an offshore turbine is installing electrical wires in order to transport the generated electricity to the main land where it will be used. These wires cannot float in the open ocean because of the change of animal or marine vessel interference. In order to prevent this, the wires can be buried under the sea floor. Many different methods exist for burying these wires; however a method that has shown to be less invasive to the surrounding ocean floor is called “hydro-plowing”. This method uses water to excavate a trench in which the wires can be placed, and this trench is eventually filled with shifting sediment.⁶⁴

⁶⁴ Malhotra, S.,14

Chapter 8: Conclusions

The purpose of this project was to design a land-based wind turbine considering various design alternatives and investigate the implications. In-depth research was conducted on wind energy and current design standards of wind turbines. This research contributed to the improved understanding of the main factors that influence the feasibility and potential efficiency of a wind turbine at a given site. This allowed for the initial establishment of suitable conditions for wind turbines. Once these conditions were determined, the group explored the available options for designing the turbine tower and foundation. The background research also provided key information on the social and political aspects involved when a suitable site is found for a turbine in a given community.

In terms of the structural design of the turbine tower, the main factors that were considered were the wind loads, dead loads from the weight of the turbine, ice loads, and lateral loads. Using the *Massachusetts State Building Code (780 CMR)* and *AISC Steel Construction Manual* as main resources for the design equations, it was determined that in terms of height, the tower is governed by the required flexural strength. A computer analysis was conducted using ANSYS, which investigated the deflection of the tower design in order to find the magnification factor of the moment on the tower. By exploring different wind speeds and tower heights, the portion of the project pertaining to the tower design resulted in a better understanding of the relationship between axial loads, moment forces, and varying wind speeds.

The foundation design procedure for the wind turbine was created primarily in reference to ACI code provisions. The foundation was designed considering different design alternatives, such as varying tower heights and soil properties. The implications of these changes were observed from the use of the integrated design spreadsheet, which used the tower data from the tower design process. This portion of the project resulted in the understanding of the checks involved in the design of a foundation including shear, bearing, and bending stresses.

The project analysis not only helped to fulfill the capstone design requirement of investigating the cost-effectiveness of a project, but also tied the construction of a wind turbine back to the initial research about the feasibility. It was important to recognize that the success of a wind turbine is not just based on the site, but also the financial status of the individuals and organizations involved.

The investigation of an offshore wind turbine provided further experience with alternative designs and the implications involved. New forces were introduced and considered, such as dynamic wave forces

and breaking wave loads. This section of the report helped to reinforce the idea of how forces and other factors can affect the wind turbine design and helped to identify the advantages of offshore wind.

The integrated spreadsheet was designed with the intention to allow users with less technical experience to explore their options in terms of turbine and foundation sizes and the financial aspects of installing a wind turbine. The spreadsheet can also theoretically eliminate a site based on feasibility and save a community or prospective builder the time and money that may have been spent on a preliminary wind study.

As stated in the background chapter, wind energy and wind turbines are relatively new technologies, and therefore, do not have an abundance of prior research. Therefore, it was difficult to find complete design methods for the turbines. The design processes utilized throughout this project was adapted from several different codes and methods because most of the published information on wind turbines are owned by wind turbine companies. In the future, when more cohesive design methods are available for wind turbine towers and foundations, the integrated spreadsheet could be modified to increase its accuracy.

For future work, it is recommended that additional design alternatives are performed such as the use of ring stiffeners or high strength steel. The consideration of the amount of steel and concrete used in the design could be investigated in order to determine the most cost-efficient design. As a result of the background research, it was found that ring stiffeners could help optimize the design of the tower by decreasing local buckling and allowing for the use of thinner steel for the shell⁶⁵. For the purpose of the integrated spreadsheet, complex design factors, such as ring stiffeners, were not considered, but it may be effective to design them for more specific projects.

In addition, research revealed that other geometries for the footing may be more efficient including circular, octagonal, or hexagonal. It is recommended that once more information is available to investigate the implications of replacing a square foundation with another shape.

⁶⁵ Uys, *Optimisation of a Steel Tower for a Wind Turbine Structure*, ScienceDirect, <http://www.sciencedirect.com/science> (November 2009).

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Appendices

Appendix A: Project Proposal

Project Number: LDA-1004

WIND TURBINE DESIGN AND IMPLEMENTATION

A Proposal for a Major Qualifying Project Report:

Submitted to Faculty of

WORCESTER POLYTECHNIC INSTITUTE

in partial fulfillment of the requirements for the

Degree of Bachelor of Science

By

Bethany Kuhn

Julie Marquis

Hilary Rotatori

Date: October 15, 2009

Approved:

Professor Leonard D. Albano, Advisor

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Problem Statement

As the demand for more environmentally-friendly energy resources grows, energy providers have recognized the importance of wind power through the development of wind turbines. In fact, wind energy is the only renewable resource that has grown faster than predicted.¹ The construction of a wind turbine is limited to specific site guidelines due to social and special constraints; therefore there is an alternative need for sites other than land-based turbines, such as off-shore and retrofit (on an existing building) turbines. Wind turbines must be designed to withstand heavy loads and moments due to extreme wind condition to prevent failures, as well as other forces that are introduced with alternative site designs.

Objective

The purpose of this project is to design a land-based wind turbine as well as explore designs for off-shore and retrofit options for the energy needs of the chosen site. To begin studying the construction of wind turbines, a suitable site must be chosen based on a given criteria. The effect of wind loads is evaluated on the wind turbine for its design. This project will investigate and draw recommendations on the process of constructing three different wind turbines with different types of foundations.

Scope of Work

This project will be split into two main sections; the first being design and the second being project management. All three group members will contribute to each section of the project. The design section will focus on detailing how a specific wind turbine tower is designed as well as how a foundation should be designed. The project management section will detail the steps that go into scheduling and analyzing the overall cost of the project. These parts contribute to fulfilling the capstone design requirement for the three group members.

Background

Project Management

The Project Management aspect of this process must commence as early as possible in order to have the most efficient decision making process. The main part of this stage is to create an accurate schedule in order to complete the tasks at hand on time. This will help keep the project on track and will set

¹ Singh; Concrete construction for wind energy towers

deadlines for different phases. Project Management will also focus the steps that would be needed to permit a wind turbine project. Later on in the project, the cost will be analyzed throughout all stages of the project to ensure that the project is still financially feasible, as well as a life-cycle cost analysis for long-term use of the turbine.

Design

In order to design the tower and foundation of a wind turbine, it is helpful to know the mechanical design and how it works. The force of the wind is applied to the blades of the turbine, which are angled to produce a rotation where the blades convene.² The blades are connected to a shaft that also spins along with the rotation of the blades, which in turns creates energy that is converted through a generator at the base of the turbine. The tower of the turbine rotates (with an axis of rotation perpendicular to the ground) based on the direction of the wind, in which the blades are positioned perpendicular to the wind load to maximize the wind exposed surface area. When the force of the wind exceeds the force at which the turbine was designed for, the blades are positioned parallel to the wind direction in order to reduce the wind exposed surface area and the spinning motion is constricted due to a rotor brake.

Power

The turbine size is determined based on the energy needs of a site. A turbine is generally classified as one of three sizes. Wind turbines can vary in size from small residential turbines to large utility-scale turbines. Table 1 shows the scale, size ranges, and uses for different classifications of wind turbines.

Table 1: Wind Turbine Classifications³

Scale	Average Size Range	Uses
Utility	900 kW - 2 MW	<ul style="list-style-type: none"> •Generate bulk energy for sale in power markets •Commonly used in “wind farms”
Industrial	50 kW - 250 kW	<ul style="list-style-type: none"> •Remote grid production •Reduce consumption of higher cost grid power •May be sold if permitted by state regulations
Residential	400 W to 50 kW	<ul style="list-style-type: none"> •Remote power •Battery charging •Net-metering type generation

² http://en.wikipedia.org/wiki/Wind_turbine_design

³ http://www.powernaturally.org/Programs/Wind/toolkit/9_windturbinetech.pdf

Tower

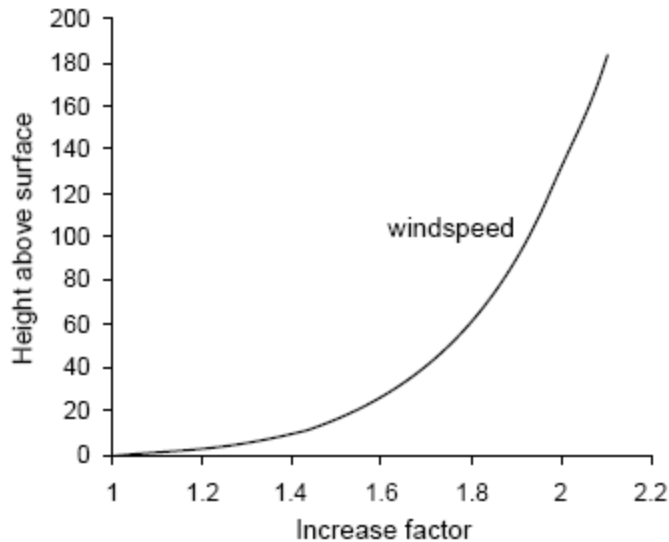


Figure 1: Relationship of Height and Increase factor of Wind

The tower is also an important factor in the design of the wind turbine. The “increase factor” of the wind speed increases exponentially as the height above a surface increases, as shown in Figure 3.⁴ From this graph, one can see that at low heights, the wind speed increases significantly at first, but at higher elevations the rate of increase is much less. The wind speed will also vary due to the surrounding area.⁴ The increase of wind speeds is higher in open areas (such as over a body of water)

than in highly vegetated areas (such as wooded areas). Through years of experience, turbine manufacturers and consultants have recommended that a turbine should be at least 30 feet higher than any object within 300 feet.⁵ This rule of thumb is used to avoid rapid changes wind velocity and direction, or turbulence. The wind speeds must be maximized and direction constant at the blades of the turbine in order to maximize the productivity of the wind turbine.

Foundation

The foundation of a wind turbine is designed for the worst-case loading. The turbine foundation has to resist great loads mainly because of the moment due to wind and must simultaneously resist forces and uplift from geotechnical conditions.⁶ Reinforced Concrete Spread footings are most common in design because it is applicable to a wide range of soil strengths, however for some weaker soils a deeper foundation is needed to stabilize the turbine tower.⁶ The main components in designing a spread footing are its width, depth, amount of steel reinforcement, and construction requirements.

The essential foundation design criteria are stiffness, strength, stability, differential settlement, durability, and economy. These minimum values for foundations are usually provided by the turbine

⁴ <http://www.greenenergyohio.org/page.cfm?pagelid=536> pg 36

⁵ <http://books.google.com/books?id=1ZKky0XK6lUC&pg=PA270&dq=turbulence+wind+turbine#> pg 270

⁶ www.garradhassan.com/.../Wind_Turbine_Foundation_Behavior_and_Design_Considerations.pdf

manufacturers' design specifications. Below lists the design factor, its design criteria, and any design codes or standards that are used in conjunction with the criteria.¹²

Table 2: Foundation Design Criteria

Design Factor	Criteria	Design Codes/Standards
Rotational Stiffness	Acceptable interaction between the flexible concrete footing and the soil supporting it due to rotational forces, allows footing to "give" with applied loads	Arya, et al. and DNV
Strength	Withstand factored loads and fatigue loads	IEC, ACI, FoS
Stability	Resist excessive translational and rotational movement under extreme loads	FoS (Factor of Safety)
Differential Settlement	Considers soil criteria to minimize short-term and long-term differential settlements	Geotechnical Standards
Durability	Resist environmental damage to concrete and maintain serviceability for the turbine's lifetime	ACI

Capstone Design

This project will address the design constraints set by the Accreditation Board for Engineering and Technology (ABET) in order to meet the requirement of capstone design experience for the Major Qualifying Project. As stated by ABET General Criterion 4, "Student must be prepared for engineering practice through the curriculum culminating in a major design experience based on the knowledge and skills acquired in earlier course work and incorporating engineering standards and realistic constraints that include most of the following considerations: economic; environmental; sustainability; manufacturability; ethical; health and safety; social; and political."⁷ In addition to these constraints, this project will consider the American Association of Civil Engineers (ASCE) Commentary on "Engineering Design."

Economic

One of the more important constraints on large-scale civil engineering projects is economic feasibility. The design of the project should consider the cost of construction and the materials used. In order to ensure that the project is affordable, a cost analysis of alternative designs and materials will be

⁷ <http://abet.soe.ucsc.edu/design.html>

performed to help choose a design. A life-cycle analysis of the final design will also be conducted to determine the economic impact of the turbine once it is in operation.

Environmental

The environmental impacts of projects have been a rising concern in the civil engineering industry in recent years. It is important to protect the land surrounding the construction site and to investigate the affects of the wind turbine on wildlife. This project will address environmental constraints by reviewing all environmental codes and regulations regarding the construction and operation of the turbine. This review will allow the group to compile a list of typical regulations that should be considered before a turbine is constructed.

Sustainability

Sustainability is an important aspect of the design of the wind turbine because the design should be durable as well as economical and environmentally-friendly. This project will address sustainability by considering the maintenance to be done on the turbine in the future. The availability of the materials used will also be taken into account. Also, by using innovative designs such as retrofit turbines (on an existing building), land can be preserved. In addition, a life-cycle cost analysis will be done including an estimation of the “payback” time for the construction of the turbine.

Constructability

The constructability of the project will address feasibility of the design and construction of the turbine. This project will address this constraint by considering ease of construction when choosing building materials, and researching the most practical methods for the construction of a turbine. The group will consider using standard dimensions for the tower height and a standard turbine power so that the turbine could be easily constructed and replicated if successful. In addition, the tower will be constructed using a modular approach to facilitate the construction process and limit the necessary site operations.

Ethical

In order to address the ethical constraints the group will use the principles outlined by the American Society of Civil Engineers (ASCE) Code of Ethics which states, “Engineers uphold and advance the integrity, honor and dignity of the engineering profession by using their knowledge and skill for the enhancement of human welfare and the environment, being honest and impartial and serving with fidelity the public, their employers and clients, striving to increase the competence and prestige of the

engineering profession, and supporting the professional and technical societies of their disciplines.”⁸
This project will consider these principles in all aspects of the design and construction plans including creating a sufficient list of regulations and factors related to the location of the turbine.

Health and Safety

In order to address health and safety considerations a structural analysis of the foundation design materials under different forces will be performed. The research performed on zoning and location restrictions will be considered in the design process to ensure that the turbine is an appropriate distance from its surroundings. In addition, the description of the construction process will include the proper safety precautions to protect laborers and residents in the nearby area.

Social and Political

Since wind energy is a relatively new technology, society has not fully embraced the idea of wind turbines being constructed in their communities. This project will address social and political constraints by considering the permitting process of constructing a wind turbine in a specific community and by researching the people and steps involved. Once these processes are understood, a flow chart of the permitting process and people or organizations are involved will be prepared. In addition, the group will examine case studies in order to determine ways that wind turbines have been successfully integrated into the social aspect of communities in the past.

Methodology

This Major Qualifying Project will focus mainly on the design of a land-based wind turbine, with substantial consideration of how this design would change for off-shore and retrofit wind turbine designs.

Schedule

The project will take place over a period of eight months. During the month of September 2009, the scope of the project will be created and initial research for design and construction components will commence. After the initial research is completed and the proposal is written, the design of a land-based turbine will be derived, considering all design factors. Then, a site that would be suitable for the use of a wind turbine will be selected and a wind turbine will be designed for that particular site. A stress analysis will be performed on the turbine foundation with the aid of computer software to check

⁸ <http://www.asce.org/inside/codeofethics.cfm>

our final design. Finally, the design of the land-based wind turbine will be modified to a suitable design for an off-shore and retrofit wind turbine. During this time the group will create relationship chart that will outline the different factors that go into installing a wind turbine, and how a turbine should be designed based on the different variables. In C term, the group will finalize the report and also finish the capstone design requirement of the project. A detailed timeline can be seen in Figure 2.

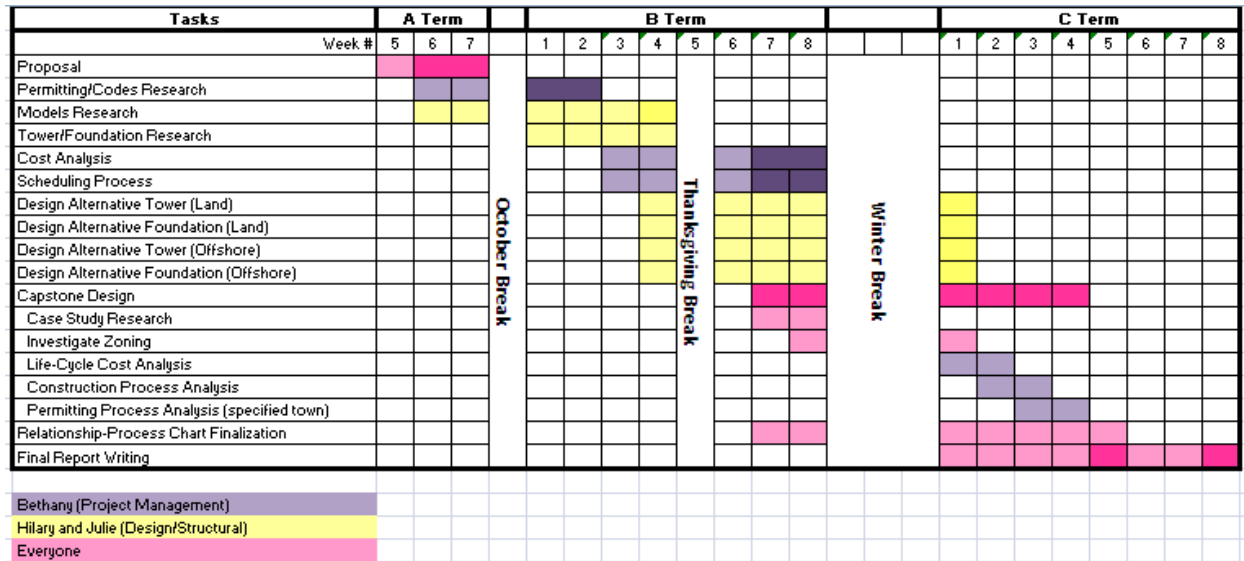


Figure 2: MQP Schedule

Land-Based Wind Turbine Design

When designing a wind turbine, there will be certain design factors associated with the site that is chosen to build upon. This project will examine several different alternatives to the design factors listed below in Table 3. The first column displays the specific design component, the second column displays alternatives to the design component to consider, and the third column displays what other design factors play into designing that particular part of the turbine.

Table 3: Design Factor Alternatives for Consideration

Design Component	Alternatives to Consider	Design Factors to Consider
Soil Profile	1. Sandy 2. Clayey 3. Rock	Strength of Soil
Foundation Type	1. Spread 2. Pile 3. Anchor	Soil Profile, Turbine Size, Number of Blades, Tower Design, Wind Speed
Turbine Size	1. Small Scale 2. Mid-Scale 3. Large Scale	Type of Site, Distribution
Number of Blades	1. Two blades 2. Three blades	Wind Speed, Tower height
Tower Design	Width, height, shape	Wind Speed, Surrounding Terrain, Turbulence
Type of Site	1. Residential 2. Commercial	Zoning, Surrounding Parcels
Wind Speed	1. Class 1-3 2. Class 4-7	Location of Site, Height off the Ground
Distribution	1. Behind the Grid 2. Sell to the Grid	Energy needs of the Site

Through research and calculations, this project will determine how the different alternatives to the design component will affect the overall design of the turbine. A turbine will then be designed for a particular site using the information from this component of the project.

Alternative Turbine Designs

Using the knowledge and experience gained from designing a land-based wind turbine, two alternative turbine designs will be generated: an off-shore wind turbine and a retrofit wind turbine on an existing building. Both cases will have the same basic concepts of designing a land-based turbine; however each design will have different loads applied and different design constraints. The off-shore wind turbine will now have an additional load on the tower from the water current, stronger winds and zero turbulence from the smooth surface of the water, and construction constraints. The retrofit wind turbine will also have stronger winds from being at an increased height (however, more turbulence if surrounding buildings are of comparable heights), and the building will have to be reinforced to handle the force created by the wind turbine. Both designs will be incorporated in this Major Qualifying Project to better understand the differences in design of the wind turbine in different environments.

Deliverables and Conclusions

The final deliverables of this project will include designs for a land-based, offshore, and retrofit wind turbine, along with a design chart identifying different components of designing a wind turbine and how these factors changed based on site conditions. A cost analysis of each design, a list of criteria used to determine potential sites, and a summary of the permitting process to construct a wind turbine will also be determined. Also, a design chart will be created as a reference to assist future turbine designs, in which all of the aforementioned design criteria will be incorporated. The designs will be completed through hand calculations and CAD drawings. The foundation design of the towers will be analyzed in a computer program such as ANSYS. The cost analysis for the land-based, offshore, and retrofit turbines will be completed and will detail the economic return of the entire life-cycle of the turbines. A list of criteria for potential sites will be created in an organized table in order to be referenced for future projects. Also, the steps that must be taken to get a wind turbine approved and permitted will be given in a detailed list. Finally, all of our results will be presented in the form of a Major Qualifying Project Report which will include how all of our deliverables took into account the ABET constraints for capstone design experience.

Appendix B: Interview Notes with Aeronautica Wind Power

Brian Kuhn (Aeronautica), Hilary Rotatori, Julie Marquis, Bethany Kuhn

Friday; October 23, 2009; 8:45AM-10:15AM

1. Aeronautica must send the information they have to a local civil engineer, located near the location where the turbine is being installed, in order to have a foundation designed.
2. Aeronautica can provide us with loading data for the foundation
3. Vestas is a fully integrated design and manufacturing company (as is Norwin)
4. Aeronautica has the design for a 60m tower for the 750kW machine
5. Goal should be to make spreadsheet for wind speeds/tower height (like Norwin has and charges for)
6. Towers are often made of steel
7. Size scales
 - a. ~15-99kW: small-scale
 - b. 100-999kW: mid-scale
 - c. 1MW and up: utility-scale
8. Retrofit is only used for micromachines- not economically feasible
 - a. Smaller machines
 - b. Higher noise
 - c. Rattling/vibrations
9. Clean air vs. Turbulent air
 - a. Clean air is located above all other obstacles
 - b. Turbulent air has more drag/friction
 - i. Located near buildings
 - ii. Another reason retrofit doesn't make sense
10. Power in wind is a cubic function of wind speed
11. Offshore turbines are often done close to the shore line with piles driven into the sand/mud. Turbines located further from the coast are often on a floating platform that is anchored to the ground.
 - a. This costs twice as much money as land-based designs
12. Process chart can depend on the customer**
 - a. Need vs. Desire
 - b. Choose the site vs. Verify the site
 - c. Design of turbine vs. Selection of turbine
13. Special permitting is often required if a bylaw doesn't exist in a town (not creating a bylaw like we had)
14. Do project management before 'design'
15. Town regulations usually refer to the hub height
16. FAA regulations usually refer to the overall height

17. Rotorsize=wingspan
18. Usually an attorney presents the application for a permit to the town (knows the zoning regulations and such)
19. Foundation Design Contacts
 - a. Atlantic Design Engineers, LLC
 - b. Patrick and Henderson (CA)
 - c. Meridian, SGE (RI), Alteris
20. MTC (Massachusetts Technology Collaborative)
 - a. Grants for renewable energy
 - i. Money can go towards wind studies or other necessary studies
21. Can cost anywhere between \$50,000-\$150,000 for permits (even more or less)
 - a. Averages \$100,000
 - b. Studies included in this price
 - c. Feasibility studies- actual vs. desktop wind data (eventually actual data is necessary)
 - i. Doppler wind provides initial data but a wind tower must be installed for at least 1 year to get real wind data (\$30,000-\$40,000)
 1. This is often needed to obtain financing for the turbine
 - ii. Feasibility (Simon's studies(noise, how it will look (pictures), flicker): \$10,000-\$20,000
 - d. Wind appraisal (first step): \$2,000-\$3,000
22. Maintenance and Debt Service are the two major costs once the turbine is installed
 - a. 7-10 year loans available, often between 8%-9%
 - b. O&M (operating and maintenance)
 - i. Twice yearly service checkups
 - ii. Manufacturers provide maintenance checklists
 - iii. \$10,000-\$15,000/year for maintenance

**Also see notes on Process Flow Chart

Appendix C: Tower Design Sheets and Hand Calculations

Design a 250' wind turbine tower in Rutton, Massachusetts for a 1.5 MW turbine.

TRY: 10' ϕ at base
9' ϕ at hub
0.75" shell thickness.

DETERMINE AXIAL LOADS.

→ Wt. of equipment @ top of tower $\sim 400^k$ for 1.5 MW turbine.

→ Wt. of steel: Approximate taper, use 5-50' sections to calculate wt. of steel:

$$V = 50' [\pi R^2 - \pi r^2], \text{ where } R = \text{outer radius and } r = \text{inner radius.}$$

$$W = V \rho_{\text{steel}}$$

Section 1: 10' ϕ

$$V = 50' [\pi (5)^2 - \pi (4.9375)^2] = 97.56 \text{ ft}^3$$

$$W = (97.56 \text{ ft}^3) (0.490 \text{ k/ft}^3) = 47.8050^k$$

Section 2: 9.75' ϕ

$$V = 50' [\pi (4.875)^2 - \pi (4.8125)^2] = 95.11 \text{ ft}^3$$

$$W = (95.11 \text{ ft}^3) (0.490 \text{ k/ft}^3) = 46.6023^k$$

Section 3: 9.5' ϕ

$$V = 50' [\pi (4.75)^2 - \pi (4.6875)^2] = 92.65 \text{ ft}^3$$

$$W = (92.65 \text{ ft}^3) (0.490 \text{ k/ft}^3) = 45.3997^k$$

Section 4: 9.25' ϕ

$$V = 50' [\pi (4.625)^2 - \pi (4.5625)^2] = 90.20 \text{ ft}^3$$

$$W = (90.20 \text{ ft}^3) (0.490 \text{ k/ft}^3) = 44.1971^k$$

Section 5: 9' ϕ

$$V = 50' [\pi (4.5)^2 - \pi (4.4375)^2] = 87.74 \text{ ft}^3$$

$$W = (87.74 \text{ ft}^3) (0.490 \text{ k/ft}^3) = 42.9914^k$$

Total Wt. of tower = $\Sigma W = 226.9985^k$

→ Wt. of ice: use 5-50' sections to approximate taper, so ϕ of tower changes.

Using spreadsheet, calculate the cross-sectional area of ice A_i for 1' sections. Sample calculation for $z = 249 - 250'$ (z will vary).

ASCE-7-05 Ch. 10 - Ice Loads.

$$A_i = \pi t_d (D_c + t_d)$$

$$t_d = 2.0 t_i f_z (K_{zt})^{0.35}$$

t = nominal ice thickness, 1" in Rutland (0.0833')

$I_i = 0.80$ for category I buildings.

$$f_z = \left(\frac{z}{33}\right)^{0.1} = \left(\frac{249.5}{33}\right)^{0.1} = 1.2212$$

$K_{zt} = 1$ assuming no wind speed-up.

$$t_d = 2.0(0.0833)(0.8)(1)^{0.35} = 0.1632$$

$$D_c = \phi \text{ tower} = 9'$$

$$A_i = \pi(0.1632)(9 + 0.1632) = 4.6989 \text{ ft}^2 \times 1' \text{ section} = \underline{4.6989 \text{ ft}^3}$$

$$W_i = (4.6989 \text{ ft}^3)(0.056 \frac{\text{lb}}{\text{ft}^3}) = 0.2631 \text{ k}$$

Repeat for all 1' sections $\sum W_i = 62.8124$ (already factored)

$$\text{Total Factored DL} = 1.2(400 \text{ k}) + 1.2(226.9985) + 62.8124 = \underline{815.21 \text{ k}}$$

DETERMINE LATERAL LOADS - (wind).
TIA/EIA-222

Approximate wind profile by using 1' tower sections. Show example for tower height = 249 - 250'.

$$F = q_z G_n [C_f A_e + \sum C_A A_A] \text{ lb} \leq 2q_z G_n A_G$$

$$q_z = 0.00256 K_z V^2 \text{ (lb/ft}^2)$$

$$K_z = \left(\frac{z}{33}\right)^{2/7} = \left(\frac{249.5}{33}\right)^{2/7} = 1.7824 ; 1 \leq K_z \leq 2.58 \checkmark$$

$V = 100$ mph in 3-second gust, convert to fastest mile wind speed

$$V_{FM} = V_3 \left[\frac{V_3 + 3600}{1.5} \right]$$

gust duration $t = (3600 \text{ s/mi}) V_3 = 3600/(100 \text{ mph}) = 36$
using figure ASCE-7-05, $V_t = 1.31$



$$V_{FM} = \frac{100(1.31)}{1.5} = 87 \text{ mph}$$

$$q_z = 0.00256(1.7824)(87)^2 = 34.5378 \text{ lb/ft}^2$$

$G_H = 1.69$ for tubular pole structures.

$$C_F: D_p = 9' + [2(\text{nominal thickness} = 1/2)] = 9.1667'$$

$$C = \sqrt{K_z V D_p} = \sqrt{1.7824(87)(9.1667)} = 1064.715 > 64 \text{ mph}\cdot\text{ft}$$

$\therefore C_F = 0.59$ according to TIA/EIA-222 table 1.

$$A_F = D_F A_F + D_R A_R R_R (\text{ft}^2)$$

~~$D_F A_F$~~ (no flat structures).

$D_R = 1.0$ for wind direction normal to surface.

$A_R =$ Projected area of round structural components in one face + tilt = $9.1667'$

$$R_R = 0.51e^2 + 0.57 \leq 1$$

$$C = \frac{A_F + A_R}{A_G} = \frac{9.1667}{9.1667} = 1$$

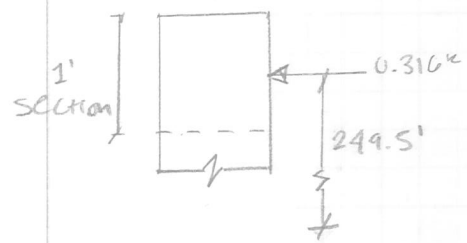
$$R_R = 0.51(1)^2 + 0.57 = 1.08, \text{ use } R_R = 1.$$

$$A_F = 0 + (1)(9.1667')(1) = 9.1667'$$

$\Sigma C_A A$ does not apply - no linear appertenances.

$$F = (34.5378)(1.69)[(0.59)(9.1667')] = 315.57 \text{ k} = \underline{0.3156 \text{ k}}$$

Determine Moment



$$M = F_z = (0.316 \text{ k})(249.5') = 78.40 \text{ ft}\cdot\text{kips}$$

Repeat for all 1' sections of the tower, sum all moments (see excel spreadsheet)

$$\Sigma M = M_{\text{max}} = \underline{8827.6} \text{ ft}\cdot\text{kips, acting at base of tower.}$$

$$\text{Total Shear} = \Sigma F = \underline{65.71 \text{ k}}$$



CHECK AXIAL STRENGTH:

Try: shell thickness = 0.75", $F_y = 36 \text{ ksi}$, $E = 29000 \text{ ksi}$ (standard for rolled steel)

$$\text{Height} = 250' \times 12" = 3000"$$

$$\text{O.D.} = 10' = 120"$$

$$\text{I.D.} = 10' - 0.75'/12' = 118.5"$$

$$KL/R:$$

$K = 2.0$ for fixed joint @ 1. end.

$$L = 3000$$

$$R = \sqrt{\frac{D^2 + d^2}{16}} = \sqrt{\frac{120^2 + 118.5^2}{16}} = 42.16$$

$$KL/R = \frac{2(3000)}{42.16} = 142.31$$

Check for compact section:

$$D/t = 120/0.75 = 160$$

λ_p for compression: N/A

$$\lambda_c \text{ for compression: } 0.11 \frac{E}{F_y} = 0.11 \frac{29000}{36} = 88.61$$

$D/t = 160 > 88.61$, member is noncompact.

$$P_n = F_{cr} A_g$$

$$[KL/R = 142.31] \geq [4.71 \sqrt{\frac{E}{F_y}} = 4.71 \sqrt{\frac{29000}{36}} = 133.68]$$

$$\text{Use } F_{cr} = 0.877 F_e$$

$$F_e = \frac{\pi^2 E}{(KL/R)^2} = \frac{\pi^2 (29000)}{(142.31)^2} = 11.13$$

$$F_{cr} = 0.877(11.13) = 9.76 \text{ ksi}$$

$$A_g = \pi R^2 - \pi r^2 = \pi (120/2)^2 - \pi (118.5/2)^2 = 280.98 \text{ in}^2$$

$$P_n = (9.76)(280.98) = 2762.69 \text{ k}$$

$$\phi P_n = 0.85(2762.69 \text{ k}) = 2348.29 \text{ k} > 815.21 \text{ k} \quad \checkmark \text{ O.K.}$$

CHECK FLEXURAL STRENGTH:

Circular hollow cross-section will yield in strength + local buckling only; choose highest of 2 values.

1. Yield in strength:

$$\phi M_r \leq \phi M_n = \phi M_p = \phi F_y Z$$

$$Z = \frac{D^3 - d^3}{6} = \frac{120^3 - 118.5^3}{6} = 10665.56$$

$$F_y = 36 \text{ ksi}$$

$$\phi M_n = (0.9)(10665.56)(36) = 34,556.4 \text{ ft-kips}$$

2. Yield in Local Buckling

→ check for compact section: if section is compact, then tower will not yield in LB:

$$D/t = 100$$

$$\lambda_p \text{ for flexure} = 0.07 \frac{E}{F_y} = 0.07 \frac{(29000)}{36} = 56.39$$

$$\lambda_c \text{ for flexure} = 0.31 \frac{E}{F_y} = 0.31 \frac{(29000)}{36} = 249.72$$

$[\lambda_p = 56.39] < [D/t = 100] < [\lambda_c = 249.72]$, tower is non-compact (check)

$$M_n = \left[\frac{0.021 E}{(D/t) \cdot F_y} + 1 \right] F_y S = \left[\frac{0.021 (29000)}{(100)(36)} + 1 \right] (36)(8324)$$

$$M_n = 331,370 \text{ k}$$

$$S = \frac{\pi(D^4 - d^4)}{32D} = \frac{\pi((120)^4 - (118.5)^4)}{32(120)} = 8324 \text{ in}^3$$

$$\phi M_n = (0.9)(331,370) = 298,233 \text{ k}$$

Yield in strength

$$[M_n = 8827 \text{ ft-k}] < [\phi M_n = 34,556.4 \text{ k}] \checkmark \text{ OK}$$



CHECK FOR COMBINED AXIAL + FLEXURAL LOADS:

$$\left[\frac{P_u}{\phi P_n} = \frac{815.21 \text{ k}}{2960 \text{ k}} = 0.275 \right] \geq 0.2; \text{ use equ. H1-1a (Stability).}$$

$$\frac{P_u}{\phi P_n} + \frac{8}{9} B_2 \left[\frac{M_u}{\phi_b M_n} \right] \leq 1.0; \text{ where } B_2 \text{ is magnification factor for stability.}$$

$$P_u = 815.21 \text{ k}$$

$$\phi P_n = 2960 \text{ k}$$

$$B_2 = \frac{1}{1 - \frac{\alpha \sum P_{nt}}{\sum P_{e2}}}$$

$$\alpha = 1.00 \text{ for LRFD}$$

$$\sum P_{nt} = 815.21 \text{ k}$$

$$\sum P_{e2} = \sum R_M \frac{\sum H L}{\Delta_H} \text{ For lateral load resisting systems}$$

$$R_M = 0.85 \text{ for moment-frame + combined systems.}$$

$$\sum H = 65.71 \text{ k}$$

$$L = 3000 \text{ ''}$$

$$\Delta_H = 6 \text{ '' from ANSYS analysis.}$$

$$\sum P_{e2} = 0.85 \left[\frac{(65.71)(3000)}{6} \right] = 27927 \text{ k}$$

$$B_2 = \frac{1}{1 - \frac{815.21}{27927}} = 1.0301$$

$$M_u = 105932 \text{ in kips}$$

$$\phi M_n = 345564 \text{ in kips.}$$

$$\frac{(815.21)}{(2960)} + \frac{8}{9} (1.0301) \frac{(105932)}{(345564)} = 0.5561 \leq 1.0 \text{ O.K.}$$

Tower is oversized, so use excel spreadsheet to adjust.

FINAL DESIGN: 250' tower, 8' ϕ at base, 7' ϕ at hub, 0.75" thickness.

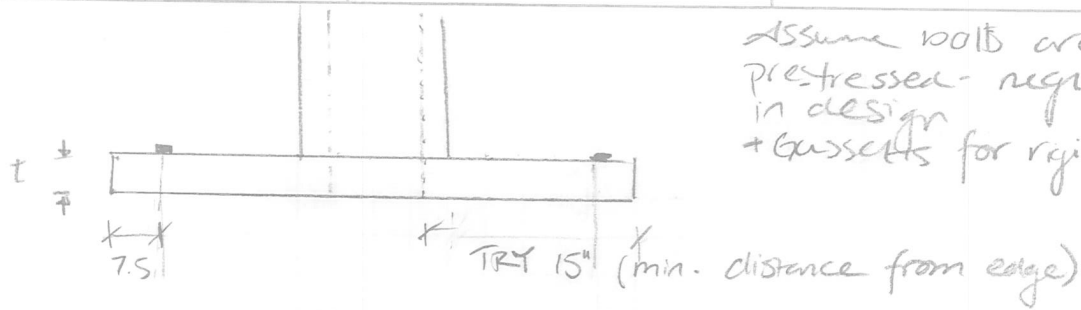
Resulting loads for baseplate + foundation design:

$$\text{Axial load: } 744.4 \text{ k}$$

$$\text{Moment load: } 6956.0 \text{ ft-k}$$

$$\text{Shear load: } 52.03 \text{ k}$$

DESIGN TOWER FLANGE



Assume bolts are prestressed - neglected in design + Gussetts for rigidity.

DESIGN FOR FLEXURAL STRENGTH:

$$\phi F_B = \phi_b F_y Z_x$$

$$Z_x = \frac{D^3 - d^3}{6} = \frac{126^3 - 96^3}{6} = 185940 \text{ in}^3$$

$$\phi F_B = 0.9(36)(185940) = 6024456 \text{ in-kips} > [M_u = 83472] \text{ OK}$$

CHECK SHEAR:

Yield strength:

$$\phi V = \phi(0.6 F_y A_g)$$

$$\phi = 0.9$$

$$F_y = 36 \text{ ksi}$$

$$A_g = \pi R^2 - \pi r^2 = \pi(126)^2 - \pi(96)^2 = 20923.0 \text{ in}^2$$

$$\phi V = 0.9(0.6)(36)(20923 \text{ in}^2) = 406743 \text{ k} > [V_u = 52.0 \text{ k}] \checkmark$$

Ultimate strength:

$$\phi V = \phi(0.6 F_u A_g)$$

$$\phi = 0.75$$

$$F_u = 80 \text{ ksi}$$

$$A_g = 20923 \text{ in}^2$$

$$\phi V = 0.75(0.6)(80)(20923) = 753228 \text{ k} > [V_u = 52.0 \text{ k}] \checkmark$$

DESIGN FLANGE THICKNESS:

$$t \geq \sqrt{\frac{6 M_u}{\phi_b F_y}}$$

$$M_u = 256 \text{ ft} \cdot \text{k} \text{ (from Baseplate design)}$$

$$\phi_b = 0.9$$

$$F_y = 36$$

$$t \geq \sqrt{\frac{6(256)}{0.9(36)}} = 6.88 \text{ in} \rightarrow \text{use } 7 \text{ in}$$

$$\text{O.R.} = 111 \text{ in} = 9.25 \text{ ft}$$

$$t = 7 \text{ in}$$

Weight of Equipment: 400 kips
 Height of tower: 250 ft
 Outer Diameter at base: 8 ft
 Outer Diameter at hub: 7 ft
 Thickness: 0.75 in
 Weight of Steel: 490 pcf
 Town (Massachusetts): Rutland

Axial Load 744.4273105
Moment Load 6956.037889
Total Shear 52.03

t_{ice} 0.083333333

Section	Diameter	Height	to	Inner Dia.	OR	IR	Ice Radius
D1	8	0	50	7.875	4	3.9375	4.25
D2	7.75	51	100	7.625	3.875	3.8125	4.125
D3	7.5	101	150	7.375	3.75	3.6875	4
D4	7.25	151	200	7.125	3.625	3.5625	3.875
D5	7	201	250	6.875	3.5	3.4375	3.75

Height	Volume of Steel	Weight of Steel	Volume of Ice	Weight of Ice	Fz	td	D1	D2	D3	D4	D5	Diameter	V(3-s)	V(f,m)	z	Kz calc	1skz	1skz2.58	q _s	Gh	Cf	Ar	Ag	e	Ae	Force	Moment
1	1.58582448	0.763676995	2.228225212	0.124780612	0.6577269	0.08769692	8					8	100	87	0.5	0.3020886	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	0.07889183
2	1.58582448	0.763676995	2.490105275	0.139445895	0.73410424	0.09788057	8					8	100	87	1.5	0.413476	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	0.23665748
3	1.58582448	0.763676995	2.622271265	0.146847191	0.77257848	0.10301046	8					8	100	87	2.5	0.478449	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	0.39445914
4	1.58582448	0.763676995	2.713184367	0.151938325	0.79901588	0.10653545	8					8	100	87	3.5	0.526729	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	0.55224228
5	1.58582448	0.763676995	2.783165184	0.15585725	0.81935075	0.10924677	8					8	100	87	4.5	0.565941	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	0.71002645
6	1.58582448	0.763676995	2.840354703	0.159059863	0.8359588	0.11146117	8					8	100	87	5.5	0.599337	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	0.86781011
7	1.58582448	0.763676995	2.888871096	0.161776781	0.85004113	0.11333882	8					8	100	87	6.5	0.628637	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.02593777
8	1.58582448	0.763676995	2.931098322	0.164415106	0.86229275	0.11497237	8					8	100	87	7.5	0.654872	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.18337742
9	1.58582448	0.763676995	2.968545048	0.166238523	0.87315333	0.11642044	8					8	100	87	8.5	0.678715	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.34116108
10	1.58582448	0.763676995	3.002228755	0.16812481	0.88291921	0.11772256	8					8	100	87	9.5	0.70063	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.49894474
11	1.58582448	0.763676995	3.032869301	0.169840681	0.89180014	0.11890669	8					8	100	87	10.5	0.720953	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.65672839
12	1.58582448	0.763676995	3.060995366	0.171435741	0.89995002	0.11999334	8					8	100	87	11.5	0.739938	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.81451205
13	1.58582448	0.763676995	3.087007058	0.172872395	0.90748532	0.12099804	8					8	100	87	12.5	0.757778	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	1.97229571
14	1.58582448	0.763676995	3.111214732	0.174228025	0.91449636	0.12193285	8					8	100	87	13.5	0.774625	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	2.13007936
15	1.58582448	0.763676995	3.133864143	0.175496392	0.92105466	0.12280729	8					8	100	87	14.5	0.790603	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	2.28786302
16	1.58582448	0.763676995	3.155153326	0.176688586	0.92721783	0.12362904	8					8	100	87	15.5	0.805812	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	2.44566438
17	1.58582448	0.763676995	3.175244228	0.17781368	0.93303299	0.1244044	8					8	100	87	16.5	0.820335	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	2.60434033
18	1.58582448	0.763676995	3.194271274	0.178879191	0.93853919	0.12513856	8					8	100	87	17.5	0.834243	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	2.76121399
19	1.58582448	0.763676995	3.212346875	0.179891425	0.94376915	0.12583589	8					8	100	87	18.5	0.847594	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	2.91897655
20	1.58582448	0.763676995	3.229566418	0.180855719	0.94875061	0.12650008	8					8	100	87	19.5	0.860349	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	3.07673187
21	1.58582448	0.763676995	3.246011378	0.181776637	0.95350723	0.12713348	8					8	100	87	20.5	0.872822	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	3.23456496
22	1.58582448	0.763676995	3.261751952	0.182658109	0.95805943	0.12774126	8					8	100	87	21.5	0.884781	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	3.39234822
23	1.58582448	0.763676995	3.276849062	0.183503547	0.96242492	0.12832322	8					8	100	87	22.5	0.896348	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	3.55013277
24	1.58582448	0.763676995	3.291355934	0.184315932	0.96661914	0.12888255	8					8	100	87	23.5	0.907554	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	3.70791593
25	1.58582448	0.763676995	3.30531935	0.185097884	0.97065571	0.12942076	8					8	100	87	24.5	0.918425	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	3.86569959
26	1.58582448	0.763676995	3.318780654	0.185851717	0.97454663	0.12993958	8					8	100	87	25.5	0.929892	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.02348324
27	1.58582448	0.763676995	3.331776573	0.186579488	0.97830257	0.13044034	8					8	100	87	26.5	0.939249	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.18126666
28	1.58582448	0.763676995	3.344339883	0.187283033	0.98193304	0.13092441	8					8	100	87	27.5	0.949242	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.33905056
29	1.58582448	0.763676995	3.35649996	0.187963998	0.98544659	0.13139288	8					8	100	87	28.5	0.958978	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.49683421
30	1.58582448	0.763676995	3.36828324	0.188623861	0.98885089	0.13184678	8					8	100	87	29.5	0.968474	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.65461787
31	1.58582448	0.763676995	3.379713597	0.189263961	0.99215286	0.13228705	8					8	100	87	30.5	0.977443	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.81240153
32	1.58582448	0.763676995	3.390812673	0.18988551	0.9953588	0.13271451	8					8	100	87	31.5	0.986797	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	4.97018518
33	1.58582448	0.763676995	3.401601145	0.190489608	0.99847442	0.13312992	8					8	100	87	32.5	0.995647	1	1	19.37664	1.69	0.59	8.166667	8.166667	1	8.166667	0.157784	5.13096541
34	1.58582448	0.763676995	3.412093959	0.191077262	1.00150492	0.13353399	8					8	100	87	33.5	1.004306	1.00430578	1.0043058	19.46007	1.69	0.59	8.166667	8.166667	1	8.166667	0.158463	5.33102325
35	1.58582448	0.763676995	3.42231053	0.19164939	1.00445507	0.13392734	8					8	100	87	34.5	1.012781	1.012781498	1.0127815	19.6243	1.69	0.59	8.166667	8.166667	1	8.166667	0.1598	5.535818
36	1.58582448	0.763676995	3.432264909	0.192206835	1.00732924	0.13431057	8					8	100	87	35.5	1.021084	1.021083506	1.0210835	19.78517	1.69	0.59	8.166667	8.166667	1	8.166667	0.16111	5.74231012
37	1.58582448	0.763676995	3.441970938	0.192750373	1.01013145	0.13468419	8					8	100	87	36.5	1.029212	1.029220112	1.0292201	19.94283	1.69	0.59	8.166667	8.166667	1	8.166667	0.162394	5.95046568
38	1.58582448	0.763676995	3.451441369	0.193280717	1.01286539	0.13504872	8					8	100	87	37.5	1.037199	1.03719901	1.037199	20.09743	1.69	0.59	8.166667	8.166667	1	8.166667	0.163653	6.16025208
39	1.58582448	0.763676995	3.460687983	0.193798527	1.01553449	0.13540466	8					8	100	87	38.5	1.045027	1.045027344	1.0450273	20.24912	1.69	0.59	8.166667	8.166667	1	8.166667	0.164888	6.37163825
40	1.58582448	0.763676995	3.469721683	0.194304414	1.01814191	0.13575225	8					8	100	87	39.5												

83	1.509437095	0.739624177	3.624906233	0.202994749	1.09595823	0.14612776	7.75			7.75	100	87	82.5	1.299263	1.299263232	1.2992632	25.17536	1.69	0.59	7.916667	7.916667	1	7.916667	0.198277	16.4232951
84	1.509437095	0.739624177	3.629357246	0.203244006	1.09727947	0.14630393	7.75			7.75	100	87	83.5	1.303743	1.303743449	1.3037435	25.26217	1.69	0.59	7.916667	7.916667	1	7.916667	0.199412	16.6793408
85	1.509437095	0.739624177	3.633760731	0.203490601	1.09858655	0.14647821	7.75			7.75	100	87	84.5	1.308186	1.308185595	1.3081856	25.34824	1.69	0.59	7.916667	7.916667	1	7.916667	0.200092	16.9362629
86	1.509437095	0.739624177	3.63811775	0.203734594	1.09987978	0.14665064	7.75			7.75	100	87	85.5	1.31259	1.312590307	1.3125903	25.43359	1.69	0.59	7.916667	7.916667	1	7.916667	0.200765	17.194054
87	1.509437095	0.739624177	3.642429329	0.203976042	1.10115947	0.14682126	7.75			7.75	100	87	86.5	1.316958	1.316958373	1.3169584	25.51823	1.69	0.59	7.916667	7.916667	1	7.916667	0.201433	17.4527009
88	1.509437095	0.739624177	3.646696459	0.204215002	1.10242591	0.14699012	7.75			7.75	100	87	87.5	1.321291	1.321290517	1.3212905	25.60217	1.69	0.59	7.916667	7.916667	1	7.916667	0.202096	17.7122145
89	1.509437095	0.739624177	3.650920098	0.204451526	1.10367939	0.14715725	7.75			7.75	100	87	88.5	1.325587	1.325587439	1.3255874	25.68543	1.69	0.59	7.916667	7.916667	1	7.916667	0.202753	17.9725698
90	1.509437095	0.739624177	3.655101175	0.204685666	1.10492019	0.14732269	7.75			7.75	100	87	89.5	1.32985	1.32984982	1.3298498	25.76802	1.69	0.59	7.916667	7.916667	1	7.916667	0.203405	18.2337659
91	1.509437095	0.739624177	3.659240585	0.204917473	1.10614857	0.14748648	7.75			7.75	100	87	90.5	1.334078	1.334078317	1.3340783	25.84996	1.69	0.59	7.916667	7.916667	1	7.916667	0.204052	18.4957961
92	1.509437095	0.739624177	3.6633392	0.205146995	1.1073648	0.14764864	7.75			7.75	100	87	91.5	1.338274	1.338273571	1.3382736	25.93125	1.69	0.59	7.916667	7.916667	1	7.916667	0.204694	18.7585339
93	1.509437095	0.739624177	3.667397859	0.20537428	1.10856913	0.14780922	7.75			7.75	100	87	92.5	1.342346	1.34246201	1.342462	26.0119	1.69	0.59	7.916667	7.916667	1	7.916667	0.20533	19.0223237
94	1.509437095	0.739624177	3.671417379	0.205599373	1.10976179	0.14796824	7.75			7.75	100	87	93.5	1.346567	1.346566811	1.3465668	26.09194	1.69	0.59	7.916667	7.916667	1	7.916667	0.205962	19.2868263
95	1.509437095	0.739624177	3.675398549	0.205822319	1.11094033	0.14812574	7.75			7.75	100	87	94.5	1.350666	1.350666894	1.3506666	26.17137	1.69	0.59	7.916667	7.916667	1	7.916667	0.206589	19.5521284
96	1.509437095	0.739624177	3.679342136	0.20604316	1.11211307	0.14828174	7.75			7.75	100	87	95.5	1.354734	1.35473429	1.3547343	26.2502	1.69	0.59	7.916667	7.916667	1	7.916667	0.207211	19.8182329
97	1.509437095	0.739624177	3.683248882	0.206261937	1.11327213	0.14843628	7.75			7.75	100	87	96.5	1.358772	1.358772279	1.3587723	26.32844	1.69	0.59	7.916667	7.916667	1	7.916667	0.207829	20.0851337
98	1.509437095	0.739624177	3.687119509	0.206478693	1.11442044	0.14858939	7.75			7.75	100	87	97.5	1.36278	1.36278049	1.3627805	26.40611	1.69	0.59	7.916667	7.916667	1	7.916667	0.208442	20.3528251
99	1.509437095	0.739624177	3.690954716	0.206693464	1.11555819	0.14874109	7.75			7.75	100	87	98.5	1.366759	1.366759443	1.3667594	26.48321	1.69	0.59	7.916667	7.916667	1	7.916667	0.209051	20.6213012
100	1.509437095	0.739624177	3.694755182	0.20690629	1.1166856	0.14889141	7.75			7.75	100	87	99.5	1.37071	1.370709645	1.3707096	26.55975	1.69	0.59	7.916667	7.916667	1	7.916667	0.209655	20.959816
101	1.46034971	0.715571358	3.581465526	0.200562069	1.11780285	0.14904038		7.5		7.5	100	87	100.5	1.374632	1.374631591	1.3746316	26.63574	1.69	0.59	7.666667	7.666667	1	7.666667	0.203615	20.9423557
102	1.46034971	0.715571358	3.585088213	0.200764621	1.11891014	0.14918802		7.5		7.5	100	87	101.5	1.378526	1.378525761	1.3785258	26.7112	1.69	0.59	7.666667	7.666667	1	7.666667	0.204192	20.746007
103	1.46034971	0.715571358	3.588667704	0.200965391	1.12000676	0.14933435		7.5		7.5	100	87	102.5	1.382393	1.382392622	1.3823926	26.78612	1.69	0.59	7.666667	7.666667	1	7.666667	0.204765	20.7117581
104	1.46034971	0.715571358	3.592221686	0.201164414	1.12109558	0.14947941		7.5		7.5	100	87	103.5	1.386233	1.386232629	1.3862326	26.86053	1.69	0.59	7.666667	7.666667	1	7.666667	0.205334	21.2813007
105	1.46034971	0.715571358	3.595745027	0.201361722	1.12217408	0.14962321		7.5		7.5	100	87	104.5	1.390046	1.390046226	1.3900462	26.93443	1.69	0.59	7.666667	7.666667	1	7.666667	0.205898	21.5457456
106	1.46034971	0.715571358	3.599238282	0.201557344	1.12324334	0.14976578		7.5		7.5	100	87	105.5	1.393834	1.393833844	1.3938338	27.00782	1.69	0.59	7.666667	7.666667	1	7.666667	0.206459	21.8109137
107	1.46034971	0.715571358	3.602701988	0.201751311	1.12430351	0.14990713		7.5		7.5	100	87	106.5	1.397596	1.397595905	1.3975959	27.08071	1.69	0.59	7.666667	7.666667	1	7.666667	0.207017	22.0768002
108	1.46034971	0.715571358	3.606136671	0.201943654	1.12535476	0.15004733		7.5		7.5	100	87	107.5	1.401333	1.401332817	1.4013328	27.15312	1.69	0.59	7.666667	7.666667	1	7.666667	0.207572	22.3430011
109	1.46034971	0.715571358	3.609542839	0.202134399	1.12639725	0.1501863		7.5		7.5	100	87	108.5	1.405045	1.405044981	1.405045	27.22505	1.69	0.59	7.666667	7.666667	1	7.666667	0.208125	22.6107089
110	1.46034971	0.715571358	3.612920989	0.202323575	1.12743112	0.15032415		7.5		7.5	100	87	109.5	1.408733	1.408732787	1.4087328	27.29651	1.69	0.59	7.666667	7.666667	1	7.666667	0.208666	22.8787218
111	1.46034971	0.715571358	3.616271605	0.20251121	1.12845653	0.15046087		7.5		7.5	100	87	110.5	1.412397	1.412396615	1.4123966	27.3675	1.69	0.59	7.666667	7.666667	1	7.666667	0.209209	23.1473442
112	1.46034971	0.715571358	3.619595158	0.202697329	1.12947362	0.15059648		7.5		7.5	100	87	111.5	1.416037	1.416036835	1.4160368	27.43804	1.69	0.59	7.666667	7.666667	1	7.666667	0.209748	23.4168416
113	1.46034971	0.715571358	3.622892105	0.202881958	1.13048254	0.15073101		7.5		7.5	100	87	112.5	1.419654	1.41965381	1.4196538	27.50812	1.69	0.59	7.666667	7.666667	1	7.666667	0.210284	23.6869395
114	1.46034971	0.715571358	3.626162893	0.203065122	1.13148341	0.15086446		7.5		7.5	100	87	113.5	1.423248	1.423247891	1.4232479	27.57726	1.69	0.59	7.666667	7.666667	1	7.666667	0.210816	23.9577236
115	1.46034971	0.715571358	3.629407959	0.203246846	1.13247639	0.15099685		7.5		7.5	100	87	114.5	1.426819	1.426819425	1.4268194	27.64697	1.69	0.59	7.666667	7.666667	1	7.666667	0.211345	24.2291896
116	1.46034971	0.715571358	3.632627725	0.203427153	1.13346158	0.15112821		7.5		7.5	100	87	115.5	1.430369	1.430368748	1.4303687	27.71574	1.69	0.59	7.666667	7.666667	1	7.666667	0.211871	24.5013331
117	1.46034971	0.715571358	3.635822607	0.203606066	1.13443913	0.15125855		7.5		7.5	100	87	116.5	1.433896	1.433896188	1.4338962	27.78409	1.69	0.59	7.666667	7.666667	1	7.666667	0.212394	24.77415
118	1.46034971	0.715571358	3.638993008	0.203783608	1.13540916	0.15138789		7.5		7.5	100	87	117.5	1.437402	1.437402066	1.4374021	27.85202	1.69	0.59	7.666667	7.666667	1	7.666667	0.212913	25.0476362
119	1.46034971	0.715571358	3.64213932	0.203959802	1.13637178	0.15151624		7.5		7.5	100	87	118.5	1.440887	1.440886866	1.4408869	27.91954	1.69	0.59	7.666667	7.666667	1	7.666667	0.213429	25.3218786
120	1.46034971	0.715571358	3.64526193	0.204134668	1.13732712	0.15164362		7.5		7.5	100	87	119.5	1.44435	1.444350385	1.4443504	27.98666	1.69	0.59	7.666667	7.666667	1	7.666667	0.213942	25.5966001
121	1.46034971	0.715571358	3.64836121	0.204308228	1.1382753	0.15177004		7.5		7.5	100	87	120.5	1.447793	1.447793432	1.4477934	28.05337	1.69	0.59	7.666667	7.666667	1	7.666667	0.214452	25.8720699
122	1.46034971	0.715571358	3.651437528	0.204480502	1.13921641	0.15189552		7.5		7.5	100	87	121.5	1.451216	1.451216129	1.4512161	28.11969	1.69	0.59	7.666667	7.666667	1	7.666667	0.214959	26.148193
123	1.46034971	0.715571358	3.654491239	0.204651509	1.14015059	0.15202008		7.5		7.5	100	87	122.5	1.454619	1.454618764	1.4546188	28.18562	1.69	0.59	7.666667	7.666667	1	7.666667	0.215463	

175	1.411262325	0.691518539	3.665119404	0.205246687	1.18121281	0.15749504					7.25	7.25	100	87	174.5	1.609355	1.609355004	1.609355	31.18389	1.69	0.59	7.416667	7.416667	1	7.416667	0.23061	40.2743225
176	1.411262325	0.691518539	3.66725893	0.2053665	1.18188799	0.15758506					7.25	7.25	100	87	175.5	1.611985	1.611984676	1.6119847	31.23485	1.69	0.59	7.416667	7.416667	1	7.416667	0.230987	40.5711173
177	1.411262325	0.691518539	3.669387562	0.205485703	1.18255971	0.15767463					7.25	7.25	100	87	176.5	1.614604	1.614603666	1.6146037	31.28559	1.69	0.59	7.416667	7.416667	1	7.416667	0.231362	40.8683952
178	1.411262325	0.691518539	3.671505418	0.205604303	1.18322801	0.15776373					7.25	7.25	100	87	177.5	1.617212	1.617212079	1.6172121	31.33614	1.69	0.59	7.416667	7.416667	1	7.416667	0.231736	41.16651546
179	1.411262325	0.691518539	3.673612612	0.205722306	1.18389293	0.15785239					7.25	7.25	100	87	178.5	1.61981	1.619810016	1.61981	31.38648	1.69	0.59	7.416667	7.416667	1	7.416667	0.232108	41.4643932
180	1.411262325	0.691518539	3.675709257	0.205839718	1.18455451	0.15794606					7.25	7.25	100	87	179.5	1.622398	1.622397578	1.6223976	31.43661	1.69	0.59	7.416667	7.416667	1	7.416667	0.232479	41.7631094
181	1.411262325	0.691518539	3.677795464	0.205956546	1.18521278	0.15802837					7.25	7.25	100	87	180.5	1.624975	1.624974864	1.6249749	31.48655	1.69	0.59	7.416667	7.416667	1	7.416667	0.232848	42.0623011
182	1.411262325	0.691518539	3.679871343	0.206072795	1.18586778	0.1581157					7.25	7.25	100	87	181.5	1.627542	1.627541971	1.627542	31.53629	1.69	0.59	7.416667	7.416667	1	7.416667	0.233216	42.3619665
183	1.411262325	0.691518539	3.681937002	0.206188472	1.18651954	0.1582026					7.25	7.25	100	87	182.5	1.630099	1.630098995	1.630099	31.58584	1.69	0.59	7.416667	7.416667	1	7.416667	0.233582	42.6210336
184	1.411262325	0.691518539	3.683992545	0.206303583	1.18716809	0.15828908					7.25	7.25	100	87	183.5	1.632646	1.632646031	1.632646	31.63519	1.69	0.59	7.416667	7.416667	1	7.416667	0.233947	42.9627108
185	1.411262325	0.691518539	3.686038079	0.206418132	1.18781346	0.15837513					7.25	7.25	100	87	184.5	1.635183	1.635183171	1.6351832	31.68436	1.69	0.59	7.416667	7.416667	1	7.416667	0.234311	43.2637861
186	1.411262325	0.691518539	3.688073704	0.206532127	1.1884557	0.15846076					7.25	7.25	100	87	185.5	1.637711	1.637710507	1.6377105	31.73333	1.69	0.59	7.416667	7.416667	1	7.416667	0.234673	43.5653277
187	1.411262325	0.691518539	3.690099523	0.206645573	1.18909483	0.15854598					7.25	7.25	100	87	186.5	1.640228	1.640228131	1.6402281	31.78211	1.69	0.59	7.416667	7.416667	1	7.416667	0.235034	43.8673338
188	1.411262325	0.691518539	3.692115634	0.206758476	1.18973088	0.15863078					7.25	7.25	100	87	187.5	1.642736	1.642736131	1.6427361	31.83071	1.69	0.59	7.416667	7.416667	1	7.416667	0.235393	44.1692708
189	1.411262325	0.691518539	3.694122136	0.20687084	1.19036388	0.15871518					7.25	7.25	100	87	188.5	1.645235	1.645234594	1.6452346	31.87912	1.69	0.59	7.416667	7.416667	1	7.416667	0.235751	44.4727325
190	1.411262325	0.691518539	3.696119126	0.206982671	1.19099387	0.15879918					7.25	7.25	100	87	189.5	1.647724	1.647723608	1.6477236	31.92735	1.69	0.59	7.416667	7.416667	1	7.416667	0.236108	44.7761216
191	1.411262325	0.691518539	3.698106697	0.207093975	1.19162088	0.15888278					7.25	7.25	100	87	190.5	1.650203	1.650203257	1.6502033	31.97539	1.69	0.59	7.416667	7.416667	1	7.416667	0.236463	45.0799682
192	1.411262325	0.691518539	3.700084944	0.207204757	1.19224493	0.15896599					7.25	7.25	100	87	191.5	1.652674	1.652673627	1.6526736	32.02326	1.69	0.59	7.416667	7.416667	1	7.416667	0.236817	45.3842706
193	1.411262325	0.691518539	3.702053958	0.207315022	1.19286605	0.15904881					7.25	7.25	100	87	192.5	1.655135	1.655134799	1.6551348	32.07095	1.69	0.59	7.416667	7.416667	1	7.416667	0.23717	45.6890271
194	1.411262325	0.691518539	3.704013831	0.207424775	1.19348428	0.15913124					7.25	7.25	100	87	193.5	1.657587	1.657586855	1.6575869	32.11846	1.69	0.59	7.416667	7.416667	1	7.416667	0.237521	45.994236
195	1.411262325	0.691518539	3.705964652	0.207534021	1.19409964	0.15921329					7.25	7.25	100	87	194.5	1.66003	1.660029877	1.6600299	32.1658	1.69	0.59	7.416667	7.416667	1	7.416667	0.237871	46.2998956
196	1.411262325	0.691518539	3.707906590	0.207642764	1.19471215	0.15929425					7.25	7.25	100	87	195.5	1.662464	1.662463943	1.6624639	32.21297	1.69	0.59	7.416667	7.416667	1	7.416667	0.23822	46.600043
197	1.411262325	0.691518539	3.709839488	0.207751011	1.19532186	0.15937625					7.25	7.25	100	87	196.5	1.664889	1.664889133	1.6648891	32.25996	1.69	0.59	7.416667	7.416667	1	7.416667	0.238567	46.9125604
198	1.411262325	0.691518539	3.71176375	0.207858766	1.19592878	0.15945717					7.25	7.25	100	87	197.5	1.667306	1.667305522	1.6673055	32.30678	1.69	0.59	7.416667	7.416667	1	7.416667	0.238914	47.2195624
199	1.411262325	0.691518539	3.713679155	0.207966033	1.19653293	0.15953722					7.25	7.25	100	87	198.5	1.669713	1.669713188	1.6697132	32.35343	1.69	0.59	7.416667	7.416667	1	7.416667	0.239259	47.5270085
200	1.411262325	0.691518539	3.715586001	0.208072817	1.19713436	0.15961791					7.25	7.25	100	87	199.5	1.672112	1.672112206	1.6721122	32.39992	1.69	0.59	7.416667	7.416667	1	7.416667	0.239603	47.8270417
201	1.36217494	0.66746572	3.592058007	0.201155248	1.19773308	0.15969774					7	7	100	87	200.5	1.674503	1.67450265	1.6745026	32.44624	1.69	0.59	7.166667	7.166667	1	7.166667	0.231857	46.5204216
202	1.36217494	0.66746572	3.59388544	0.201257585	1.19832911	0.15977722					7	7	100	87	201.5	1.676885	1.676884593	1.6768846	32.49239	1.69	0.59	7.166667	7.166667	1	7.166667	0.232187	46.8187829
203	1.36217494	0.66746572	3.595704769	0.201359467	1.19892249	0.15985633					7	7	100	87	202.5	1.679258	1.679258107	1.6792581	32.53838	1.69	0.59	7.166667	7.166667	1	7.166667	0.232515	47.1175672
204	1.36217494	0.66746572	3.597516068	0.2014609	1.19951324	0.15993551					7	7	100	87	203.5	1.681623	1.681623264	1.6816233	32.58421	1.69	0.59	7.166667	7.166667	1	7.166667	0.232843	47.4167732
205	1.36217494	0.66746572	3.599319413	0.201561887	1.20010139	0.16001352					7	7	100	87	204.5	1.68398	1.683980133	1.6839801	32.62988	1.69	0.59	7.166667	7.166667	1	7.166667	0.233169	47.7163994
206	1.36217494	0.66746572	3.601114878	0.201662433	1.20068695	0.16009159					7	7	100	87	205.5	1.686329	1.686328785	1.6863288	32.67539	1.69	0.59	7.166667	7.166667	1	7.166667	0.233494	48.0164442
207	1.36217494	0.66746572	3.602902534	0.201762542	1.20126995	0.16016933					7	7	100	87	206.5	1.688669	1.688669287	1.6886693	32.72074	1.69	0.59	7.166667	7.166667	1	7.166667	0.233819	48.3169063
208	1.36217494	0.66746572	3.604682453	0.201862217	1.20185401	0.16024672					7	7	100	87	207.5	1.691002	1.691001707	1.6910017	32.76593	1.69	0.59	7.166667	7.166667	1	7.166667	0.234142	48.6177841
209	1.36217494	0.66746572	3.606454707	0.201961464	1.20242836	0.16032378					7	7	100	87	208.5	1.693326	1.693326112	1.6933261	32.81097	1.69	0.59	7.166667	7.166667	1	7.166667	0.234463	48.9190763
210	1.36217494	0.66746572	3.608219364	0.202060284	1.20300383	0.16040051					7	7	100	87	209.5	1.695643	1.695642568	1.6956426	32.85586	1.69	0.59	7.166667	7.166667	1	7.166667	0.234784	49.2207815
211	1.36217494	0.66746572	3.609976493	0.202158684	1.20357682	0.16047691					7	7	100	87	210.5	1.697951	1.697951139	1.6979511	32.90059	1.69	0.59	7.166667	7.166667	1	7.166667	0.235104	49.5228981
212	1.36217494	0.66746572	3.611726162	0.202256665	1.20414737	0.16055298					7	7	100	87	211.5	1.700252	1.70025189	1.7002519	32.94517	1.69	0.59	7.166667	7.166667	1	7.166667	0.235422	49.8254249
213	1.36217494	0.66746572	3.613468438	0.202354233	1.2047155	0.16062873					7	7	100	87	212.5	1.702545	1.702544883	1.7025449	32.9896	1.69	0.59	7.166667	7.166667	1	7.166667	0.23574	50.1283604
214	1.36217494	0.66746572	3.615203385	0.20245139	1.20528123	0.16070416					7	7	100	87	213.5	1.70483	1.704830182	1.7048302	33.03388	1.69	0.59	7.166667	7.166667	1	7.166667		

ANSYS Analysis for Deflection

PRINT U NODAL SOLUTION PER NODE

***** POST1 NODAL DEGREE OF FREEDOM LISTING *****

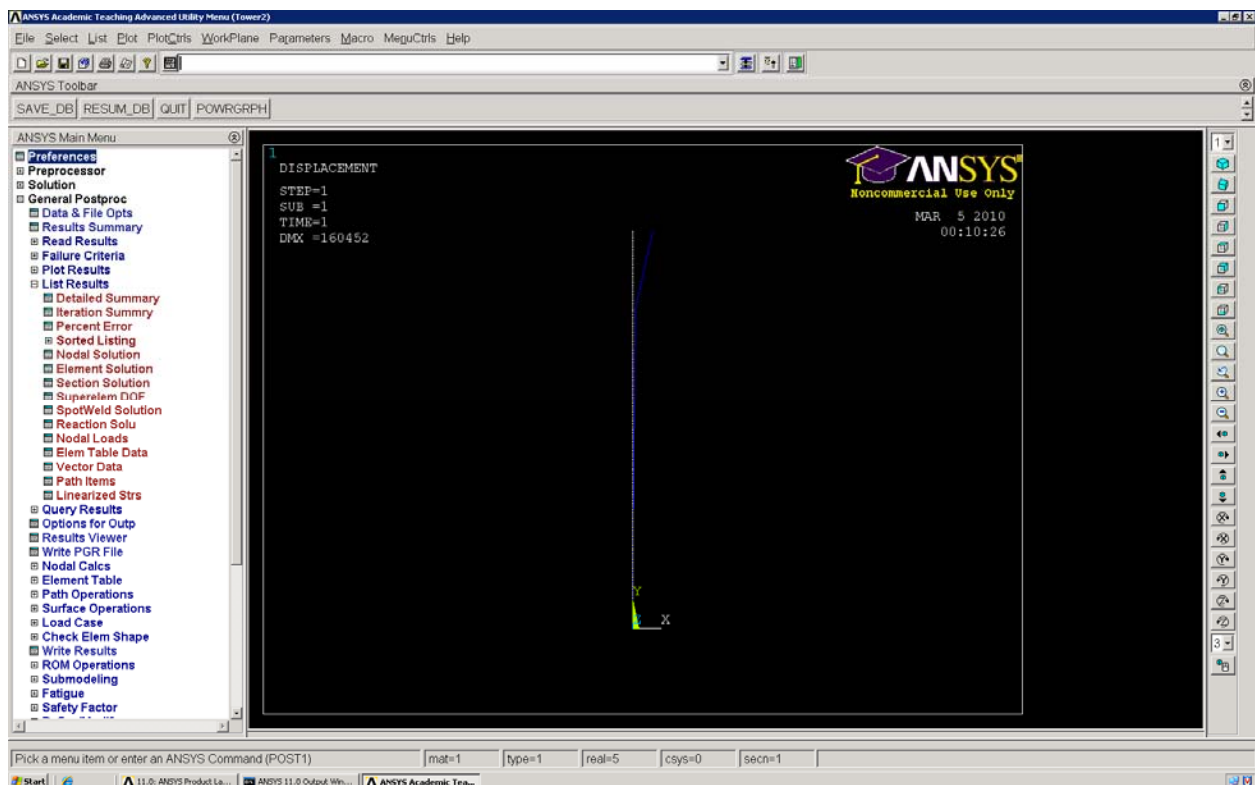
LOAD STEP= 1 SUBSTEP= 1
TIME= 1.0000 LOAD CASE= 0

THE FOLLOWING DEGREE OF FREEDOM RESULTS ARE IN THE GLOBAL COORDINATE SYSTEM

NODE	UX
1	0.0000
2	-1.56623
3	-2.1016
4	-2.6027
5	-3.0657
6	-5.4877

MAXIMUM ABSOLUTE VALUES

NODE 6
VALUE -5.4877



User Input				Axial Strength		Flexure Strength		Combined Loads				
Overall height	250	ft =	3000	in	Factored Load	744.4273105	Factored Load	83472.45	Combined Loading	0.841748	≤1.0	OK
Diameter at Base	8	ft =	96	in	Factored Strength	1508.537309	Factored Strength	220467.83	Pu/Pc	0.493476		
Shell Thickness	0.75	in			Pn	1774.749775	Yielding Mn	244964.25	For Pu/Pc≥0.2	0.841748		
Strength of Steel	36	ksi			Fcr	7.907897098	Local Buckling	216128.8075	For Pu/Pc<0.2	0.59501		
Modulus of Elasticity	29000	ksi			KL/r	178.1631148	Check for Compact section:		Magnification Factors			
					4.71*sqrt(E/Fy)	133.680683	D/t	128	B2	1.03484		
ID	94.5				Fe	9.016986429	λp	56.38888889	Pnt	744.4273		
S	5302.757785				Fcr for KL/r≤4.71*sqrt(E/Fy)	6.769799368	λc	249.7222222	Pe2	22111.49		
Z	6804.5625				Fcr for KL/r>4.71*sqrt(E/Fy)	7.907897098	non-compact		ΣH	52.03		
r	33.67700439				Ag	224.4275252	Flexure Strength:	OK	ΔH	6		
l	254532.3737				Check for Compact section:				L	3000		
					D/t	128						
					λp							
					λc	88.61111111						
					noncompact							
					Axial Strength:	OK						

Appendix D: Tower Design Alternatives

Weight of Equipment: 400 kips
 Height of tower: 250 ft
 Outer Diameter at base: 8.25 ft
 Outer Diameter at hub: 7.5 ft
 Thickness: 0.75 in
 Weight of Steel: 490 pcf
 Town (Massachusetts): Belmont

Axial Load: 757.7449302
 Moment Load: 7878.008784
 Total Shear: 58.54

Section	Diameter	Height	t	Inner Dia.	OR	IR	Ice Radius
D1	8.25	0	50	8.125	4.125	4.0625	4.375
D2	8.0625	51	100	7.975	4.03125	3.96875	4.28125
D3	7.875	101	150	7.735	3.9375	3.875	4.1875
D4	7.6875	151	200	7.5625	3.84375	3.78125	4.09375
D5	7.5	201	250	7.375	3.75	3.6875	4

t_{ice} 0.083333333

Height	Volume of Steel	Weight of Steel	Volume of Ice	Weight of Ice	Fz	td	D1	D2	D3	D4	D5	Diameter	V(3-s)	V(f,m)	z	Kz calc	1skz	1skzs2.58	q _z	Gh	Cf	Ar	Ag	e	Ae	Force	Moment	
1	1.607611866	0.787729814	2.297102212	0.128637724	0.6577269	0.08769692	8.25	8.25	105	90	0.5	0.3020886	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	0.08701093					
2	1.607611866	0.787729814	2.566980491	0.143750908	0.73410424	0.09788057	8.25	8.25	105	90	1.5	0.413476	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	0.2610328					
3	1.607611866	0.787729814	2.703175494	0.151377828	0.77257848	0.10301046	8.25	8.25	105	90	2.5	0.478449	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	0.4350567					
4	1.607611866	0.787729814	2.796857114	0.156623998	0.79901588	0.10653545	8.25	8.25	105	90	3.5	0.526729	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	0.60907654					
5	1.607611866	0.787729814	2.868967395	0.160662174	0.81935075	0.10924677	8.25	8.25	105	90	4.5	0.565941	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	0.78309841					
6	1.607611866	0.787729814	2.927896104	0.163962182	0.8359588	0.1146117	8.25	8.25	105	90	5.5	0.599337	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	0.95712028					
7	1.607611866	0.787729814	2.97787196	0.166761683	0.85004113	0.11333882	8.25	8.25	105	90	6.5	0.628637	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	1.13112415					
8	1.607611866	0.787729814	3.021397408	0.169198255	0.86229275	0.11497237	8.25	8.25	105	90	7.5	0.654872	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	1.30516402					
9	1.607611866	0.787729814	3.059981447	0.171358961	0.8731533	0.11642044	8.25	8.25	105	90	8.5	0.678715	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	1.47918588					
10	1.607611866	0.787729814	3.094687838	0.173302519	0.88291921	0.11772256	8.25	8.25	105	90	9.5	0.70063	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	1.65320775					
11	1.607611866	0.787729814	3.126258393	0.17507047	0.89180014	0.11890669	8.25	8.25	105	90	10.5	0.720953	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	1.82722962					
12	1.607611866	0.787729814	3.155237912	0.176693233	0.89995002	0.11999334	8.25	8.25	105	90	11.5	0.739938	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	2.00125149					
13	1.607611866	0.787729814	3.182038698	0.178194167	0.90748532	0.12099804	8.25	8.25	105	90	12.5	0.757778	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	2.17527336					
14	1.607611866	0.787729814	3.206980567	0.179590912	0.91449636	0.12193285	8.25	8.25	105	90	13.5	0.774625	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	2.34929523					
15	1.607611866	0.787729814	3.230316762	0.180897739	0.92105466	0.12280729	8.25	8.25	105	90	14.5	0.790603	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	2.5233171					
16	1.607611866	0.787729814	3.252251335	0.182126076	0.92721783	0.12362904	8.25	8.25	105	90	15.5	0.805812	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	2.69733897					
17	1.607611866	0.787729814	3.272951267	0.183285217	0.93303299	0.1244044	8.25	8.25	105	90	16.5	0.820335	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	2.87136084					
18	1.607611866	0.787729814	3.292554868	0.184383073	0.93853919	0.12513856	8.25	8.25	105	90	17.5	0.834243	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	3.0453827					
19	1.607611866	0.787729814	3.31117815	0.185425976	0.94376915	0.12583589	8.25	8.25	105	90	18.5	0.847594	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	3.21940457					
20	1.607611866	0.787729814	3.328919349	0.186419484	0.94875061	0.12650088	8.25	8.25	105	90	19.5	0.860349	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	3.39342444					
21	1.607611866	0.787729814	3.345862422	0.187368296	0.95350723	0.1271343	8.25	8.25	105	90	20.5	0.872822	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	3.56744831					
22	1.607611866	0.787729814	3.362079701	0.188276463	0.95805943	0.12774126	8.25	8.25	105	90	21.5	0.884781	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	3.74147018					
23	1.607611866	0.787729814	3.377633964	0.189147502	0.96242492	0.12832332	8.25	8.25	105	90	22.5	0.896348	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	3.91549205					
24	1.607611866	0.787729814	3.392580054	0.18984483	0.96661914	0.12888255	8.25	8.25	105	90	23.5	0.907554	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	4.08951392					
25	1.607611866	0.787729814	3.406966178	0.190790106	0.97065571	0.12942076	8.25	8.25	105	90	24.5	0.918425	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	4.26353579					
26	1.607611866	0.787729814	3.420834938	0.191566757	0.97454663	0.12993982	8.25	8.25	105	90	25.5	0.929892	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	4.43755765					
27	1.607611866	0.787729814	3.434224179	0.192316554	0.97830257	0.13040434	8.25	8.25	105	90	26.5	0.939249	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	4.61157952					
28	1.607611866	0.787729814	3.447167671	0.19304139	0.98193304	0.13092441	8.25	8.25	105	90	27.5	0.949242	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	4.78560319					
29	1.607611866	0.787729814	3.459695686	0.193742958	0.98544659	0.13139288	8.25	8.25	105	90	28.5	0.958789	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	4.95962326					
30	1.607611866	0.787729814	3.471835462	0.194422786	0.98885089	0.13184678	8.25	8.25	105	90	29.5	0.968474	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	5.13364513					
31	1.607611866	0.787729814	3.483611602	0.19508225	0.99215286	0.13227075	8.25	8.25	105	90	30.5	0.977443	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	5.307667					
32	1.607611866	0.787729814	3.495046403	0.195722599	0.9953588	0.13271451	8.25	8.25	105	90	31.5	0.986797	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	5.48168887					
33	1.607611866	0.787729814	3.506161042	0.196344968	0.99847442	0.13312992	8.25	8.25	105	90	32.5	0.995647	1	1	20.736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	5.65679493					
34	1.607611866	0.787729814	3.516971309	0.196950393	1.00150492	0.13353299	8.25	8.25	105	90	33.5	1.004306	1.00430578	1.0043058	20.82528	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	5.83174771					
35	1.607611866	0.787729814	3.527496819	0.197539822	1.00445507	0.13392734	8.25	8.25	105	90	34.5	1.012781	1.012781498	1.0127815	21.00104	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	6.00672659					
36	1.607611866	0.787729814	3.5375752181	0.198114122	1.00732924	0.13431057	8.25	8.25	105	90	35.5	1.021084	1.021083506	1.0210835	21.17319	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	6.18227659					
37	1.607611866	0.787729814	3.547751656	0.198674093	1.01013145	0.13468419	8.25	8.25	105	90	36.5	1.029212	1.029220112	1.0292201	21.34191	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	6.35828435					
38	1.607611866	0.787729814	3.557508385	0.199220047	1.01286539	0.13504872	8.25	8.25	105	90	37.5	1.037199	1.03719901	1.037199	21.50736	1.69	0.59	8.416667	8.416667	1	8.416667	0.174022	6.53432063					
39	1.607611866																											

83	1.570796327	0.7696902	3.76836683	0.211028542	1.09595823	0.14612776	8.063				0.8625	105	90	82.5	1.299263	1.299263223	1.2992632	26.94152	1.69	0.59	8.229167	8.229167	1	8.229167	0.212063	18.2692328
84	1.570796327	0.7696902	3.77290792	0.211287484	1.09727947	0.14630393	8.063				0.8625	105	90	83.5	1.303743	1.303743449	1.3037435	27.03443	1.69	0.59	8.229167	8.229167	1	8.229167	0.212186	18.5540573
85	1.570796327	0.7696902	3.77756374	0.211543661	1.09858565	0.14647821	8.063				0.8625	105	90	84.5	1.308186	1.308185599	1.3081856	27.12654	1.69	0.59	8.229167	8.229167	1	8.229167	0.222581	18.8398568
86	1.570796327	0.7696902	3.782091677	0.211797134	1.09987978	0.14665064	8.063				0.8625	105	90	85.5	1.31259	1.312590307	1.3125903	27.21787	1.69	0.59	8.229167	8.229167	1	8.229167	0.223331	19.1266231
87	1.570796327	0.7696902	3.786570766	0.212047963	1.10115947	0.14682126	8.063				0.8625	105	90	86.5	1.316958	1.316958373	1.3169584	27.30845	1.69	0.59	8.229167	8.229167	1	8.229167	0.224074	19.4143479
88	1.570796327	0.7696902	3.791003673	0.212296206	1.10242591	0.14699012	8.063				0.8625	105	90	87.5	1.321291	1.321290517	1.3212905	27.39828	1.69	0.59	8.229167	8.229167	1	8.229167	0.224811	19.7030235
89	1.570796327	0.7696902	3.795391393	0.212541918	1.10367939	0.14715725	8.063				0.8625	105	90	88.5	1.325587	1.325587439	1.3255874	27.48738	1.69	0.59	8.229167	8.229167	1	8.229167	0.225542	19.9926421
90	1.570796327	0.7696902	3.799734889	0.212785154	1.10492019	0.14732269	8.063				0.8625	105	90	89.5	1.32985	1.32984982	1.3298498	27.57577	1.69	0.59	8.229167	8.229167	1	8.229167	0.226267	20.283196
91	1.570796327	0.7696902	3.804035095	0.213025965	1.10614857	0.14748648	8.063				0.8625	105	90	90.5	1.334078	1.334078317	1.3340783	27.66345	1.69	0.59	8.229167	8.229167	1	8.229167	0.226987	20.5746778
92	1.570796327	0.7696902	3.808292913	0.213264403	1.1073648	0.14764864	8.063				0.8625	105	90	91.5	1.338274	1.338273571	1.3382736	27.75044	1.69	0.59	8.229167	8.229167	1	8.229167	0.227701	20.8670801
93	1.570796327	0.7696902	3.812509218	0.213500516	1.10856913	0.14780922	8.063				0.8625	105	90	92.5	1.342346	1.342346201	1.3423462	27.83676	1.69	0.59	8.229167	8.229167	1	8.229167	0.228409	21.1603958
94	1.570796327	0.7696902	3.816684857	0.213734352	1.10976179	0.14796824	8.063				0.8625	105	90	93.5	1.346567	1.346566811	1.3465668	27.92241	1.69	0.59	8.229167	8.229167	1	8.229167	0.229112	21.4546178
95	1.570796327	0.7696902	3.820820651	0.213965956	1.11094033	0.14812574	8.063				0.8625	105	90	94.5	1.350666	1.350665984	1.350666	28.00741	1.69	0.59	8.229167	8.229167	1	8.229167	0.229809	21.7497392
96	1.570796327	0.7696902	3.824917396	0.214195374	1.11211307	0.14828174	8.063				0.8625	105	90	95.5	1.354734	1.354734229	1.3547343	28.09177	1.69	0.59	8.229167	8.229167	1	8.229167	0.230501	22.0457531
97	1.570796327	0.7696902	3.828975864	0.214422268	1.11327213	0.14843628	8.063				0.8625	105	90	96.5	1.358772	1.358772079	1.3587723	28.1755	1.69	0.59	8.229167	8.229167	1	8.229167	0.231189	22.342653
98	1.570796327	0.7696902	3.832996803	0.214647821	1.11442044	0.14858939	8.063				0.8625	105	90	97.5	1.36278	1.362780449	1.3627805	28.25862	1.69	0.59	8.229167	8.229167	1	8.229167	0.23187	22.6404322
99	1.570796327	0.7696902	3.836980942	0.214870933	1.11555819	0.14874109	8.063				0.8625	105	90	98.5	1.366759	1.366759443	1.3667594	28.34112	1.69	0.59	8.229167	8.229167	1	8.229167	0.232547	22.9398043
100	1.570796327	0.7696902	3.840928985	0.215092023	1.1166856	0.14889141	8.063				0.8625	105	90	99.5	1.37071	1.370709645	1.3707096	28.42304	1.69	0.59	8.229167	8.229167	1	8.229167	0.23322	22.9730738
101	1.533980788	0.751650586	3.757049587	0.210394777	1.11780285	0.14904038		7.875			7.875	105	90	100.5	1.374632	1.374631591	1.3746316	28.50364	1.69	0.59	8.041667	8.041667	1	8.041667	0.228558	20.026508
102	1.533980788	0.751650586	3.760840507	0.210607068	1.11891014	0.14918802		7.875			7.875	105	90	101.5	1.378526	1.378525761	1.3785258	28.58511	1.69	0.59	8.041667	8.041667	1	8.041667	0.229205	20.2970205
103	1.533980788	0.751650586	3.764598066	0.210817493	1.12000766	0.14933435		7.875			7.875	105	90	102.5	1.382393	1.382392622	1.3823926	28.66529	1.69	0.59	8.041667	8.041667	1	8.041667	0.229848	20.522191
104	1.533980788	0.751650586	3.76832968	0.211026086	1.12109558	0.14947941		7.875			7.875	105	90	103.5	1.386233	1.386232629	1.3862326	28.74492	1.69	0.59	8.041667	8.041667	1	8.041667	0.230487	20.882408
105	1.533980788	0.751650586	3.77201572	0.21123288	1.12217408	0.14962321		7.875			7.875	105	90	104.5	1.390046	1.390046229	1.3900462	28.824	1.69	0.59	8.041667	8.041667	1	8.041667	0.231121	21.1850799
106	1.533980788	0.751650586	3.775676933	0.211437908	1.12324334	0.14976578		7.875			7.875	105	90	105.5	1.393834	1.393833844	1.3938338	28.90254	1.69	0.59	8.041667	8.041667	1	8.041667	0.231751	21.4827309
107	1.533980788	0.751650586	3.779307171	0.211641202	1.12430351	0.14990713		7.875			7.875	105	90	106.5	1.397596	1.397595959	1.3975959	28.98055	1.69	0.59	8.041667	8.041667	1	8.041667	0.232376	21.7811882
108	1.533980788	0.751650586	3.782906993	0.211842791	1.12535476	0.1500473		7.875			7.875	105	90	107.5	1.401333	1.401332811	1.4013328	29.05804	1.69	0.59	8.041667	8.041667	1	8.041667	0.232997	21.9500446
109	1.533980788	0.751650586	3.786476904	0.212042707	1.12639725	0.1501863		7.875			7.875	105	90	108.5	1.405045	1.405044981	1.405045	29.13501	1.69	0.59	8.041667	8.041667	1	8.041667	0.233615	22.3805003
110	1.533980788	0.751650586	3.790017455	0.212240977	1.12743112	0.15032415		7.875			7.875	105	90	109.5	1.408733	1.408732787	1.4087328	29.21148	1.69	0.59	8.041667	8.041667	1	8.041667	0.234228	25.6813445
111	1.533980788	0.751650586	3.793529142	0.212437632	1.12845653	0.15046087		7.875			7.875	105	90	110.5	1.412397	1.412396615	1.4123966	29.28746	1.69	0.59	8.041667	8.041667	1	8.041667	0.234837	25.9827399
112	1.533980788	0.751650586	3.7971012459	0.212632698	1.12947362	0.15059648		7.875			7.875	105	90	111.5	1.416037	1.416036835	1.4160368	29.36294	1.69	0.59	8.041667	8.041667	1	8.041667	0.235442	26.2853835
113	1.533980788	0.751650586	3.800467886	0.212826202	1.13048254	0.15073101		7.875			7.875	105	90	112.5	1.419654	1.419653811	1.4196538	29.43794	1.69	0.59	8.041667	8.041667	1	8.041667	0.236044	26.5885682
114	1.533980788	0.751650586	3.803895892	0.21301817	1.13148341	0.15086446		7.875			7.875	105	90	113.5	1.423248	1.423247891	1.4232479	29.51247	1.69	0.59	8.041667	8.041667	1	8.041667	0.236641	26.8925231
115	1.533980788	0.751650586	3.807296323	0.213208628	1.13247639	0.15099685		7.875			7.875	105	90	114.5	1.426819	1.426819425	1.4268194	29.58653	1.69	0.59	8.041667	8.041667	1	8.041667	0.237235	27.1972434
116	1.533980788	0.751650586	3.810671454	0.213397601	1.13346158	0.15112821		7.875			7.875	105	90	115.5	1.430369	1.430368748	1.4303687	29.66013	1.69	0.59	8.041667	8.041667	1	8.041667	0.237825	27.5027243
117	1.533980788	0.751650586	3.814019889	0.213585114	1.13443913	0.15125855		7.875			7.875	105	90	116.5	1.433896	1.433896188	1.4338962	29.73327	1.69	0.59	8.041667	8.041667	1	8.041667	0.238412	27.808961
118	1.533980788	0.751650586	3.817342661	0.213771189	1.13540916	0.15138789		7.875			7.875	105	90	117.5	1.437402	1.437402066	1.4374021	29.80597	1.69	0.59	8.041667	8.041667	1	8.041667	0.238995	28.1159499
119	1.533980788	0.751650586	3.820640183	0.21395585	1.13637178	0.15151624		7.875			7.875	105	90	118.5	1.440887	1.440886666	1.4408867	29.87823	1.69	0.59	8.041667	8.041667	1	8.041667	0.239574	28.4236836
120	1.533980788	0.751650586	3.823912857	0.21413912	1.13732712	0.15164362		7.875			7.875	105	90	119.5	1.444435	1.444350385	1.4443504	29.95005	1.69	0.59	8.041667	8.041667	1	8.041667	0.24015	28.7321605
121	1.533980788	0.751650586	3.827161076	0.21432102	1.1382753	0.15177004		7.875			7.875	105	90	120.5	1.447793	1.447793342	1.4477934	30.02144	1.69	0.59	8.041667	8.041667	1	8.041667	0.240722	29.0431751
122	1.533980788	0.751650586	3.830385223	0.214501573	1.13921641	0.15189552		7.875			7.875	105	90	121.5	1.451216	1.451216129	1.4512161	30.09242	1.69	0.59	8.041667	8.041667	1	8.041667	0.241291	29.351323

User Input				Axial Strength		Flexure Strength		Combined Loads				
Overall height	250	ft =	3000	in	Factored Load	757.7449302	Factored Load	94536.11	Combined Loading	0.827171	≤1.0	OK
Diameter at Base	8.25	ft=	99	in	Factored Strength	1655.606644	Factored Strength	234573.98	Pu/Pc	0.457684		
Shell Thickness	0.75	in			Pn	1947.772522	Yielding Mn	260637.75	For Pu/Pc≥0.2	0.827171		
Strength of Steel	36	ksi			Fcr	8.413845629	Local Buckling	229197.9926	For Pu/Pc<0.2	0.598329		
Modulus of Elasticity	29000	ksi			KL/r	172.7233417	Check for Compact section:		Magnification Factors			
					4.71*sqrt(E/Fy)	133.680683	D/t	132	B2	1.031415		
ID	97.5				Fe	9.593894674	λp	56.38888889	Pnt	757.7449		
S	5643.375308				Fcr for KL/r≤4.71*sqrt(E/Fy)	7.485413811	λc	249.7222222	Pe2	24878.07		
Z	7239.9375				Fcr for KL/r>4.71*sqrt(E/Fy)	8.413845629	non-compact		ΣH	58.54		
r	34.73763269				Ag	231.4961087	Flexure Strength:	OK	ΔH	6		
l	279347.0777				Check for Compact section:				L	3000		
					D/t	132						
					λp							
					λc	88.61111111						
					noncompact							
					Axial Strength:	OK						

Weight of Equipment: 400 kips
 Height of tower: 250 ft
 Outer Diameter at base: 8.25 ft
 Outer Diameter at hub: 7.5 ft
 Thickness: 0.50 in
 Weight of Steel: 490 pcf
 Town (Massachusetts): Lee

Axial Load: 669.7351275
 Moment Load: 5587.946162
 Total Shear: 41.52

t_{ice}: 0.0625

Section	Diameter	Height	t	Inner Dia.	OR	IR	Ice Radius
D1	8.25	0	50	8.166666667	4.125	4.083333	4.375
D2	8.0625	51	100	7.979166667	4.03125	3.989583	4.28125
D3	7.875	101	150	7.791666667	3.9375	3.895833	4.1875
D4	7.6875	151	200	7.604166667	3.84375	3.802083	4.09375
D5	7.5	201	250	7.416666667	3.75	3.708333	4

Height	Volume of Steel	Weight of Steel	Volume of Ice	Weight of Ice	Fz	td	D1	D2	D3	D4	D5	Diameter	V(3-s)	V(f,m)	z	Kz calc	1skz	1skzs2.58	q _s	Gh	Cf	Ar	Ag	e	Ae	Force	Moment
1	1.074468321	0.526489477	1.718296433	0.0962246	0.6577269	0.06577269	8.25	8.25	90	76	0.5	0.302086	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.06173916				
2	1.074468321	0.526489477	1.919591926	0.107497148	0.73410424	0.07341042	8.25	8.25	90	76	1.5	0.413476	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.18521747				
3	1.074468321	0.526489477	2.021131134	0.113183343	0.77257848	0.07725785	8.25	8.25	90	76	2.5	0.478449	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.30869578				
4	1.074468321	0.526489477	2.09095725	0.117093606	0.79901588	0.07990159	8.25	8.25	90	76	3.5	0.526729	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.43217409				
5	1.074468321	0.526489477	2.144695335	0.120102939	0.81935075	0.08193508	8.25	8.25	90	76	4.5	0.565941	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.5565524				
6	1.074468321	0.526489477	2.188603977	0.122561823	0.8359588	0.08359588	8.25	8.25	90	76	5.5	0.599337	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.67913071				
7	1.074468321	0.526489477	2.225848662	0.124647525	0.85004113	0.08500411	8.25	8.25	90	76	6.5	0.628637	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.80260902				
8	1.074468321	0.526489477	2.258261631	0.126462651	0.86229275	0.08622927	8.25	8.25	90	76	7.5	0.654872	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	0.92608734				
9	1.074468321	0.526489477	2.287002286	0.128072128	0.8731533	0.08731533	8.25	8.25	90	76	8.5	0.678115	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.04956566				
10	1.074468321	0.526489477	2.312852489	0.129519739	0.88291921	0.08829192	8.25	8.25	90	76	9.5	0.70063	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.17304395				
11	1.074468321	0.526489477	2.336365354	0.13083646	0.89180014	0.08918001	8.25	8.25	90	76	10.5	0.720953	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.29652227				
12	1.074468321	0.526489477	2.357947076	0.132045036	0.89995002	0.089995	8.25	8.25	90	76	11.5	0.739938	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.42000058				
13	1.074468321	0.526489477	2.377905042	0.133162682	0.90748532	0.09074853	8.25	8.25	90	76	12.5	0.757778	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.54347889				
14	1.074468321	0.526489477	2.396477675	0.13420275	0.91449636	0.09144964	8.25	8.25	90	76	13.5	0.774625	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.6669572				
15	1.074468321	0.526489477	2.413853758	0.13517581	0.92105466	0.09210547	8.25	8.25	90	76	14.5	0.790603	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.79043552				
16	1.074468321	0.526489477	2.430185411	0.136090383	0.92721783	0.09272178	8.25	8.25	90	76	15.5	0.805812	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	1.91391383				
17	1.074468321	0.526489477	2.445597066	0.136953436	0.93303299	0.0933033	8.25	8.25	90	76	16.5	0.820335	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.03739214				
18	1.074468321	0.526489477	2.460191851	0.137770744	0.93853919	0.09385392	8.25	8.25	90	76	17.5	0.834243	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.16087054				
19	1.074468321	0.526489477	2.474056221	0.138547148	0.94376915	0.09437692	8.25	8.25	90	76	18.5	0.847594	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.28434876				
20	1.074468321	0.526489477	2.487263396	0.13928675	0.94875061	0.09487506	8.25	8.25	90	76	19.5	0.860349	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.40782707				
21	1.074468321	0.526489477	2.499875947	0.139993053	0.95350723	0.09535072	8.25	8.25	90	76	20.5	0.872822	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.53130538				
22	1.074468321	0.526489477	2.511947781	0.140669076	0.95805943	0.09580594	8.25	8.25	90	76	21.5	0.884781	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.6547837				
23	1.074468321	0.526489477	2.523525683	0.141317438	0.96242492	0.09624249	8.25	8.25	90	76	22.5	0.896348	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.77826201				
24	1.074468321	0.526489477	2.534650524	0.141940429	0.96661914	0.09666191	8.25	8.25	90	76	23.5	0.907554	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	2.90174032				
25	1.074468321	0.526489477	2.545358226	0.142540061	0.97065571	0.09706557	8.25	8.25	90	76	24.5	0.918425	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.02521863				
26	1.074468321	0.526489477	2.555680538	0.14311811	0.97454663	0.09745466	8.25	8.25	90	76	25.5	0.928982	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.14869694				
27	1.074468321	0.526489477	2.565645658	0.143676157	0.97830257	0.09783026	8.25	8.25	90	76	26.5	0.939249	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.27171525				
28	1.074468321	0.526489477	2.575278753	0.14421561	0.98193304	0.0981933	8.25	8.25	90	76	27.5	0.949242	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.39565356				
29	1.074468321	0.526489477	2.584602377	0.144737733	0.98544659	0.09854466	8.25	8.25	90	76	28.5	0.958789	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.51913188				
30	1.074468321	0.526489477	2.593636826	0.145243662	0.98885089	0.09888509	8.25	8.25	90	76	29.5	0.968474	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.64261109				
31	1.074468321	0.526489477	2.602400431	0.145734424	0.99215286	0.09921529	8.25	8.25	90	76	30.5	0.977473	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.7660885				
32	1.074468321	0.526489477	2.610909806	0.146210949	0.9953588	0.09953588	8.25	8.25	90	76	31.5	0.986797	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	3.88956681				
33	1.074468321	0.526489477	2.619180058	0.146674083	0.99847442	0.09984744	8.25	8.25	90	76	32.5	0.995647	1	1	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.01279507				
34	1.074468321	0.526489477	2.627224964	0.147124598	1.00150492	0.10015049	8.25	8.25	90	76	33.5	1.004306	1.00430578	1.0043058	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.17195161				
35	1.074468321	0.526489477	2.635057124	0.147563199	1.00445507	0.10044551	8.25	8.25	90	76	34.5	1.012781	1.012781498	1.0127815	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.33221966				
36	1.074468321	0.526489477	2.642688094	0.147990533	1.00732924	0.10073292	8.25	8.25	90	76	35.5	1.021084	1.021083506	1.0210835	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.49381623				
37	1.074468321	0.526489477	2.650113146	0.148407196	1.01011315	0.10101314	8.25	8.25	90	76	36.5	1.02922	1.029220112	1.0292201	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.65671458				
38	1.074468321	0.526489477	2.657388128	0.148813735	1.01286539	0.10128654	8.25	8.25	90	76	37.5	1.037199	1.03719901	1.037199	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.82088919				
39	1.074468321	0.526489477	2.664476024	0.149210657	1.01553449	0.10155345	8.25	8.25	90	76	38.5	1.045027	1.045027344	1.0450273	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	4.98631575				
40	1.074468321	0.526489477	2.671400556	0.149598431	1.01814191	0.10181419	8.25	8.25	90	76	39.5	1.052172	1.052171756	1.0521718	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	5.15297099				
41	1.074468321	0.526489477	2.678169486	0.149977491	1.02069058	0.10206906	8.25	8.25	90	76	40.5	1.060258	1.06025844	1.0602584	14.78656	1.69	0.59	8.375	8.375	1	8.375	1.23478	5.32083269				
42	1.074468321	0.526489477	2.684790029	0.150348242	1.02318324	0.10231832	8.25	8.25	90	76	41.5	1.067673	1.067673173	1.0676732	14.78656	1.69	0.59	8.375	8.375	1	8.375						

83	1.049924628	0.514463068	2.813669777	0.157567031	1.09595823	0.10959582	8.063				0.0625	90	76	82.5	1.299263	1.299263232	1.2992632	19.21163	1.69	0.59	8.1875	8.1875	1	8.1875	0.156839	12.9615796
84	1.049924628	0.514463068	2.817134603	0.157759538	1.09727947	0.10972795	8.063				0.0625	90	76	83.5	1.303743	1.303743449	1.3037435	19.27788	1.69	0.59	8.1875	8.1875	1	8.1875	0.15738	13.1635657
85	1.049924628	0.514463068	2.820535483	0.157949987	1.09858655	0.10985865	8.063				0.0625	90	76	84.5	1.308186	1.308185595	1.3081856	19.34756	1.69	0.59	8.1875	8.1875	1	8.1875	0.157916	13.3664236
86	1.049924628	0.514463068	2.823900437	0.158138424	1.09987978	0.10998798	8.063				0.0625	90	76	85.5	1.31259	1.312590307	1.3125903	19.4087	1.69	0.59	8.1875	8.1875	1	8.1875	0.158448	13.5698773
87	1.049924628	0.514463068	2.827230258	0.158324894	1.10115947	0.11011595	8.063				0.0625	90	76	86.5	1.316958	1.316958373	1.3169584	19.47328	1.69	0.59	8.1875	8.1875	1	8.1875	0.158975	13.7740111
88	1.049924628	0.514463068	2.830525714	0.15850944	1.10242591	0.11024259	8.063				0.0625	90	76	87.5	1.321291	1.321290177	1.3212901	19.53734	1.69	0.59	8.1875	8.1875	1	8.1875	0.159498	13.9788195
89	1.049924628	0.514463068	2.833787545	0.158692103	1.10367939	0.11036794	8.063				0.0625	90	76	88.5	1.325857	1.325857439	1.3258574	19.60088	1.69	0.59	8.1875	8.1875	1	8.1875	0.160017	14.1842988
90	1.049924628	0.514463068	2.83701647	0.158872922	1.10492019	0.11049202	8.063				0.0625	90	76	89.5	1.32985	1.32984982	1.3298498	19.6639	1.69	0.59	8.1875	8.1875	1	8.1875	0.160531	14.3904378
91	1.049924628	0.514463068	2.840213182	0.159051938	1.10614857	0.11061486	8.063				0.0625	90	76	90.5	1.334078	1.334078317	1.3340783	19.72643	1.69	0.59	8.1875	8.1875	1	8.1875	0.161042	14.5973271
92	1.049924628	0.514463068	2.843378354	0.159229188	1.1073648	0.11073648	8.063				0.0625	90	76	91.5	1.338274	1.338273571	1.3382736	19.78846	1.69	0.59	8.1875	8.1875	1	8.1875	0.161548	14.8046895
93	1.049924628	0.514463068	2.846512636	0.159404708	1.10856913	0.11085691	8.063				0.0625	90	76	92.5	1.342346	1.342346201	1.3423462	19.85001	1.69	0.59	8.1875	8.1875	1	8.1875	0.162051	15.0127899
94	1.049924628	0.514463068	2.849616659	0.159578533	1.10976179	0.11097618	8.063				0.0625	90	76	93.5	1.346567	1.346566811	1.3465668	19.91109	1.69	0.59	8.1875	8.1875	1	8.1875	0.162549	15.2215333
95	1.049924628	0.514463068	2.852691034	0.159750698	1.11094033	0.1110943	8.063				0.0625	90	76	94.5	1.350666	1.350665984	1.350666	19.9717	1.69	0.59	8.1875	8.1875	1	8.1875	0.163044	15.4309148
96	1.049924628	0.514463068	2.855736355	0.159921236	1.11211307	0.11121131	8.063				0.0625	90	76	95.5	1.354734	1.35473429	1.3547343	20.03186	1.69	0.59	8.1875	8.1875	1	8.1875	0.163535	16.0409296
97	1.049924628	0.514463068	2.858753195	0.160090179	1.11327213	0.11132721	8.063				0.0625	90	76	96.5	1.358772	1.358772279	1.3587723	20.09157	1.69	0.59	8.1875	8.1875	1	8.1875	0.164023	15.8515728
98	1.049924628	0.514463068	2.861742112	0.160257558	1.11442044	0.11144204	8.063				0.0625	90	76	97.5	1.36278	1.36278049	1.3627805	20.15084	1.69	0.59	8.1875	8.1875	1	8.1875	0.164507	16.06284
99	1.049924628	0.514463068	2.864703646	0.160423404	1.11555819	0.1115582	8.063				0.0625	90	76	98.5	1.366759	1.366759443	1.3667594	20.20967	1.69	0.59	8.1875	8.1875	1	8.1875	0.164987	16.2747265
100	1.049924628	0.514463068	2.867638324	0.160587746	1.1166856	0.11166856	8.063				0.0625	90	76	99.5	1.37071	1.370709645	1.3707096	20.26808	1.69	0.59	8.1875	8.1875	1	8.1875	0.165464	16.2978568
101	1.025380936	0.502436658	2.804702632	0.157063347	1.11780285	0.11178028		7.875			7.875	90	76	100.5	1.374632	1.374631591	1.3746316	20.32607	1.69	0.59	8	8	1	8	0.162137	16.3178889
102	1.025380936	0.502436658	2.807519887	0.157221114	1.11891014	0.11189101		7.875			7.875	90	76	101.5	1.378526	1.378525761	1.3785258	20.38365	1.69	0.59	8	8	1	8	0.162596	16.5267123
103	1.025380936	0.502436658	2.810312346	0.157377491	1.12000766	0.11200077		7.875			7.875	90	76	102.5	1.382393	1.382392622	1.3823926	20.44083	1.69	0.59	8	8	1	8	0.163052	16.7361237
104	1.025380936	0.502436658	2.813804688	0.157532506	1.12109558	0.11210956		7.875			7.875	90	76	103.5	1.386233	1.386232629	1.3862326	20.49761	1.69	0.59	8	8	1	8	0.163505	16.9461191
105	1.025380936	0.502436658	2.815824697	0.157686183	1.12217408	0.11221741		7.875			7.875	90	76	104.5	1.390046	1.390046226	1.3900462	20.554	1.69	0.59	8	8	1	8	0.163955	17.1566943
106	1.025380936	0.502436658	2.818545464	0.157838546	1.12324334	0.11232433		7.875			7.875	90	76	105.5	1.393834	1.393833804	1.3938338	20.61001	1.69	0.59	8	8	1	8	0.164402	17.3678454
107	1.025380936	0.502436658	2.82124319	0.157989619	1.12430351	0.11243035		7.875			7.875	90	76	106.5	1.397596	1.39759595	1.3975959	20.66564	1.69	0.59	8	8	1	8	0.164846	17.5795685
108	1.025380936	0.502436658	2.823918283	0.158139424	1.12535476	0.11253548		7.875			7.875	90	76	107.5	1.401333	1.401332817	1.4013328	20.72089	1.69	0.59	8	8	1	8	0.165286	17.7918598
109	1.025380936	0.502436658	2.826571142	0.158287984	1.12639725	0.11263972		7.875			7.875	90	76	108.5	1.405045	1.405044981	1.405045	20.77578	1.69	0.59	8	8	1	8	0.165724	18.0047155
110	1.025380936	0.502436658	2.829202154	0.158435321	1.12743112	0.11274311		7.875			7.875	90	76	109.5	1.408733	1.408732787	1.4087328	20.83031	1.69	0.59	8	8	1	8	0.166159	18.2181319
111	1.025380936	0.502436658	2.831811695	0.158581455	1.12845653	0.11284565		7.875			7.875	90	76	110.5	1.412397	1.412396615	1.4123966	20.88449	1.69	0.59	8	8	1	8	0.166591	18.4321053
112	1.025380936	0.502436658	2.834400134	0.158726407	1.12947362	0.11294736		7.875			7.875	90	76	111.5	1.416037	1.416036835	1.4160368	20.93831	1.69	0.59	8	8	1	8	0.167021	18.6466321
113	1.025380936	0.502436658	2.836967827	0.158870198	1.13048254	0.11304825		7.875			7.875	90	76	112.5	1.419654	1.41965381	1.4196538	20.9918	1.69	0.59	8	8	1	8	0.167447	18.8617088
114	1.025380936	0.502436658	2.839515123	0.159012847	1.13148341	0.11314834		7.875			7.875	90	76	113.5	1.423248	1.423247891	1.4232479	21.04494	1.69	0.59	8	8	1	8	0.167871	19.0773319
115	1.025380936	0.502436658	2.842042362	0.159154372	1.13247639	0.11324764		7.875			7.875	90	76	114.5	1.426819	1.426819425	1.4268194	21.09775	1.69	0.59	8	8	1	8	0.168293	19.2934979
116	1.025380936	0.502436658	2.844549876	0.159294793	1.13346158	0.11334616		7.875			7.875	90	76	115.5	1.430369	1.430368748	1.4303687	21.15023	1.69	0.59	8	8	1	8	0.168711	19.5020735
117	1.025380936	0.502436658	2.847037986	0.159434127	1.13443913	0.11344391		7.875			7.875	90	76	116.5	1.433896	1.433896188	1.4338962	21.20239	1.69	0.59	8	8	1	8	0.169127	19.7274453
118	1.025380936	0.502436658	2.849570707	0.159572392	1.13540916	0.11354092		7.875			7.875	90	76	117.5	1.437402	1.437402066	1.4374021	21.25423	1.69	0.59	8	8	1	8	0.169541	19.945227
119	1.025380936	0.502436658	2.851957427	0.159709606	1.13637178	0.11363718		7.875			7.875	90	76	118.5	1.440887	1.440886696	1.4408867	21.30576	1.69	0.59	8	8	1	8	0.169952	20.1635245
120	1.025380936	0.502436658	2.854389006	0.159845784	1.13732712	0.11373271		7.875			7.875	90	76	119.5	1.444435	1.444350385	1.4443504	21.35697	1.69	0.59	8	8	1	8	0.170306	20.3823554
121	1.025380936	0.502436658	2.856802575	0.159980944	1.13827553	0.11382753		7.875			7.875	90	76	120.5	1.447793	1.447793342	1.4477934	21.40788	1.69	0.59	8	8	1	8	0.170706	20.6017096
122	1.025380936	0.502436658	2.859198241	0.160115101	1.13921641	0.11392164		7.875			7.875	90	76	121.5	1.451216	1.451216129	1.4512161	21.45849	1.69	0.59	8	8	1	8	0.171117	20.8215841
123	1.025380936	0.502436658	2.861576281	0.160248272	1.14015059	0.11401506		7.875			7.875	90	76	122.5	1.454619	1.454618764	1.4546188	21.50881	1.69	0.59	8	8	1	8	0.171571	21.0419758
124	1.025380936	0.502436658	2.863936967	0.16038047	1.14107792	0.11410779		7.875			7.875	90	76	123.5	1.458002	1.458001615	1.4580016	21.55883	1.69	0.59	8	8	1	8	0.171917	21.2628816
125	1.025380936	0.50243																								

175	1.000837243	0.490410249	2.896579801	0.162208469	1.18121281	0.11812128			7.688	7.6875	90	76	174.5	1.609355	1.609355004	1.609355	23.79682	1.69	0.59	7.8125	7.8125	1	7.8125	0.185374	32.3741365
176	1.000837243	0.490410249	2.898260536	0.16230259	1.18188799	0.1181888			7.688	7.6875	90	76	175.5	1.611985	1.611984676	1.6119847	23.8371	1.69	0.59	7.8125	7.8125	1	7.8125	0.185676	33.6217211
177	1.000837243	0.490410249	2.899932703	0.162396231	1.18255971	0.11825597			7.688	7.6875	90	76	176.5	1.614604	1.614603666	1.6146037	23.87443	1.69	0.59	7.8125	7.8125	1	7.8125	0.185978	32.8516762
178	1.000837243	0.490410249	2.901596393	0.162489398	1.18322801	0.1183228			7.688	7.6875	90	76	177.5	1.617212	1.617212079	1.6172121	23.913	1.69	0.59	7.8125	7.8125	1	7.8125	0.186279	33.0910273
179	1.000837243	0.490410249	2.903251696	0.162582095	1.18389293	0.11838929			7.688	7.6875	90	76	178.5	1.61981	1.619810016	1.61981	23.95142	1.69	0.59	7.8125	7.8125	1	7.8125	0.186578	33.3706736
180	1.000837243	0.490410249	2.904898701	0.162674327	1.18455451	0.11845545			7.688	7.6875	90	76	179.5	1.622398	1.622397578	1.6223976	23.98968	1.69	0.59	7.8125	7.8125	1	7.8125	0.186876	33.5708838
181	1.000837243	0.490410249	2.906537497	0.1627661	1.18521278	0.11852128			7.688	7.6875	90	76	180.5	1.624975	1.624974864	1.6249749	24.02779	1.69	0.59	7.8125	7.8125	1	7.8125	0.187173	33.8211862
182	1.000837243	0.490410249	2.908168168	0.162857417	1.18586678	0.11858678			7.688	7.6875	90	76	181.5	1.627542	1.627541971	1.627542	24.06575	1.69	0.59	7.8125	7.8125	1	7.8125	0.187468	34.0522694
183	1.000837243	0.490410249	2.909790801	0.162948285	1.18651954	0.11865195			7.688	7.6875	90	76	182.5	1.630099	1.630098995	1.630099	24.10356	1.69	0.59	7.8125	7.8125	1	7.8125	0.187763	34.2935319
184	1.000837243	0.490410249	2.911405477	0.163038707	1.18716809	0.11871681			7.688	7.6875	90	76	183.5	1.632646	1.632646031	1.632646	24.14122	1.69	0.59	7.8125	7.8125	1	7.8125	0.188056	34.5351721
185	1.000837243	0.490410249	2.913012228	0.163128668	1.18781346	0.11878135			7.688	7.6875	90	76	184.5	1.635183	1.635183171	1.6351832	24.17873	1.69	0.59	7.8125	7.8125	1	7.8125	0.188349	34.7771887
186	1.000837243	0.490410249	2.914611289	0.163218232	1.1884557	0.11884557			7.688	7.6875	90	76	185.5	1.637711	1.637710507	1.6377105	24.2161	1.69	0.59	7.8125	7.8125	1	7.8125	0.18864	35.0195801
187	1.000837243	0.490410249	2.916202585	0.163307345	1.18909483	0.11890948			7.688	7.6875	90	76	186.5	1.640228	1.640228131	1.6402281	24.25333	1.69	0.59	7.8125	7.8125	1	7.8125	0.18893	35.2623449
188	1.000837243	0.490410249	2.917786246	0.16339603	1.18973088	0.11897309			7.688	7.6875	90	76	187.5	1.642736	1.642736131	1.6427361	24.29042	1.69	0.59	7.8125	7.8125	1	7.8125	0.189219	35.5054816
189	1.000837243	0.490410249	2.919362349	0.163484292	1.19036388	0.11903639			7.688	7.6875	90	76	188.5	1.645235	1.645234594	1.6452346	24.32736	1.69	0.59	7.8125	7.8125	1	7.8125	0.189506	35.7489899
190	1.000837243	0.490410249	2.920930969	0.163572134	1.19099387	0.11909939			7.688	7.6875	90	76	189.5	1.647724	1.647723608	1.6477236	24.36416	1.69	0.59	7.8125	7.8125	1	7.8125	0.189793	35.9928654
191	1.000837243	0.490410249	2.922492182	0.163659562	1.19162088	0.11916209			7.688	7.6875	90	76	190.5	1.650203	1.650203257	1.6502033	24.40083	1.69	0.59	7.8125	7.8125	1	7.8125	0.190079	36.2371097
192	1.000837243	0.490410249	2.924046061	0.163746579	1.19224493	0.11922449			7.688	7.6875	90	76	191.5	1.652674	1.652673627	1.6526736	24.43736	1.69	0.59	7.8125	7.8125	1	7.8125	0.190363	36.4817203
193	1.000837243	0.490410249	2.925592678	0.16383319	1.19286605	0.11928661			7.688	7.6875	90	76	192.5	1.655135	1.655134799	1.6551348	24.47375	1.69	0.59	7.8125	7.8125	1	7.8125	0.190647	36.7266959
194	1.000837243	0.490410249	2.927132105	0.163919398	1.19348428	0.11934843			7.688	7.6875	90	76	193.5	1.657587	1.657586855	1.6575869	24.51001	1.69	0.59	7.8125	7.8125	1	7.8125	0.190929	36.9720352
195	1.000837243	0.490410249	2.928664413	0.164005207	1.19409964	0.11940996			7.688	7.6875	90	76	194.5	1.66003	1.660029877	1.6600299	24.54613	1.69	0.59	7.8125	7.8125	1	7.8125	0.191211	37.2177369
196	1.000837243	0.490410249	2.93018967	0.164090622	1.19471215	0.11947122			7.688	7.6875	90	76	195.5	1.662464	1.662463943	1.6624639	24.58212	1.69	0.59	7.8125	7.8125	1	7.8125	0.191491	37.4637995
197	1.000837243	0.490410249	2.931707945	0.164175645	1.19532186	0.11953219			7.688	7.6875	90	76	196.5	1.664889	1.664889133	1.6648891	24.61798	1.69	0.59	7.8125	7.8125	1	7.8125	0.19177	37.7102217
198	1.000837243	0.490410249	2.933219305	0.164260281	1.19592878	0.11959288			7.688	7.6875	90	76	197.5	1.667306	1.667305522	1.6673055	24.65371	1.69	0.59	7.8125	7.8125	1	7.8125	0.192049	37.9570024
199	1.000837243	0.490410249	2.934723817	0.164344534	1.19653293	0.11965329			7.688	7.6875	90	76	198.5	1.669713	1.669713188	1.6697132	24.68931	1.69	0.59	7.8125	7.8125	1	7.8125	0.192326	38.2041401
200	1.000837243	0.490410249	2.936221545	0.164428407	1.19713436	0.11971344			7.688	7.6875	90	76	199.5	1.672112	1.672112206	1.6721122	24.72479	1.69	0.59	7.8125	7.8125	1	7.8125	0.192602	38.4598949
201	0.97629355	0.47838384	2.867160253	0.160560974	1.19773308	0.11977331			7.5	7.5	90	76	200.5	1.674503	1.67450265	1.6745026	24.76013	1.69	0.59	7.625	7.625	1	7.625	0.188249	37.770694
202	0.97629355	0.47838384	2.868609497	0.160642132	1.19832911	0.11983291			7.5	7.5	90	76	201.5	1.676885	1.676884593	1.6768846	24.79535	1.69	0.59	7.625	7.625	1	7.625	0.188516	38.0129384
203	0.97629355	0.47838384	2.870052305	0.160722929	1.19892249	0.11989225			7.5	7.5	90	76	202.5	1.679258	1.679258107	1.6792581	24.83045	1.69	0.59	7.625	7.625	1	7.625	0.188783	38.2552623
204	0.97629355	0.47838384	2.871488737	0.160803369	1.19951324	0.11995132			7.5	7.5	90	76	203.5	1.681623	1.681623264	1.6816233	24.86542	1.69	0.59	7.625	7.625	1	7.625	0.189049	38.4984565
205	0.97629355	0.47838384	2.872918852	0.160883456	1.20010139	0.12001014			7.5	7.5	90	76	204.5	1.683998	1.683980133	1.6839801	24.90027	1.69	0.59	7.625	7.625	1	7.625	0.189314	38.7417279
206	0.97629355	0.47838384	2.874342708	0.160963192	1.20066895	0.12006895			7.5	7.5	90	76	205.5	1.686329	1.686328785	1.6863288	24.935	1.69	0.59	7.625	7.625	1	7.625	0.189578	38.9853393
207	0.97629355	0.47838384	2.875760363	0.16104258	1.20126995	0.12012699			7.5	7.5	90	76	206.5	1.688669	1.688669287	1.6886693	24.96961	1.69	0.59	7.625	7.625	1	7.625	0.189841	39.2292893
208	0.97629355	0.47838384	2.877171875	0.161121625	1.20185041	0.12018504			7.5	7.5	90	76	207.5	1.691002	1.691001707	1.6910017	25.0041	1.69	0.59	7.625	7.625	1	7.625	0.190103	39.4735777
209	0.97629355	0.47838384	2.878577298	0.161200329	1.20242836	0.12024284			7.5	7.5	90	76	208.5	1.693326	1.693326112	1.6933261	25.03847	1.69	0.59	7.625	7.625	1	7.625	0.190365	39.7182011
210	0.97629355	0.47838384	2.879976689	0.161278695	1.20300383	0.12030038			7.5	7.5	90	76	209.5	1.695643	1.695642568	1.6956426	25.07272	1.69	0.59	7.625	7.625	1	7.625	0.190625	39.9631604
211	0.97629355	0.47838384	2.881370101	0.161356726	1.20357682	0.12035768			7.5	7.5	90	76	210.5	1.697951	1.697951139	1.6979511	25.10686	1.69	0.59	7.625	7.625	1	7.625	0.190885	40.2084539
212	0.97629355	0.47838384	2.882757589	0.161434425	1.20414737	0.12041474			7.5	7.5	90	76	211.5	1.700252	1.70025189	1.7002519	25.14088	1.69	0.59	7.625	7.625	1	7.625	0.191143	40.4548084
213	0.97629355	0.47838384	2.884139206	0.161511796	1.2047155	0.12047155			7.5	7.5	90	76	212.5	1.702545	1.702544883	1.7025449	25.17478	1.69	0.59	7.625	7.625	1	7.625	0.191401	40.7003827
214	0.97629355	0.47838384	2.885515004	0.16158884	1.20528123	0.12052812			7.5	7.5	90	76	213.5	1.70483	1.704830182	1.7048302	25.20857	1.69	0.59	7.625	7.625	1	7.625	0.191658	40.9463278
215	0.97629355	0.47838384	2.886885034	0.161665562	1.20584458	0.12058446			7.5	7.5	90	76	214.5	1.707108	1.707107849	1.7071078	25.24225	1.69	0.59	7.625	7.625	1	7.625	0.191914	41.1929465
216	0.97629355	0.47838384	2.888249347	0.161741963	1.20640557	0.12064056			7.5	7.5	90	76	215.5	1.709378	1.709377943	1.70937									

User Input				Axial Strength		Flexure Strength		Combined Loads				
Overall height	250	ft =	3000	in	Factored Load	669.7351275	Factored Load	67055.35	Combined Loading	0.996377	≤1.0	OK
Diameter at Base	8.25	ft=	99	in	Factored Strength	1112.14855	Factored Strength	157177.80	Pu/Pc	0.602199		
Shell Thickness	0.5	in			Pn	1308.410059	Yielding Mn	174642	For Pu/Pc≥0.2	0.996377		
Strength of Steel	36	ksi			Fcr	8.456443794	Local Buckling	148133.055	For Pu/Pc<0.2	0.695277		
Modulus of Elasticity	29000	ksi			KL/r	172.2877575	Check for Compact section:		Magnification Factors			
					4.71*sqrt(E/Fy)	133.680683	D/t	198	B2	1.039447		
ID	98				Fe	9.642467267	λp	56.38888889	Pnt	669.7351		
S	3790.919594				Fcr for KL/r≤4.71*sqrt(E/Fy)	7.544869482	λc	249.7222222	Pe2	17647.76		
Z	4851.166667				Fcr for KL/r>4.71*sqrt(E/Fy)	8.456443794	non-compact		ΣH	41.52		
r	34.82545764				Ag	154.7234382	Flexure Strength:	OK	ΔH	6		
l	187650.5199				Check for Compact section:				L	3000		
					D/t	198						
					λp							
					λc	88.61111111						
					noncompact							
					Axial Strength:	OK						

Weight of Equipment: 400 kips
 Height of tower: 250 ft
 Outer Diameter at base: 8.5 ft
 Outer Diameter at hub: 7.5 ft
 Thickness: 0.75 in
 Weight of Steel: 490 pcf

Axial Load 748.6849367
 Moment Load 10552.98717

Total Shear 78.83
 t_{ice} 0.0625

Yarmouth

Section	Diameter	Height	t	Inner Dia.	OR	IR	Ice Radius
D1	8.5	0	50	8.375	4.25	4.1875	4.5
D2	8.25	51	100	8.125	4.125	4.0625	4.375
D3	8	101	150	7.875	4	3.9375	4.25
D4	7.75	151	200	7.625	3.875	3.8125	4.125
D5	7.5	201	250	7.375	3.75	3.6875	4

Height	Volume of Steel	Weight of Steel	Volume of Ice	Weight of Ice	Fz	td	D1	D2	D3	D4	D5	Diameter	v(3-s)	v(f,m)	z	Kz calc	1skz	1skzs2.58	q _s	Gh	Cf	Ar	Ag	e	Ae	Force	Moment
1	1.656699251	0.811782633	1.769954184	0.099117434	0.6577269	0.06577269	8.5	8.5	120	104	0.5	0.302086	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	0.11906235				
2	1.656699251	0.811782633	1.977248338	0.110725907	0.73410424	0.073410424	8.5	8.5	120	104	1.5	0.4134476	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	0.35718706				
3	1.656699251	0.811782633	2.081809306	0.116581321	0.77257848	0.07725785	8.5	8.5	120	104	2.5	0.4784489	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	0.59531177				
4	1.656699251	0.811782633	2.15371181	0.120607861	0.79901588	0.079901588	8.5	8.5	120	104	3.5	0.5267299	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	0.83343648				
5	1.656699251	0.811782633	2.209406993	0.123706632	0.81935075	0.081935075	8.5	8.5	120	104	4.5	0.5659941	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	1.07561199				
6	1.656699251	0.811782633	2.254260028	0.126238562	0.8359588	0.08359588	8.5	8.5	120	104	5.5	0.599337	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	1.30968599				
7	1.656699251	0.811782633	2.292610737	0.128386201	0.85004113	0.085004113	8.5	8.5	120	104	6.5	0.628637	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	1.54781061				
8	1.656699251	0.811782633	2.325985945	0.130255213	0.86229275	0.086229275	8.5	8.5	120	104	7.5	0.654872	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	1.78593532				
9	1.656699251	0.811782633	2.355579586	0.131912457	0.87315333	0.087315333	8.5	8.5	120	104	8.5	0.678715	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	2.02406003				
10	1.656699251	0.811782633	2.382196801	0.133403021	0.88291921	0.088291921	8.5	8.5	120	104	9.5	0.70063	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	2.26218474				
11	1.656699251	0.811782633	2.406407173	0.134758802	0.89180014	0.089180014	8.5	8.5	120	104	10.5	0.720953	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	2.50030945				
12	1.656699251	0.811782633	2.428628985	0.136003223	0.89995002	0.089995	8.5	8.5	120	104	11.5	0.739938	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	2.73843416				
13	1.656699251	0.811782633	2.449178772	0.137154011	0.90748532	0.090748532	8.5	8.5	120	104	12.5	0.757778	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	2.97655887				
14	1.656699251	0.811782633	2.468302051	0.138224915	0.91449636	0.09144964	8.5	8.5	120	104	13.5	0.774625	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	3.21468358				
15	1.656699251	0.811782633	2.486193222	0.13922682	0.92105466	0.09210547	8.5	8.5	120	104	14.5	0.790603	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	3.45280829				
16	1.656699251	0.811782633	2.503008929	0.1401685	0.92721783	0.09272178	8.5	8.5	120	104	15.5	0.805812	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	3.690933				
17	1.656699251	0.811782633	2.518877306	0.141057129	0.93303299	0.0933033	8.5	8.5	120	104	16.5	0.820335	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	3.92905771				
18	1.656699251	0.811782633	2.533904546	0.141896655	0.93853919	0.09385392	8.5	8.5	120	104	17.5	0.834243	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	4.16718742				
19	1.656699251	0.811782633	2.548179677	0.142698062	0.94376915	0.09437692	8.5	8.5	120	104	18.5	0.847594	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	4.40530713				
20	1.656699251	0.811782633	2.561778095	0.143459573	0.94875061	0.09487506	8.5	8.5	120	104	19.5	0.860349	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	4.64343184				
21	1.656699251	0.811782633	2.57476423	0.144186797	0.95350723	0.09535072	8.5	8.5	120	104	20.5	0.872822	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	4.88155685				
22	1.656699251	0.811782633	2.587193599	0.144882841	0.95805943	0.09580594	8.5	8.5	120	104	21.5	0.884781	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	5.11968126				
23	1.656699251	0.811782633	2.599114359	0.145550404	0.96242492	0.09624249	8.5	8.5	120	104	22.5	0.896348	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	5.35780597				
24	1.656699251	0.811782633	2.610568614	0.146191842	0.96661914	0.09666191	8.5	8.5	120	104	23.5	0.907554	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	5.59593068				
25	1.656699251	0.811782633	2.621593348	0.146809227	0.97065571	0.09706557	8.5	8.5	120	104	24.5	0.918425	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	5.83405539				
26	1.656699251	0.811782633	2.632221251	0.147404939	0.97454663	0.09745466	8.5	8.5	120	104	25.5	0.928982	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	6.0721801				
27	1.656699251	0.811782633	2.642481362	0.147978956	0.97830257	0.09783026	8.5	8.5	120	104	26.5	0.939249	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	6.31034041				
28	1.656699251	0.811782633	2.652399594	0.148534377	0.98193304	0.0981933	8.5	8.5	120	104	27.5	0.949242	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	6.54842952				
29	1.656699251	0.811782633	2.661999171	0.149071954	0.98544659	0.09854466	8.5	8.5	120	104	28.5	0.959878	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	6.78655423				
30	1.656699251	0.811782633	2.671300993	0.149592856	0.98885089	0.09888509	8.5	8.5	120	104	29.5	0.968474	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	7.02467894				
31	1.656699251	0.811782633	2.680323935	0.150098414	0.99215286	0.09921529	8.5	8.5	120	104	30.5	0.977443	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	7.26280365				
32	1.656699251	0.811782633	2.689085104	0.150588766	0.9953588	0.09953588	8.5	8.5	120	104	31.5	0.986797	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	7.50092836				
33	1.656699251	0.811782633	2.697600056	0.151065603	0.99847442	0.09984744	8.5	8.5	120	104	32.5	0.995647	1	1	27.68896	1.69	0.59	8.625	8.625	1	8.625	0.238125	7.75599984				
34	1.656699251	0.811782633	2.705882976	0.151529447	1.00150492	0.10015049	8.5	8.5	120	104	33.5	1.004306	1.00430578	1.0043058	27.80818	1.69	0.59	8.625	8.625	1	8.625	0.238125	8.04555001				
35	1.656699251	0.811782633	2.713946841	0.151981023	1.00445507	0.10044551	8.5	8.5	120	104	34.5	1.012781	1.012781498	1.0127815	28.04287	1.69	0.59	8.625	8.625	1	8.625	0.241168	8.35457286				
36	1.656699251	0.811782633	2.721803548	0.152420999	1.00732924	0.10073292	8.5	8.5	120	104	35.5	1.021084	1.021083506	1.0210835	28.27774	1.69	0.59	8.625	8.625	1	8.625	0.243145	8.66620765				
37	1.656699251	0.811782633	2.729464037	0.152849986	1.01013145	0.10101314	8.5	8.5	120	104	36.5	1.02922	1.029220112	1.0292201	28.49803	1.69	0.59	8.625	8.625	1	8.625	0.245083	8.98035287				
38	1.656699251	0.811782633	2.73693839	0.15326855	1.01286539	0.10128654	8.5	8.5	120	104	37.5	1.037199	1.03719901	1.037199	28.71896	1.69	0.59	8.625	8.625	1	8.625	0.246983	9.29659934				
39	1.656699251	0.811782633	2.744235916	0.153677211	1.01553449	0.10155345	8.5	8.5	120	104	38.5	1.045027	1.045027344	1.0450273	28.93572	1.69	0.59	8.625	8.625	1	8.625	0.248847	9.61598014				
40	1.656699251	0.811782633	2.751365234	0.154076453	1.01814191	0.10181419	8.5	8.5	120	104	39.5	1.052172	1.052171756	1.0521718	29.14489	1.69	0.59	8.625	8.625	1	8.625	0.250677	9.93737045				
41	1.656699251	0.811782633	2.758334337	0.154466723	1.02069058	0.10206906	8.5	8.5	120	104	40.5	1.060258	1.06025844	1.0602584	29.35745	1.69	0.59	8.625	8.625	1	8.625	0.252474	10.2610874				
42	1.656699251	0.811782633	2.765150652	0.154848437	1.02318324	0.10231832	8.5	8.5	120	104	41.5	1.067673	1.067673173	1.0676732	29.56276	1.69	0.59	8.625	8.625	1	8.625						

83	1.607611866	0.787729814	2.878254245	0.161182238	1.09595823	1.0959582	8.25			8.25	120	104	82.5	1.299263	1.29926323	1.2992632	35.97525	1.69	0.59	8.375	8.375	1	8.375	0.300419	24.827382
84	1.607611866	0.787729814	2.881766999	0.161379103	1.09727947	1.0972795	8.25			8.25	120	104	83.5	1.303743	1.30374349	1.3037435	36.0993	1.69	0.59	8.375	8.375	1	8.375	0.301455	25.2144507
85	1.607611866	0.787729814	2.885247572	0.161573864	1.09858655	1.0985865	8.25			8.25	120	104	84.5	1.308186	1.30818599	1.3081856	36.2223	1.69	0.59	8.375	8.375	1	8.375	0.302482	25.6028443
86	1.607611866	0.787729814	2.888688703	0.161766567	1.09987978	1.0998798	8.25			8.25	120	104	85.5	1.31259	1.312590307	1.3125903	36.34426	1.69	0.59	8.375	8.375	1	8.375	0.30305	25.9925517
87	1.607611866	0.787729814	2.892093905	0.161957259	1.10115947	1.1011595	8.25			8.25	120	104	86.5	1.316958	1.316958373	1.3169584	36.46521	1.69	0.59	8.375	8.375	1	8.375	0.30451	26.3856518
88	1.607611866	0.787729814	2.89546396	0.162145982	1.10242591	1.1024259	8.25			8.25	120	104	87.5	1.321291	1.321290517	1.3212905	36.58516	1.69	0.59	8.375	8.375	1	8.375	0.305512	26.775864
89	1.607611866	0.787729814	2.898799628	0.162332779	1.10367939	1.1036794	8.25			8.25	120	104	88.5	1.325587	1.325587439	1.3255874	36.70414	1.69	0.59	8.375	8.375	1	8.375	0.306506	27.1694476
90	1.607611866	0.787729814	2.902101642	0.162517692	1.10492019	1.1049202	8.25			8.25	120	104	89.5	1.32985	1.32984982	1.3298498	36.82216	1.69	0.59	8.375	8.375	1	8.375	0.307491	27.5643023
91	1.607611866	0.787729814	2.905370712	0.16270076	1.10614857	1.1061486	8.25			8.25	120	104	90.5	1.334078	1.334078317	1.3340783	36.93924	1.69	0.59	8.375	8.375	1	8.375	0.308469	27.960418
92	1.607611866	0.787729814	2.908607525	0.162882021	1.1073648	1.1073648	8.25			8.25	120	104	91.5	1.338274	1.338273571	1.3382736	37.0554	1.69	0.59	8.375	8.375	1	8.375	0.309439	28.3577846
93	1.607611866	0.787729814	2.911812747	0.163061514	1.10856913	1.1085691	8.25			8.25	120	104	92.5	1.342436	1.342436201	1.3424362	37.17066	1.69	0.59	8.375	8.375	1	8.375	0.310402	28.7563926
94	1.607611866	0.787729814	2.914987024	0.163239273	1.10976179	1.1097618	8.25			8.25	120	104	93.5	1.346567	1.346566811	1.3465668	37.28503	1.69	0.59	8.375	8.375	1	8.375	0.311357	29.1562321
95	1.607611866	0.787729814	2.91813098	0.163415335	1.11094033	1.1109403	8.25			8.25	120	104	94.5	1.350666	1.350666684	1.3506667	37.39854	1.69	0.59	8.375	8.375	1	8.375	0.312304	29.5752939
96	1.607611866	0.787729814	2.921245222	0.163589732	1.11211307	1.1121131	8.25			8.25	120	104	95.5	1.354734	1.35473429	1.3547343	37.51118	1.69	0.59	8.375	8.375	1	8.375	0.313245	29.99505687
97	1.607611866	0.787729814	2.924330336	0.163762499	1.11327213	1.1132721	8.25			8.25	120	104	96.5	1.358772	1.358772279	1.3587723	37.62299	1.69	0.59	8.375	8.375	1	8.375	0.314179	30.3630474
98	1.607611866	0.787729814	2.927386894	0.163933666	1.11442044	1.1144204	8.25			8.25	120	104	97.5	1.36278	1.36278049	1.3627805	37.73397	1.69	0.59	8.375	8.375	1	8.375	0.315106	30.7377211
99	1.607611866	0.787729814	2.930415448	0.164103265	1.11555819	1.1155582	8.25			8.25	120	104	98.5	1.366759	1.366759443	1.3667594	37.84415	1.69	0.59	8.375	8.375	1	8.375	0.316026	31.175811
100	1.607611866	0.787729814	2.933416535	0.164271326	1.1166856	1.1166856	8.25			8.25	120	104	99.5	1.37071	1.370709645	1.3707097	37.95352	1.69	0.59	8.375	8.375	1	8.375	0.316939	31.1078033
101	1.58582448	0.76367995	2.848598648	0.159521524	1.11780285	1.1178028	8			8	120	104	100.5	1.374632	1.374631591	1.3746316	38.06212	1.69	0.59	8.125	8.125	1	8.125	0.308358	31.0339351
102	1.58582448	0.76367995	2.851459385	0.159681726	1.11891014	1.1189101	8			8	120	104	101.5	1.378526	1.378525761	1.3785258	38.16994	1.69	0.59	8.125	8.125	1	8.125	0.309321	31.4130829
103	1.58582448	0.76367995	2.854294944	0.159840517	1.12000766	1.1200077	8			8	120	104	102.5	1.382393	1.382392622	1.3823926	38.27701	1.69	0.59	8.125	8.125	1	8.125	0.310099	31.8293489
104	1.58582448	0.76367995	2.857105789	0.159997924	1.12109558	1.1210956	8			8	120	104	103.5	1.386233	1.386232629	1.3862326	38.38334	1.69	0.59	8.125	8.125	1	8.125	0.31096	32.2287524
105	1.58582448	0.76367995	2.85998237	0.160153973	1.12217408	1.1221741	8			8	120	104	104.5	1.390046	1.390046226	1.3900462	38.48893	1.69	0.59	8.125	8.125	1	8.125	0.311816	32.6290207
106	1.58582448	0.76367995	2.862655126	0.160308687	1.12324334	1.1232433	8			8	120	104	105.5	1.393834	1.393833844	1.3938338	38.59381	1.69	0.59	8.125	8.125	1	8.125	0.312665	33.0307794
107	1.58582448	0.76367995	2.865394485	0.160462091	1.12430351	1.1243035	8			8	120	104	106.5	1.397596	1.397595905	1.3975959	38.69798	1.69	0.59	8.125	8.125	1	8.125	0.313509	33.4334919
108	1.58582448	0.76367995	2.868110861	0.160614208	1.12535476	1.1253548	8			8	120	104	107.5	1.401333	1.401332817	1.4013328	38.80145	1.69	0.59	8.125	8.125	1	8.125	0.314348	33.8371849
109	1.58582448	0.76367995	2.870804658	0.160765061	1.12639725	1.1263972	8			8	120	104	108.5	1.405045	1.405044981	1.405045	38.90423	1.69	0.59	8.125	8.125	1	8.125	0.315158	34.2420014
110	1.58582448	0.76367995	2.87347627	0.160914671	1.12743112	1.1274311	8			8	120	104	109.5	1.408733	1.408732787	1.4087328	39.00635	1.69	0.59	8.125	8.125	1	8.125	0.316007	34.6478842
111	1.58582448	0.76367995	2.876126079	0.16106306	1.12845653	1.1284565	8			8	120	104	110.5	1.412397	1.412396615	1.4123966	39.10779	1.69	0.59	8.125	8.125	1	8.125	0.316829	35.0548263
112	1.58582448	0.76367995	2.878754459	0.16121025	1.12947362	1.1294736	8			8	120	104	111.5	1.416037	1.416036835	1.4160368	39.20859	1.69	0.59	8.125	8.125	1	8.125	0.317646	35.462821
113	1.58582448	0.76367995	2.881361772	0.161356259	1.13048254	1.1304825	8			8	120	104	112.5	1.419654	1.41965381	1.4196538	39.30874	1.69	0.59	8.125	8.125	1	8.125	0.318457	35.8718614
114	1.58582448	0.76367995	2.883948373	0.161501109	1.13148341	1.1314834	8			8	120	104	113.5	1.423248	1.423247891	1.4232479	39.40825	1.69	0.59	8.125	8.125	1	8.125	0.319264	36.281941
115	1.58582448	0.76367995	2.886514606	0.161644481	1.13247639	1.1324764	8			8	120	104	114.5	1.426819	1.426819425	1.4268194	39.50715	1.69	0.59	8.125	8.125	1	8.125	0.320065	36.6930531
116	1.58582448	0.76367995	2.889060808	0.161787405	1.13346158	1.1334616	8			8	120	104	115.5	1.430369	1.430368748	1.4303687	39.60542	1.69	0.59	8.125	8.125	1	8.125	0.320861	37.1051914
117	1.58582448	0.76367995	2.891587306	0.161928889	1.13443913	1.1344391	8			8	120	104	116.5	1.433896	1.433896188	1.4338962	39.70309	1.69	0.59	8.125	8.125	1	8.125	0.321652	37.5183495
118	1.58582448	0.76367995	2.894094421	0.162069288	1.13540916	1.1354092	8			8	120	104	117.5	1.437402	1.437402066	1.4374021	39.80017	1.69	0.59	8.125	8.125	1	8.125	0.322439	37.9325212
119	1.58582448	0.76367995	2.896582463	0.162208618	1.13637178	1.1363718	8			8	120	104	118.5	1.440887	1.440886696	1.4408867	39.89665	1.69	0.59	8.125	8.125	1	8.125	0.323232	38.3477002
120	1.58582448	0.76367995	2.899051738	0.162346897	1.13732712	1.1373271	8			8	120	104	119.5	1.44435	1.444350385	1.4443504	39.99256	1.69	0.59	8.125	8.125	1	8.125	0.323997	38.7638805
121	1.58582448	0.76367995	2.901502542	0.162484142	1.13827533	1.1382753	8			8	120	104	120.5	1.447793	1.447793432	1.4477934	40.08789	1.69	0.59	8.125	8.125	1	8.125	0.32477	39.1810561
122	1.58582448	0.76367995	2.903935165	0.162620369	1.13921641	1.1392164	8			8	120	104	121.5	1.451216	1.451216129	1.4512161	40.18267	1.69	0.59	8.125	8.125	1	8.125	0.325537	39.5922111
123	1.58582448	0.76367995	2.90634989	0.162755594	1.14015059	1.1401506	8			8	120	104	122.5	1.454619	1.454618764	1.4546188	40.27688	1.69	0.59	8.125	8.125	1	8.125	0.326301	40.0183698
124	1.58582448	0.76367995	2.908746993	0.162889832	1.14107792	1.1410779	8			8	120	104	123.5	1.458002	1.458001615	1.4580016	40.37055	1.69	0.59	8.125	8.125	1	8.125	0.327059	40.4384963
125	1.58582448	0.76367995	2.911126743	0.163023098	1.14199852	1.1419985	8			8	120	104	124.5	1.461365	1.461364957										

User Input				Axial Strength		Flexure Strength		Combined Loads				
Overall height	250	ft =	3000	in	Factored Load	748.6849367	Factored Load	126635.85	Combined Loading	0.875381	≤1.0	OK
Diameter at Base	8.5	ft=	102	in	Factored Strength	1811.937034	Factored Strength	249117.53	Pu/Pc	0.413196		
Shell Thickness	0.75	in			Pn	2131.690628	Yielding Mn	276797.25	For Pu/Pc≥0.2	0.875381		
Strength of Steel	36	ksi			Fcr	8.935482487	Local Buckling	242648.892	For Pu/Pc<0.2	0.668783		
Modulus of Elasticity	29000	ksi			KL/r	167.6058981	Check for Compact section:		Magnification Factors			
					4.71*sqrt(E/Fy)	133.680683	D/t	136	B2	1.022859		
ID	100.5				Fe	10.18869155	λp	56.38888889	Pnt	748.6849		
S	5994.595697				Fcr for KL/r≤4.71*sqrt(E/Fy)	8.204172194	λc	249.7222222	Pe2	33501.45		
Z	7688.8125				Fcr for KL/r>4.71*sqrt(E/Fy)	8.935482487	non-compact		ΣH	78.83		
r	35.79826288				Ag	238.5646921	Flexure Strength:	OK	ΔH	6		
l	305724.3805				Check for Compact section:				L	3000		
					D/t	136						
					λp							
					λc	88.61111111						
					noncompact							
					Axial Strength:	OK						

Appendix E: Foundation Design Sheets and Hand Calculations

Design of a Square Spread Footing

$$P_u = 744.43 \text{ K} \quad M_u = 6956.04 \text{ ft-K}$$

column Base Diameter, $D = 8 \text{ ft}$

$$q_{\text{allowable}} = 4500 \text{ psf}$$

$$f'_c = 3000 \text{ psi}$$

$$f_y = 40,000 \text{ psi}$$

a) Estimate footing pressure

Assume 3 ft footing $\therefore 450 \text{ psf}$

$$p_{\text{net}} = 4500 - 450 = 4050 \text{ psf}$$

$$\begin{aligned} &\rightarrow \text{check } q_{\text{equiv}} \leq q_a \\ q_{\text{equiv}} &= \frac{P + W_f}{B^2} = \frac{744.43 \text{ K} + 88.2 \text{ K}}{14^2} \end{aligned}$$

$$q_{\text{equiv}} = 4400 \text{ psf}$$

$$4400 \text{ psf} \leq q_a = 4500 \text{ psf}$$

$$\text{required } A = \frac{DL}{p_{\text{net}}} = \frac{744.43 \text{ K}}{4.05 \text{ Ksf}} = 183.81 \text{ ft}^2 \quad \underline{\text{OK}} \quad 4500 \text{ psf}$$

$$\sqrt{183.81} = 13.56 \text{ ft} \rightarrow \text{use } 14 \text{ ft}$$

$$W_f = 450 \text{ lb/ft}^2 \times 14^2$$

$$W_f = 88.2 \text{ K}$$

b) check eccentricity

$$e = \frac{M_u}{P_u + W_f} = \frac{6956.04}{744.43 + (14' \times 450)} = 0.078 \text{ ft}$$

$$B/6 = \frac{14}{6} = 2.33 \text{ ft} \quad e < B/6 \quad \underline{\text{OK}}$$

c) Determine depth based on shear

$$\text{avg } d = 36 - 3 (\text{over}) - 2 (\text{conserv. bar diameter}) = 31 \text{ in}$$

$$V_u = p_{\text{net}} \times \text{area} = 4.05 \text{ Ksf} (14^2) = 793.8 \text{ K}$$

$$b_o \Rightarrow 2\pi r = 25.13 \text{ ft} - 2(1/2) = 271 \text{ in}$$

ACI (11-33)

$$V_c = \left(2 + \frac{4}{1}\right) \sqrt{f'_c} b_o d = (2+4) \sqrt{3000} (271) (31) \frac{1}{1000}$$

$$V_c = 2760.92 \text{ K}$$

$$V_c = \left(\frac{d_s}{b_o d} + 2\right) (\sqrt{f'_c}) b_o d = \left(\frac{40}{271/31} + 2\right) \sqrt{3000} (271 \times 31) \frac{1}{1000}$$

$d_s = 40$ for column

$$V_c = 3025.75 \text{ K}$$

$$V_c = 4 \sqrt{f'_c} b_o d = 4 \sqrt{3000} (271 \times 31) \frac{1}{1000}$$

$$V_c = 1840.61 \text{ K} \quad \times \text{ smallest governs}$$

Two Way Shear

$B_c = \frac{14'}{14'} = 1 \leq 2$ OK $b_o/d = 271''/31'' = 8.74 < 20$ OK

$\phi V_c = (0.75)(1840.61) = 1380.46 \rightarrow V_u = 793.8k$
OK

one way shear

$V_u = 405(0.42)(14')$

$V_u = 23.6k$

ACI-11.2.1.1 and 11.3.1.1

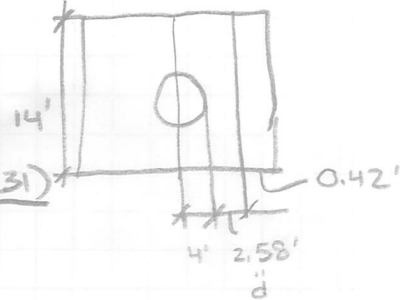
$V_c = \frac{2\sqrt{f'_c}bw d}{1000} = \frac{2\sqrt{3000}(14)(2)(31)}{1000}$

$V_{cu} = 570.51k$

$\phi V_c = 0.75(570.51k) = 427.88k > V_u = 23.6k$

OK

*NO Shear reinforcement needed.



CAMPAD

Bearing Strength

Design of baseplate

cont. Next page

DESIGN OF BASEPLATE - try 128" x 128"

CHECK BEARING PRESSURE:

$$\phi_c P_p = \phi_c 0.85 f'_c A_1 \sqrt{\frac{A_2}{A_1}} \leq \phi 1.7 f'_c A_1 \quad > P_u = 774.4 \text{ k}$$

$$\phi_c = 0.60$$

$$f'_c = 3 \text{ ksi}$$

$$A_1 = \text{Area of baseplate, try } 128" \times 128", A_1 = 16384 \text{ in}^2$$

$$A_2 = \text{Area of concrete footing} = 36864 \text{ in}^2$$

$$[\phi_c P_p = (0.60)(0.85)(3 \text{ ksi})(16384 \text{ in}^2) \sqrt{\frac{36864}{16384}} = 37601.2 \text{ k} \leq$$

$$[(0.6)(1.7)(3 \text{ ksi})(16384) = 50135 \text{ k}]$$

use $[\phi_c P_p = 37601 \text{ k}] > [P_u = 774 \text{ k}]$ OK

CHECK OVERTURNING MOMENT:

$$f_{max} \leq \phi_c P_p / A_1$$

$$f_{max} = -\frac{P_u}{A_1} + \frac{6 M_u}{bd^2}$$

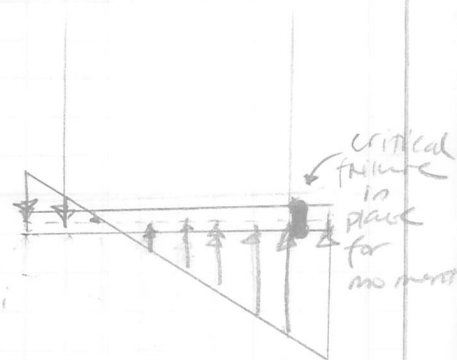
$$P_u = 774.4 \text{ k}$$

$$A_1 = 16384 \text{ in}^2$$

$$M_u = 6956 \text{ ft-kips} \cdot 12" = 83472 \text{ in-kips}$$

$$b = d = 128"$$

$$f_{max} = \frac{-774.4 \text{ k}}{16384 \text{ in}^2} + \frac{6(83472)}{128^3} = \begin{cases} -0.286 \text{ ksi} \\ 0.192 \text{ ksi} \end{cases}$$



$[f_{max} = 0.286 \text{ ksi}] \leq [\phi_c P_p / A_1 = 2.15 \text{ ksi}]$ ✓ OK

DESIGN THICKNESS:

$$m = 0.5(N - 0.8d)$$

$$m = 0.5(128 - 0.8(96)) = 25.6"$$

$$n' = 0.25d$$

$$n' = 0.25(96) = 24"$$

$$\lambda = \frac{2\sqrt{x}}{1 + \sqrt{1-x}} \leq 1$$

$$x = \frac{P_u}{\phi_c P_p} = \frac{774.4 \text{ k}}{32521 \text{ k}} = 0.022$$

$$\lambda = \frac{2\sqrt{0.022}}{1 + \sqrt{1-0.022}} = 0.15 \leq 1$$

$$\lambda n' = 3.51"$$

$$t_{min} = L / \sqrt{2 P_u / (0.9 F_y N^2)}$$

$$L = \max(m, 2r'); \quad L = 25.6'$$

$$P_u = 774.4 \text{ k}$$

$$F_y = 36 \text{ ksi}$$

$$N^2 = 128''$$

$$0.192 \left| \begin{array}{c} \triangle \\ x \end{array} \right. \quad 0.432 \left| \begin{array}{c} \triangle \\ 128'' \end{array} \right.$$

$$\frac{0.192}{x} = \frac{0.432}{128} \quad x = 50.99''$$

$$t_{min} = 25.6 \sqrt{2 (774.4 \text{ k}) / (0.9 (36 \text{ ksi}) (128'')^2)} = 1.36'' \quad , \text{Try } t = 1.5''$$

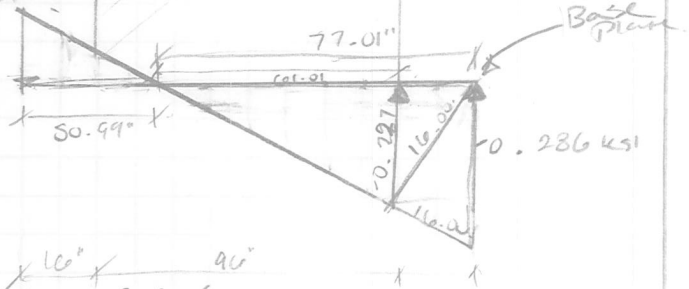
CHECK FOR FLEXURAL STRENGTH:

$$t \geq \sqrt{\frac{6 M_u}{\phi_b F_y}}$$

M_u = Moment to right @ center of right flange

$$= (16.00) \left(\frac{16''}{3} \right) + (16.00) \left(\frac{2}{3} \cdot 16.00 \right) = 256 \text{ ft-k}$$

0.192 ksi



$$\frac{77.01''}{-0.286} = \frac{61.01''}{x}$$

$$\phi_b = 0.9$$

$$F_y = 36 \text{ ksi}$$

$$t \geq \sqrt{\frac{6 (256)}{0.9 (36)}} = 6.88'' \rightarrow \text{use } \underline{\underline{7''}}$$

Final design: 128'' x 128'' x 7''

Design for Bending moment strength

$$\text{Required } R_n = \frac{m_u}{\phi b d^2} = \frac{6956.04 (12000)}{(69)(168)^2 (31^2)} = 574.47 \text{ psi}$$

section 3.8 eqn. (3.8.5) Wang, Salmon

$$p = \frac{1}{m} \left(1 - \sqrt{1 - \frac{2mR_n}{f_y}} \right) \quad m = \frac{40000}{0.85(3000)} = 15.7$$

$$p = \frac{1}{15.7} \left(1 - \sqrt{1 - \frac{2(15.7)(574.4)}{40000}} \right) = 0.015$$

$$\text{required } A_s = p b d = 0.015 (168)(31) = 85.91 \text{ in}^2$$

$$\text{min required } A_s = 0.002 (168)(36) = 12.10 \text{ in}^2$$

Bar size try # 9

$$\text{Try } 8\#9 \quad A_s = 86 \text{ in}^2$$

Check bar spacing

$$\hookrightarrow \frac{168}{86} = 1.95 \text{ in } \text{OK} \checkmark$$

$$\text{New } d = 36 - 3 - \underset{\substack{\uparrow \\ d_b}}{1.13} \text{ in} = 31.87 \text{ in}$$

check for balanced cond.
 $\phi_c = 0.85$ for $f'_c = 3000 \text{ psi}$
 $p_b = \frac{0.85 \cdot 3000}{40000} (0.85) \left(\frac{87000}{87000 + f_y} \right)$
 $p_b = 0.037$
 $p \leq 0.75 p_b$
 $p \leq 0.75 (0.037)$
 $0.02 \leq 0.028 \checkmark$
OK

$$C = 0.85 f'_c b a = 0.85 (3)(14)(12) a = 428.4 a$$

$$T = A_s f_y = 86(40) = 3440 \text{ K}$$

$$a = 8.03 \text{ in}$$

$$\phi M_n = 0.9 (3440) \left(\left[31.87 - 0.5(8.03) \right] \frac{1}{12} \right)$$

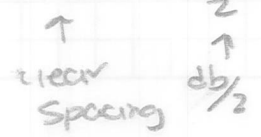
$$\phi M_n = 7186.6 \text{ K} > \phi 6956.04 \text{ ft-K}$$

OK

Development of reinforcement

bottom + side cover :

$$1.5 + \frac{1.13}{2} = 2.06 \text{ in}$$



one half center to center spacing

$$1.95''/2 = 0.98 \text{ in} \leftarrow c_b \text{ smaller}$$

$$c_b = 0.98 \text{ in. NO stirrups } k_{tr} = 0$$

$$\text{ACI 12-1. } L_d = \left(\frac{3}{40} \frac{f_y}{\sqrt{f_c}} \frac{\psi + \psi_e \psi_s \lambda}{\left(\frac{c_b + k_{tr}}{d_b} \right)} \right) d_b =$$

$$\frac{c_b + k_{tr}}{d_b} = \frac{0.98 + 0}{1.13} = 0.867 < 2.5 \text{ max } \underline{\text{OK}}$$

$$L_d = \frac{3}{40} \frac{40,000}{\sqrt{3000}} \frac{1 \times 1 \times 1 \times 1}{0.867} = 71.35 \text{ in}$$

$$\text{actual embedment} = (14' - 8') \frac{12''}{1''} - 2 \text{ in} = 34 \text{ in} < L_d$$

↑
cover

NO good

Recheck Try increasing side length to 16'

check eccentricity: $\rightarrow q_{\text{egun}} = \frac{774.40 + (450 \times 16 \times 16)}{16} = 3475 \text{ psf} \times$

$$e = \frac{M_u}{P_u W_{eff}} = \frac{6956.04}{744.43 \times (16^2 \times 450)} = 0.06 \text{ ft}$$

$$B/c = 16/6 = 2.67 \text{ ft} \quad e < B/c \quad \underline{\text{OK}}$$

Determine depth based on shear:

$$\text{avg } d = 36 - 3(\text{cover}) - 2(\text{conser. } d_b) = 31 \text{ in}$$

$$V_u = p_{\text{net}} \times \text{area} = 4.05 \text{ ksf} (16^2) = 1036.8 \text{ k}$$

$$b_o \approx 2\pi r = 25.13 \text{ ft} - 31/12 = 271 \text{ in}$$

$$V_c = (2 + 4) \sqrt{3000} (271)(31) \frac{1}{1000} = 2760.92 \text{ k}$$

$$V_c = \left(\frac{40}{271/31} + 2 \right) \sqrt{3000} (271 \times 31) \frac{1}{1000} = 3025.75 \text{ k}$$

$$V_c = 4 \sqrt{f_c} b_o d = 4 \sqrt{3000} (271 \times 31) \frac{1}{1000}$$

$$V_c = 1840.61 \text{ k}$$

*smallest governs

TWO-way shear (same)

$B_c = 1 \leq 2$ OK $b_o/d = 271"/31" = 8.74 < 20$ OK

$\phi V_c = (0.75)(1840.61) = 1380.46 \text{ k} > V_u = 793.8 \text{ k}$ OK

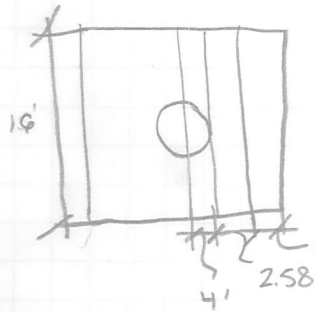
check one-way shear

$V_u = 4.05(1.42)(16') = 92 \text{ k}$

$V_c = \frac{2\sqrt{3000}(16)(12)(31)}{1000}$

$V_{cu} = 652 \text{ k}$

$\phi V_{cu} = 0.75(652 \text{ k}) = 489 \text{ k}$ OK



* No shear reinforcement

Design of Baseplate → same

Design of Bending Moment strength

Required $R_n = \frac{6956.04(12000)}{(0.9)(192)(31^2)} = 502.7 \text{ psi}$

$m = 15.7$ (same)

check balanced cond.

$\sqrt{0.85} \rho_b = 0.028$

$\rho = \frac{1}{15.7} \left(1 - \sqrt{1 - \frac{2(15.7)(502.6)}{40,000}} \right) = 0.0141 < 0.028$ OK

Required $A_s = \rho b d = (0.0141)(192)(31) = 84.12 \text{ in}^2$

Min reqd $A_s = 0.002(192)(36) = 13.824 \text{ in}^2$

Try #9 bars → 85 ⇒ $A_s = 85 \text{ in}^2$

check bar spacing

$\frac{192 \text{ in}}{85 \text{ bars}} = 2.26 \text{ in/bar} > d_b =$ OK

New $d = 36 - 3 - 1.13 = 31.87 \text{ in}$

$C = 0.85 f'_c b a = 0.85(3)(16)(12)a = 489.6a$

$T = A_s F_y = 85(40) = 3400 \text{ k}$

$a = 6.94 \text{ in}$

$$\phi M_n = 0.9 (3400) (31.87 - 0.5 (6.94)) \frac{1}{12}$$

$$\phi M_n = 7242 \text{ k} > 69.56, 04 \text{ k}$$

OK

Development of reinforcement

$$\text{bottom + side cover} = 1.5 + \frac{1.13}{2} = 2.06 \text{ in}$$

$$\frac{1}{2} \text{ cto c spacing} = 2.26/2 = 1.13 \text{ in} \leftarrow \text{cb b/c smaller}$$

$$c_b = 1.06 \text{ No stirrups } k_{tr} = 0$$

$$L_d = \left(\frac{3}{40} \frac{40,000}{\sqrt{3000}} \frac{1 \times 1 \times 1.3 \times 1}{1.01} \right) 1.13 = 80.5 \text{ in}$$

$$\frac{1.13}{1.13} = 1$$

$$\text{Actual embedment} = (16-8) (12''/1'') - 2 \text{ in (cover)}$$

$$= 94 \text{ in} > L_d$$

OK

check shear w/ new d

$$d = 31.87 \text{ in}$$

$$V_u = 1036.8 \text{ k}$$

$$b_o = 2\pi r - d_b$$

$$= 2\pi(4) - 31.87/2'' = 270.13 \text{ in}$$

$$V_c = \left(2 + \frac{4}{1}\right) \sqrt{f'_c} b_o d = (2+4) (\sqrt{3000}) (270.13 \times 31.87) \frac{1}{1000}$$

$$V_c = 2829.2 \text{ k}$$

$$V_c = \left(\frac{0.9}{k/d} + 2\right) (\sqrt{f'_c}) b_o d = \left(\frac{40}{270.13/31.87} + 2\right) (\sqrt{3000}) (270.13 \times 31.87) \frac{1}{1000}$$

$$V_c = 3168.35 \text{ k}$$

$$V_c = 4\sqrt{3000} (270.13 \times 31.87) \frac{1}{1000}$$

$$V_c = 1886.2 \text{ k} \quad * \text{ smallest governs}$$

Two-way shear

$B_c = \frac{16'}{16'} = 1 \leq 2$ OK $b_o/d = 270.13/31.87 = 8.5'' < 20''$

$\phi V_c = (0.75)(1886.3) = 1414.65 \text{ k} > V_u = 1036.8 \text{ k}$ OK

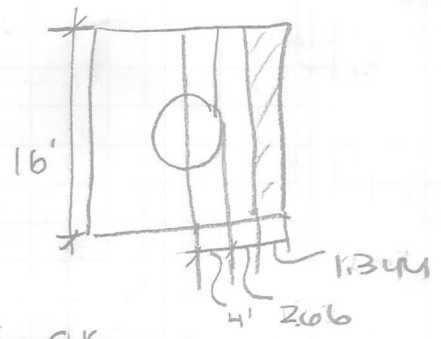
oneway shear

$V_u = 4.05(1.344)(16') = 87.09 \text{ k}$

$V_c = \frac{2\sqrt{3000}(16)(12)(31.87)}{1000}$

$V_c = 670.3 \text{ k}$

$\phi V_c = 0.75(670.3) = 502.76 \text{ k}$ OK



*NO shear reinforcement needed

Design complete.

use 16' x 16' x 3' concrete footing
with 85 # 9 @ 2 1/4" spacing

CAMPAD

250' Tower

Initial Inputs

Axial Load, Pu	744.43	k
Moment, Mu	6956.04	ft-k
Column Base Diameter, D	8.00	ft
Allowable Bearing Stress, qa	4500.00	psf
f'c	3000.00	psi
fy	40000.00	psi

Determine Footing Size

Footing Depth	3.00	ft	
Weight of Footing	450.00	psf	
p net	4050.00	psf	
Required Area	183.81	ft ²	
Suggested side length	13.56	ft	
Chosen side length	16.00	ft	OK q equiv < qa

Check Eccentricity

Eccentricity, e	0.06	ft	
B/6	2.67	ft	OK

Determine Depth based on Shear

average d	31.00	in
Vu	1036.80	k
bo	271.01	in
Bc	1.00	(for squares)
Alpha s	40.00	(for columns)

Choose smallest of:

Vc	2760.92	k
Vc	3025.75	k
Vc	1840.61	k
Governing Vc	1840.61	k

Check Two-Way Shear

Bc	1.00	OK	
bo/d	8.74	OK	
Shear Capacity (phi c, Vc)	1380.46	OK	No Shear Reinforcement

Check One-Way Shear

Vu	91.80	k	
Vcu	652.01	k	
Phi*Vc	489.01	OK	No Shear Reinforcement

Design of Baseplate

Try base plate width	128.00	in	OK
----------------------	--------	----	----

Check Bearing Pressure:

$\Phi_c P_p$	37601.28	>	744.4	OK
$\Phi_c * 0.85 * f'_c * A_1 * \sqrt{A_2/A_1}$	37601.28	k	3	
$1.7 * f'_c * A_1$	50135.04	k		
fmax	-0.28	ksi		OK
Mu	256.00	in-k		
tmin	6.89	in		
Choose t	7	in		OK

Design for Bending Moment Strength

Required Rn	502.66	psi
m	15.69	
p	0.014	

Check balanced condition

pb	0.028		OK
Required As	84.12	in ²	
Min Required As	13.82	in ²	
Bar Size, #	9.00		
Diameter, db	1.13	in	
Area, a	1.00	in ²	
Suggested # of bars, As/a	84.12		
Chosen # of Bars	85.00		
As	85.00	in ²	

Check Bar Spacing

Calculated bar spacing	2.26	in/bar	OK
Actual bar spacing	2.26	in/bar	
New d	31.87	in	
Compressive Strength, C	489.60	a	
Tensile Strength, T	3400.00	k	
a	6.94	in	
Phi Mn	7241.94	ft-k	OK

Development of Reinforcement

bottom and side cover	2.06	in	
one half c-c spacing	1.13	in	
cb	1.13	in	
Cb/db	1.00		OK
Actul Cb/db	1.00		

Ld	80.22	in	
Actual Embedment	94.00	in	OK

Check Shear with New d

average d	31.87	in
Vu	1036.80	k
bo	270.13	in
Bc	1.00	(for squares)
Alpha s	40.00	(for columns)

Choose smallest of:

Vc	2829.44	k
Vc	3168.71	k
Vc	1886.30	k
Governing Vc	1886.30	k

Check Two-Way Shear

Bc	1.00	OK
bo/d	8.48	OK
Shear Capacity (phi c, Vc)	1414.72	k

OK No Shear Reinforcement

Check One-Way Shear

Vu	87.09	k
Vcu	670.35	k
Phi*Vc	502.76	k

OK No Shear Reinforcement

Appendix F: Foundation Design Alternatives

350' Tower

Initial Inputs

Axial Load, Pu	1026.88	k
Moment, Mu	21637.72	ft-k
Column Base Diameter, D	12.00	ft
Allowable Bearing Stress, qa	4500.00	psf
f'c	3000.00	psi
fy	40000.00	psi

Determine Footing Size

Footing Depth	6.00	ft	
Weight of Footing	900.00	psf	
p net	3600.00	psf	
Required Area	285.24	ft ²	
Suggested side length	16.89	ft	
Chosen side length	24.00	ft	OK q equiv < qa

Check Eccentricity

Eccentricity, e	0.04	ft	
B/6	4.00	ft	OK

Determine Depth based on Shear

average d	67.00	in
Vu	2073.60	k
bo	386.01	in
Bc	1.00	(for squares) (for columns)
Alpha s	40.00	columns)

Choose smallest of:

Vc	8499.33	k
Vc	12668.02	k
Vc	5666.22	k
Governing Vc	5666.22	k

Check Two-Way Shear

Bc	1.00	OK	
bo/d	5.76	OK	
Shear Capacity (phi c, Vc)	4249.67	OK	No Shear Reinforcement

Check One-Way Shear

Vu	36.00	k	
Vcu	2113.77	k	
Phi*Vc	1585.33	k	OK No Shear Reinforcement

Design of Baseplate

Try base plate width	180.00	in	OK
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Check Bearing Pressure:

$\Phi_c P_p$	79315.20	>	1026.88	OK
$\Phi_c * 0.85 * f'_c * A_1 * \sqrt{A_2/A_1}$	79315.20	k		
$1.7 * f'_c * A_1$	99144.00	k		
fmax	-0.30	ksi	OK	
Mu	324.01	in-k		
tmin	7.75	in		
Choose t	8	in	OK	

Design for Bending Moment Strength

Required Rn	223.16	psi
m	15.69	
p	0.006	

Check balanced condition

pb	0.028	OK
Required As	112.82	in^2
Min Required As	41.47	in^2
Bar Size, #	9.00	
Diameter, db	1.13	in
Area, a	1.00	in^2
Suggested # of bars, As/a	112.82	
Chosen # of Bars	113.00	
As	113.00	in^2

Check Bar Spacing

Calculated bar spacing	2.55	in/bar	OK
Actual bar spacing	2.55	in/bar	
New d	67.87	in	
Compressive Strength, C	734.40	a	
Tensile Strength, T	4520.00	k	
a	6.15	in	
Phi Mn	21965.39	ft-k	OK

Development of Reinforcement

bottom and side cover	2.06	in	
one half c-c spacing	1.27	in	
cb	1.27	in	
Cb/db	1.13	OK	
Actul Cb/db	1.13		
Ld	71.09	in	
Actual Embedment	142.00	in	OK

Check Shear with New d

average d	67.87	in
Vu	2073.60	k
bo	385.14	in
Bc	1.00	(for squares) (for columns)
Alpha s	40.00	columns)

Choose smallest of:

Vc	8590.50	k
Vc	12956.07	k
Vc	5727.00	k
Governing Vc	5727.00	k

Check Two-Way Shear

Bc	1.00	OK	
bo/d	5.67	OK	
Shear Capacity (phi c, Vc)	4295.25	OK	No Shear Reinforcement

Check One-Way Shear

Vu	29.72	k	
Vcu	2141.28	k	
Phi*Vc	1605.96	k	OK No Shear Reinforcement

250' Tower with Clayey Soil

Initial Inputs

Axial Load, Pu	744.43	k
Moment, Mu	6956.04	ft-k
Column Base Diameter, D	8.00	ft
Allowable Bearing Stress, qa	2000.00	psf
f'c	3000.00	psi
fy	40000.00	psi

Determine Footing Size

Footing Depth	3.00	ft	
Weight of Footing	450.00	psf	
p net	1550.00	psf	
Required Area	480.28	ft ²	
Suggested side length	21.92	ft	
Chosen side length	22.00	ft	OK q equiv < qa

Check Eccentricity

Eccentricity, e	0.03	ft	
B/6	3.67	ft	OK

Determine Depth based on Shear

average d	31.00	in
Vu	750.20	k
bo	271.01	in
Bc	1.00	squares)
Alpha s	40.00	columns)

Choose smallest of:

Vc	2760.92	k
Vc	3025.75	k
Vc	1840.61	k
Governing Vc	1840.61	k

Check Two-Way Shear

Bc	1.00		OK
bo/d	8.74		OK
Shear Capacity (phi c, Vc)	1380.46	k	OK No Shear Reinforcement

Check One-Way Shear

Vu	150.61	k	
Vcu	896.51	k	
Phi*Vc	672.38	k	OK No Shear Reinforcement

Design of Baseplate

Try base plate width	128.00	in	OK
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Check Bearing Pressure:

$\Phi_c P_p$	50135.04	>	744.4	OK
$\Phi_c * 0.85 * f'_c * A_1 * \sqrt{A_2/A_1}$	51701.76	k	3	
$1.7 * f'_c * A_1$	50135.04	k		
fmax	-0.28	ksi		OK
Mu	256.00	in-k		
tmin	6.89	in		
Choose t	7	in		OK

Design for Bending Moment Strength

Required Rn	365.57	psi
m	15.69	
p	0.010	

Check balanced condition

pb	0.028		OK
Required As	81.10	in ²	
Min Required As	19.01	in ²	
Bar Size, #	9.00		
Diameter, db	1.13	in	
Area, a	1.00	in ²	
Suggested # of bars, As/a	81.10		
Chosen # of Bars	82.00		
As	82.00	in ²	

Check Bar Spacing

Calculated bar spacing	3.22	in/bar	OK
Actual bar spacing	3.22	in/bar	
New d	31.87	in	
Compressive Strength, C	673.20	a	
Tensile Strength, T	3280.00	k	
a	4.87	in	
Phi Mn	7241.23	ft-k	OK

Development of Reinforcement

bottom and side cover	2.06	in	
one half c-c spacing	1.61	in	
cb	1.61	in	
Cb/db	1.43		OK
Actul Cb/db	1.43		

Ld	56.28	in	
Actual Embedment	166.00	in	OK

Check Shear with New d

average d	31.87	in
Vu	750.20	k
bo	270.13	in
Bc	1.00	(for squares)
Alpha s	40.00	(for columns)

Choose smallest of:

Vc	2829.44	k
Vc	3168.71	k
Vc	1886.30	k
Governing Vc	1886.30	k

Check Two-Way Shear

Bc	1.00	OK	
bo/d	8.48	OK	
Shear Capacity (phi c, Vc)	1414.72	k	OK

No Shear Reinforcement

Check One-Way Shear

Vu	148.13	k	
Vcu	921.73	k	
Phi*Vc	691.30	k	OK

No Shear Reinforcement

250' Tower with Rocky Soil

Initial Inputs

Axial Load, Pu	744.43	k
Moment, Mu	6956.04	ft-k
Column Base Diameter, D	8.00	ft
Allowable Bearing Stress, qa	6500.00	psf
f'c	3000.00	psi
fy	40000.00	psi

Determine Footing Size

Footing Depth	4.00	ft	
Weight of Footing	600.00	psf	
p net	5900.00	psf	
Required Area	126.17	ft^2	
Suggested side length	11.23	ft	
Chosen side length	14.00	ft	OK q equiv < qa

Check Eccentricity

Eccentricity, e	0.06	ft	
B/6	2.33	ft	OK

Determine Depth based on Shear

average d	43.00	in
Vu	1156.40	k
bo	259.01	in
Bc	1.00	squares)
Alpha s	40.00	columns)

Choose smallest of:

Vc	3660.08	k
Vc	5270.98	k
Vc	2440.05	k
Governing Vc	2440.05	k

Check Two-Way Shear

Bc	1.00		OK
bo/d	6.02		OK
Shear Capacity (phi c, Vc)	1830.04	k	OK No Shear Reinforcement

Check One-Way Shear

Vu	-48.18	k	
Vcu	791.35	k	
Phi*Vc	593.51	k	OK No Shear Reinforcement

Design of Baseplate

Try base plate width	128.00	in	OK
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Check Bearing Pressure:

$\Phi_c P_p$	32901.12	>	744.4	OK
$\Phi_c * 0.85 * f'_c * A_1 * \sqrt{A_2/A_1}$	32901.12	k	3	
$1.7 * f'_c * A_1$	50135.04	k		
fmax	-0.28	ksi		OK
Mu	256.00	in-k		
tmin	6.89	in		
Choose t	7	in		OK

Design for Bending Moment Strength

Required Rn	298.58	psi
m	15.69	
p	0.008	

Check balanced condition

pb	0.028		OK
Required As	57.51	in ²	
Min Required As	16.13	in ²	
Bar Size, #	9.00		
Diameter, db	1.13	in	
Area, a	1.00	in ²	
Suggested # of bars, As/a	57.51		
Chosen # of Bars	58.00		
As	58.00	in ²	

Check Bar Spacing

Calculated bar spacing	2.90	in/bar	OK
Actual bar spacing	2.90	in/bar	
New d	43.87	in	
Compressive Strength, C	428.40	a	
Tensile Strength, T	2320.00	k	
a	5.42	in	
Phi Mn	7162.58	ft-k	OK

Development of Reinforcement

bottom and side cover	2.06	in	
one half c-c spacing	1.45	in	
cb	1.45	in	
Cb/db	1.28		OK
Actul Cb/db	1.28		

Ld	62.56	in		
Actual Embedment	70.00	in	OK	
Check Shear with New d				
average d	43.87	in		
Vu	1156.40	k		
bo	258.13	in		
Bc	1.00	(for squares)		
Alpha s	40.00	(for columns)		
Choose smallest of:				
Vc	3721.73	k		
Vc	5457.50	k		
Vc	2481.16	k		
Governing Vc	2481.16	k		
Check Two-Way Shear				
Bc	1.00		OK	
bo/d	5.88		OK	
Shear Capacity (phi c, Vc)	1860.87	k	OK	No Shear Reinforcement
Check One-Way Shear				
Vu	-54.19	k		
Vcu	807.40	k		
Phi*Vc	605.55	k	OK	No Shear Reinforcement

Appendix G: Anchor Bolt Design

DESIGN OF ANCHOR BOLTS:

Use standard steel rods, cut to length and threaded + ASTM A490 rods. Tensile capacity = 114 ksi; shear capacity = 60 ksi

Try 20 - 1 1/2" rods.

Minimum Embedment depth: $12(d) = 12(1.5") = 18"$

Minimum Edge distance: Max of $5(d) = 5(1.5") = 7.5"$; 4"

Design for tensile strength: (ASCE-7-05 Table 7-2)

$$T_u = \frac{P_u}{n} = \frac{M_x d_i A_b}{I_x}$$

$$P_u = 774.4 \text{ k}$$

$$n = 20$$

$$A_b = 1.77 \text{ in}^2$$

$$M_x = 6956.0 \text{ ft-kips}$$

$$d_i = \phi_{\text{tracer}}/2 + \text{min edge dist}; d_i = 96/2 + 7.5" = 55.5"$$

$$I_x = \sum_{i=1}^n A_b d_i^2$$

$$A_b = 1.77; d_i \text{ (varies - found dist. using AutoCad.)}$$

$$I_x = 1.77 [55.5^2 + 2(52.78^2) + 2(44.90^2) + 2(32.62^2) + 2(17.15^2)]$$

$$I_x = 27260 \text{ in}^4$$

$$T_u = \frac{774.4 \text{ k}}{20} = \frac{(6956 \text{ ft-kip})(12")}{27260} (55.5)(1.77 \text{ in}^2) = 262.1 \text{ ksi tension}$$

$$262.1 \text{ ksi} > 148 \text{ ksi} \quad \checkmark \text{ OK}$$

Find ϕR_n for combined shear + tension:

$$\phi R_n = \phi F_{nt} A_b$$

$$F_{nt} = 1.3 F_{nt} - \frac{F_{nv}}{\phi F_{nv}} f_v \leq F_{nt}$$

$$F_{nt} = 113 \text{ ksi}$$

$$\text{Table 3.2} \left\{ \begin{array}{l} F_{nv} = 75 \text{ ksi} \\ \phi = 0.75 \end{array} \right.$$

$$f_v = \frac{V_u}{n A_b} = \frac{52.0 \text{ k}}{(20)(1.77)} = \text{from 25' frame design}$$

$$f_v = 1.469 \text{ ksi}$$

$$F_{nt} = 1.3(113) - \frac{(113)}{(0.75)(75)} \times 1.469 \text{ ksi}$$

$$F_{nt} = 143.4 \text{ ksi} \not\leq [F_{nt} = 113 \text{ ksi}]; \text{ use } F_{nt} = 113 \text{ ksi.}$$

$$\phi R_n = (0.75)(113 \text{ ksi})(1.77) = 150 \text{ ksi}$$

* Shear stress is minimal, therefore neglected when checking for combined shear + tensile stress.

$$\phi R_n \geq T_u$$

$$[\phi R_n = 150 \text{ ksi}] \not\geq [T_u = 201.1 \text{ ksi}] \quad \text{NG.}$$

Try 24 - $1\frac{1}{2}$ " rods. (for 2-way symmetry)

$$T_u = \frac{P_u}{n} - \frac{M_x d_i A_b}{I_x}$$

$$P_u = 774.4 \text{ k}$$

$$n = 24$$

$$A_b = 1.77$$

$$M_x = 6956.0 \text{ ft-kips}$$

$$d_i = 55.5$$

$$I_x = 32712 \text{ in}^4$$

$$T_u = \frac{774.4}{24} - \frac{(6956)(12)(55.5)(1.77)}{32712} = 218 \text{ k} > \phi R_n; \text{ N.G.}$$

Try 28 - $1\frac{1}{2}$ " rods

$$T_u = \frac{P_u}{n} - \frac{M_x d_i A_b}{I_x}$$

$$P_u = 774.4$$

$$n = 28$$

$$A_b = 1.77$$

$$M_x = 6956.0 \text{ ft-kips}$$

$$d_i = 55.5$$

$$I_x = 70877 \text{ in}^4$$

$$T_u = \frac{774.4}{28} - \frac{(6956)(12)(55.5)(1.77)}{70876.53} = 88 \text{ k} \leq [\phi R_n = 150 \text{ ksi}] \text{ OK}$$

Final design:

28 $1\frac{1}{2}$ " rods

Embedment length = 18"

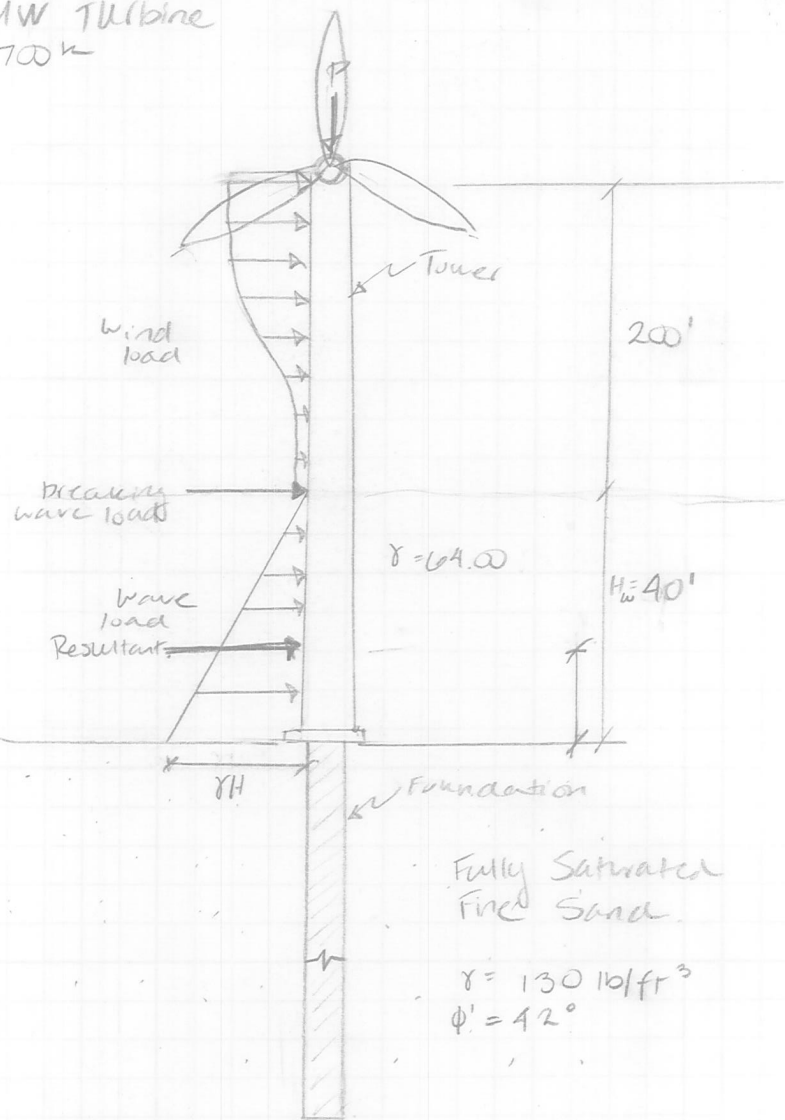
Edge distance = 7.5"

Spacing $\sim 1'-5\frac{1}{8}"$

Appendix H: Offshore Wind Turbine Design Sheets and Hand Calculations

OFF-SHORE WIND TURBINE DESIGN

Use 3-MW Turbine
 $P_{\text{equipment}} = 700 \text{ k}$



TOWER DESIGN (with use of tower design spreadsheet for determining loads.)

TRY: 12.5' ϕ at base
 11' ϕ at hub
 1.00" shell thickness.

DETERMINE AXIAL LOADS.

- Wt. of equipment @ top of tower $\sim 700 \text{ k}$ for 3.0 MW Turbine.
- Wt of steel: $= 359.2 \text{ k}$ (see excel spreadsheet for multiple calcs of section wt.)
- Wt of ice $= 47.03 \text{ k}$ (see excel spreadsheet)
 *only accounts for portion of tower above sea level.

$$\text{Total Factored DL} = 1.2(700 \text{ k}) + 1.2(359.2) + 47.03 \text{ k}$$

$$= \underline{\underline{1318.1 \text{ k}}}$$

DETERMINE LATERAL LOADS

→ Wind loads for portion of tower above sea level:
(see excel sheet for calculations).

$$F_{wind} = 106.7 \text{ k}$$
$$M_{wind} = 15370.2 \text{ ft-k}$$

$$V_{fastest \text{ mile}} = 110 \text{ ft/s}$$

→ Breaking wave load: (ASCE 7-05 5.4.4)

$$F_D = 0.5 \gamma_w C_D D H_b^2$$

$$\gamma_w = 0.064 \text{ kip/ft}^3$$

$$C_D = 1.75 \text{ for round columns}$$

$$D = \text{Column diameter} = 12.5'$$

$$H_b = 0.78 d_s = 0.78 \times 40' = 31.2'$$

$$F_D = 0.5(0.064)(1.75)(12.5)(31.2)^2 = 681.4 \text{ k}$$

$$M_D = (681.4)(40') = 27256 \text{ ft-kips}$$

→ Water pressure (ASCE 7-05 5.4.3); hydrodynamic power.

$$F_{water} = \left[I \frac{1}{2} \gamma h^2 d \right]$$

$$I = 2.0$$

$$\gamma = 0.064 \text{ kip/ft}^3$$

$$h = 40'$$

$$d = 12.5'$$

$$F_{water} = (2) \frac{1}{2} (0.064)(40)^2 (12.5) = 1280 \text{ k}$$

$$M_{water} = (1280 \text{ k}) \left(\frac{1}{3} (40') \right) = 17066.7 \text{ ft-kips}$$

$$\text{Total Lateral Shear} = 106.7 \text{ k} + 681.4 \text{ k} + 1280 \text{ k} = \underline{\underline{2068.1 \text{ k}}}$$

$$\text{Total Moment acting @ base} = 15370.2 + 27256 + 17066.7$$

$$= \underline{\underline{59693.2 \text{ ft-kips}}}$$

CHECK AXIAL STRENGTH:

Height = 240' = 2880"

O.D. = 12.5" = 150"

I.D. = 150 - 2(1") = 148" "

$F_y = 36 \text{ ksi}$

$E = 29000 \text{ ksi}$

KL/R

$K = 2.0$

$L = 2880$

$R = \sqrt{\frac{D^2 + d^2}{16}} = \sqrt{\frac{150^2 + 148^2}{16}} = 52.68$

$KL/R = \frac{2(2880)}{52.68} = 109.4$

Check for compact section:

$D/t = 150/1 = 150$

λ_p for compression: N/A

λ_c for compression = 88.61

$D/t = 150 > 88.61$, member is noncompact.

$P_n = F_{cr} A_g$

$[KL/R = 109.4 < 133.68] \leq [4.71 \sqrt{\frac{E}{F_y}} = 133.68]$

Use $F_{cr} = 0.658^{F_y/E} \times F_y$

$F_e = \frac{\pi^2 E}{(KL/R)^2} = \frac{\pi^2 (29000)}{109.4^2} = 23.94 \text{ ksi}$

$F_{cr} = 0.658^{(36/23.9)} (36) = 14.20 \text{ ksi}$

$A_g = \pi R^2 - \pi r^2 = \pi (150/2)^2 - \pi (148/2)^2 = 468.1 \text{ in}^2$

$P_n = (14.20 \text{ ksi}) (468.1 \text{ in}^2) = 8480.8 \text{ k}$

$\phi_c P_n = 0.85 (8480.8 \text{ k}) = 7633 \text{ k} > [P_n = 1318.1 \text{ k}] \checkmark \text{ OK}$



CHECK FLEXURAL STRENGTH:

1. Yield in strength:

$$\phi M_n \leq \phi M_n = \phi M_p = \phi F_y Z$$

$$Z = \frac{D^3 - d^3}{6} = \frac{150^3 - 148^3}{6} = 122201.33 \text{ in}^3$$

$$F_y = 36 \text{ ksi}$$

$$\phi M_n = (0.9)(122201.3)(36) = 719323 \text{ in} \cdot \text{kips}$$

2. Yield in Local Buckling:

→ check for compact section.

$$D/t = 150$$

$$\lambda_p \text{ for flexure} = 56.39$$

$$\lambda_c \text{ for flexure} = 249.72$$

$$[\lambda_p = 56.39] < [D/t = 150] < [\lambda_c = 249.72], \text{ tower is non-compact}$$

$$M_n = \left[\frac{0.021}{\rho_t} \frac{E}{F_y} + 1 \right] f_y S = \left[\frac{0.021(29000)}{(150)(36 \text{ ksi})} + 1 \right] (36)(17321.2)$$

$$M_n = 693886 \text{ ft} \cdot \text{kips}$$

$$S = \frac{\pi(D^4 - d^4)}{32D} = \frac{\pi((150)^4 - (148)^4)}{32(150)} = 17321.2$$

$$\phi M_n = (0.9)(693886) = 624497 \text{ in} \cdot \text{kips}$$

Yield in strength:

$$[M_n = 716318.4 \text{ in} \cdot \text{kip}] \leq [\phi M_n = 719323.2 \text{ in} \cdot \text{kips}] \checkmark \text{ OK}$$

CHECK FOR COMBINED AXIAL + FLEXURAL LOADS.

$$\left[\frac{P_u}{\phi P_n} = \frac{1318.1}{7633.6} = 0.175 \right] \leq 0.2; \text{ use eqn. H2-1b.}$$

$$\frac{P_u}{2\phi P_n} + \frac{8}{9} B_2 \left[\frac{M_u}{\phi_b M_n} \right] \leq 1.0$$

$$P_u = 1318.1$$

$$\phi P_n = 7633.6$$

$$B_2 = \frac{1}{1 - \frac{\alpha \sum P_{nt}}{\sum P_{c2}}}$$

$\alpha = 1.00$ for LRFD

$$\sum P_{nt} = 1318.1$$

$$\sum P_{c2} = \sum R_M \frac{Z_H L}{\Delta_H}$$

$R_M = 0.85$ for moment-frame combined systems

$$\sum Z_H = 2068 \text{ k}$$

$$L = 2880 \text{ in}$$

$\Delta_H = 6 \text{ in}$ from ANSYS Analysis.

$$\sum P_{c2} = 0.85 \frac{(2068)(2880)}{6} = 843745 \text{ k}$$

$$B_2 = \frac{1}{1 - \frac{(1)(1318)}{843745}} = 1.002$$

$$M_u = 716318.4 \text{ in-kips}$$

$$\phi M_n = 719323.27 \text{ in-kips}$$

$$\frac{1318 \text{ k}}{7633 \text{ k}} + \frac{8}{9} (1.002) \frac{716318.4}{719323.2} = 0.9729 \leq 1.0 \text{ OK}$$

FINAL TOWER DESIGN:

Height: 240'

ϕ_{base} : 12.5'

ϕ_{hub} : 11.0'

thickness: 1"

FOUNDATION DESIGN

Since there are large moment loads acting on tower, foundation will be designed for lateral load capacity for shear + moment loads using the p-y method (Coulter 601).

Acceptable lateral deflection = 0.50"

Try $B = 125''$

DESIGN FOR SHEAR LOAD VS. LATERAL DEFLECTION.

$$V_c = \lambda B^2 ER_L \left(\frac{\sigma_p}{ER_L} \right)^m (E_{s0})^n$$

$$\lambda = 1.0 \text{ for sand}$$

$$B = 12.5 \times 12 = 150$$

$$E = 29,000 \text{ ksi}$$

$$R_L = 1.00 \text{ for solid circular cross-section}$$

$$\sigma_p = 2C_{pp} \gamma B \tan^2(45 + \phi'/2)$$

$$C_{pp} = \phi'/10 = 42/10 = 4.2$$

$$\gamma = 130 \text{ lb/ft}^3 = 7.58 \text{ e-5 k/ft}^3$$

$$B = 138''$$

$$\phi' = 42^\circ$$

$$\sigma_p = 2(4.2)(7.58 \text{ e-5}) \tan^2(45 + 42/2) = 0.4432$$

$$m = 0.57 \text{ for shear}$$

$$n = -0.22 \text{ for shear}$$

$$E_{s0} = 0.002 \text{ for sand}$$

$$V_c = (1.0)(150)^2(29000)(1) \left(\frac{0.4432}{29000} \right)^{0.57} (0.002)^{-0.22}$$

$$V_c = 17113897 \text{ k}$$

$$\frac{V}{V_c} = \frac{2068 \text{ k}}{17113897} = 0.000121$$

Using Fran's + Duncan Shear load vs. lateral deflection for sand;

$$\frac{y_t}{B} = 0.0015 ; B = 150'' : y_t = (150)(0.0015) \\ y_t = 0.225'' < 0.50'' \text{ OK}$$

DESIGN FOR MOMENT LOAD VS. LATERAL DEFLECTION:

$$M_c = \lambda B^3 ER_L \left(\frac{\sigma_p}{ER_L} \right)^m (E_{s0})^n$$

Same values as shown above, except:

$$m = 0.46 \text{ for moment}$$

$$n = -0.15 \text{ for moment}$$

$$M_c = (1.0)(150'')^3(29000)(1) \left(\frac{0.4432}{29000} \right)^{0.46} (0.002)^{-0.15}$$

$$M_c = 15144330637 \text{ ft} \cdot \text{kips}$$

$$\frac{M}{M_c} = \frac{59693.2 \text{ ft} \cdot \text{kip}}{15144330637 \text{ ft} \cdot \text{kip}} = 0.00004$$

Using Evans + Duncan Moment load vs. lateral deflection for sand:

$$\frac{y_f}{B} = 0.0003; B = 150'' : y_f = (150)(0.0003)$$

$$y_f = 0.045'' < 0.50'' \text{ OK.}$$

DESIGN FOR MAXIMUM MOMENT FROM SHEAR LOADS

$$\frac{V}{V_c} = 0.000121$$

Using Evans + Duncan Shear load vs. maximum moment for sand:

$$\frac{M_{max}}{M_c} = 0.00011; M_{max} = 0.0001(M_c) = M_{max} @ 0.0001(15144330637)$$

$$M_{max} = 1514433 \text{ ft} \cdot \text{kip} > M_u = 59693 \text{ ft} \cdot \text{kip} \text{ OK.}$$

$$D_{req} = 20 \cdot \phi = 20(12.5) = \underline{250'}$$

Wind	83	3.152500961	1.544725471	4.212437791	0.235896516	1.09595823	0.10959582	12.13			12.125	110	82.5	1.2992663	1.299263223	1.2992632	40.24598	1.69	0.59	12.25	12.25	1	12.25	0.4915883	40.6257032
Wind	84	3.152500961	1.544725471	4.217561683	0.236183454	1.09727947	0.10972795	12.13			12.125	110	83.5	1.3037443	1.303734349	1.3037435	40.38476	1.69	0.59	12.25	12.25	1	12.25	0.493279	41.259074
Wind	85	3.152500961	1.544725471	4.222630751	0.236467322	1.09858655	0.10985865	12.13			12.125	110	84.5	1.308186	1.308185595	1.3081856	40.52236	1.69	0.59	12.25	12.25	1	12.25	0.494959	41.664127
Wind	86	3.152500961	1.544725471	4.227646218	0.236748188	1.09987978	0.10998798	12.13			12.125	110	85.5	1.31259	1.312590307	1.3125903	40.6588	1.69	0.59	12.25	12.25	1	12.25	0.496626	42.533021
Wind	87	3.152500961	1.544725471	4.23260927	0.237026119	1.10115947	0.11011595	12.13			12.125	110	86.5	1.316958	1.316958373	1.3169584	40.7941	1.69	0.59	12.25	12.25	1	12.25	0.498279	43.1721215
Wind	88	3.152500961	1.544725471	4.23752105	0.237301179	1.10242591	0.11024259	12.13			12.125	110	87.5	1.321291	1.321290517	1.3212905	40.9283	1.69	0.59	12.25	12.25	1	12.25	0.499918	43.8140559
Wind	89	3.152500961	1.544725471	4.242382666	0.237573429	1.10367939	0.11036794	12.13			12.125	110	88.5	1.325587	1.325587439	1.3255874	41.0614	1.69	0.59	12.25	12.25	1	12.25	0.501543	44.4580872
Wind	90	3.152500961	1.544725471	4.247195188	0.237842931	1.10492019	0.11049202	12.13			12.125	110	89.5	1.32985	1.32984982	1.3298498	41.19343	1.69	0.59	12.25	12.25	1	12.25	0.503156	45.1041984
Wind	91	3.152500961	1.544725471	4.251959633	0.238109741	1.10614857	0.11061486	12.13			12.125	110	90.5	1.334078	1.334078317	1.3340783	41.32441	1.69	0.59	12.25	12.25	1	12.25	0.504756	45.752373
Wind	92	3.152500961	1.544725471	4.256677062	0.238373915	1.1073648	0.11073648	12.13			12.125	110	91.5	1.338274	1.338273571	1.3382736	41.45436	1.69	0.59	12.25	12.25	1	12.25	0.506343	46.4025946
Wind	93	3.152500961	1.544725471	4.261348389	0.23863551	1.10856913	0.11085691	12.13			12.125	110	92.5	1.342436	1.342436201	1.3424362	41.5833	1.69	0.59	12.25	12.25	1	12.25	0.507918	47.0548473
Wind	94	3.152500961	1.544725471	4.265974574	0.238894576	1.10976179	0.11097618	12.13			12.125	110	93.5	1.346567	1.346566811	1.3465668	41.71125	1.69	0.59	12.25	12.25	1	12.25	0.509481	47.7091154
Wind	95	3.152500961	1.544725471	4.270556239	0.239151166	1.11094303	0.11109433	12.13			12.125	110	94.5	1.350666	1.350665984	1.350666	41.83823	1.69	0.59	12.25	12.25	1	12.25	0.511032	48.3653834
Wind	96	3.152500961	1.544725471	4.275095139	0.239405328	1.11211307	0.11211311	12.13			12.125	110	95.5	1.354734	1.354734924	1.3547343	41.96425	1.69	0.59	12.25	12.25	1	12.25	0.512571	48.7208484
Wind	97	3.054326191	1.496619834	4.148436978	0.232312471	1.11327123	0.11132721	11.75		11.75	11.75	110	96.5	1.358772	1.358772279	1.3587723	42.08933	1.69	0.59	11.875	11.875	1	11.875	0.498361	48.1629247
Wind	98	3.054326191	1.496619834	4.152756163	0.232554345	1.11442044	0.11144204	11.75		11.75	11.75	110	97.5	1.36278	1.36278049	1.3627805	42.21349	1.69	0.59	11.875	11.875	1	11.875	0.499831	48.8048323
Wind	99	3.054326191	1.496619834	4.157035743	0.232794002	1.11555819	0.11155582	11.75		11.75	11.75	110	98.5	1.366759	1.366759443	1.3667594	42.33674	1.69	0.59	11.875	11.875	1	11.875	0.501291	49.4486215
Wind	100	3.054326191	1.496619834	4.161276478	0.233031483	1.1166856	0.11166856	11.75		11.75	11.75	110	99.5	1.37071	1.370709645	1.3707096	42.4591	1.69	0.59	11.875	11.875	1	11.875	0.50274	50.0942787
Wind	101	3.054326191	1.496619834	4.165479105	0.233266883	1.1178028	0.11178028	11.75		11.75	11.75	110	100.5	1.374632	1.374631591	1.3746316	42.58059	1.69	0.59	11.875	11.875	1	11.875	0.504178	50.7417906
Wind	102	3.054326191	1.496619834	4.169644434	0.233500083	1.11891014	0.11189101	11.75		11.75	11.75	110	101.5	1.378526	1.378525761	1.3785258	42.70121	1.69	0.59	11.875	11.875	1	11.875	0.505606	51.3911438
Wind	103	3.054326191	1.496619834	4.173772882	0.233731281	1.1200076	0.11200767	11.75		11.75	11.75	110	102.5	1.382393	1.382392622	1.3823926	42.82099	1.69	0.59	11.875	11.875	1	11.875	0.507025	52.0423255
Wind	104	3.054326191	1.496619834	4.17786505	0.233960643	1.12109558	0.11210956	11.75		11.75	11.75	110	103.5	1.386233	1.386232629	1.3862326	42.93994	1.69	0.59	11.875	11.875	1	11.875	0.508433	52.6952329
Wind	105	3.054326191	1.496619834	4.181922568	0.234187664	1.12217408	0.11221741	11.75		11.75	11.75	110	104.5	1.390046	1.390046226	1.3900462	43.05807	1.69	0.59	11.875	11.875	1	11.875	0.509832	53.3501033
Wind	106	3.054326191	1.496619834	4.185944501	0.234412921	1.12324334	0.11232433	11.75		11.75	11.75	110	105.5	1.393834	1.393833844	1.3938338	43.1754	1.69	0.59	11.875	11.875	1	11.875	0.511221	54.006715
Wind	107	3.054326191	1.496619834	4.189933354	0.234636268	1.12430351	0.11243035	11.75		11.75	11.75	110	106.5	1.397596	1.397595905	1.3975959	43.29193	1.69	0.59	11.875	11.875	1	11.875	0.512661	54.6650851
Wind	108	3.054326191	1.496619834	4.193888204	0.234857739	1.12535476	0.11253548	11.75		11.75	11.75	110	107.5	1.401333	1.401332817	1.4013328	43.40769	1.69	0.59	11.875	11.875	1	11.875	0.513971	55.325214
Wind	109	3.054326191	1.496619834	4.19781015	0.235077368	1.12639725	0.11263972	11.75		11.75	11.75	110	108.5	1.405045	1.405044981	1.405045	43.52767	1.69	0.59	11.875	11.875	1	11.875	0.515333	55.987222
Wind	110	3.054326191	1.496619834	4.201699766	0.235295187	1.12743113	0.11274311	11.75		11.75	11.75	110	109.5	1.408733	1.408732787	1.4087328	43.63691	1.69	0.59	11.875	11.875	1	11.875	0.516686	56.6507494
Wind	111	3.054326191	1.496619834	4.205557608	0.235511226	1.12845653	0.11284565	11.75		11.75	11.75	110	110.5	1.412397	1.412396615	1.4123966	43.7504	1.69	0.59	11.875	11.875	1	11.875	0.518029	57.316117
Wind	112	3.054326191	1.496619834	4.209384222	0.235725516	1.12947362	0.11294736	11.75		11.75	11.75	110	111.5	1.416037	1.416036835	1.4160368	43.86316	1.69	0.59	11.875	11.875	1	11.875	0.519364	57.9832054
Wind	113	3.054326191	1.496619834	4.213180135	0.235938088	1.13048254	0.11304825	11.75		11.75	11.75	110	112.5	1.419654	1.41965381	1.4196538	43.9752	1.69	0.59	11.875	11.875	1	11.875	0.520691	58.6520038
Wind	114	3.054326191	1.496619834	4.216945866	0.236148968	1.13148341	0.11314834	11.75		11.75	11.75	110	113.5	1.423248	1.423247891	1.4232479	44.08653	1.69	0.59	11.875	11.875	1	11.875	0.522009	59.3225011
Wind	115	3.054326191	1.496619834	4.220681915	0.236358771	1.13247639	0.11324764	11.75		11.75	11.75	110	114.5	1.426819	1.426819425	1.4268194	44.19716	1.69	0.59	11.875	11.875	1	11.875	0.523319	59.9946868
Wind	116	3.054326191	1.496619834	4.224388775	0.236565771	1.13346158	0.11334616	11.75		11.75	11.75	110	115.5	1.430369	1.430368748	1.4303687	44.3071	1.69	0.59	11.875	11.875	1	11.875	0.524621	60.6685502
Wind	117	3.054326191	1.496619834	4.228066921	0.236771748	1.13443913	0.11343913	11.75		11.75	11.75	110	116.5	1.433896	1.433896188	1.4338962	44.41637	1.69	0.59	11.875	11.875	1	11.875	0.525915	61.3440811
Wind	118	3.054326191	1.496619834	4.231716821	0.236976142	1.13540916	0.11354092	11.75		11.75	11.75	110	117.5	1.437402	1.437402066	1.4374021	44.52197	1.69	0.59	11.875	11.875	1	11.875	0.527201	62.0212692
Wind	119	3.054326191	1.496619834	4.235338929	0.23717898	1.13637178	0.11363718	11.75		11.75	11.75	110	118.5	1.440887	1.440886696	1.4408867	44.63291	1.69	0.59	11.875	11.875	1	11.875	0.528479	62.7001043
Wind	120	3.054326191	1.496619834	4.238933689	0.237380287	1.13732712	0.11373271	11.75		11.75	11.75	110	119.5	1.444435	1.444350385	1.4443504	44.7402	1.69	0.59	11.875	11.875	1	11.875	0.529749	63.3805766
Wind	121	3.054326191	1.496619834	4.242501533	0.237580086	1.1382753	0.11382753	11.75		11.75	11.75	110	120.5	1.447793	1.447793432	1.4477934	44.84685	1.69	0.59	11.875	11.875	1	11.875	0.531012	64.0627664
Wind	122	3.054326191	1.496619834	4.246042884	0.237778401	1.13921641	0.11392164	11.75		11.75	11.75	110	121.5	1.451216	1.451216129	1.4512161	44.95287	1.69	0.59	11.875	11.875	1	11.875	0.532267	64.7463938
Wind	123	3.054326191	1.496619834	4.249558154	0.237975257	1.14015059	0.11401506	11.75		11.75	11.75	110	122.5	1.454619	1.454618764	1.4546188	45.05827	1.69	0.59	11.875	11.875	1	11.875	0.533515	

User Input				Axial Strength		Flexure Strength		Combined Loads				
Overall height	240	ft =	2880	in	Factored Load	1318.058907	Factored Load	716318.40	Combined Loading	0.972893	≤1.0	OK
Diameter at Base	12.5	ft=	150	in	Factored Strength	7633.644538	Factored Strength	719323.20	Pu/Pc	0.172664		
Shell Thickness	1	in			Pn	8980.75828	Yielding Mn	799248	For Pu/Pc≥0.2	1.059225		
Strength of Steel	36	ksi			Fcr	19.18566541	Local Buckling	693885.6946	For Pu/Pc<0.2	0.972893	624497.1	
Modulus of Elasticity	29000	ksi			KL/r	109.3380761	Check for Compact section:		Magnification Factors			
					4.71*sqrt(E/Fy)	133.680683	D/t	150	B2	1.001565		
ID	148				Fe	23.94165422	λp	56.38888889	Pnt	1318.059		
S	17321.16062				Fcr for KL/r≤4.71*sqrt(E/Fy)	19.18566541	λc	249.7222222	Pe2	843794.86		
Z	22201.33333				Fcr for KL/r>4.71*sqrt(E/Fy)	20.99683075	non-compact		ΣH	2068.12		
r	52.68064161				Ag	468.0973054	Flexure Strength:	OK	ΔH	6		
l	1299087.047				Check for Compact section:				L	2880		
					D/t	150						
					λp							
					λc	88.61111111						
					noncompact							
					Axial Strength:	OK						